

FAYETTEVILLE AVE. - HWY. 162

(ALMA) (S)

CRAWFORD COUNTY
ROUTE 64 SECTION 2C

FEDERAL AID PROJ. NHPP-0017(42)

FEDERAL AID PROJ. RTP-0017(42)

JOB 040721

NOT TO SCALE

STA. 206+26.31 BEGIN JOB 040721 LOG MILE 0.04

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ARK. HWY. DIST. NO. 4

DESIGN TRAFFIC DATA

DESIGN YEAR	2012
2023 ADT	1600
2043 ADT	2200
2043 DHV	242
DIRECTIONAL DISTRIBUTION	
TRUCKS	
DESIGN SPEED3	5 MPH



APPROVED



CHIEF ENGINEER - PRECONSTRUCTION

SEP 1 9 2023

BRIDGE DATA

(1) STA. 209+59.47 BR. END
BR. NO. 07590
30'-0" CLEAR ROADWAY
131'-0 3/4" TOTAL LENGTH
130'-0" INTEGRAL CONTINUOUS W-BEAM UNIT
(40'-0", 50'-0", 40'-0")
STA. 210+90.53 BR. END

LENGTH OF PROJECT CALCULATED ALONG C.L.

GROSS LENGTH OF PROJECT 881.90 FEET OR 0.167 MILES
NET ROADWAY 750.84 0.142 MILES
NET BRIDGES 131.06 0.025 MILES
NET PROJECT 881.90 0.167 MILES

R 31 W R 30 W

7/19/2023

h39735 7/19/2023 040721.DGN

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS				
		6	ARK.	040721	2	64				
		INDEX OF SHEETS								

ARKANSAS
LICENSED
LICENSED
ENGINEER
No. 11425
LICENSED
ENGINEER
No. 11425

Digitally signed by Trinity Smith Date: 2023,09.18 13:14:43-05'00'

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PERMANENT PAVEMENT MARKING DETAILS		
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DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS			
		6	ARK.	040721	3	64			
		GOVERNING SPECS.							

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09-18-2023

GOVERNING SPECIFICATIONS (1 OF 2)

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS:

TITLE

FHWA-1273	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTCE TO CONTRACTORS
FHWA-1273	SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 14C)
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
FHWA-1273	SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
FHWA-1273	SUPPLEMENT - WAGE RATE DETERMINATION
100-3	CONTRACTOR'S LICENSE
100-4	DEPARTMENT NAME CHANGE
102-2	ISSUANCE OF PROPCSALS
	MAINTENANCE DURING CONSTRUCTION
	RESTRAINING CONDITIONS
108-1	LIQUIDATED DAMAGES
108 2	WORK ALLOWED PRIOR TO ISSUANCE OF WORK ORDER
110-1	PROTECTION OF WATER QUALITY AND WETLANDS
210-1	UNCLASSIFIED EXCAVATICN
303-1	AGGREGATE BASE COURSE
	QUALITY CONTROL AND ACCEPTANCE
307-1	CEMENT
308-1	
400-1	TACK COATS
400-4	DESIGN AND QUALITY CONTROL OF ASPHALT MIXTURES
400-5	PERCENT AIR VOIDS FOR ACHM MIX DESIGNS
	LIQUID ANTI-STRIP ADDITIVE
400-7	TRACKLESS TACK
404-3	DESIGN OF ASPHALTMIXTURES
409-2	ASPHALT LABORATORY FACILITY
410-1	CONSTRUCTION REQUIREMENTS AND ACCEPTANCE OF ASPHALT CONCRETE PLANT MIX COURSES
410-2	DEVICES FOR MEASURING DENSITY FOR ROLLING PATTERNS
410-4	EVALUATION OF ACHM SUBLOT REPLACEMENT MATERIAL
416-1	RECYCLED ASPHALTPAVEMENT
501-2	CEMENT
505-1	PORTLAND CEMENT CONCRETE DRIVEWAY
600-2	INCIDENTAL CONSTRUCTION
603-1	LANE CLOSURE NOTIFICATION
604-1	RETROREFLECTIVE SHEETING FOR TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES
	TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES (MASH)
605-1	CONCRETE DITCH PAVING

ERRATA____ ERRATA FOR THE BOOK OF STANDARD SPECIFICATIONS

GUARDRAIL DELINEATORS

GENERAL REQUIREMENTS FOR SIGNS

REINFORCING STEEL FOR STRUCTURES

_ INSTALLATION OF ELASTOMERIC BEARINGS

CHANNEL POST SIGN SUPPORT

CONCRETE FOR STRUCTURES

CONCRETE WALKS, CONCRETE STEPS, AND HAND RAILING

MULCH COVER

_FILTER SOCKS

_STRUCTURES

STEEL STRUCTURES

_ ELASTOMERIC BEARINGS

CEMENT

GOVERNING SPECIFICATIONS (2 OF 2)

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FCLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS:

NUMBER	TITLE
	ASSESSMENT OF WORKING DAYS – MAINTENANCE OF TRAFFIC
	BIDDING REQUIREMENTS AND CONDITIONS
	BROADBAND INTERNET SERVICE FOR ASPHALT CONCRETE PLANT
	BROADBAND INTERNET SERVICE FOR FIELD OFFICE BUY AMERICA - CONSTRUCTION MATERIALS
	CARGO PREFERENCE ACT REQUIREMENTS
	CLASS C FLY ASH IN PORTLAND CEMENT CONCRETE PAVEMENT AND CLASS S(AE) CONCRETE
JOB 040721	COLD MILLING - COUNTY PROPERTY
	CONCRETE BRIDGE DECK CURING AND SURFACE TREATMENT RESTRICTIONS
	CONCRETE WALKS (TYPE SPECIAL)
	CONSTRUCTION IN SPECIAL FLOOD HAZARD AREAS
	CONTACT INFORMATION FOR MOTORIST DAMAGE CLAIMS
	DESIGN AND QUALITY CONTROL ASPHALT MIXTURES DESIGN OF ASPHALT MIXTURES - AGGREGATES
	DESIGN OF ASPHALT MIXTURES - AGGREGATES DIRECT TENSION INDICATORS FOR HIGH STRENGTH BOLT ASSEMBLIES
	DISADVANTAGED BUSINESS ENTERPRISE BIDDER'S RESPONSIBILITIES
	DRILLED SHAFT FOUNDATIONS
JOB 040721	ESTABLISHING CONTRACT TIME – WORKING DAY CONTRACT
	FLEXIBLE BEGINNING OF WORK
	GOALS FOR DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION
	LIQUIDATED DAMAGES PROCEDURE FOR BID LETTINGS
	MAINTENANCE OF TRAFFIC
	MANDATORY ELECTRONIC CONTRACT
	MANDATORY ELECTRONIC DOCUMENT SUBMITTAL NESTING SITES OF MIGRATORY BIRDS
	NONDESTRUCTIVE TESTING OF DRILLED SHAFTS
	OFF-SITE RESTRAINING CONDITIONS FOR AMERICAN BURYING BEETLE
JOB 040721	OFF-SITE RESTRAINING CONDITIONS FOR INDIANA AND NORTHERN LONG-EARED BATS
JOB 040721	PARTNERING REQUIREMENTS
	PLASTIC PIPE
	PRICE ADJUSTMENT FOR ASPHALT BINDER
	PRICE ADJUSTMENT FOR FUEL PROHIBITION OF CERTAIN TELECOMMUNICATIONS AND VIDEO SURVEILLANCE SERVICES OR EQUIPMENT
	REMOVAL OF HISTORIC TRUSS SPAN OF BRIDGE NUMBER M1144
	SECTION 404 NATIONWIDE 14 PERMIT REQUIREMENTS
	SELECT GRANULAR BACKFILL
	SHORING FOR CULVERTS
	SOIL STABILIZATION
	STORM WATER POLLUTION PREVENTION PLAN
	SUBMISSION OF ASPHALT CONCRETE HOT MIX ACCEPTANCE TEST RESULTS
	TEXTURED COATING FINISH (CAST-IN PLACE RETAINING WALLS)
	TOTAL SOLAR ECLIPSE
	TRAFFIC SIGNAL CONTROLLER (MODIFICATION)
	UTILITY ADJUSTMENTS VALUE ENGINEERING
JOB 040721	_ VALUE LINGUINEERING

JOB 040721__ WARM MIX ASPHALT

NUMBER

617-2_

620-1_ 621-1_

633-1_

634-1

723-1_ 729-1_

800-1

802-3_

802-4_

804-2_ 807-2_

808-1_

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS			
10-05-2023		6	ARK.	040721	4	64			
		STANDARD DRAWING & CENERAL NOTES							

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ENGINEER

N. 11425

Digitally signed by Trinity Smith Date: 2023.10.09 16:07:39-05'00'

BRIDGE STANDARD DRAWINGS

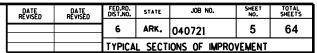
DRWG.NO.	TITLE	DATE
55000	STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS	02-27-14
55001	_ STANDARD DETAILS FOR DUMPED RIPRAP AND FILTER BLANKET AND COMPUTING EXCAVATION FOR STRUCTURES	02-27-14
55005	STANDARD DETAILS FOR PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS	03-24-16
55006	STANDARD GENERAL NOTES FOR STEEL BRIDGE STRUCTURES	09-02-15
55007	_ STANDARD DETAILS FOR STEEL BRIDGE STRUCTURES	02-11-16
55010	STANDARD DETAILS FOR TYPE D BRIDGE NAME PLATE	04-14-23
55013A	STANDARD DETAILS FOR TRANSITIONAL APPROACH RAILING TYPE SSTR36	04-08-21
55015	STANDARD DETAILS FOR TYPE H2 RAILING	06-25-20
55020	STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS	03-24-16
55040F1	STANDARD DETAILS FOR TYPE F APPROACH SLAB	09-07-23

ROADWAY STANDARD DRAWINGS

DRWG.NO	D. TITLE	DATE
CDP-1	_ CONCRETE DITCH PAVING	12-08-16
CG-1	_ CURBING DETAILS	11-29-07
DR-1	_ DETAILS OF DRIVEWAYS & ISLANDS	05-19-22
FES-1	_ FLARED END SECTION	10-18-96
FES-2	_ FLARED END SECTION	
FPC-9E	_ DETAILS OF DROP INLETS (TYPE C)	08-22-02
FPC-9M	DETAILS OF DROP INLET (TYPE MO)	08-22-02
GR-5	GUARDRAIL DETAILS (TYPE C) STREET/ROAD BARRICADE OR TEMPORARY INSTALLLATION	11-07-19
PCC-1	_ CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING	02-27-14
PCM-1	METAL PIPE CULVERT FILL HEIGHTS & BEDDING	02-27-14
PCP-1	PLASTIC PIPE CULVERT (HIGH_DENSITY POLYETHYLENE)	02-27-14
PCP-2	PLASTIC PIPE CULVERT (PVC F949)	
PCP-3		02-27-20
PM-1		02-27-20
PU-1	_ DETAILS OF PIPE UNDERDRAIN	12-08-16
SE-3	TABLES AND METHOD OF SUPERELEVATION FOR TWO-WAY TRAFFIC (4% MAXIMUM)	11-07-19
SHS-1	_ STANDARD HIGHWAY SIGNS AND SUPPORTS ASSEMBLIES	09-12-13
SHS-2	U-CHANNEL POST ASSEMBLIES	07-25-19
SI-1	_ DETAILS OF SPECIAL ITEMS	10-25-18
SI-3	_ CONCRETE WALK (TYPE SPECIAL)	11-05-20
TC-1	_ STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	11-07-19
TC-2	_ STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	05-20-21
TC-3	_ STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	08-12-21
TEC-1	TEMPORARY EROSION CONTROL DEVICES	11-16-17
TEC-3	_ TEMPORARY EROSION CONTROL DEVICES	11-03-94
WF-3	_ CHAIN LINK FENCE	11-17-10

GENERAL NOTES

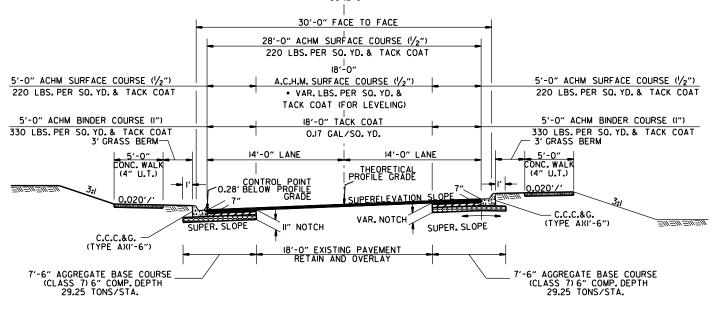
- 1. GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- 2. ALL PIPE LINES, POWER, TELEPHONE, AND TELEGRAPH LINES TO BE MOVED OR LOWERED BY THE RESPECTIVE OWNERS AS PER AGREEMENT WITH SUCH OWNERS.
- ANY EQUIPMENT OR APPURTENANCE THAT INTERFERES WITH THE PROPOSED CONSTRUCTION AND WHICH MAY BE THE PROPERTY OF UTILITY SERVICE ORGANIZATIONS SHALL BE MOVED BY THE OWNERS UNLESS OTHERWISE PROVIDED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING U. S. MAILBOXES WITHIN THE PROJECT LIMITS IN SUCH A MANNER THAT THE PUBLIC MAY RECEIVE CONTINUED MAIL SERVICE. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS BID ITEMS.
- ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- 6. ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED AS DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO ENSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A FENCE TO CONTROL LIVESTOCK IN AREAS WHERE PASTURES ARE SEVERED. WIRE FENCE MAY BE CONSTRUCTED INITIALLY, OR IN LIEU THEREOF, THE CONTRACTOR AT HIS OWN EXPENSE, MAY ELECT TO PROVIDE TEMPORARY FENCING SUITABLE TO CONTAIN LIVESTOCK.
- 8. THE SEQUENCE AS SHOWN ON THE MAINTENANCE OF TRAFFIC PLANS IS A GENERAL OUTLINE FOR THE CONSTRUCTION OF THIS PROJECT, AND IN NO WAY IS IT INTENDED TO COVER EVERY ITEM IN THE PROJECT. ITEMS NOT CRITICAL TO THE CONSTRUCTION SEQUENCE MAY BE CONSTRUCTED IN ANY STAGE AS APPROVED BY THE RESIDENT ENGINEER.
- ALL FLEXIBLE BASE AND ASPHALTIC PAVEMENTS REMOVED SHALL BE PAID FOR UNDER THE ITEM NO. 210 - UNCLASSIFIED EXCAVATION.
- 10. THE EXISTING ASPHALT PAVEMENT TO BE REMOVED FROM THE REMAINING PAVEMENT SHALL BE SEPARATED BY SAWING ALONG A NEAT LINE. AFTER SAWING, THE PAVEMENT TO BE REMOVED SHALL BE CAREFULLY REMOVED IN A MANNER THAT WILL NOT DAMAGE THE PAVEMENT THAT IS TO REMAIN. ANY DAMAGE OF THE ASPHALT PAVEMENT THAT IS TO REMAIN IN PLACE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.





09-18-2023

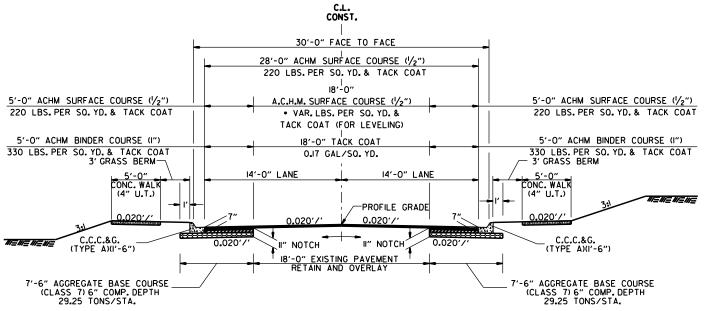
C.L. CONST.



HWY. 64B - NOTCH, WIDEN, AND OVERLAY SECTION SUPERELEVATION

STA. 206+26.3ITO STA. 209+24.47

•TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER



HWY. 64B - NOTCH, WIDEN AND OVERLAY SECTION

STA. 211+25.53 TO STA. 215+08.21

NOTES:

- I. REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.
- 2. THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT AN DEFICIENT THICKNESS THAT DOES NOT MEET TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.
- 3. ASPHALT FOR LEVELING OF EXISTING PAVEMENT SHALL BE PLACED ONLY IF AND WHERE DIRECTED BY THE ENGINEER. CALCULATIONS FOR THE AMOUNT OF LEVELING AND/OR LEVELING OPERATIONS SHALL BE PERFORMED BEFORE CONSTRUCTING NOTCH AND WIDENING. CALCULATIONS WILL NOT BE PAID FOR DIRECTLY, BUT PAYMENT WILL BE CONSIDERED INCLUDED IN THE VARIOUS PAY ITEMS.
- 4. THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.
- 5. PRIOR TO AND DURING PLACEMENT OF PAVEMENT IN FRONT OF THE CURB AND GUTTER, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AT ALL TIMES. THE METHOD(S) USED SHALL BE APPROVED BY THE ENGINEER. PAYMENT FOR THIS WORK SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

DATE REVISED PATE FED.AD. STATE JOB NO. SHEET TOTAL SHEETS

6 ARK. 040721 6 64

SPECIAL DETAILS

FINE OF ARKANSAS

ARKANSAS

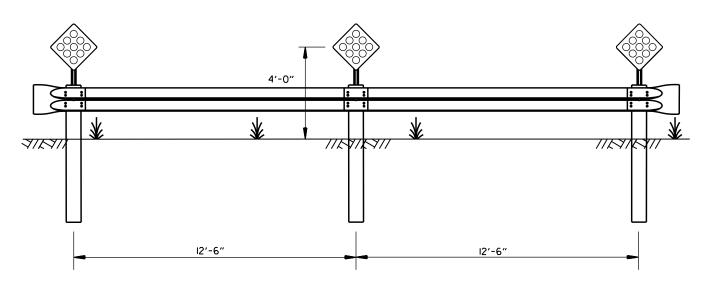
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ENGINEER

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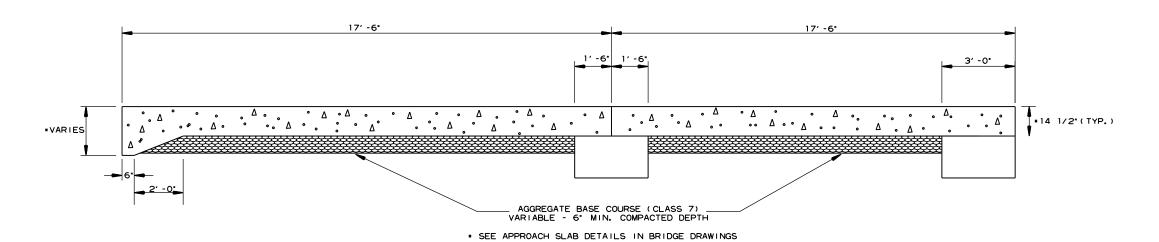
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CONSTRUCT
25 LIN.FT.TYPE "C" GUARDRAIL
WITH 3 RED DIAMOND REFLECTORS
MOUNTED ON U-CHANNEL POSTS
DIRECTLY BEHIND THE GUARDRAIL
AT A HEIGHT OF 4'-0".

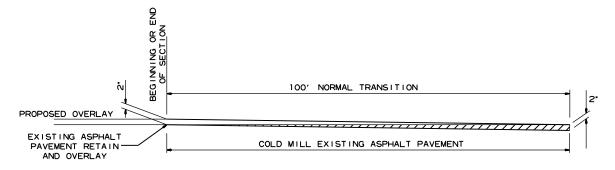


ROAD CLOSED DETAIL

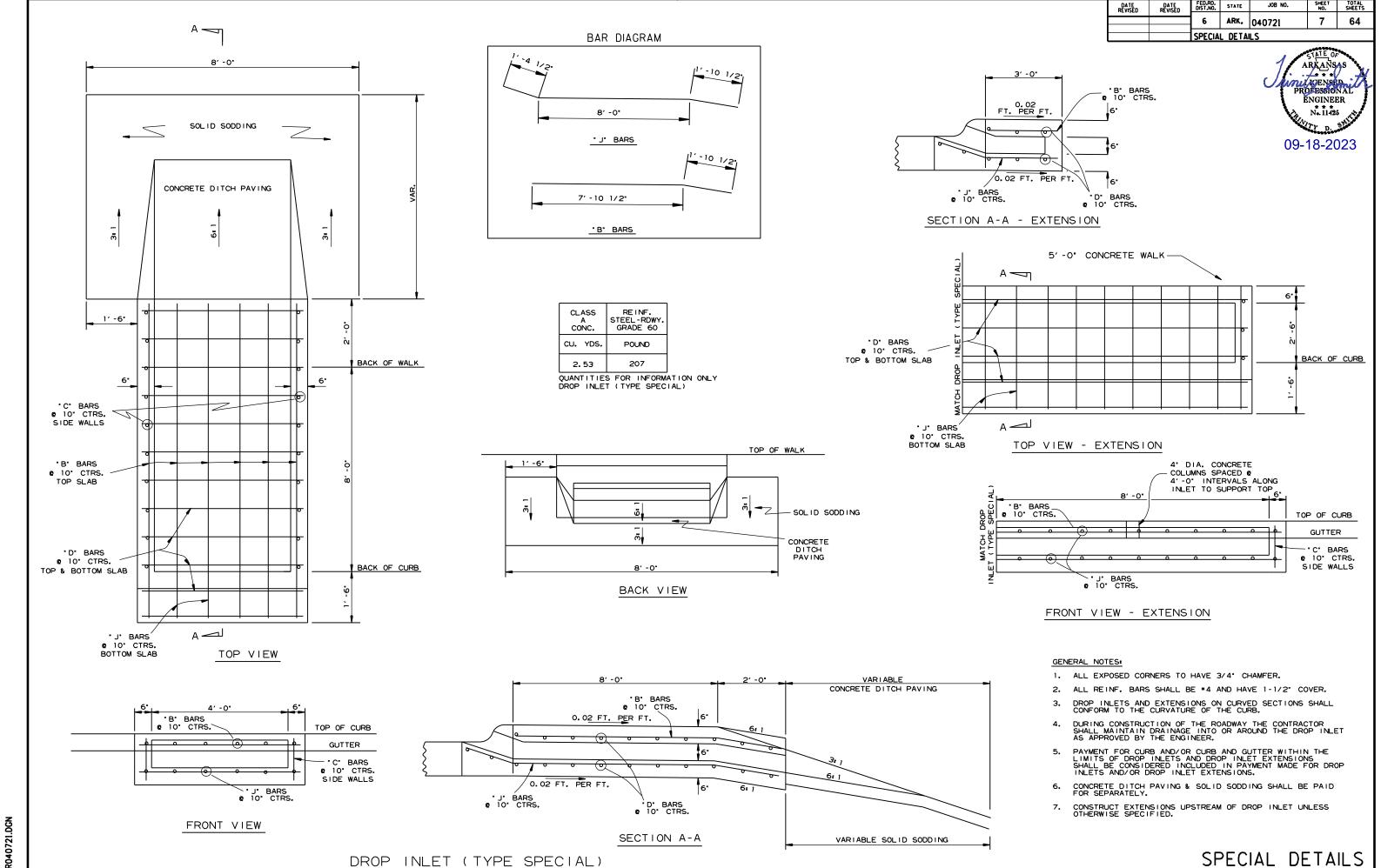
TO BE USED FOR PERMANENTLY CLOSING SOUTH MOUNTAIN GROVE RD. SEE PLAN SHEETS FOR LOCATIONS SEE STD. DWG. GR-5 FOR MORE DETAILS.

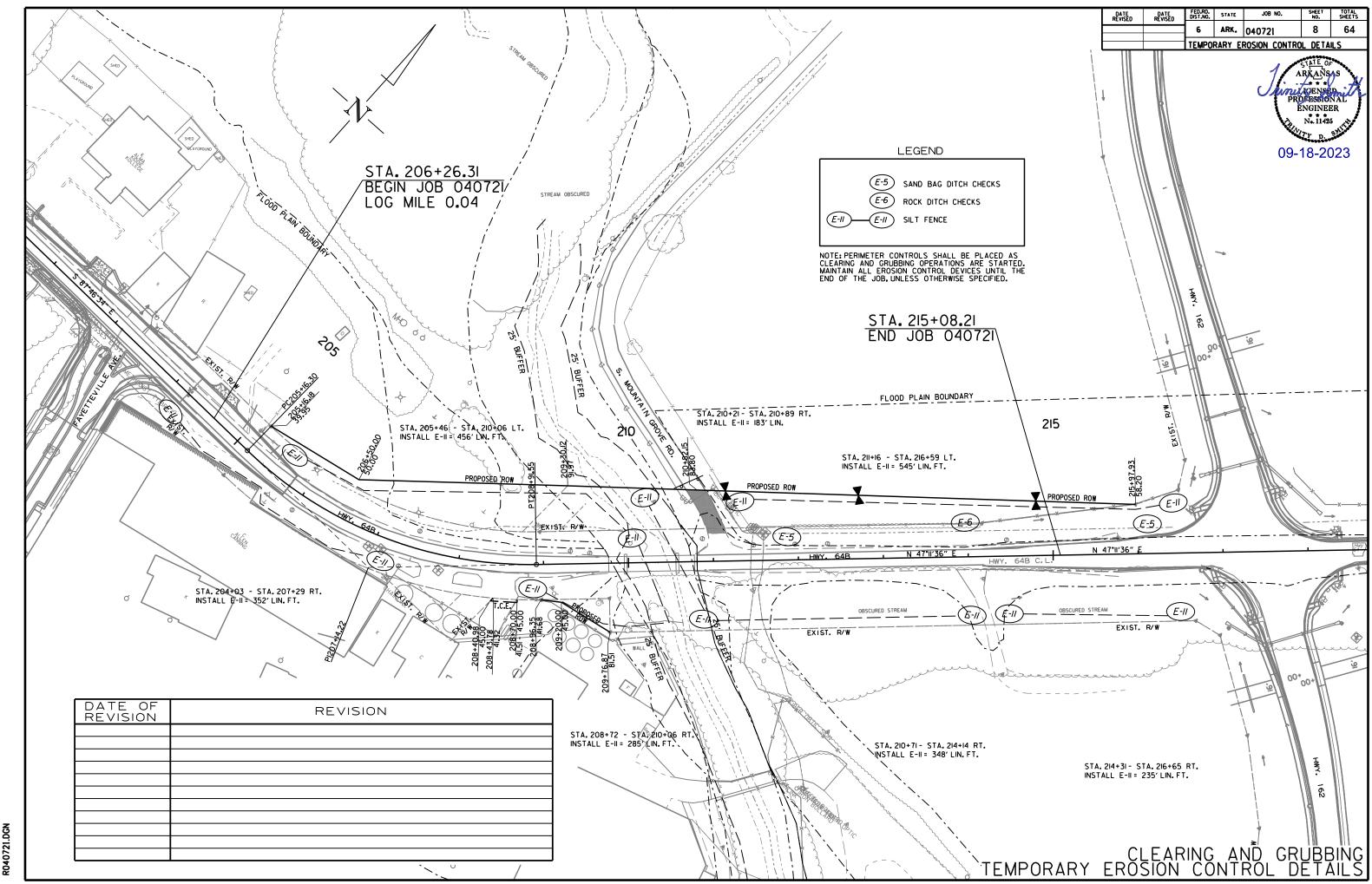


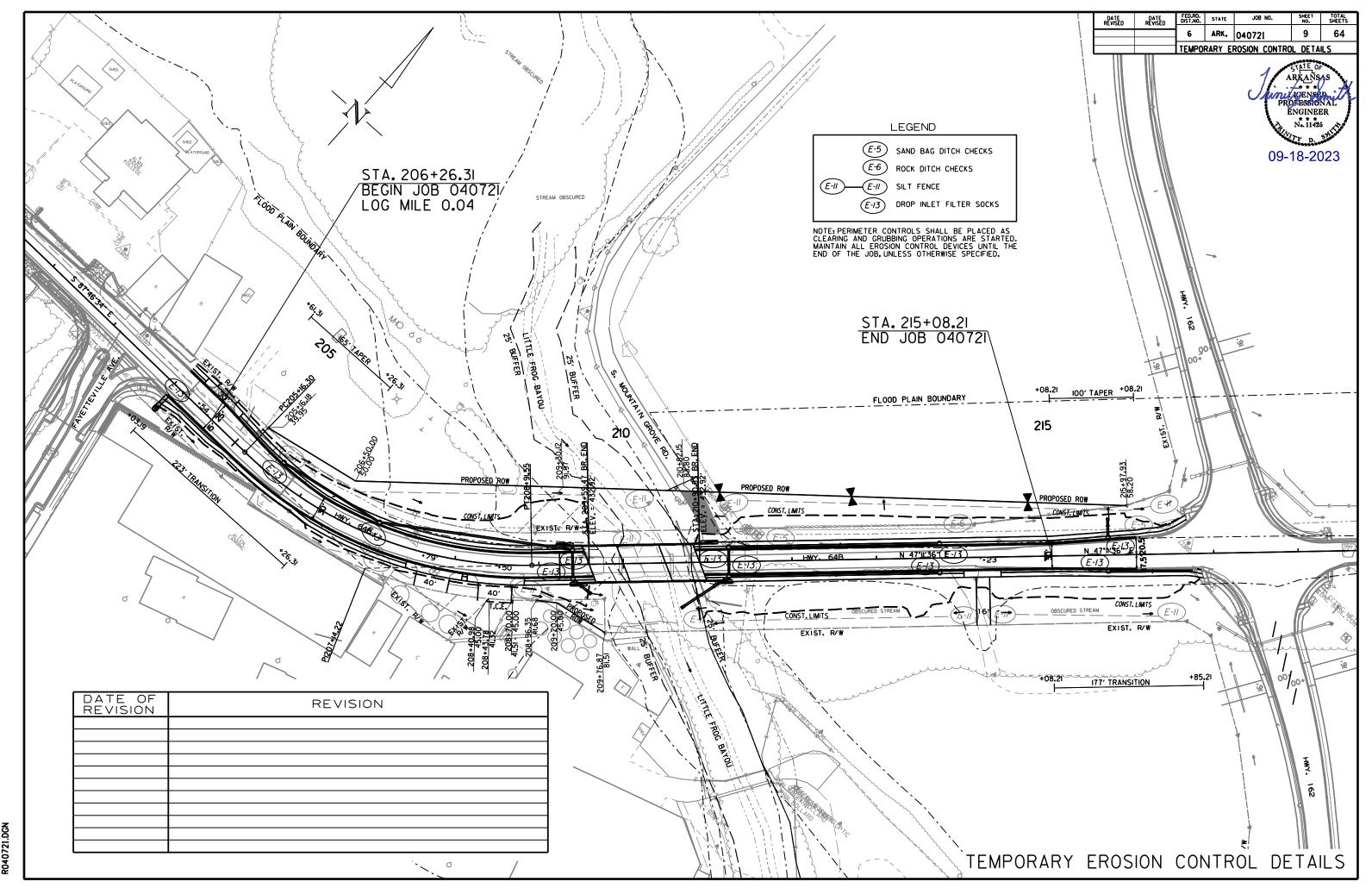
SECTION OF APPROACH SLAB



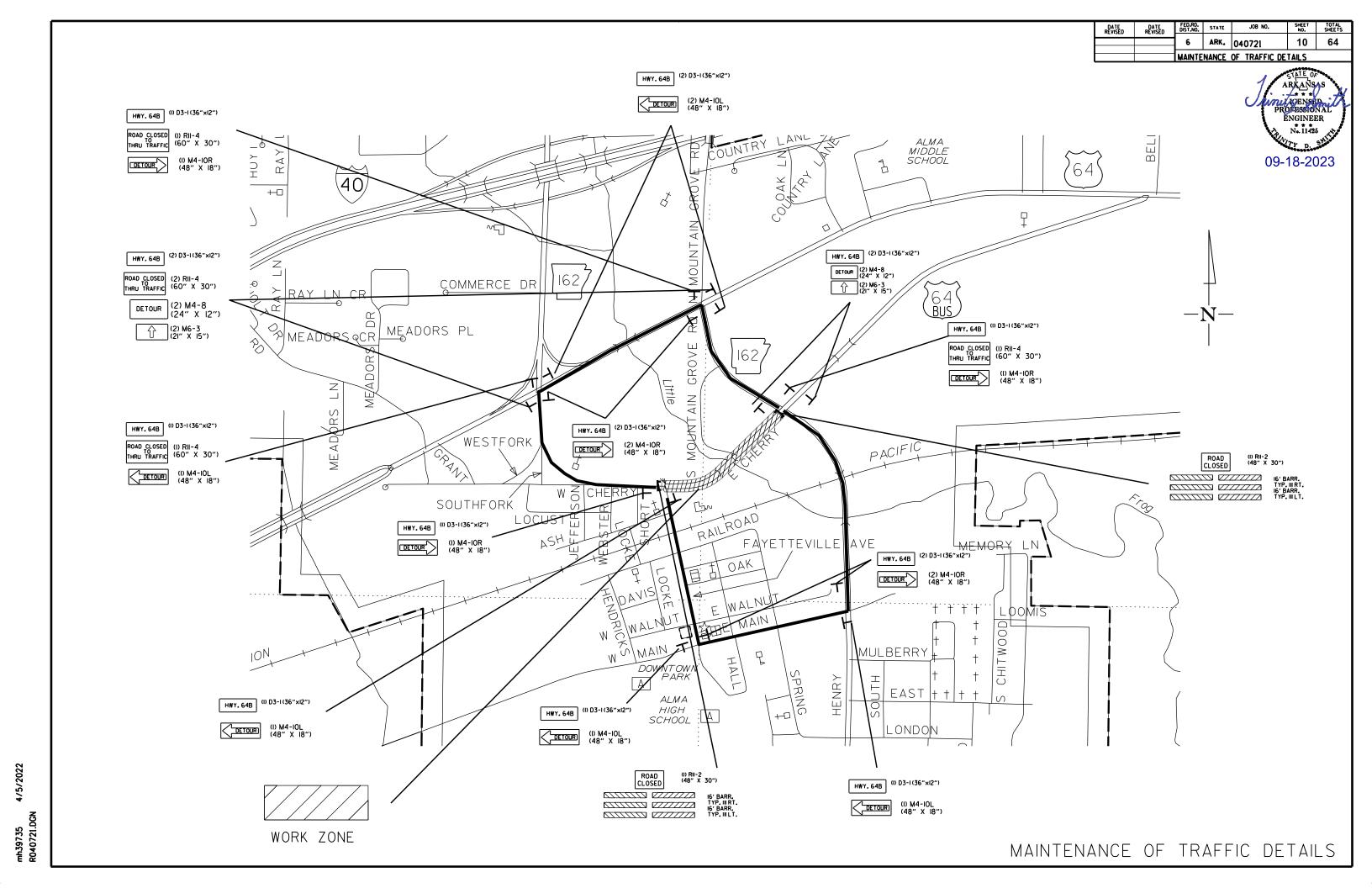
DETAIL FOR TRANSITIONS

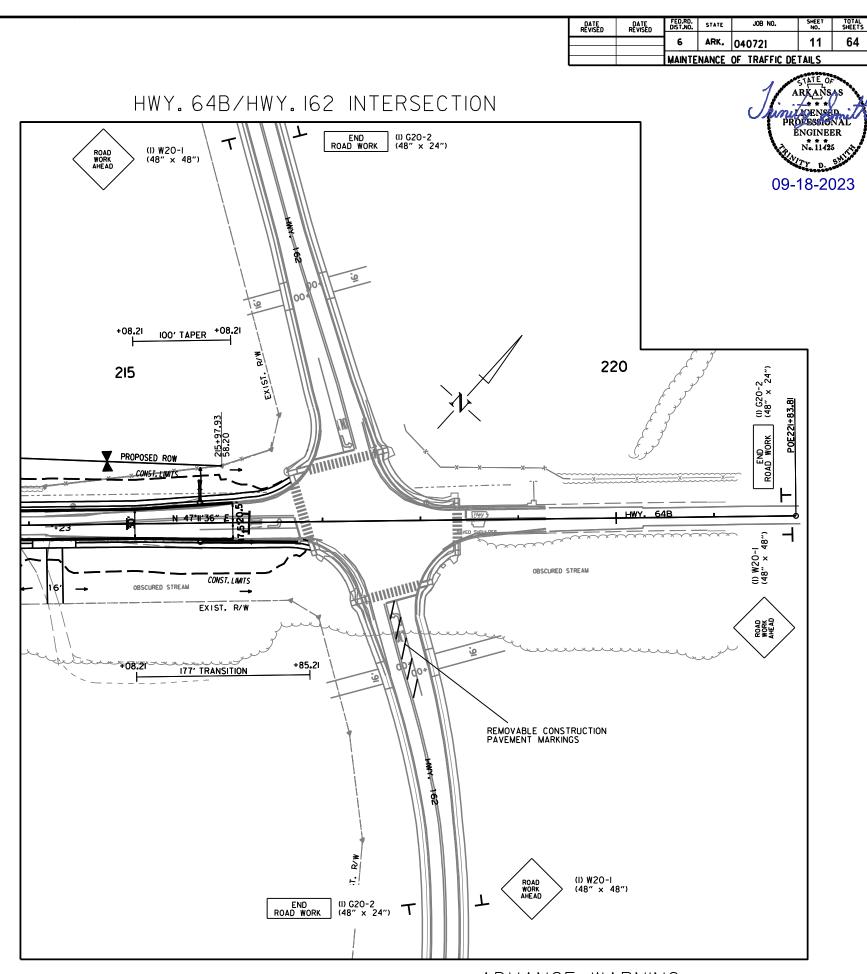






35 4/5/2022





ROAD (I) WZO-I
ROAD (

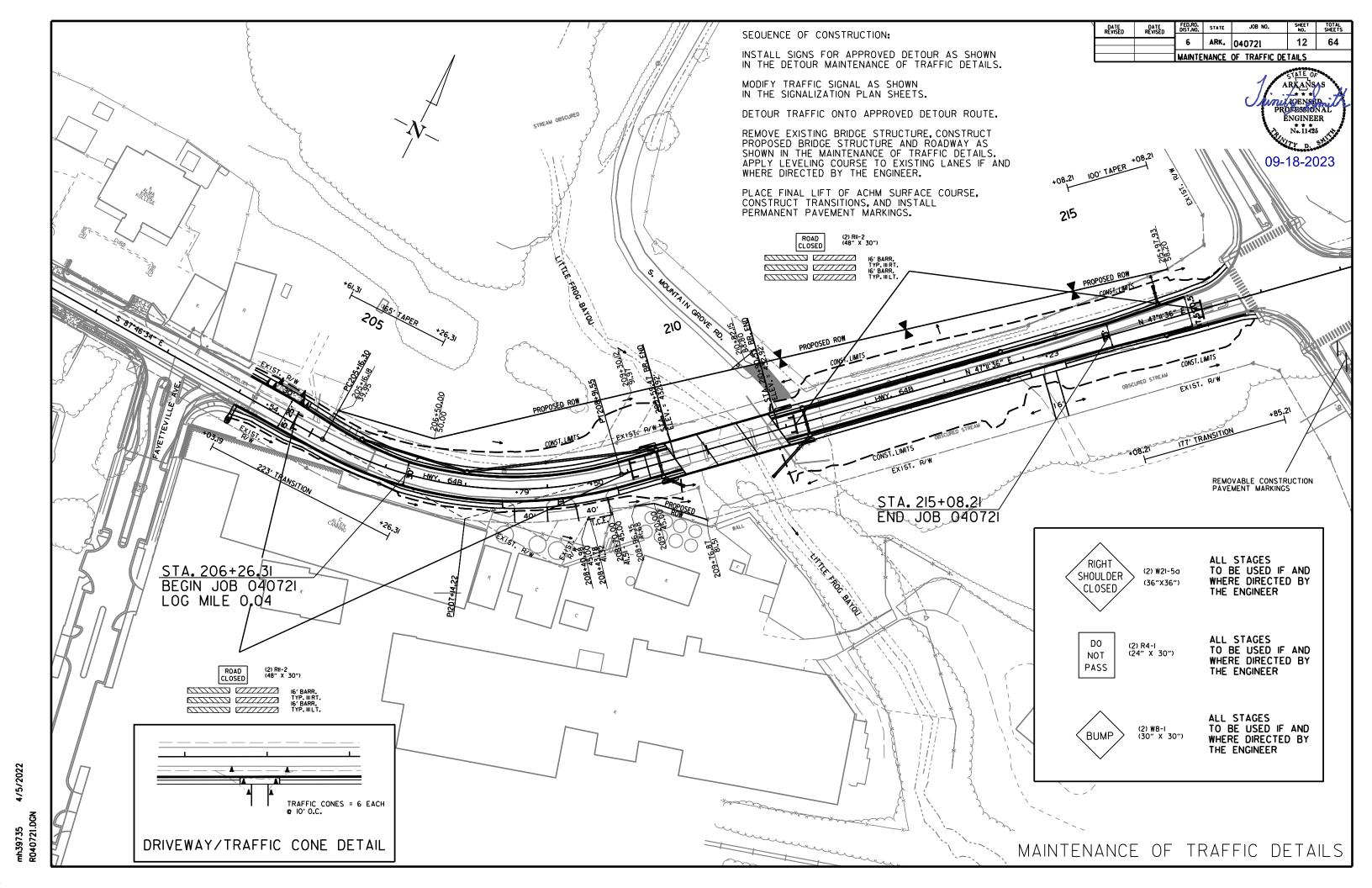
END (I) G20-2 T

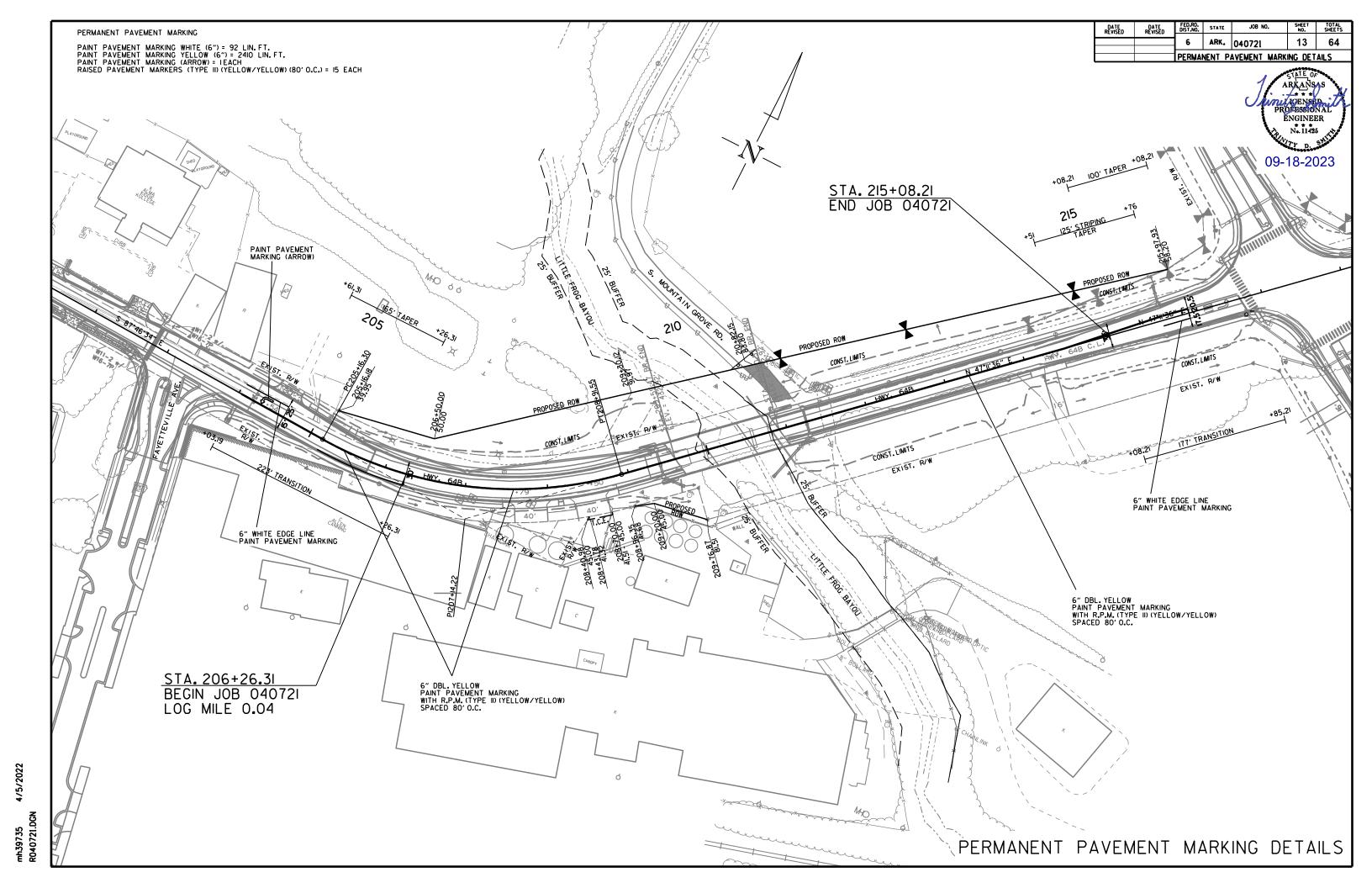
(I) G20-2 (48" × 24") HWY. 64B/FAYETTEVILLE AVE. INTERSECTION

223' TRANSITION

(1) W2O-1 (48" × 48")

> ADVANCE WARNING MAINTENANCE OF TRAFFIC DETAILS





DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS			
		6	ARK.	040721	14	64			
		QUANTITIES							

ARKANSAS
ARKANSAS
PROPESSIONAL
ENGINEER
No. 11425

ADVANCE WARNING SIGNS AND DEVICES

SIGN NUMBER	DESCRIPTION	SIGN SIZE	ENTIRE JOB	MAXIMUM NUMBER REQUIRED	TOTAL SIGN	S REQUIRED	TRAFFIC CONE	BARRICADE	ES (TYPE III)
	_		LIN. FT EACH		NO.	SQ. FT.	EACH	LIN.	FT.
W20-1	ROAD WORK AHEAD	48"x48"	5	5	5	80.0			
G20-2	END ROAD WORK	48"x24"	5	5	5	40.0			
D3-1	HIGHWAYNAME	36"x12"	16	16	16	48.0			
M4-10L	DETOUR WITH ARROW LEFT	48"x18"	6	6	6	36.0			
M4-10R	DETOUR WITH ARROW RIGHT	48"x18"	7	7	7	42.0			
M4-8	DETOUR	24"x12"	4	4	4	8.0			
M6-3	ARROW	21"x15"	4	4	4	8.8			
R11-4	ROAD CLOSED TO THRU TRAFFIC	60"x30"	5	5	5	62.5			
R11-2	ROAD CLOSED	48"x30"	4	4	4	40.0			
R4-1	DO NOT PASS	24"x30"	2	2	2	10.0			
W21-5a	RIGHT SHOULDER CLOSED	36"x36"	2	2	2	18.0			
W8-1	BUMP	30"x30"	2	2	2	12.5			
	TRAFFIC CONES		6	6			6		
	TYPE III BARRICADE-RT. (16')		4	4				64	
	TYPE III BARRICADE-LT. (16')		4	4		_			64
TOTALS:						405.8	6	64	64

NOTE: THIS IS A LOW TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

CONSTRUCTION PAVEMENT MARKINGS AND PERMANENT PAVEMENT MARKINGS

CONSTRUCTION PAVEMENT MARKINGS AND PERMANENT PAVEMENT MARKINGS										
DESCRIPTION	END OF JOB	REMOVABLE CONSTRUCTION PAVEMENT	RAISED PAVEMENT MARKERS	REFLECTORIZED PAVEMENT MARKING	REFLECTORIZED PAINT PAVEMENT MARKING					
		MARKINGS	TYPE II							
			(YELLOW/YELLOW)	ARROWS	WHITE	YELLOW				
	LIN. FT EACH	LIN. FT.	EACH	EACH	LIN	.FT.				
REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	110	110								
RAISED PAVEMENT MARKERS TYPE II (YELLOW/YELLOW)	15		15							
REFLECTORIZED PAVEMENT MAR <ing (arrows)<="" td=""><td>1</td><td></td><td></td><td>1</td><td></td><td></td></ing>	1			1						
REFLECTORIZED PAINT PAVEMENT MARKING WHITE (6")	92				92					
REFLECTORIZED PAINT PAVEMENT MARKING YELLOW (6")	2410					2410				
TOTALS:		110	15	1	92	2410				

NOTE: THIS IS A LOW TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

NOTE: THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT.

THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING.

CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	15	64
		OUANTI	TIES			

CLEARING AND GRUBBING

OLLY INVINO 7 IND ONOBBING						
STATION	STATION STATION LOCATION		CLEARING	GRUBBING		
			STA	TION		
204+03	217+00	HWY 64B LT. & RT.	13	13		
TOTALS:			13	13		

BENCH MARKS

STATION	STATION LOCATION	
		EACH
209+52	209+52 LT. SIDE OF BRIDGE	
TOTAL:	1	
NOTE: CHO	ANT COD INCODMATION ONLY DENIGHTAD	/ C

NOTE: SHOWN FOR INFORMATION ONLY. BENCH MARKS SHALL BE FURNISHED AND PLACED BY STATE FORCES.

REMOVAL AND DISPOSAL OF ITEMS

STATION	STATION	LOCATION	CURB AND GUTTER	CONCRETE DRIVEWAYS	GUARDRAIL
			LIN. FT.	SQ. YD.	LIN. FT.
204+03	205+82	HWY 64B LT. & RT.	175		
	204+54	HWY 64B LT.		40	
209+45	209+76	HWY 64B LT.			31
209+21	209+76	HWY 64B RT.			55
210+77	210+91	HWY 64B LT.			14
210+77	211+09	HWY 64B RT.			32
ALS:			175	40	132

NOTE: THE QUANTITY SHOWN ABOVE FOR THE REMOVAL AND DISPOSAL OF GUARDRAIL SHALL INCLUDE THE REMOVAL AND DISPOSAL OF ALL GUARDRAIL TERMINALS AND TERMINAL ANCHOR POSTS.

STATION

215+97

STATION

211+15

TOTAL:

REMOVAL AND DISPOSAL OF FENCE

HWY 64B LT.

LOCATION

FENCE

LIN. FT.

490

490

STATION	STATION	LOCATION	LENGTH	CONCRETE WALKS	
			LIN. FT.	SQ.YD.	
204+03	204+25	HWY 64B LT.	22	12	
204+03	205+25	HWY 64B RT.	122	68	
204+83	209+53	HWY 64B LT.	470	261	
206+25	207+46	HWY 64B RT.	121	67	
208+11	208+18	HWY 64B RT.	7	4	
208+82	209+67	HWY 64B RT.	85	47	
210+82	216+67	HWY 64B LT.	585	325	
210+98	214+04	HWY 64B RT.	306	170	
214+47	216+87	HWY 64B RT.	240	133	
TOTAL:		<u> </u>	<u> </u>	1087	

4" PIPE UNDERDRAIN

	STATION	STATION	LOCATIONS	4" PIPE UNDERDRAINS LIN. FT.		
*	ENTIRE PRO	OJECT TO B	E USED IF AND	500		
ı	WHERE DIRECTED BY THE ENGINEER					
I						
I	TOTALS:	500				

* NOTE: QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

UNDERDRAINS SHALL BE STUBBED INTO THE PROPOSED DROP INLET IF AND WHERE DIRECTED BY THE ENGINEER. PAYMENT FOR THIS TO BE INCLUDED IN THE UNIT PRICE BID FOR 4" PIPE UNDERDRAIN.

CONCRETE WALKS (TYPE SPECIAL) (RTP-0017(42))

STATION	STATION	LOCATION	CONCRETE WALKS (TYPE SPECIAL)	HAND RAILING	TEXTURED COATING FINISH	
			SQ. YD.	LIN. FT.	SQ. YD.	
205+25	206+25	HWY. 64B RT.	56	100	6	
TOTALS:			56	100	6	

EARTHWORK

STATION	STATION	LOCATION / DESCRIPTION	UNCLASSIFIED EXCAVATION	COMPACTED EMBANKMENT
			CU.	YD.
ENTIRE	PROJECT	MAIN LANES	251	7285
ENTIRE	PROJECT	APPROACHES		145
BRIDGE	END	BENT 1	604	
BRIDGE	END	BENT4	487	
210+90.00	211+15.00	S. MTN. GROVE RD. OBLITERATION	80	
TOTALS:			1422	7430

NOTE: EARTHWORK QUANTITIES SHALL BE PAID AS PLAN QUANTITY.

SOIL STABILIZATION

STATION	STATION	LOCATION / DESCRIPTION	SOIL STABILIZATION TON
ENTIRE	PROJECT	TO BE USED IF AND WHERE	100
		DIRECTED BY THE ENGINEER	
TOTAL:			100
		·	

QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

CONCRETE COMBINATION CURB AND GUTTER

STATION	STATION	LOCATION	TYPE A (1' 6")
			LIN. FT.
204+03	209+25	HWY 64B RT.	522
204+03	209+25	HWY 64B LT.	522
211+25	216+08	HWY 64B LT.	483
211+25	216+08	HWY 64B RT.	483
TOTAL:			2010

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	16	64
		OUANTITIES				

SELECTED PIPE BEDDING

SELECTED PIPE BEDDING				
CU.YD.				
100				
100				

NOTE: QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

COLD MILLING ASPHALT PAVEMENT

STATION	STATION	LOCATION	AVG. WIDTH	COLD MILLING ASPHALT PAVEMENT						
			FEET	SQ. YD.						
204+03.19	206+26.31	MAIN LANES	VAR.	716.44						
215+08.21	216+85.21	MAIN LANES	VAR.	731.90						
										
TOTAL:				1448.34						
NOTE: COLD	NAUL LINIO CTO	OKDII E I OOATION								

NOTE: COLD MILLING STOCKPILE LOCATION 5811 ARKHOLA ROAD, VAN BUREN, AR 72956

PATTY D. SHIT

09-10-202

FENCING

STATION	STATION	LOCATION	* 4' CHAIN LINK FENCE LIN. FT.			
211+15	215+97	HWY 64B LT.	483			
TOTAL:	TOTAL:					
* DEMOTEC /	U TE DAIA TE D	ID ITEM				

* DENOTES ALTERNATE BID ITEM.

ACHM PATCHING OF EXISTING ROADWAY

DESCRIPTION	TON
ENTIRE PROJECT - TO BE USED IF AND WHERE	25
DIRECTED BY THE ENGINEER	
TOTAL:	25
NOTE: OUANTITY ESTIMATED	

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

CONCRETE DITCH PAVING

STATION	LOCATION	LENGTH	"W"	"B"	CONC. DITCH PAVING (TYPE A)	SOLID SODDING	WATER
		LIN. FT.	FEET	FEET	SQ. YD.	SQ. YD.	M. GAL.
204+04.00	HWY 64B RT.	10.00	6.00	4.00	6.67	4.44	0.06
TOTALS:					6.67	4.44	0.06

BASIS OF ESTIMATE:

WATER.....12.6 GAL. / SQ. YD. OF SOLID SODDING.

STONE BACKFILL

STATION	STATION	LOCATION / DESCRIPTION	STONE BACKFILL TON
210+63.02	211+39.12	UNDER HWY 64B BRIDGE - FOR USE IN RETAINING WALL	324
TOTAL:			324

EROSION CONTROL

						ERU	JOION CONTR	OL								
				PERMANENT EROSION CONTROL				TEMPORARY EROSION CONTROL								
STATION	STATION	LOCATION	SEEDING	LIME	MULCH COVER	WATER	SECOND SEEDING APPLICATION	SOLID SODDING	TEMPORARY SEEDING	MULCH COVER	WATER	SAND BAG DITCH CHECKS	ROCK DITCH CHECKS	SILT FENCE	DROP INLET FILTER SOCK (12")	*SEDIMENT REMOVAL & DISPOSAL
							APPLICATION					(E-5)	(E-6)	(E-11)	(E-13)	DISPUSAL
			ACRE	TON	ACRE	M.GAL.	ACRE	SQ.YD.	ACRE	ACRE	M.GAL.	BAG	CU.YD.	LIN. FT.	LIN. FT.	CU. YD.
ENTIRE	PROJECT	CLEARING AND GRUBBING							2.63	2.63	53.7	44	3	2404		92
ENTIRE	PROJECT	ENTIRE PROJECT	0.99	1.98	0.99	108.9	0.99	628	2.63	2.63	53.7				253	9
																
*ENTIRE PRO	I DJECT TO BE U I	J JSED IF AND WHERE DIRECTED BY THE ENGINEER.	0.20	0.40	0.20	22.0	0.20	126	1.05	1.05	21.4	44	6	481	69	24
TOTALS:	I		1.19	2.38	1.19	130.9	1.19	754	6.31	6.31	128.8	88	9	2885	322	125

BASIS OF ESTIMATE:

SAND BAG DITCH CHECKS.......22 BAGS / LOCATION ROCK DITCH CHECKS.......3 CU.YD./LOCATION

FILTER SOCKS........23 LN. FT./4' DIA. INLET

NOTE: THE TEMPORARY EROSION CONTROL DEVICES SHOWN ABOVE AND ON THE PLANS SHALL BE INSTALLED IN SUCH A SEQUENCE
AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION

*QUANTITIES ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

GUARDRAIL

	OARDINE										
STATION	STATION		GUARDRAIL (TYPE C)	STD. SIGN OM4-1 RED DIAMOND REFLECTORIZED END OF ROAD MARKER	CHANNEL POST SIGN SUPPORT (TYPE C)						
			LIN. FT.	SQ. FT.	EACH						
210+63.00	210+88.00	HWY. 64B LT.	25	6.75	3						
TOTALS:			25	6.75	3						

PAVEMENT REPAIR OVER CULVERTS (ASPHALT)

STATION	LOCATION	WIDTH	LENGTH	TON
		FEET		
209+40	HWY. 64B	7.92	.8	8
211+25	HWY. 64B	7.92	, 8	8
TOTAL:				16
AVG. DEPTH	= 9"			

		OUANTI	TIES			
		6	ARK.	040721	17	64
DATE EVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS

09-18-2023

APPROACH SLABS

STATION	LOCATION	APPROACH SLABS (TYPE F)	REINFORCING STEEL-RDWY. (GR. 60)	BASE CRS. (CLASS 7)
		CU.YD.	POUND	TON
209+59.47	HWY 64B	56.61	6680	19.60
211+25.53	HWY 64B	56.61	6680	19.60
•				
_		113.22	13360	39.20
	209+59.47	209+59.47 HWY 64B	STATION LOCATION (TYPE F) CU.YD. 209+59.47 HWY 64B 56.61 211+25.53 HWY 64B 56.61	STATION LOCATION APPROACH SLABS (TYPE F) STEEL-RDWY. (GR. 60) CU.YD. POUND 209+59.47 HWY 64B 56.61 6680 211+25.53 HWY 64B 56.61 6680

	RE	TAINING WALLS
		CLASS S
STATION	LOCATION	CONCRETE-

STATION	STATION	LOCATION	CLASS S CONCRETE- ROADWAY	REINF. STEEL- ROADWAY (GRADE 60)	SELECT GRANULAR MATERIAL	UNCL.EXC. FOR STR ROADWAY
			CU.YDS.	POUNDS	TON	CU.YDS.
210+63.02	211+39.12	UNDER HWY. 64 BRIDGE	216.00	14101	1319	1024
TOTALS:			216.00	14101	1319	1024

DRIVEWAYS

STATION	SIDE	LOCATION	WIDTH	**MODIFIE	ED CURB	PORTLAND CEMENT CONCRETE DRIVEWAY	ACHM SURFACE C LBS. PER SQ.	` '	AGGREGATE BASE COURSE (CLASS 7)
			FEET	STATION	STATION	SQ. YD.	SQ. YD.	TON	TON
204+54	LT.	HWY. 64B	30	204+25	204+83	51.56			
207+79	RT.	HWY. 64B	40	207+45	208+13	140.08			
208+50	RT.	HWY. 64B	40	208+16	208+84	156.08			
214+23	RT.	HWY. 64B	16	214+01	214+45	39.11	101.42	11.16	41.41
* ENTIRE PROJECT	TEMPORARY DRIVI	I ≣S							40.00
TOTALS:						386.83	101.42	11.16	81.41

BASIS OF ESTIMATE:

ACHM SURFACE COURSE (1/2")......93.9% MIN. AGGR...... ..6.1% ASPHALT BINDER

MAXIMUM NUMBER OF GYRATIONS = 115 FOR PG 64-22

* QUANTITY ESTIMATED

SEE SECTION 104.03 OF THE STD. SPECS.

TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC

LOCATION	TON	TACK COAT
ENTRE PROJECT - TO BE USED IF AND WHERE	15	30
DIRECTED BY THE ENGINEER		
TOTALS:	15	30

* QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

STRUCTURES

					OTIVOOTORE	•					
		F	PIPE CULVERT ALTE	RNATES	PIPE CULVERT STORM DRAIN ALTERNATES 2, 3, 4, & 5	FLARED END SECTION ALTERNATES FOR PIPE		P INLETS	SOLID	WATER	
STATION	DESCRIPTION	ALT. 1 (CLASS III)	ALT. 1 (CLASS III)	ALT. 2, 3, 4, 5, 6, AND 7	ALTERNATES 2, 3, 4, & 5	CULVERT ALTERNATES		TYPE	SODDING	WAIEK	STD. DWG. NOS.
		18"	18"	18"	18"	18"	MO	SPECIAL			
				LIN. FT.		EACH	E	ACH	SQ.YD.	M.GAL.	
204+04	CONST. D.I. ON RT.							1			SPECIAL DETAILS
205+60	CONST. D.I. ON LT. W/PIPE OUTLET	130			130		1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
207+00	CONST. D.I. ON LT. W/PIPE OUTLET	228			228		1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
209+40	CONST. D.I. ON LT. W/PIPE OUTLET		32	32			1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
209+40	CONST. D.I. ON RT. W/ PIPE OUTLET & FES	22			26	1	1		5	0.06	FES-1, FES-2, FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
211+25	CONST. D.I. ON LT. W/PIPE OUTLET		32	32			1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
211+25	CONST. D.I. ON RT. W. PIPE OUTLET & FES	65			69	1	1		5	0.06	FES-1, FES-2, FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
213+75	CONST. D.I. ON LT. W/PIPE OUTLET	246			246		1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
213+75	CONST. D.I. ON RT. W/ PIPE OUTLET	246			246		1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
215+75	CONST. D.I. ON LT. W/PIPE OUTLET & FES	16			20	1	1		5	0.06	FES-1, FES-2, FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
215+75	CONST. D.I. ON RT. W/ PIPE OUTLET	98			98		1				FPC-9E, FPC-9M, PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
•											
TOTALS:		1051	64	64	1063	3	10	1	15	0.18	

BASIS OF ESTIMATE:

..12.6 GAL. / SQ. YD. OF SOLID SODDING

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED.

NOTE: FOR C.M. AND PLASTIC PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	18	64
		OUANTI	TIES		•	

09-18-2023

									В	ASE AND S	SURFACIN	G													
			LENGTH	AGGREGA COURSE					TACK COAT				Į ,	ACHM BINDE	R COURSE (1	")				ACHM S	RFACE COU	₹SE (1/2")			
STATION	STATION	LOCATION	LENGTH	TON /			GAL. PER SC			GAL. PER SQ	. YD.)	TOTAL	AVG. WID.		POUND /	PG 64-22	AVG. WID.		POUND /	PG 64-22	AVG. WID.		POUND /	PG 64-22	TOTAL
				STATION	TON	TOTAL WID.	SQ.YD.	GALLON	TOTAL WID.	SQ.YD.	GALLON	GALLONS		SQ.YD.	SQ.YD.			SQ.YD.	SQ.YD.			SQ.YD.	SQ.YD.		PG 64-22
			FEET	CIATION		FEET		CALLON	FEET	J 04.15.	OALLON	CALLONS	FEET	<u></u>		TON	FEET		J 04.15.	TON	FEET		J 54.15.	TON	TON
	LANES																								
204+03.19	204+61.31	TRANSITION & CURB & GUTTER LT. & RT.	58.12	19.50	11.33																38.00	245.40	220.00	26.99	26.99
204+61.31	206+26.31	TRANSITION & NOTCH AND WIDEN CURB & GUTTER (LT. TAPER)	165.00	VAR.	51.02	VAR.	107.66	5.38				5.38	VAR.	53.83	330.00	8.88	VAR.	53.83	220.00	5.92	VAR.	601.74	220.00	66.19	72.11
206+26.31		NOTCH AND WIDEN CURB & GUTTER	298.16	58.50	174.42	20.00	662.58	33.13				33.13	10.00	331.29	330.00	54.66	10.00	331.29	220.00	36.44	28.00	927.61	220.00	102.04	138.48
211+25.53	215+08.21	NOTCH AND WIDEN CURB & GUTTER	382.68	58.50	223.87	20.00	850.40	42.52				42.52	10.00	425.20	330.00	70.16	10.00	425.20	220.00	46.77	28.00	1190.56	220.00	130.96	177.73
215+08.21	216+08.21	TRANSITION & CURB & GUTTER (TAPEF)	100.00	19.50	19.50	32.00	355.56	17.78	32.00	355.56	60.45	78.23									32.00	355.56	220.00	39.11	39.11
		· · · · · · · · · · · · · · · · · · ·																							
															<u> </u>										
ADD	TIONAL FOR	LEVELING		•		•	•					•	•			•	•	•		•			•		
204+03.19	206+26.31	NOTCH AND WIDEN CURB & GUTTER	223.12			VAR.	804.59	40.23	VAR.	804.59	136.78	177.01									VAR.	804.59	VAR.	80.46	80.46
206+26.31	209+24.47	NOTCH AND WIDEN CURB & GUTTER	298.16			18.00	59632	29.82	18.00	596.32	101.37	131.19			<u> </u>						36.00	119264	VAR.	238.53	238.53
211+25.53	215+08.21	NOTCH AND WIDEN CURB & GUTTER	382.68			18.00	765.36	38.27	18.00	765.36	130.11	168.38			 						36.00	1530.72	VAR.	382.68	382.68
															 										
TOTALS:				•	480.14		4142.47	207.13		2521.83	428.71	635.84		B10.32		133.70		810.32		89.13		6848.82	İ	1066.96	1156.09

DATE REVISED	DATE REVISED	FED. RO. DIST. NO.	STATE	JOB NO.	SMEET NO.	TOTAL SHEETS
11211323		6	ARK.	040721	19	64
			0759	0 - QUANTITIES - 6	55374	

SCHEDULE OF BRIDGE QUANTITIES - JOB NO. 040721

			ITEM NO.	SP & 205	801	SP, SS, & 802	SP, SS, & 802	SP & 803	SS & 804	SS & 804	SS & 805	SS & 805	SS & 806	SS & 806	SP, SS, & 807	SS & 807	SS & 808	812	SS & 816	SS & 816	SP JOB 040721	SP JOB 040721	SP JOB 040721	SP JOB 040721
RIDGE NO.	AME PLATE TITLE	UNIT OF STRUCTURE	ITEM	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO)	UNCLASSIFIED EXCAVATION FOR STRUCTURES - BRIDGE	CLASS S CONCRETE - BRIDGE	CLASS S(AE) CONCRETE - BRIDGE	CLASS 2 PROTECTIVE SURFACE TREATMENT	REINFORCING STEEL - BRIDGE (GRADE 60)	EPOXY COATED REINFORCING STEEL (GRADE 60)	STEEL PILING (HP 12X53)	PREBORING	TRANSITIONAL APPROACH RAILING	METAL BRIDGE RAILING (TYPE H2)	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50W)	PAINTING STRUCTURAL STEEL	ELASTOMERIC BEARINGS	BRIDGE NAME PLATE (TYPE D)	FILTER BLANKET	DUMPED RIPRAP	DRILLED SHAFT (60" DIA.)	PERMANENT STEEL CASING (72" DIA.)	CROSSHOLE SONIC LOGGING (60" DIA.)	CORING DRILLED SHAFT
			UNIT	LUMP SUM	CU. YD.	CU. YD.	CU. YD.	SQ. YD.	LB.	LB.	LIN. FT.	LIN. FT.	EACH	LIN. FT.	LB.	TON	CU. IN.	EACH	SQ. YD.	CU. YD.	LIN. FT.	LIN. FT.	EACH	LIN. FT.
		DENT 4				10.10			2.420	420	462								272	100				
		BENT 1			53	18.12			2,120	428	162	90	2						372	198		20		
	ΑĞ	BENT 2				35,98			7,345								1,700.0				50	30	2	25
	BA,	BENT 3				35.98			7,345								1,700.0				50	30	2	
8	64B OVER OG BAYOU	BENT 4			62	18.12			2,120	428	180		2						53	33				
6	≱₭																							
	HIGHW/ LITTLE	130'-0" INTEGRAL W-BEAM	UNIT				276,50	737.0		60,384				250	99,490	2,8		1						
	보드																							
		SITE NO. 1 (EXISTING BRID	GE NO. M1144)	1																				
тот	ALS FOR JO	DB NO. 040721			115	108.20	276.50	737.0	18,930	61,240	2 342	90	4	250	99,490	3 2.8	3,400.0	1	425	231	100	2)60	4	2)25

All steel piling shall be Grade 50 and are required to have QPL approved driving points which will not be paid for directly, but will be considered subsidiary to the Item "Steel Piling (HP 12x53)". All piles shall conform to Standard Drawing No. 55020.

② Quantity shown is for estimating and bidding purposes only. Actual quantity will be determined in the field.

(3) The color of paint shall be Brown equal or close to Fed. Std. 595 B, Color Chip No. 30070 and as approved by the Engineer.

THOMAS GERARD DESIGN SECTION SUPERVISOR

TABLE OF APPROACH SLAB QUANTITIES

(FOR INFORMATION ONLY)

BDIDCE NO	ITEM	REINFORCING STEEL	CONCRETE
DRIDGE NO.	UNIT	LB.	CU. YDS.
07500	Begin Bridge	6680	56.61
0/590	End Bridge	6680	56.61
	BRIDGE NO. 07590	BRIDGE NO. UNIT Begin Bridge 07590	BRIDGE NO. UNIT LB. Begin Bridge 6680 07590



SCHEDULE OF BRIDGE QUANTITIES FAYETTEVILLE AVE. - HWY. 162 (ALMA) (S) CRAWFORD COUNTY

ROUTE 64 SEC. 2C ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

 DRAWN BY:
 BAB
 DATE: 6/22/2022
 FILENAME: b040721_q1.dgn

 CHECKED BY:
 TMG
 DATE: 8/10/2022
 SCALE: No Scale
 DESIGNED BY:

BRIDGE NO. 07590

DRAWING NO. 65374

BRIDGE ENGINEER

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	20	64
		SHMMA	DV OF	OLIANTITIES		

ARKANSAS

ARKANSAS

PROFESSIONAL
ENGINEER

No.11425

AVERY D. SWIFT

09-18-2023

DENOTES ALTERNATE BID ITEMS.

		SUMMA	DV 05	OLIANTITIES &	REVISION	
10-05-2023		6	ARK.	040721	21	64
DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS

SUMMARY OF QUANTITIES (BOX 2 OF 2)

ITEM NUMBER	ITEM	QUANTITY (NHPP-0017(42))	QUANTITY (RTP-0017(42))	TOTAL QUANTITY	UNIT
	STRUCTURES OVER 20' SPAN				
SP & 205	REMOVAL OF EX STING BRIDGE STRUCTURE (SITE NO. 1)	1		1	LUMP SUM
636	BRIDGE CONSTRUCTION CONTROL	1.00		1.00	LUMP SUM
801	UNCLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE	115		115	CU. YD.
SP, SS, & 802	CLASS S CONCRETE-BRIDGE	108.20		108.20	CU. YD.
SP, SS, & 802	CLASS S(AE) CONCRETE-BRIDGE	276.50		276.50	CU. YD.
SP & 803	CLASS 2 PROTECTIVE SURFACE TREATMENT	737.0		737.0	SQ. YD.
SS & 804	REINFORCING STEEL-BRIDGE (GRADE 60)	18930		18930	POUND
SS & 804	EPOXY COATED REINFORCING STEEL (GRADE 60)	61240		61240	POUND
SS & 805	STEEL PILING (HP 12X53)	342		342	LIN. FT.
SP	CORING DRILLED SHAFT	25		25	LIN. FT.
SP	DRILLED SHAFT (60" DIAMETER)	100		100	LIN. FT.
SP	PERMANENT STEEL CASING (72" DIAMETER)	60		60	LIN. FT.
SS & 805	PREBORING	90		90	LIN. FT.
SP	CROSSHOLE SONIC LOGGING (60" DIAMETER)	4		4	EACH
SS & 806	METAL BRIDGE RAILING (TYPE H2)	250		250	LIN. FT.
SS & 806	TRANSITIONAL APPROACH RAILING	4		4	EACH
SP, SS, & 807	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50)	99490		99490	POUND
SS & 807	PAINTING STRUCTURAL STEEL	2.8		2.8	TON
SS & 808	ELASTOMERIC BEARINGS	3400.0		3400.0	CU. IN.
812	BRIDGE NAME PLATE (TYPE D)	1		1	EACH
SS & 816	FILTER BLANKET	425		425	SQ. YD.
SS & 816	DUMPED RIPRAP	231		231	CU. YD.

REVISIONS

	TEVIOLOTO .	
DATE	REVISION	SHEET NUMBER
10/5/2023	REVISED THE STANDARD DRAWINGS LIST TO DISPLAY THE CORRECT DATE FOR THE FOLLOWING STANDARD DRAWINGS: BRIDGE DRAWING 55010 AND ROADWAY DRAWING SHS-2. REVISED THE STANDARD DRAWINGS ATTACHMENT TO INCLUDE THE CORRECT VERSION OF THE STANDARD DRAWING DR-1. REVISED THE SPECIAL PROVISIONS ATTACHMENT TO REMOVE THE DUPLICATE OF CONSTRUCTION IN SPECIAL FLOOD HAZARD AREA SPECAL PROVISION. REVISED THE PROPOSAL TO INCLUDE THE FOLLOWING SUPPLEMENTAL SPECIFICATIONS: SS 617-2 GUARDRAIL DELINEATORS AND SS 723-1 GENERAL REQUIREMENTS FOR SIGNS.	4 & 21

FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED 6 ARK. 040721 22 64 SURVEY CONTROL DETAILS

> ARKANSAS MULICENSER PROFESSIONAL ENGINEER No. 11425 09-18-2023

SURVEY CONTROL COORDINATES

Project Name: s040721 Alma
Date: 5/31/2017
Coordinate System: ARKANSAS STATE PLANE - NORTH ZONE BASED ON GPS CONTROL, 170007 - 170007A &

170016 - 170016A

PROJECTED TO GROUND.
Units: U.S. SURVEY FOOT

Easting Elev Feature Description

765 650533, 6933 437, 156 CTL AHTD STD. MON. STAMPED PN: 1

765 650847, 2599 434, 757 CTL AHTD STD. MON. STAMPED PN: 2

875 650865, 5568 433, 539 CTL AHTD STD. MON. STAMPED PN: 3

123 651242, 8140 431, 544 CTL AHTD STD. MON. STAMPED PN: 4

454 651526, 6682 430, 739 CTL AHTD STD. MON. STAMPED PN: 5

458 651899, 6382 427, 756 CTL AHTD STD. MON. STAMPED PN: 5

673 652537, 5376 426, 007 CTL AHTD STD. MON. STAMPED PN: 6

673 652537, 5376 426, 007 CTL AHTD STD. MON. STAMPED PN: 7

677 651217, 1048 425, 065 CTL AHTD STD. MON. STAMPED PN: 7

715 646400, 6414 465, 411 GPS AHTD GPS *170007

715 646400, 6414 465, 411 GPS AHTD GPS *170017A

6465704, 5127 416, 459 GPS AHTD GPS *170016A

688 646959, 5117 414, 294 GPS AHTD GPS *170016A

6929 651931, 4477 429, 465 TBM CHISELED SQUARE AT BASE OF SIGNAL POLE

681 650760, 1772 436, 322 TBM CHISELED SQUARE 1N CONC ISLAND

715 648376, 0337 435, 938 TBM CHISELED SQUARE 1N CONC ISLAND Point Name Northing 425072, 7765 425110, 1584 424660, 1875 425144, 4123 425388, 0454 425750, 8458 426321, 1673 425479, 907 427118, 7430 427292, 8715 420991, 7285 421365, 4068 425749, 7929 425169, 3681 425830, 1335

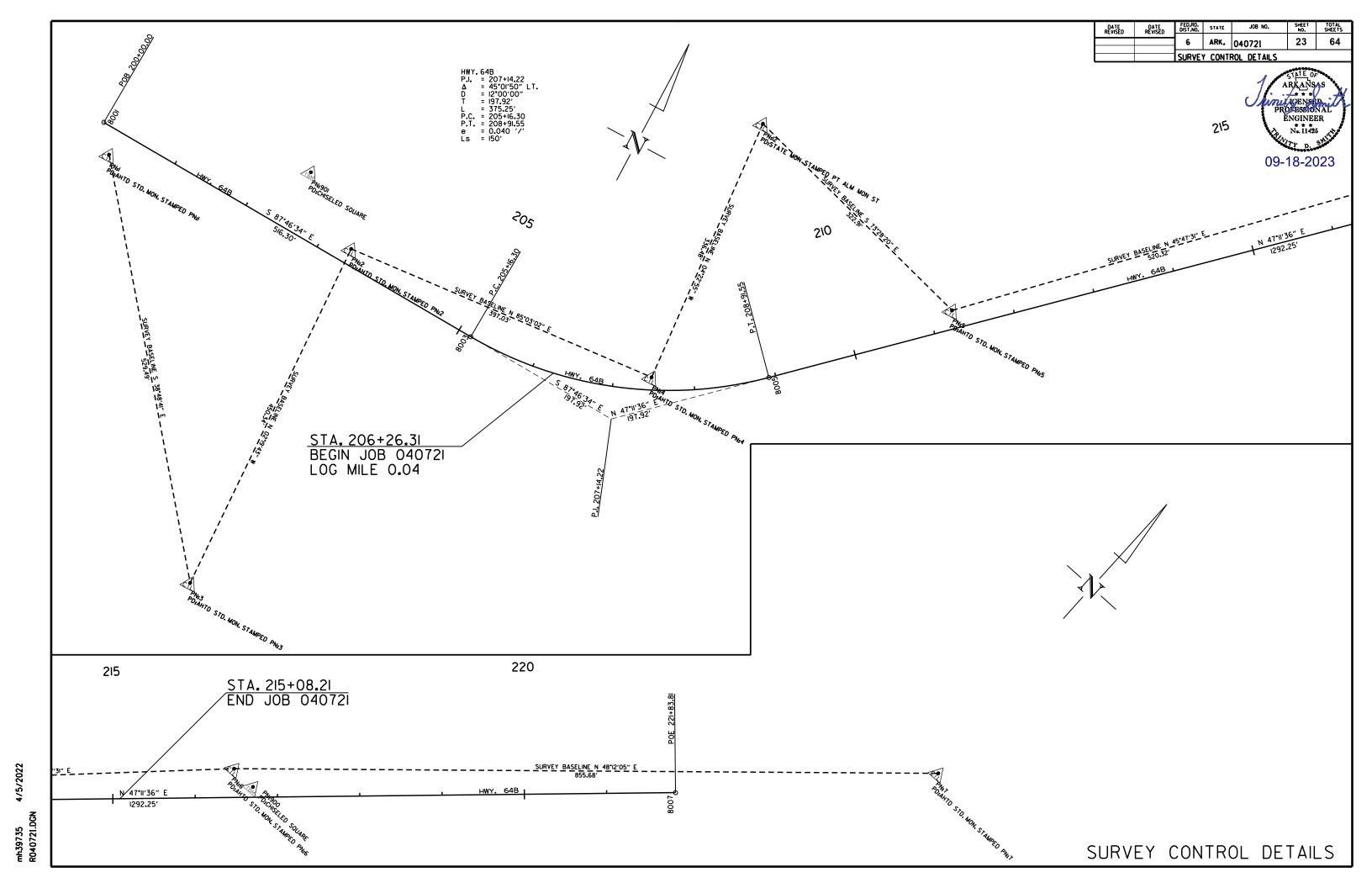
*Note - Rebar and Cap - Standard - 5/8' Rebar with 2' Aluminum Cap stamped
*(standard markings common to all caps), or as indicated
(other markings indicated in the point description of the individual point),
ALL DISTANCES ARE GROUND,
USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT,
A PROJECT CAF OF 0.9999/76161 HAS BEEN USED TO COMPUTE THE ABOVE GROUND COORDINATES.
THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS,
GRID DISTANCE - GROUND DISTANCE X CAF,
GRID DISTANCE - GROUND DISTANCE X CAF,
GRID COORDINATES ARE STORED UNDER FILE NAME s040721 LITTLE FROG BAYOUgi.CTL
HORIZONTAL DATUM: NAD 83 (1997)
VERTICAL DATUM: NAVD 88 POSITIONAL ACCURACY THIRD ORDER, UNLESS SPECIFIED OTHERWISE
AT A SPECIFIC POINT.

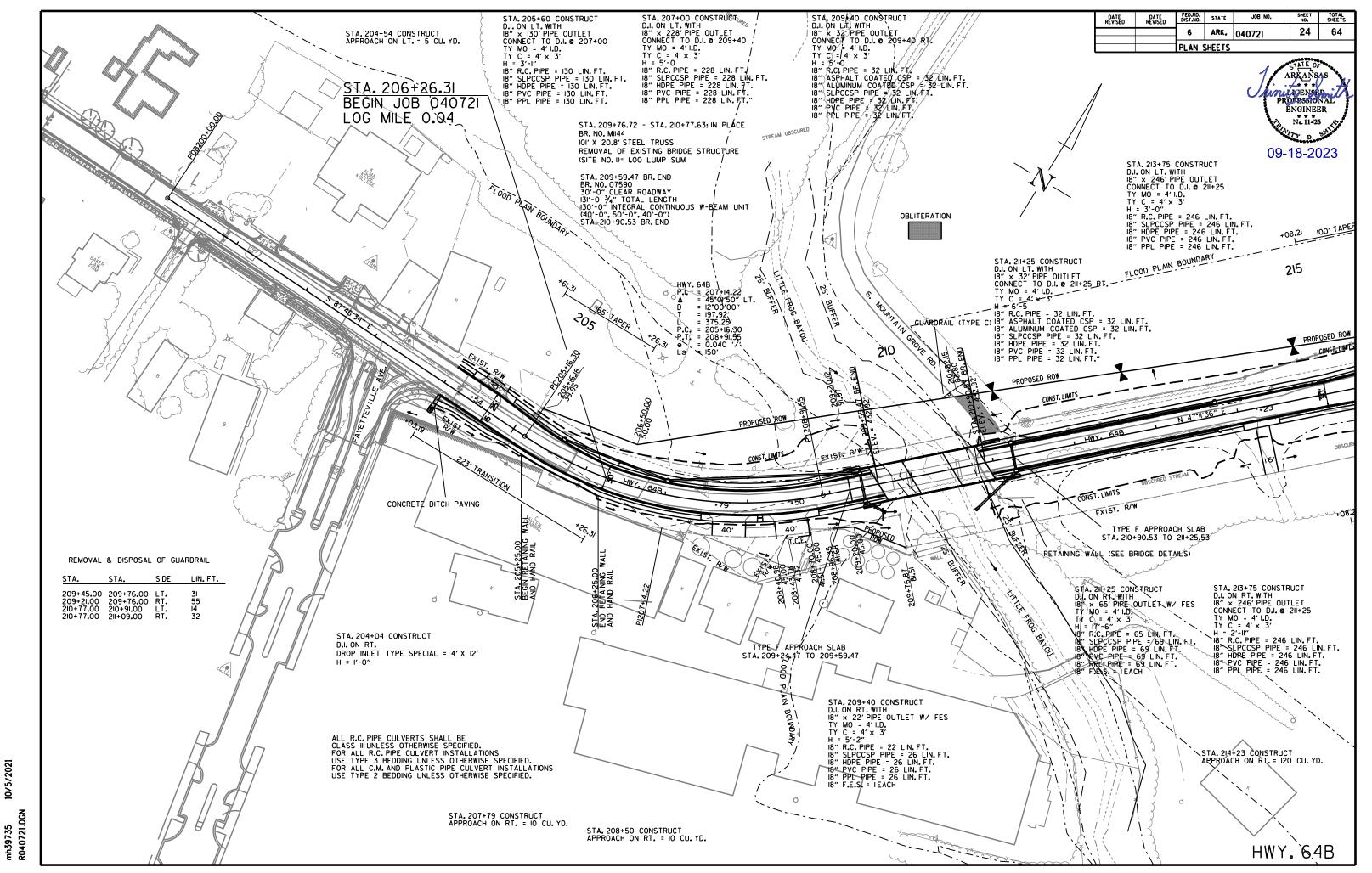
REFERENCE POINTS (1500 SERIES) ARE TO BE USED TO ESTABLISH CONTROL IF THE PRIMARY CONTROL POINTS LISTED ABOVE HAVE BEEN DESTROYED. REFERENCE POINTS ARE NOT TO BE USED FOR VERTICAL CONTROL

BASIS OF BEARING:
ARKANSAS STATE PLANE GRID BEARINGS - 0301-NORTH ZONE
DETERMINED FROM GPS CONTROL POINTS: 170007 - 170007A & 170016 - 170016A
CONVERGENCE ANGLE: 01 17 30. 42 LEFT AT PN: 5 LT:N 35-28-53, 33 LG:W 094-13-11,79
GRID AZIMUTH - ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

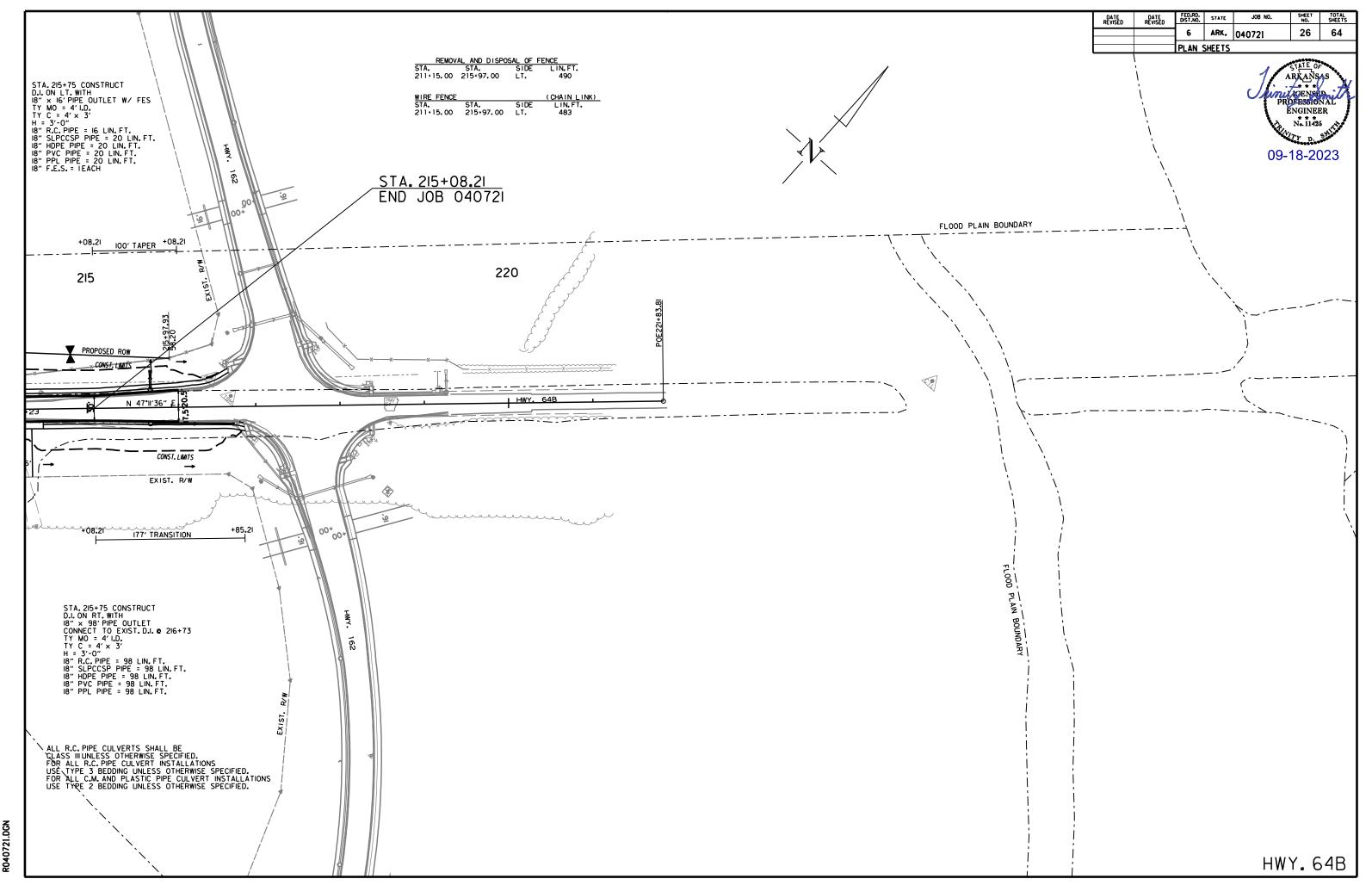
HWY. 64B

POINT NO.	TYPE	STATION	NORTHING	EASTING
8001	POB	200+00.00	425104, 4368	650509, 7235
8003	PC	205+16.30	425084, 4016	651025, 6348
8005	PT	208+91.55	425561, 5068	651044, 1629
8007	POE	221+83.81	425211, 2136	651368, 6121





FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED ARKANSAS 6 ARK. 040721 25 64 LICENSEI PROFILE SHEETS ENGINEER No. 11425 STA. 204.03.80 BEGIN SUPERELEVATION STA. 205+53.80 MAX SUPERELEVATION (0.040'/') STA. 208+00.00 MAX SUPERELEVATION (0.040'/') STA. 209+50.00 END SUPERELEVATION 09-18-2023 HWY. 64B LT. SIDE STORM SEWER 445 440 440 STA. 205+60 TOP ELEV.=434.33 F.L. ELEV.=431.26 5 STA. 207+00 5 TOP ELEV.=433.95 6 F.L. ELEV.=428.95 STA. 209+40 VC= 200' e= -0.05' K= 952.38 TOP ELEV.=433.12 6 6 F VC= 150' e= 0.08' K= 365.85 VC= 200' e= -0.39' K= I29.87 STA. 213+75 TOP ELEV.=430.42 F.L. ELEV.=427.42 435 435 e= 0.06' / K= 227.27 -0.44% 18"× 130' @ 1.59% e= 0.18' K= 69.44 430 430 18"x 228 18"× 246' @ 0.30% P.V.I. 214+75.00 428.67' 31.3 425 425 D.A. = 16.2 SQ. MI. STA. 206+26.31 BEGIN JOB 040721 420 420 FOR THE CONSTRUCTION OF TEMPORARY WORK RAMPS OR HAUL ROADS. 025 DESIGN THIS STREAM, STA. 210+20 TO STA. 210+60, IS CLASSIFIED AS HIGH WATER 421.30 LOG MILE 0.04 A PERENNIAL STREAM. THE TOP OF CHANNEL ELEVATION IS 413 FT. MSL. REFER TO SECTION 110.05(c) OF THE 2014 STANDARD SPECIFICATIONS. 415 415 410 410 405 205+00 200+00 201+00 202+00 203+00 204+00 206+00 207+00 208+00 209+00 210+00 211+00 212+00 213+00 214+00 215+00 STA. 209+59.47 BR. END STA. 210+90.53 BR. END ELEV. = 432.92' 'ELEV. = 432.92' 445 REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA. HWY. 64B RT. SIDE STORM SEWER 445 A34.89 A34.89 A34.89 A34.89 440 440 VC= 200' e= -0.05' K= 952.38 VC= 150' e= 0.08' K= 365.85 VC= 100' e= 0.06' K= 227.27 VC= 200 435 STA. 213+75 76 65 TOP ELEV.=430.42 57 F.L. ELEV.=427.47 57 435 e= -0.39' K= 129.87 AC= 100, e= 0.18' K= 69.44 430 430 18"× 246' @ 0.30% 18"x 22' © 1.90% 213 P.V.I. 214+75.1 428.67 425 425 D.A. = 16.2 SQ. MI. NOTE: 420 420 FOR THE CONSTRUCTION OF TEMPORARY WORK RAMPS OR HAUL ROADS, THIS STREAM, STA. 210+20 TO STA. 210+60, IS CLASSIFIED AS A PERENNIAL STREAM. THE TOP OF CHANNEL ELEVATION IS 413 FT. MSL. REFER TO SECTION IIO.05(G) OF THE 2014 STANDARD SPECIFICATIONS. 025 DESIGN HIGH WATER 421.30 1 18" × 65' € 3.00% 415 415 410 410 405 200+00 201+00 202+00 203+00 204+00 205+00 206+00 207+00 208+00 209+00 210+00 211+00 212+00 213+00 214+00 215+00



10/5/2021

FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED ARKANSAS PROFESSIONAL 27 ARK. 040721 6 64 PROFILE SHEETS ENGINEER No. 11425 09-18-2023 HWY. 64B LT. SIDE STORM SEWER 445 445 440 440 STA. 2|5+75 TOP ELEV.=428.70 F.L. ELEV.=425.70 F.L. OUTLET=42|.86 435 435 VC= 100' e= 0.18' K= 69.44 430 430 -0.10% 425 425 420 420 STA. 215+08.21 END JOB 040721 415 415 410 410 405 405 220+00 221+00 223+00 227+00 228+00 215+00 216+00 217+00 218+00 219+00 222+00 224+00 225+00 226+00 229+00 230+00 445 REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA. HWY. 64B RT. SIDE STORM SEWER 445 440 440 STA. 215+75 TOP ELEV.=428.75 F.L. ELEV.=425.75 000 4. 000 4. 000 4. 000 4. 000 6. 435 435 VC= 100' e= 0.18' K= 69.44 430 430 -0.10% 425 425 420 420 415 415 410 410 223+00 227+00 215+00 216+00 217+00 218+00 219+00 220+00 221+00 222+00 224+00 225+00 226+00 228+00 229+00 230+00

mh39735 10/5/2021 R040721.DGN

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	28	64
		SUMMA	RY OF	TRAFFIC SIGNAL	OHANI	ITIES

ARKANSAS

ARKANSAS

PROFESSIONAL

ENGINEER

N. 11425

109-18-2023

SUMMARY OF TRAFFIC SIGNAL QUANTITIES

ITEM NUMBER	ITEM	TRAFFIC	RETURN TO EXISTING OPERATIONS HWY. 162 & HWY. 64B	QUANTITY	UNIT
SP	TRAFFIC SIGNAL CONTROLLER (MODIFICATION)	1	1	2	EACH

MAINTENANCE OF TRAFFIC:

SEE MAINTENANCE OF TRAFFIC DETAILS FOR SEQUENCE OF CONSTRUCTION. ONCE THE SOUTH LEG OF THE INTERSECTION IS CLOSED TO TRAFFIC BAG THE FOLLOWING SIGNAL HEADS 1, 4, 5, 6, AND 21 AS SHOWN IN THE SIGNAL PLANS. OMIT PHASES 1, 3, AND 8 FROM THE CONTROLLER. THE FOLLOWING DETECTION ZONES WILL NOT BE DETECTED DURING MAINTENANCE OF TRAFFIC Vz11, Vz12, Vz31, Vz32, Vz81, AND Vz82. RETURN TO EXISTING SIGNAL OPERATIONS:

PRIOR TO REOPENING THE SOUTH LEG OF THE INTERSECTION UNBAG SIGNAL HEADS 1, 4, 5, 6, AND 21. ADD PHASES 1, 3, AND 8 BACK INTO THE CONTROLLER. ADD DETECTION ZONES Vz11, Vz12, Vz31, Vz32, Vz81, AND Vz82. RETURN TRAFFIC SIGNAL TO THE EXISTING OPERATION PRIOR TO MAINTENANCE OF TRAFFIC AS SHOWN IN THE SIGNAL PLANS.

LOCATION: HIGHWAY 162 AND HIGHWAY 64B (E. CHERRY ST.)

SCALE: N/A

CITY: ALMA

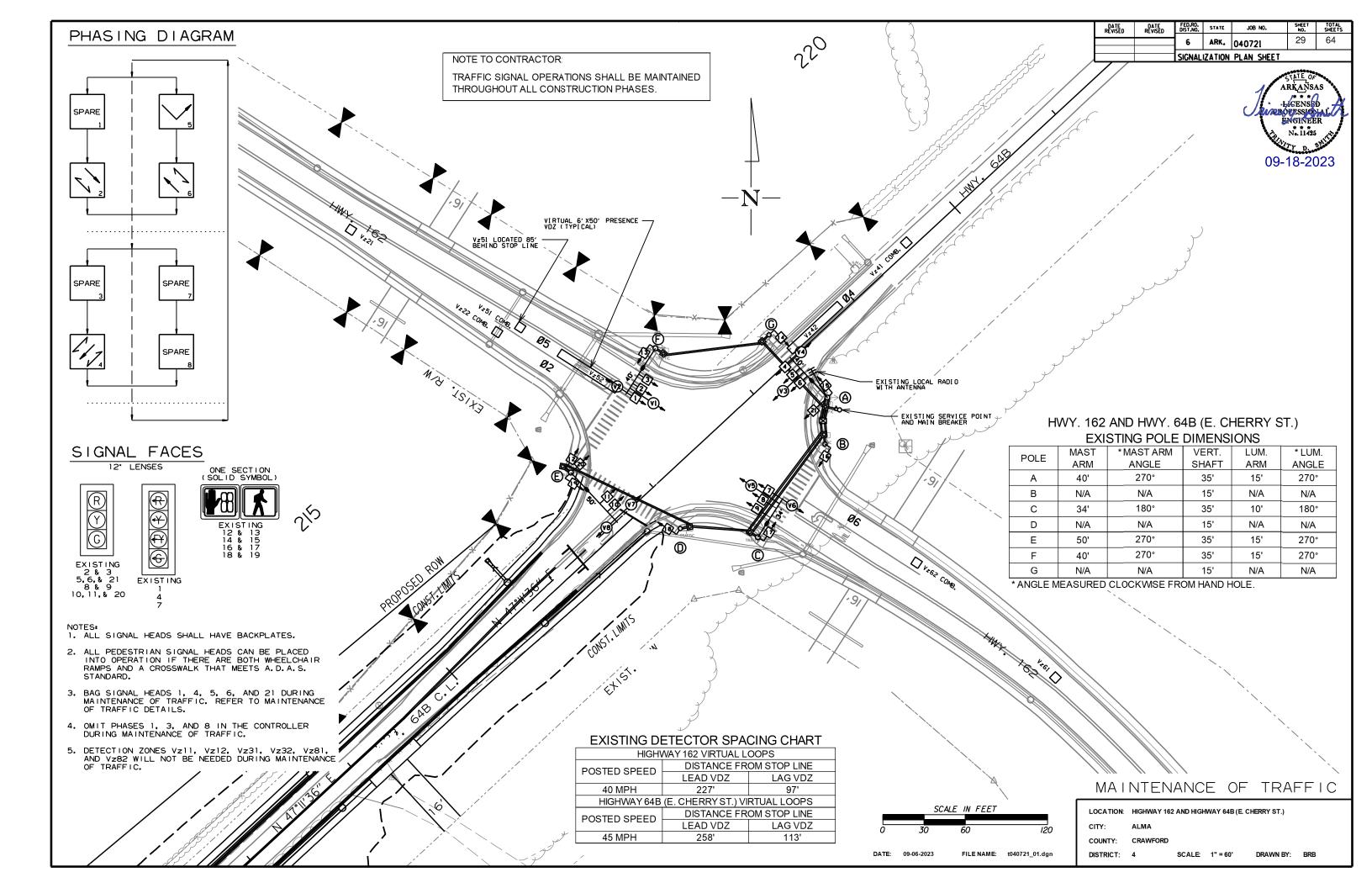
DISTRICT: 4

FILE NAME: t040721_01.dgn

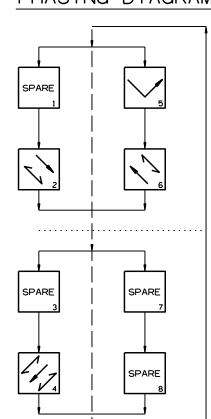
DATE: 09-06-2023

COUNTY: CRAWFORD

DRAWN BY: BRB

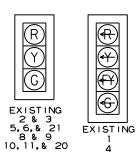


PHASING DIAGRAM

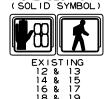


SIGNAL FACES

12" LENSES ONE SECTION (SOLID SYMBOL)



EXISTING



1. ALL SIGNAL HEADS SHALL HAVE BACKPLATES.

- 2. ALL PEDESTRIAN SIGNAL HEADS CAN BE PLACED INTO OPERATION IF THERE ARE BOTH WHEELCHAIR RAMPS AND A CROSSWALK THAT MEETS A.D.A.S.
- 3. BAG SIGNAL HEADS 1, 4, 5, 6, AND 21 DURING MAINTENANCE OF TRAFFIC. REFER TO MAINTENANCE OF TRAFFIC DETAILS.
- 4. OMIT PHASES 1, 3, AND 8 IN THE CONTROLLER DURING MAINTENANCE OF TRAFFIC.
- 5. DETECTION ZONES Vz11, Vz12, Vz31, Vz32, Vz81, AND Vz82 WILL NOT BE NEEDED DURING MAINTENANCE OF TRAFFIC.

DETECTOR CHART

	DETECTOR SYSTEM DESCRIPTION: JOB 040721										
HWY.	162 AND HWY. 64B (E. CH	ERRYST	.)	HARD	HARDWARE INPUTS			PROGRAM ASSIGNMENTS			
	DETECTOR ASSIGNMEN	ITS		B١	BYSUPPLIER		LOCAL MASTER SYSTEM			COMMENTS	TUBE
DET. ID#	LOCATION DIRECTION	TYPE	DET.#	CAB.	AMP	CON.	PHS	SYSTEM	DETECTOR	COMMENTS	LENGTHS
DL1.1D#	EGCATION BIRECTION	1111	DL1.#	TRM.#	CHN.#	IMP.#	FIIS	DET.#	NUMBERS		
Vz21	EB ADVANCE	LOCAL			5	V2	2			CAMERA V2	74"
V z 22	EB NEAR	COMB.			6	V10	2	2		CAMERA V5	74"
Vz41	SB ADVANCE	LOCAL			13	V4	4			CAMERA V4	74"
Vz42	SB NEAR	COMB.			14	V12	4	4		CAMERA V7	74"
Vz51	EB LEFT TURN FAR	COMB.			7	V13	5	5		CAMERA V5	74"
Vz52	EB LEFT TURN	LOCAL			8	V5	5			CAMERA V5	74"
Vz61	WB ADVANCE	LOCAL			3	V6	6			CAMERA V6	74"
Vz62	WB NEAR	COMB.			4	V14	6	6		CAMERA V1	74"
PB2 A&B	HWY 64B S. LEG	PED.				P2	2				
PB4 A&B	HWY 162 W. & E. LEGS	PED.				P4	4				
PB6 A&B	HWY. 64B N. LEG	PED.				P6	6				
					SPARE:	1, 2, 9, 10	0, 11, 12,	15, & 16		<u> </u>	

CONTROLLER INPUT ABBREVIATIONS:

V = VEHICLE INPUT

D = SYSTEM OR AUXILIARY INPUT

P = PEDESTRIAN INPUT

NOTE: "AMP CHN =" REFERS TO THE RACK OUTPUT POSITION.

THIS IS WIRED TO CONTROLLER INPUT DETECTOR NUMBER WHICH IS PROGRAMMED TO ACTUATE THE DESIGNATED PHASE.

EXAMPLE: V9 = SYSTEM DETECTOR 1, V10 = SYSTEM DETECTOR 2

INTERVAL CHART

	HW	/. 162 AN		FLASH					
SIGNAL FACES	2+5	CLR.	2+6	CLR.	4	CLR.		SEQUENCE	
2 & 3	R	R	G	Υ	R	R		R	
7	< G	←¥	< F Y	*	←R	∢R		←R	
8 & 9	G	**	G	**	R	R		R	
10, 11, & 20	R	R	R	R	G	Y		R	
12 & 13	DW	DW	DW	DW	W	FDW		BLK	
14 & 15	DW	DW	W	FDW	DW	DW		BLK	
16 & 17	DW	DW	DW	DW	W	FDW		BLK	
18 & 19	W	FDW	W	FDW	DW	DW		BLK	

- * DENOTES GREEN OR YELLOW ARROW DEPENDING ON NEXT PHASE
- ** DENOTES GREEN OR YELLOW BALL DEPENDING ON NEXT PHASE

MAINTENANCE OF TRAFFIC

FED.RD. DIST.NO. STATE

6

JOB NO.

ARK. 040721

SIGNALIZATION PLAN SHEET

30

ARIANSAS

MUJUENSEE

PROFESSIONAL

ENGINEER * * * No. 11425

09-18-2023

64

DATE REVISED

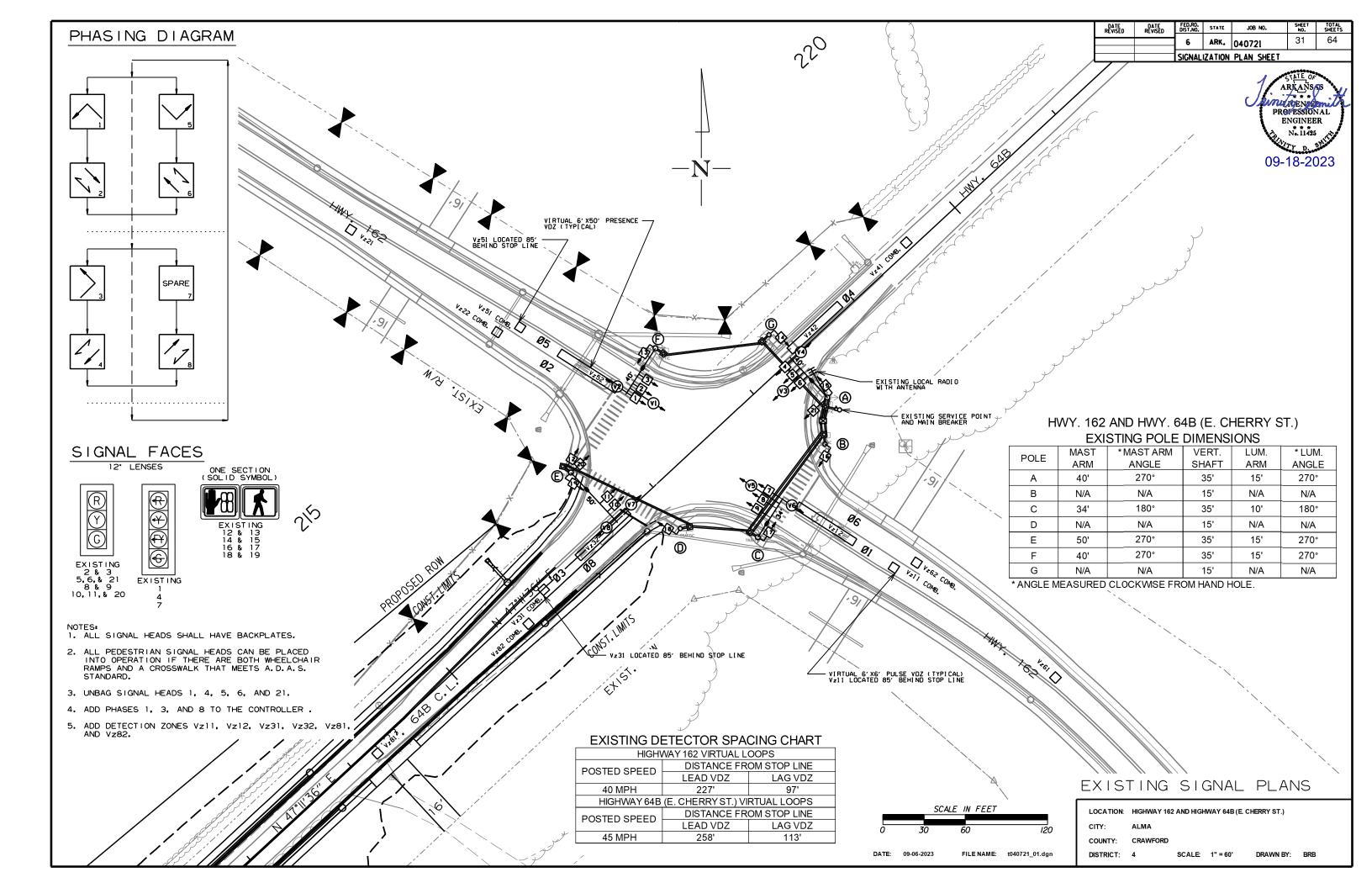
DATE REVISED

LOCATION: HIGHWAY 162 AND HIGHWAY 64B (E. CHERRY ST.)

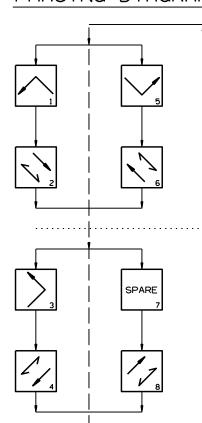
ALMA COUNTY: CRAWFORD

DISTRICT: 4 SCALE: N/A DRAWN BY: BRB

FILE NAME: t040721_01.dgn DATE: 09-06-2023

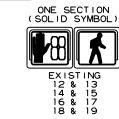


PHASING DIAGRAM



SIGNAL FACES

12" LENSES



EXISTING 2 % 3 5,6,8 21 EXISTING 8 % 9 1 10,11,8 20 4

NOTES:

1. ALL SIGNAL HEADS SHALL HAVE BACKPLATES.

- 2. ALL PEDESTRIAN SIGNAL HEADS CAN BE PLACED INTO OPERATION IF THERE ARE BOTH WHEELCHAIR RAMPS AND A CROSSWALK THAT MEETS A.D.A.S. STANDARD.
- 3. UNBAG SIGNAL HEADS 1, 4, 5, 6, AND 21.
- 4. ADD PHASES 1, 3, AND 8 TO THE CONTROLLER .
- 5. ADD DETECTION ZONES Vz11, Vz12, Vz31, Vz32, Vz81, AND Vz82,

DETECTOR CHART

			DETE	CTOR SY	STEM DI	ESCRIPT	ION: JOB	040721			
HWY.	162 AND HWY. 64B (E. CH	IERRY ST	.)	HARDWARE INPUTS			PROGRAM ASSIGNMENTS				
	DETECTOR ASSIGNMEN	NTS		B١	SUPPLIE	ER	L	LOCAL MASTER SYSTEM			TUBE
DET. ID#	LOCATION DIRECTION	TYPE	DET.#	CAB. TRM.#	AMP CHN.#	CON.	PHS	SYSTEM DET.#	DETECTOR NUMBERS	COMMENTS	LENGTHS
Vz11	WB LEFT TURN FAR	сомв.			1	V9	1	1		CAMERA V1	74"
Vz12	WB LEFT TURN	LOCAL			2	V1	1			CAMERA V1	74"
11.01	55 45 44 44				_	- 10				0.111551.115	
Vz21	EB ADVANCE	LOCAL			5	V2	2			CAMERA V2	74"
Vz22	EB NEAR	COMB.			6	V10	2	2		CAMERA V5	74"
Vz31	NB LEFT TURN FAR	COMB.			9	V11	3	3		CAMERA V3	74"
Vz32	NB LEFT TURN	LOCAL			10	V3	3			CAMERA V3	74"
1/-44	OD ADVANOE	1.00.41			40)//	4			OAMEDA 1/4	7.411
Vz41 Vz42	SB ADVANCE SB NEAR	LOCAL COMB.			13 14	V4 V12	4	4		CAMERA V4	74" 74"
V Z-1Z	OBINEAUX	OOMB.			17	, 12		7		O WILL VY	, ,
Vz51	EB LEFT TURN FAR	COMB.			7	V13	5	5		CAMERA V5	74"
Vz52	EB LEFT TURN	LOCAL			8	V5	5			CAMERA V5	74"
Vz61	WB ADVANCE	LOCAL			3	V6	6			CAMERA V6	74"
Vz62	WB NEAR	COMB.			4	V14	6	6		CAMERA V1	74"
Vz81	NB ADVANCE	LOCAL			44	V8	8			CAMEDANO	7.411
Vz81 Vz82	NB ADVANCE NB NEAR	COMB.	<u> </u>		11 12	V8 V16	8	8		CAMERA V8	74" 74"
7202	THE THEFT	001113			12	7.10	Ů	-		07111214140	
PB2 A&B	HWY 64B S. LEG	PED.				P2	2				
PB4 A&B	HWY 162 W. LEG	PED.				P4	4				
PB6 A&B	HWY. 64B N. LEG	PED.				P6	6				
PB8 A&B	HWY 162 E. LEG	PED.				P8	8			_	
			_		SPARE:	15 & 16			<u> </u>		

CONTROLLER INPUT ABBREVIATIONS:

V = VEHICLE INPUT

D = SYSTEM OR AUXILIARY INPUT

P = PEDESTRIAN INPUT

TE: "AMP CHN =" REFERS TO THE RACK OUTPUT POSITION.

THIS IS WIRED TO CONTROLLER INPUT DETECTOR NUMBER WHICH IS PROGRAMMED TO ACTUATE THE DESIGNATED PHASE.

EXAMPLE: V9 = SYSTEM DETECTOR 1, V10 = SYSTEM DETECTOR 2

INTERVAL CHART

						_ \		` '					
				ΗW	Y. 162 AN	ND HWY.	64B (E.	CHERRY	'ST.)				FLASH
SIGNAL FACES	1+5	CLR.	1+6	CLR.	2+5	CLR.	2+6	CLR.	3+8	CLR.	4+8	CLR.	SEQUENCE
1	←6	*	< C	*	< FY	***	< F ¥	***	←R	←R	←R	←R	< R−
2 & 3	R	R	G	**	R	R	G	**	R	R	R	R	R
4	←R	←R	←R	< R	<r< del=""></r<>	<r< del=""></r<>	< Γ	< R	< 6	*	← FY	***	<r-< del=""></r-<>
5, 6, & 21	R	R	R	R	R	R	R	R	G	**	G	**	R
7	< C	*	< F ¥	***	< 6	*	< F ¥	***	< R	←R	←R	←R	<r< del="">−</r<>
8 & 9	R	R	R	R	G	**	G	**	R	R	R	R	R
10, 11, & 20	R	R	R	R	R	R	R	R	R	R	G	**	R
12 & 13	DW	DW	DW	DW	DW	DW	DW	DW	DW	DW	W	FDW	BLK
14 & 15	DW	DW	W	FDW	DW	DW	W	FDW	DW	DW	DW	DW	BLK
16 & 17	DW	DW	DW	DW	DW	DW	DW	DW	W	FDW	W	FDW	BLK
18 & 19	DW	DW	DW	DW	W	FDW	W	FDW	DW	DW	DW	DW	BLK

- * DENOTES GREEN OR YELLOW ARROW DEPENDING ON NEXT PHASE
- ** DENOTES GREEN OR YELLOW BALL DEPENDING ON NEXT PHASE
- *** DENOTES FLASHING YELLOW ARROW OR YELLOW ARROW DEPENDING ON NEXT PHASE

EXISTING SIGNAL PLANS

FED.RD. DIST.NO. STATE

6

JOB NO.

ARK. 040721

SIGNALIZATION PLAN SHEET

32

ARKANSAS

MUZDENSSEMIA

PROFESSIONAL

ENGINEER

No. 11425

09-18-2023

64

DATE REVISED DATE REVISED

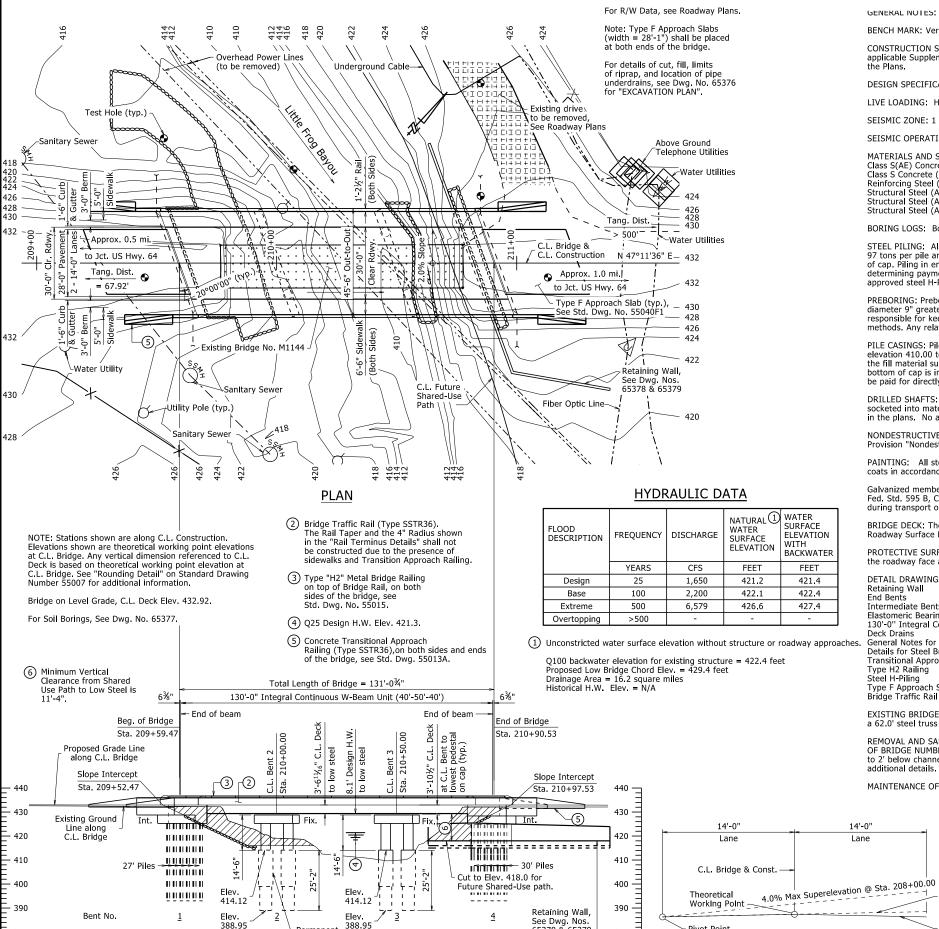
LOCATION: HIGHWAY 162 AND HIGHWAY 64B (E. CHERRY ST.)

CITY: ALMA
COUNTY: CRAWFORD

DISTRICT: 4 SCALE: N/A

DRAWN BY: BRB

DATE: 09-06-2023 FILE NAME: t040721_01.dgn



Permanent

Casing (typ.)

ELEVATION

- 380

- 370 🖇

65378 & 65379

211

380 -

370 —

CROSS SLOPE TRANSITION SKETCH

Looking Ahead

GENERAL NOTES:

BENCH MARK: Vertical Control Data are shown on the Survey Control Data Sheets.

DATE REVISED	DATE REVISED	FED. RO. DIST. NO.	STATE	JOB NO.	SHEET NO.	SHEETS		
		6	ARK.	040721	33	64		
		- 07590 - LAYOUT - 65375						

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 edition) with ental Specifications and Special Provisions. Section and Subsection refer to the Standard Construction Specifications unless otherwise noted in

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications, Eighth Edition (2017).

LIVE LOADING: HL-93

SEISMIC ZONE: 1 SD1 = 0.091SITE CLASS: C

SEISMIC OPERATIONAL CLASSIFICATION: Other

MATERIALS AND STRENGTHS: Class S(AE) Concrete (Superstructure) Class S Concrete (Substructure) f'c = 4,000 psif'c = 3,500 psiReinforcing Steel (Grade 60, AASHTO M 31 or M 322, Type A) Structural Steel (ASTM A709, Gr. 50W) fy = 60,000 psiFv = 50.000 psiStructural Steel (ASTM A709, Gr. 50) Fy = 50,000 psiStructural Steel (ASTM A709, Gr. 36) Fy = 36,000 psi

BORING LOGS: Boring logs may be obtained from the Construction Contract Development Section of the Program Management Division.

STEEL PILING: All piling shall be HP 12 x 53 (Grade 50) and shall be driven with an approved air, steam, or diesel hammer to a minimum safe bearing capacity of 97 tons per pile and into the material designated as slightly weathered or unweathered shale on the boring legend. Minimum penetration shall be 15' below botton of cap. Piling in end bents shall be driven after embankment to bottom of cap is in place. Lengths of piling shown are for estimating quantities and for use in determining payment for cut-off and build-up in accordance with Section 805. Actual pile lengths are to be determined in the field. The Contractor shall use approved steel H-Pile driving points on all piles.

PREBORING: Preboring is required for all piling at Bent 1. The depth of preboring shall be to a depth of 10' below bottom of cap. Prebored holes shall have a diameter 9" greater than the diagonal of the pile. After driving is completed, the prebored holes shall be backfilledwith sand or pea gravel. The Contractor shall be responsible for keeping prebored holes free of debris prior to driving piles and backfilling which may require the use of temporary casings or other approved methods. Any related cost for backfilling and temporary casing will not be paid for directly, but shall be considered subsidiary to the item "Preboring"

PILE CASINGS: Pile casings are required for piling at Bent 4. Casings shall be installed prior to or during retaining wall construction and shall extend from elevation 410.00 to bottom of cap. Pile casing material shall be of sufficient strength to retain its original form free from harmful distortions after compaction of the fill material surrounding it. The minimum inside diameter of the casing shall be 18". Piles shall be driven through the open casings after embankment to bottom of cap is in place. After driving is complete, the pile casings shall be filled with sand of pea gravel to completely fill voids. Pile casings and backfill will not be paid for directly but shall be considered subsidiary to the item "Steel Piling (HP 12x53)".

DRILLED SHAFTS: Drilled shafts at Bents 2 and 3 shall be constructed in accordance with Special Provision "Drilled Shaft Foundations". Drilled shafts shall be socketed into material designated as slightly weathered to unweathered shale on the boring legend and to the minimum rock penetrations and tip elevations show in the plans. No adjustment to plan tip elevations shall be made without prior approval from the Engineer. Temporary casing may be required.

NONDESTRUCTIVE TESTING: Crosshole Sonic Logging (CSL) shall be performed on each drilled shaft. Testing shall be performed in accordance with Special Provision "Nondestructive Testing of Drilled Shafts"

PAINTING: All steel surfaces within 5 feet of the beam ends, including the section encased in concrete, shall be painted as specified in Section 807. All three coats in accordance with Subsection 807.76 will be required. ASTM F3125, Grade A325 Type 3 bolts shall be used within these painted zones and shall be painted

Galvanized members and surfaces in contact with concrete shall not be painted unless otherwise noted above. The color of paint shall be Brown equal or close Fed. Std. 595 B, Color Chip No. 30070 and as approved by the Engineer. The finish system may be applied in the shop. Any damage to the paint system occurring during transport or installation shall be corrected according to the manufacturer's recommendations at no cost to the Department.

BRIDGE DECK: The concrete bridge deck, except sidewalks, shall be given a tine finish as specified for final finishing in Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish, Sidewalks shall be given a Class 6 Broomed Finish.

PROTECTIVE SURFACE TREATMENT: Class 2 Protective Surface Treatment shall be applied to the roadway surface, sidewalk surface (including curbing), and to the roadway face and top of the concrete bridge rails in accordance with Section 803

DETAIL DRAWINGS DRAWING NOS. Retaining Wall 65378 - 65379 End Bents 65380 - 65381 65382 - 65384 Intermediate Bents Elastomeric Bearings 65385 130'-0" Integral Continuous W-Beam Unit - 65394 Deck Drains 65395 General Notes for Steel Bridge Structures 55006 Details for Steel Bridge Structures 55007 Transitional Approach Railing 55013A Type H2 Railing 55015 Steel H-Pilina 55020 Type F Approach Slab 55040F Bridge Traffic Rail

EXISTING BRIDGE: Existing Bridge No. M1144 (Log Mile 0.122) is 20.8' wide (18.0' clear roadway) and 101.0' long and consists of 19.5' RCDG approach spans and a 62.0' steel truss center span (3 spans total) supported by concrete piers

REMOVAL AND SALVAGE: The Contractor shall remove existing Bridge No. M1144 in accordance with Section 205 and Job SP "REMOVAL OF HISTORIC TRUSS SPAN OF BRIDGE NUMBER M1144." All other material from the existing bridge shall become the property of the Contractor. Existing bridge foundations shall be removed to 2' below channel or completely where they interfere with the proposed foundations as directed by the Engineer, see "FOUNDATION PLAN" on Dwg. 65377 for

MAINTENANCE OF TRAFFIC: The road will be closed during the construction of this project.

Reverse Crown @

2.0% Normal Crown

@ Sta. 209+50.00

Sta. 208+75.00

ARKANSAS LICENSED PROFESSIONAL ENGINEER No. 9235 HARLES BRIDGE ENGINEER

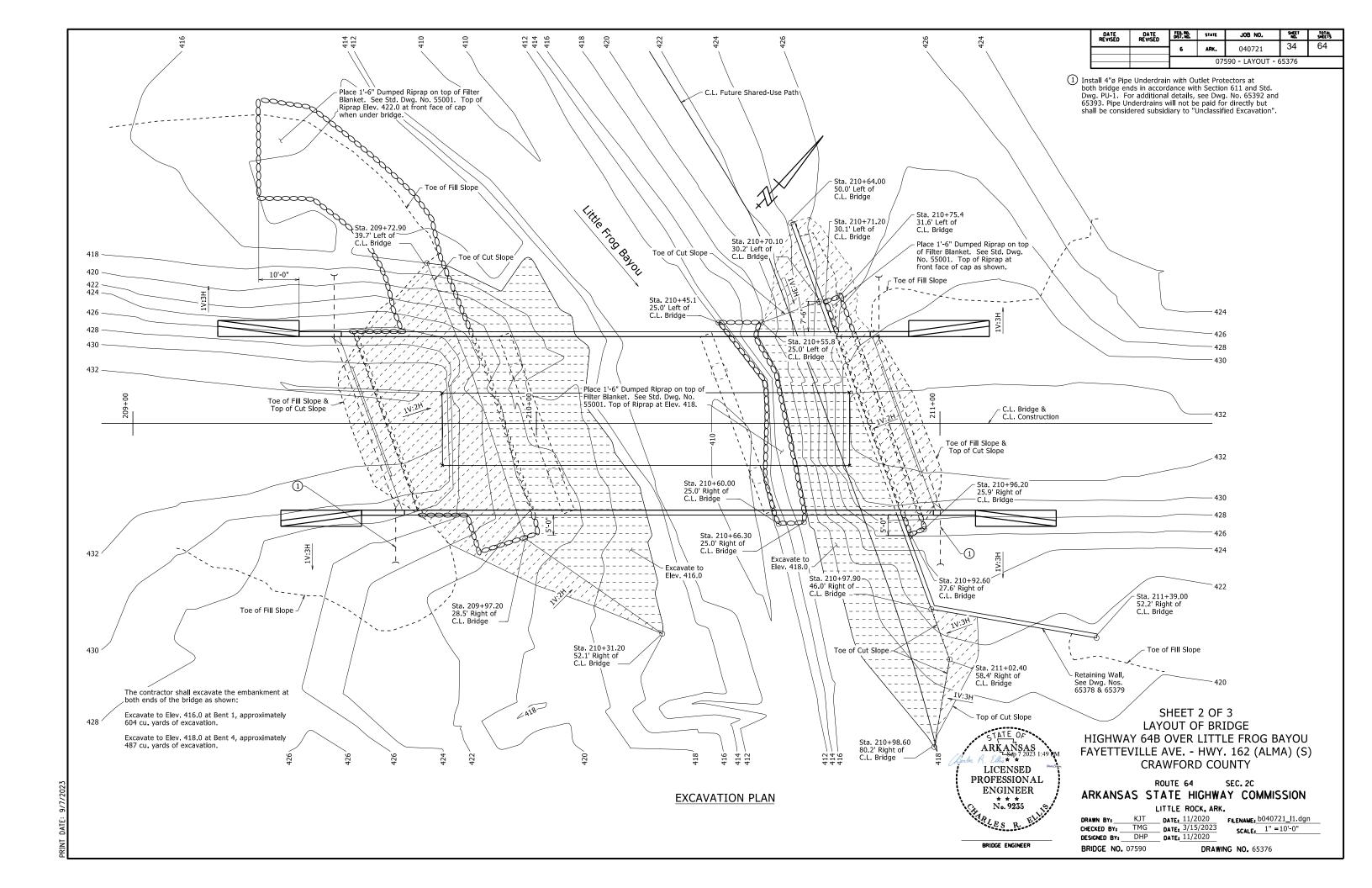
SHEET 1 OF 3 LAYOUT OF BRIDGE HIGHWAY 64B OVER LITTLE FROG BAYOU FAYETTEVILLE AVE. - HWY. 162 (ALMA) (S) **CRAWFORD COUNTY**

ROUTE 64 SEC. 2C ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

KIT DATE: 11/2020 FILENAME: b040721_l1.dgn TMG DATE: 3/15/2023 SCALE: 1"=20'-0" CHECKED BY: DESIGNED BY: DHP DATE: 11/2020

BRIDGE NO. 07590 **DRAWING NO.** 65375



BORING LEGEND



E2-SHALE - Unweathered, Medium Hard, Occasional Fractures, Frequent Slickensides, Dark Gray

J2-SHALE - Weathered with Highly Weathered Layers, Medium Hard with Soft Layers, Dark Gray

K2-SHALE - Slightly Weathered, Occasional Fractures, Medium Hard, Dark Gray L2-SHALE - Slightly Weathered, Frequent Slickensides, Medium Hard, Dark Gray

T2-SHALE - Slightly Weathered, Medium Hard, Frequent Slickensides, Dark Gray

"N" VALUES

F2-Wet, Very Loose, Brown Fine Sandy Silt G2-Wet, Loose, Brown Fine Sandy Silt

N2-Moist, Loose, Brown Silty Fine Sand P2-Wet, Very Loose, Brown Silty Fine Sand

H2-Wet, Very Dense, Gray Silty Fine Sand with Some Gravel

Q2-Wet, Very Loose, Gray Silty Fine Sand with Some Gravel R2-SHALE - Highly Weathered, Medium Hard, Dark Gray

M2-SHALE - Unweathered, Medium Hard Dark Gray

Sta. 209+45 - 6' Right of C.L. Construction	Sta. 210+85 - 75' Left of C.L. Construction
5.0- 6.0, N=4	5,7- 6,7, N=5
10.0- 11.0, N=7	10.7- 11.7, N=0
15.0- 16.0, N=0	15.7- 16.7, N=2
20.0- 21.0, N=2	20.7- 21.7, N=20
25.0- 26.0, N=51	•
30.0- 30.2, N=30(2")	Sta. 210+85 - 28' Left of C.L. Construction
Cha. 200 : E3. E3! Laft of C.L. Comptunishing	5.7- 6.7, N=3
Sta. 209+53 - 52' Left of C.L. Construction	10.7- 11.7, N=5
0.5- 1.5, N=16	15.7- 16.7, N=2
2.0- 3.0, N=4	20.7- 21.7, N=3
5.1- 6.1, N=0	25.7- 26.7, N=58
10.1- 11.1, N=20	0. 044.40 (10.1.40.00.00.1.40
14.6- 15.0, N=30(5")	Sta. 211+13 - 6' Right of C.L. Construction
Cta 210 LOZ 201 Loft of C.L. Construction	5.0- 6.0, N=6
Sta. 210+07 - 39' Left of C.L. Construction	10.0- 11.0, N=7
0.5- 1.5, N=5	15.0- 16.0, N=3
2.0- 3.0, N=4	20.0- 21.0, N=3
3.5- 4.5, N=3	25.0- 26.0, N=2
5.0- 6.0, N=2	30.0- 31.0, N=3
6.5- 7.5, N=2	34.5- 34.9, N=40(5")
8.0- 9.0, N=10	
9.5- 10.5, N=19	
15.0- 15.1, N=20(1")	
	SHEET 3 OF 3
	LAVOUT OF RDIDG

ARKANSAS
ARKANSAS
ARKANSAS
LICENSED
PROFESSIONAL
ENGINEER
No. 9235

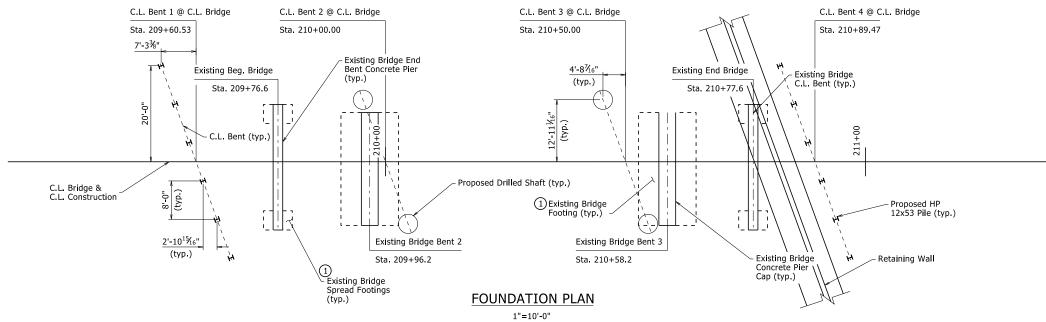
BRIDGE ENGINEER

LAYOUT OF BRIDGE HIGHWAY 64B OVER LITTLE FROG BAYOU FAYETTEVILLE AVE. - HWY. 162 (ALMA) (S) CRAWFORD COUNTY

ROUTE 64 SEC. 2C
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE	ROCK, ARK.	
2475	11/2020	

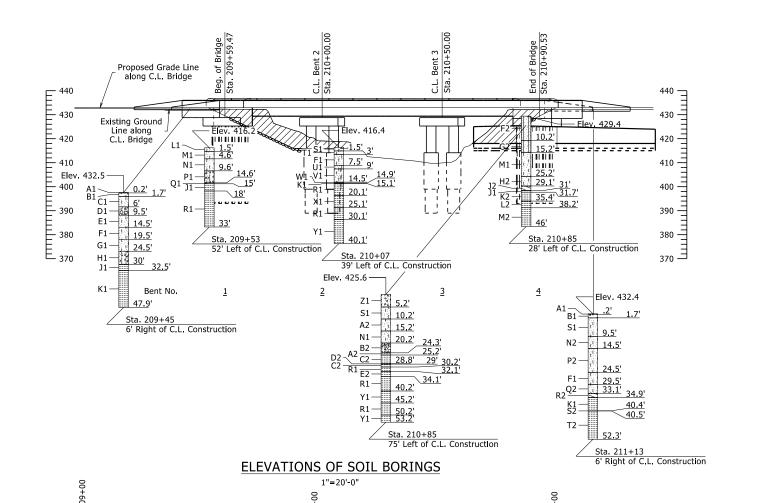
DRAWN BY:	KJT	DATE: 11/2020 F	ILENAME: b040721_l1.dgn
CHECKED BY:	TMG	DATE: 3/15/2023	SCALE: As Noted
DESIGNED BY:	DHP	DATE: 11/2020	
BRIDGE NO.	07590	DRAWING	NO. 65377



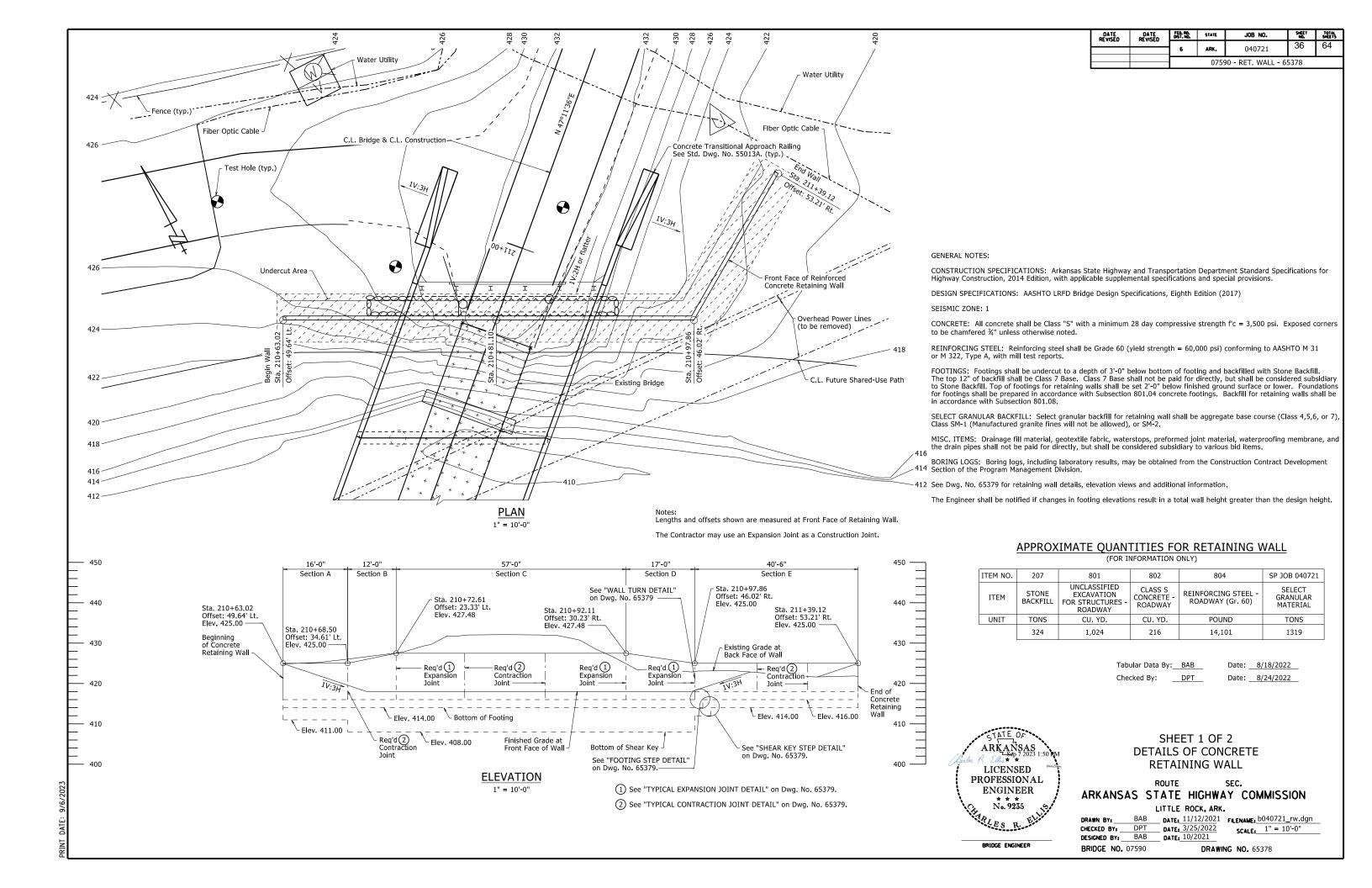
Note: Location of existing substructure elements shown are approximate only. Existing substructure assumed to be on spread

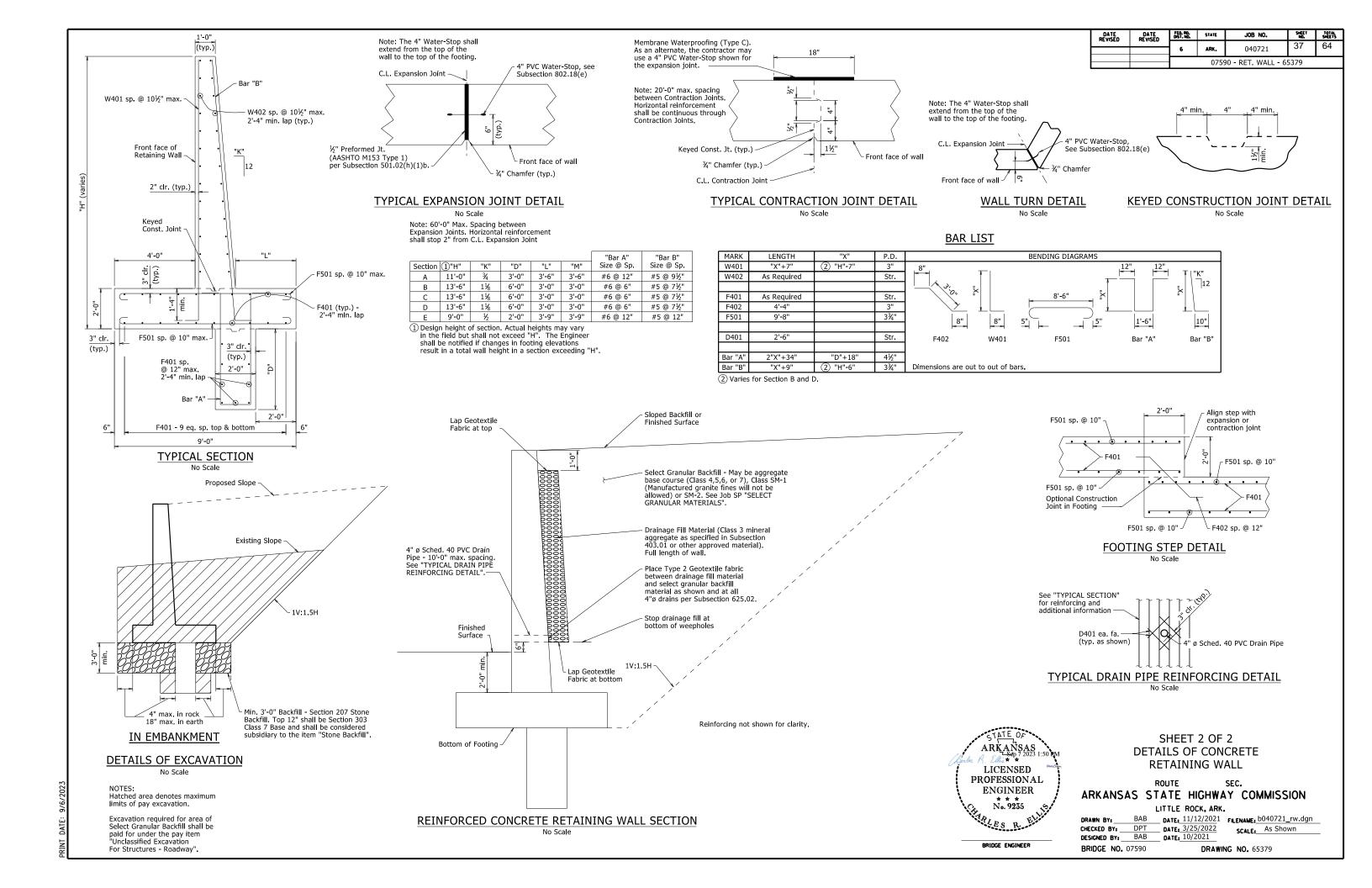
footings, the Contractor shall field verify the existing substructure type and dimensions and its relationship to the proposed work.

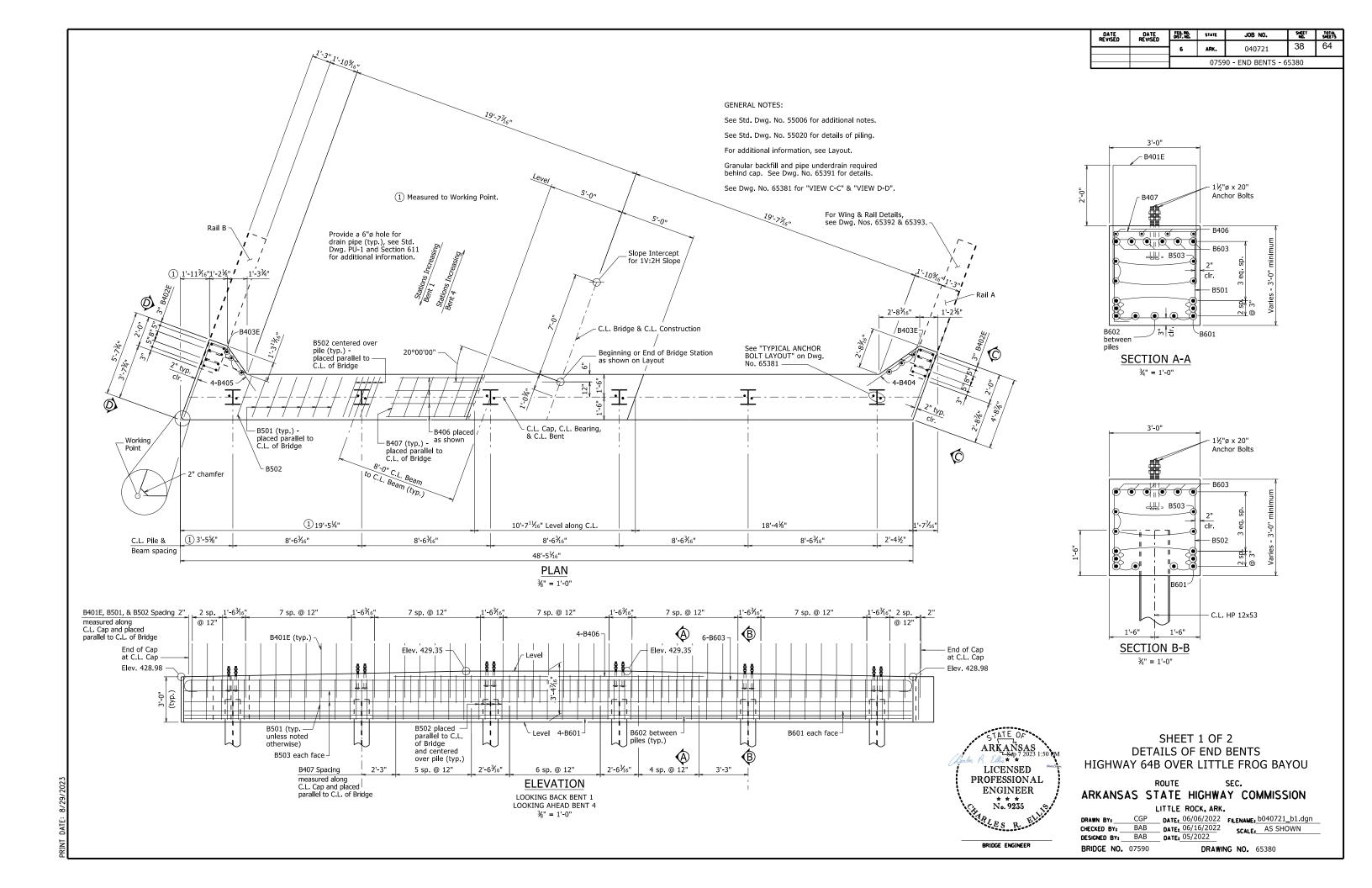
1 Estimated limits of existing foundation.



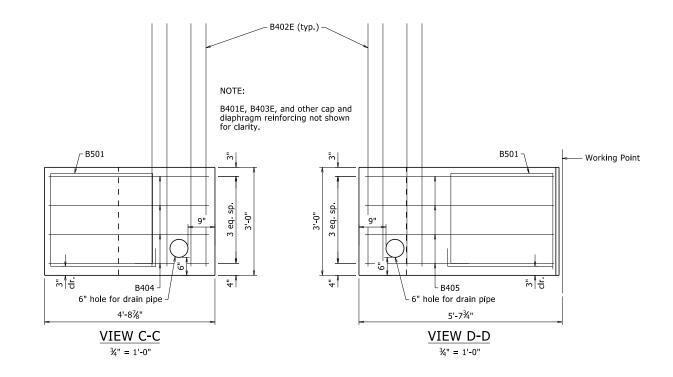
211





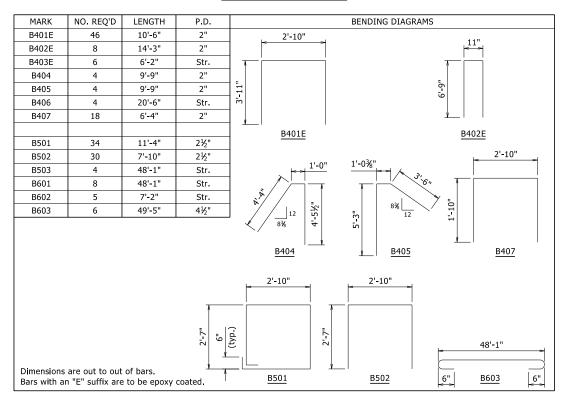


DATE REVISED	DATE REVISED	FED. RD. DIST. NO.	STATE JOB NO.		SHEET NO.	TOTAL SHEETS				
	112 11323	6	ARK.	040721	39	64				
		07590 - END BENTS - 65381								



C.L. Beam C.L. Beam C.L. Cap & C.L. Bearing C.L. Anchor Bolts TYPICAL ANCHOR BOLT LAYOUT NO SCALE

BAR LIST-PER BENT



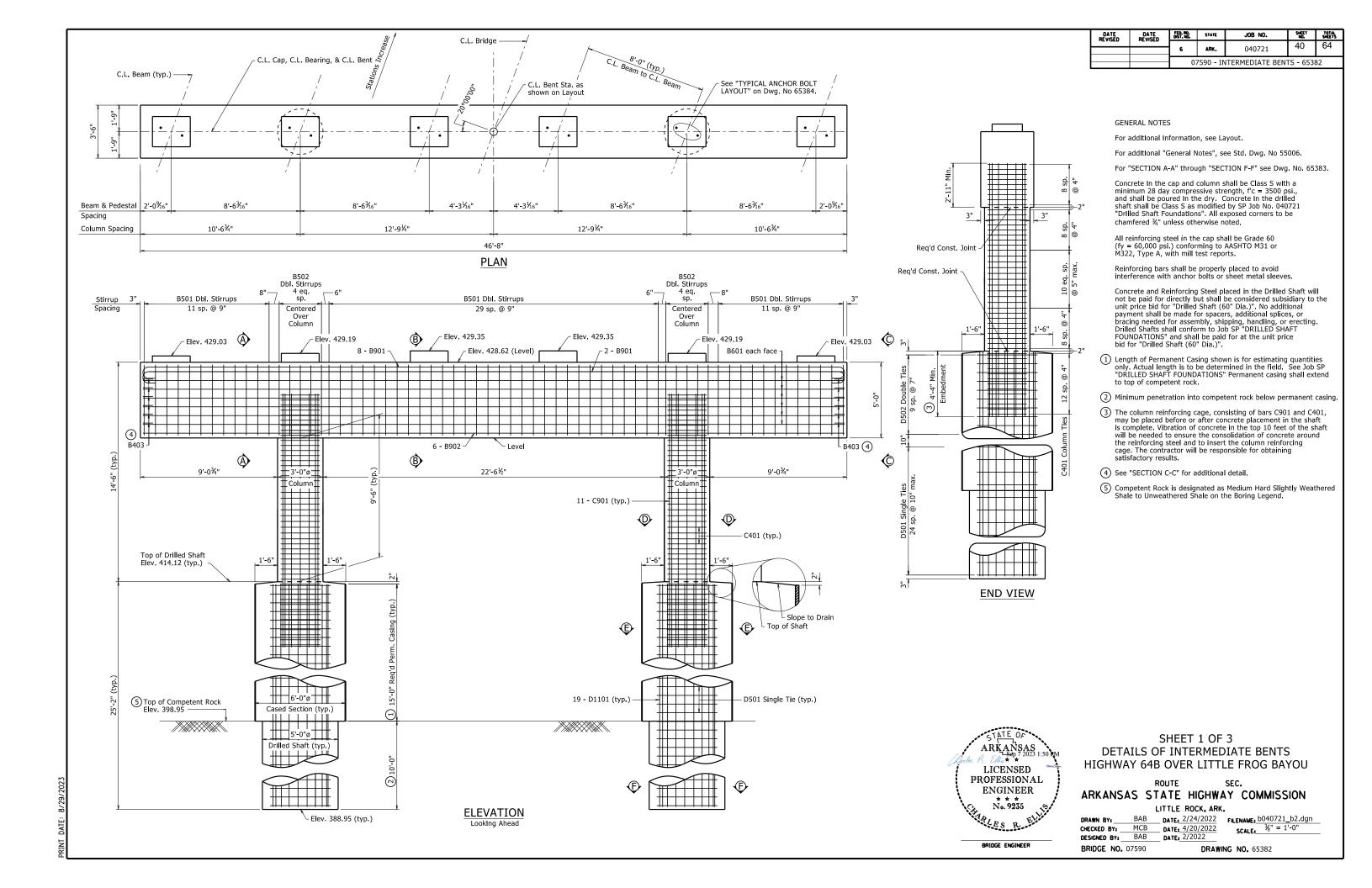


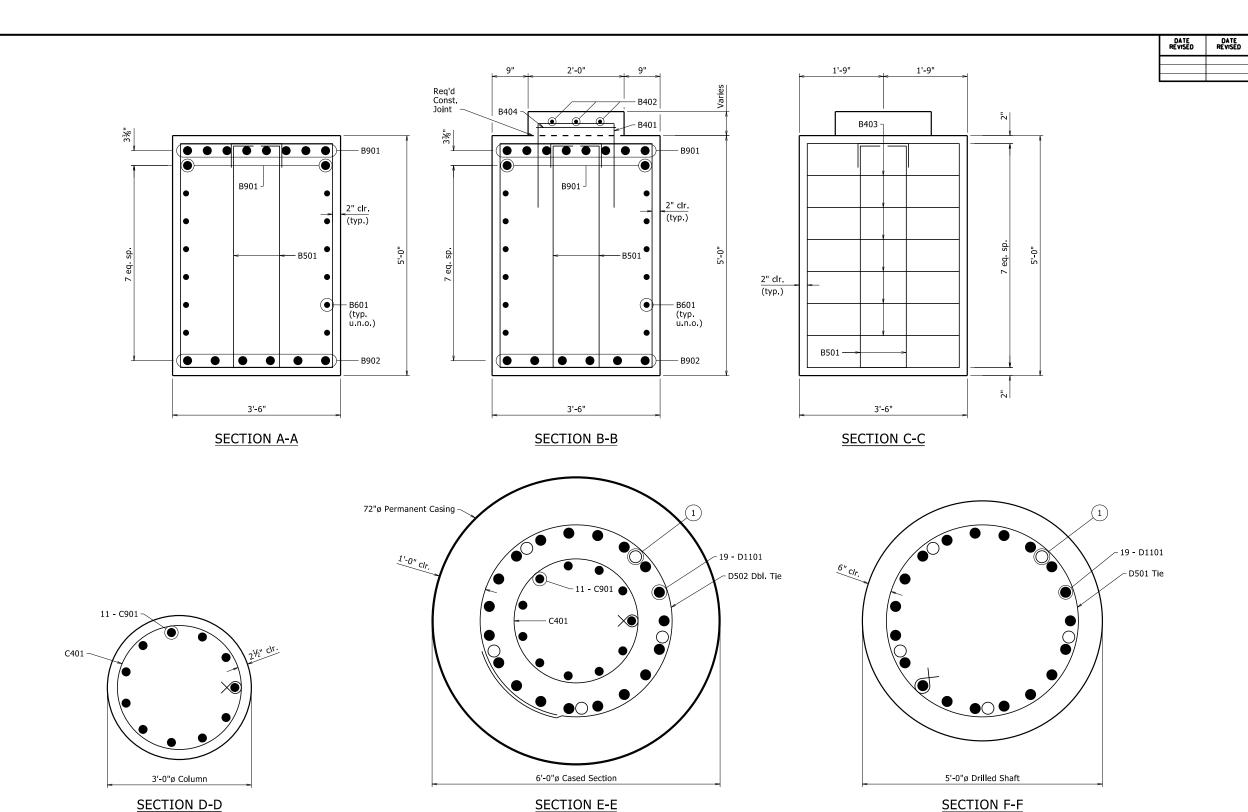
SHEET 2 OF 2
DETAILS OF END BENTS
HIGHWAY 64B OVER LITTLE FROG BAYOU

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION

DESIGNED BY: BAB DATE: 05/2022

BRIDGE NO. 07590 DRAWING NO. 65381





1 5 - 1½"ø Min. Schedule 40 Steel Pipes equally spaced. See Job SP "NONDESTRUCTIVE TESTING OF DRILLED SHAFTS".



SHEET 2 OF 3 DETAILS OF INTERMEDIATE BENTS HIGHWAY 64B OVER LITTLE FROG BAYOU

SHEET TOTAL NO. SHEETS

41

64

FEO. RO. STATE

ARK.

JOB NO.

040721 07590 - INTERMEDIATE BENTS - 65383

ROUTE ARKANSAS STATE HIGHWAY COMMISSION

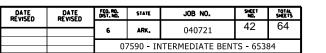
LITTLE ROCK, ARK.

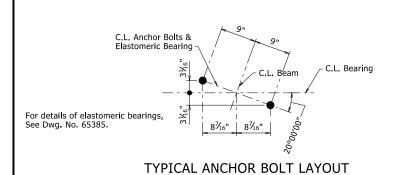
DRAWN BY: BAB DATE: 2/24/2022
CHECKED BY: MCB DATE: 4/20/2022
DESIGNED BY: BAB DATE: 2/2022

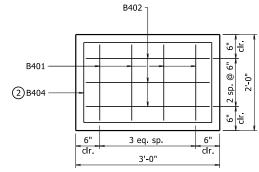
DESIGNED BY: BAB DATE: 2/2022

DESIGNED BY: BAB DATE: 2/2022

BRIDGE NO. 07590 DRAWING NO. 65383







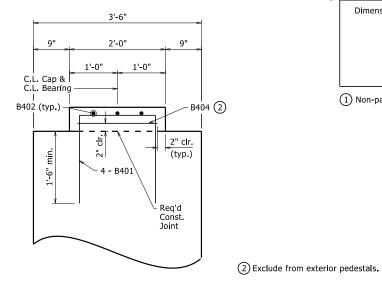
PEDESTAL PLAN 1" = 1'-0"

1'-6" 1'-6" - C.L. Beam & C.L. Pedestal 2)B404 -- B401 (typ.) 2" clr. (typ. − Req'd Const. Joint ∕- 3 - B402

NO SCALE



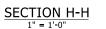
(1)



BAR LIST - PER BENT

_							
	MARK	NO. REQ'D	"A"	"B"	LENGTH	P.D.	BENDING DIAGRAMS
	B401	24	1'-7"	2'-1"	5'-7"	2"	
	B402	18	2'-7"	2'-1"	6'-7"	2"	"A"
	B403	12	3'-2"	8"	4'-4"	2"	
	B404	4			9'-0"	2"	
	B501	108			14'-0"	2½"	"B" "8" 1-8" 447" 1-8"
	B502	20	2'-1"	4'-8"	11'-3"	2½"	
	B601	12			46'-4"	Str.	B401, B402, B403, & B502 B404 B501
	B901	10			48'-10"	9"	2'-2"
	B902	6			46'-4"	Str.	min.
	C401	98	5"	2'-7"	9'-2"	3"	
	C901	22			16'-9"	Str.	\$\left(\alpha\) \ \bar{\alpha}\ \bar{\alpha}\ \alpha\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\
							\$\frac{\lambda}{\hat{\hat{\hat{\hat{\hat{\hat{\hat
1	D501	50	6¼"	4'-0"	13'-11"	3¾"	
1	D502	40			14'-7"	3¾"	V
1	D1101	38			24'-10"	Str.	<u>C401 & D501</u> <u>D502</u>
	Dimensio	ns are out to o	ut of bars.				46'-4"
							10"
- 1							1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1

1) Non-pay Item - Subsidiary to the item "Drilled Shaft (60" Dia.)".





SHEET 3 OF 3 DETAILS OF INTERMEDIATE BENTS HIGHWAY 64B OVER LITTLE FROG BAYOU

<u>B901</u>

ROUTE ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARK.

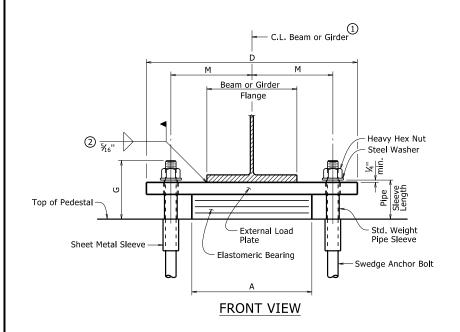
 DRAWN BY:
 BAB
 DATE:
 2/24/2022
 FILENAME:
 b040721_b2.dgn

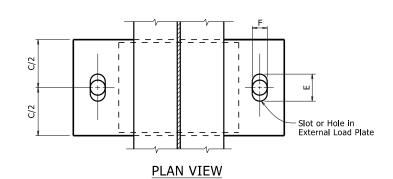
 CHECKED BY:
 MCB
 DATE:
 4/20/2022
 SCALE:
 As Shown

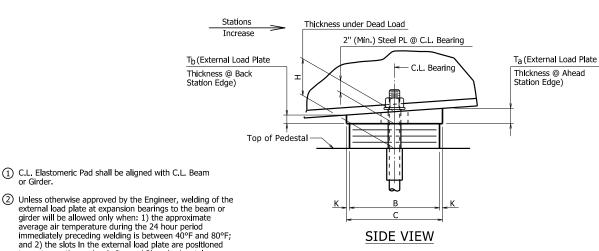
 DESIGNED BY:
 BAB
 DATE:
 2/2022
 SCALE:
 As Shown

BRIDGE NO. 07590

BRIDGE ENGINEER







Elastomeric Bearing shall be vulcanized to the external load plate ¼" clr. Steel Laminae 50 Durometer (typ.) Elastomer Number of layers

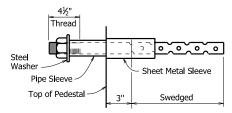
 $t_{\mbox{\scriptsize e}}\mbox{=}$ Thickness of elastomer cover on top and bottom of pad

t i = Thickness of elastomer between steel laminae

 $N = Number of elastomer layers of thickness t_i$

ELASTOMERIC BEARING





ANCHOR BOLT DETAIL

Anchor Bolts may be cast In place or drilled and grouted into place. If Anchor Bolts are to be cast in place, the Galvanized Sheet Metal Sleeves will not be required.

If Anchor Bolts are to be drilled and grouted in place, the Galvanized Sheet Metal Sleeves shall be cast In place as shown. Sleeves shall be dry packed with styrofoam, urethane foam or approved equal prior to pouring of concrete. After pouring of the cap and prior to erection of Structural Steel, the dry pack shall be removed and holes for the anchor bolts shall be accurately drilled into the concrete. Bolts placed in drilled holes shall be accurately set and fixed using a QPL approved epoxy or non-shrink grout that completely fills the holes. Galvanized Sheet Metal Sleeves shall meet the requirements of ASTM 653, CS Type B or approved equivalent, be of minimum 16 gage thickness, and be galvanized according to ASTM B695, Class 50. Sheet Metal Sleeves will not be pald for directly, but will be considered subsidiary to the item "Structural Steel in Beam Spans (A709, Gr. 50W)".

GENERAL NOTES

Elastomeric Bearings shall conform to Section 808 and shall be paid for at the unit price bid for

External load plates shall conform to ASTM A709, Grade 50W. Pipe sleeves shall be ASTM A500, Grade B, and shall be galvanized to conform to AASHTO M 232, Class C or ASTM B695, Class 50.

External load plates shall be completely fabricated (including bevel, bolt holes and all shop welding) and shall be cleaned before vulcanizing to the elastomeric bearing. The surface in contact with the elastomeric bearing shall be cleaned in accordance with Subsection 808,03, Other surfaces shall be blast cleaned in accordance with Subsection 807.84(b) for painted steel.

Anchor Bolts, Washers and Nuts shall conform to Subsection 807.07. The anchor bolt grade of steel shall be as specified in the "Table of Fabricator Variables". Indentations shall be circular with rounded bottoms and staggered as shown in the details.

Pipe Sleeves, Anchor Bolts, Washers and Nuts shall be paid for at the unit price bid for "Structural Steel in Beam Spans (A709, Gr. 50W)", External load plates will not be measured or paid for separately, but will be considered incidental to the unit price bid for "Elastomeric Bearings".

Bearings shall be seated in accordance with Subsection 808.08. This work and materials are considered subsidiary to the item "Elastomeric Bearings" and will not be paid for directly.

of thickness = t

TABLE OF FABRICATOR VARIABLES

to center on the anchor bolts; and 3) no horizontal deformation of the elastomeric pad is evident. If welding at other temperatures is required, the Engineer will provide

Care shall be taken to ensure that the external load plate is in full and complete contact with the beam or girder

The direction of bevel of the external load plate

may not be accurately depicted with respect to Ta and Tb values shown in the "Table of Fabricator

Prior to erection of the beams or girders, the Contractor shall verify the orientation of the bearing

adjustment data.

flange before welding begins.

with respect to Ta and Tb.

		(3) Maximum	Design L	oad = Servic	e 1 Limit State	5				ELA	STO	MER	RIC PAD			EXT	ERN.	AL L	. O A E) РІ	_ A T E				ANCHOR	BOLT	
BRIDGE	B N	BENT	CATION BEAM OR GIRDER NO.			3MAXIMUM DESIGN LOAD (KIPS)	G	н	А	В	Ν	ti	t _e	NO. & THICKESS OF STEEL LAMINAE	Т	С	D	Е	F	К	М	Та	Т _b	ANCHOR E	BOLT GRADE	SLEEVE SIZE	SHEET METAL SLEEVE SIZE (Ø x L)	STEEL WASHER SIZE (O.D.)
		2	All	Fix.	6	163	7¼"	3 ¹ ¾ ₁₆ "	12½"	12½"	2	1⁄2"	¼"	3 @ 12 ga.	$1^{13}/_{16}$ "	13½"	24½"	3%"	31/8"	1⁄2"	9"	2.00"	2.00"	2"ø x 29"	55	2½"ø x 4½"	4"ø x 6"	3¾"
290		3	All	Fix	6	163	7¼"	3 ¹ 3⁄16"	12½"	12½"	2	1⁄2"	¼"	3 @ 12 ga.	1 ¹ ¾ ₁₆ "	13½"	24½"	3%"	31/8"	1⁄2"	9"	2.00"	2.00"	2"ø x 29"	55	2½"ø x 4½"	4"ø x 6"	3¾"
075																												

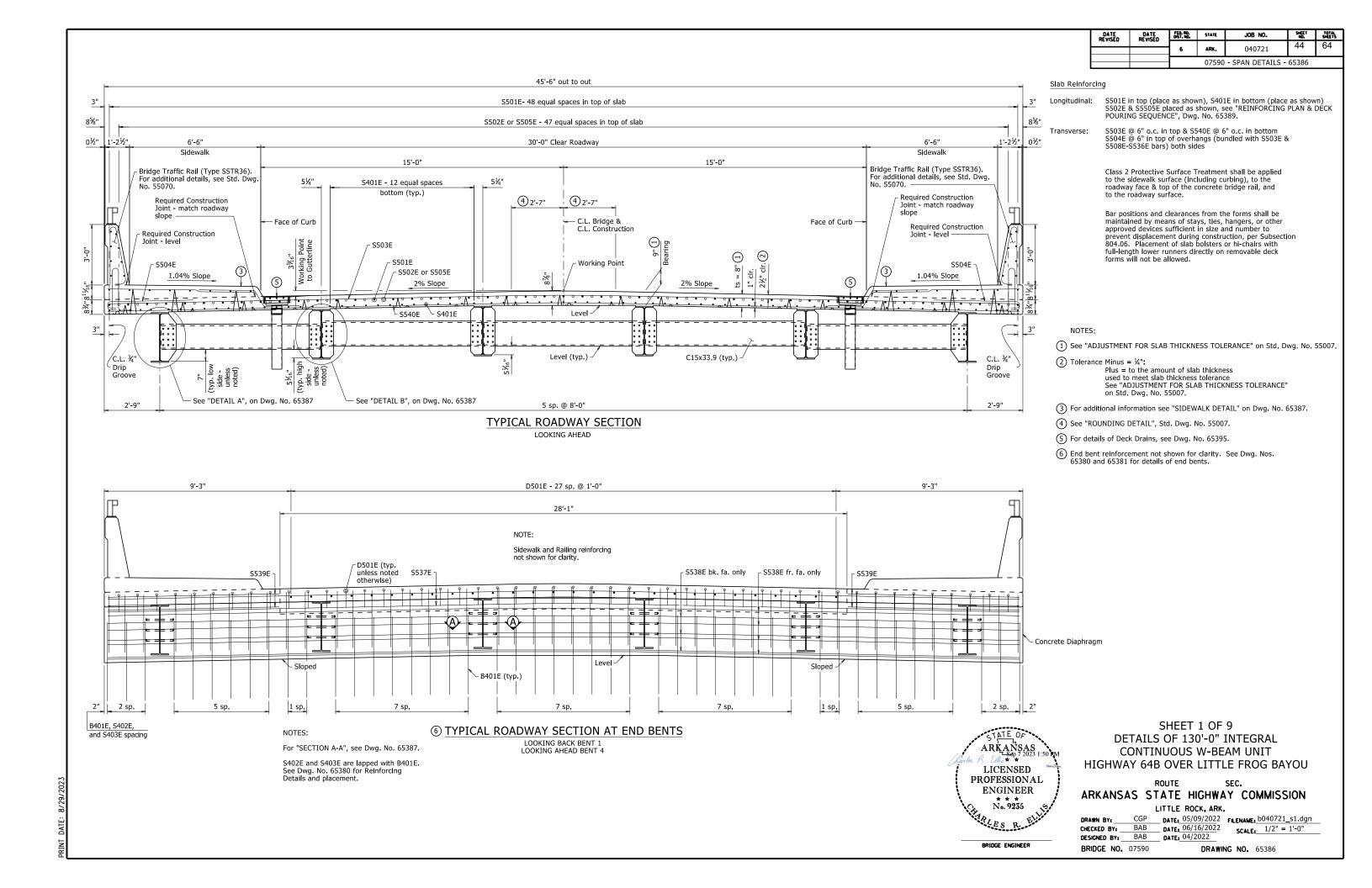


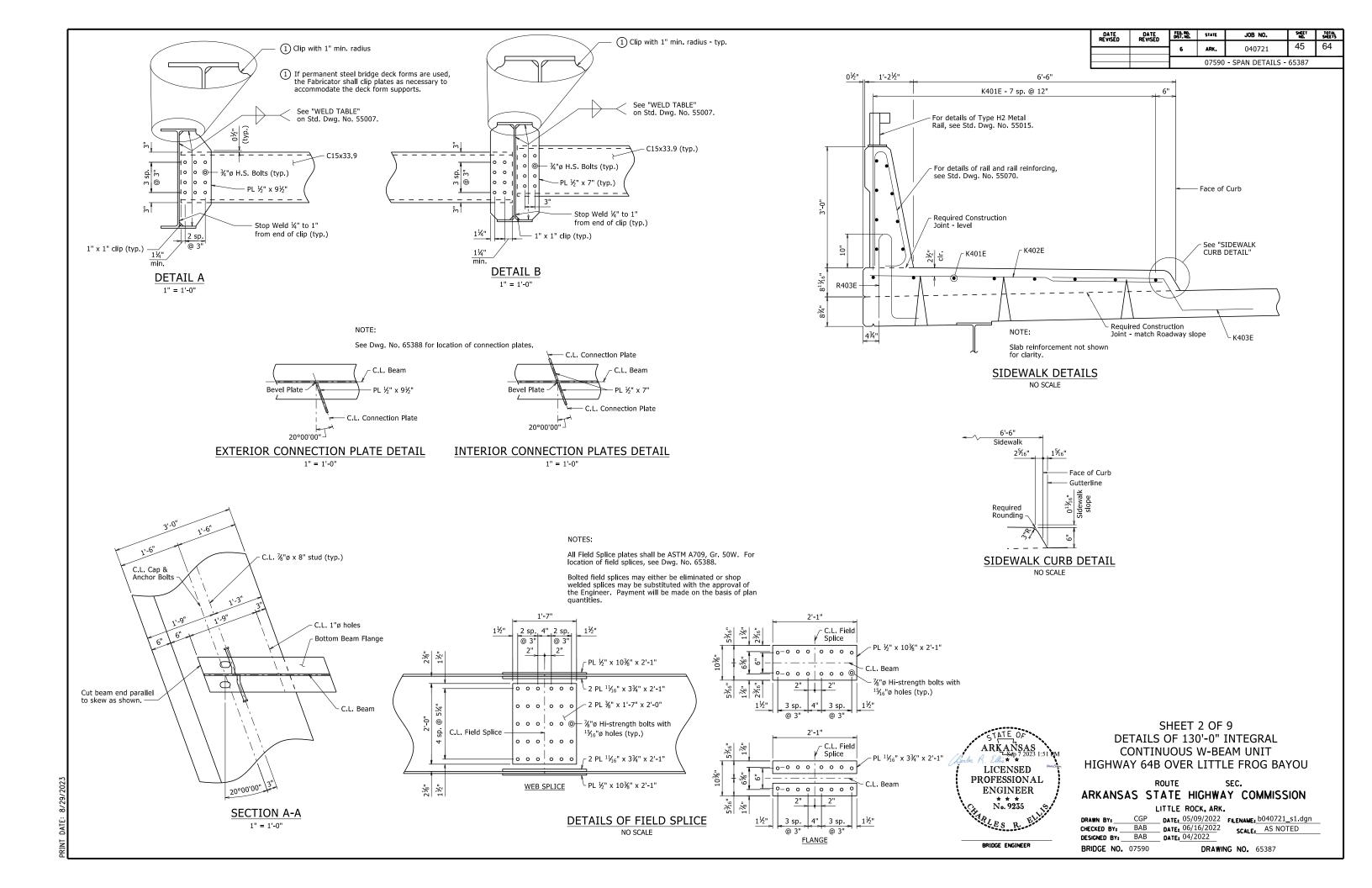
DETAILS OF ELASTOMERIC BEARINGS HIGHWAY 64B OVER LITTLE FROG BAYOU

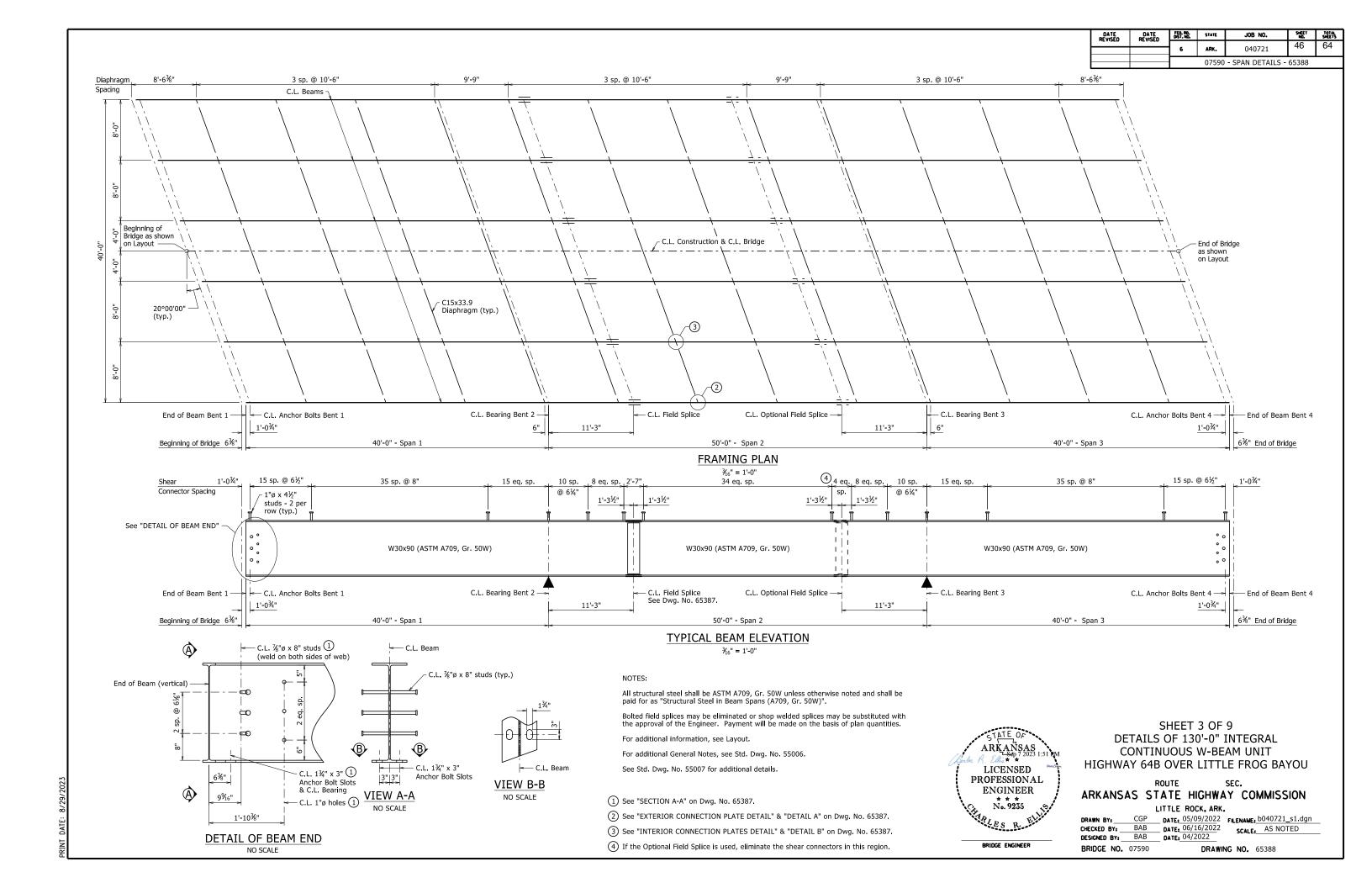
ROUTE ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARK.

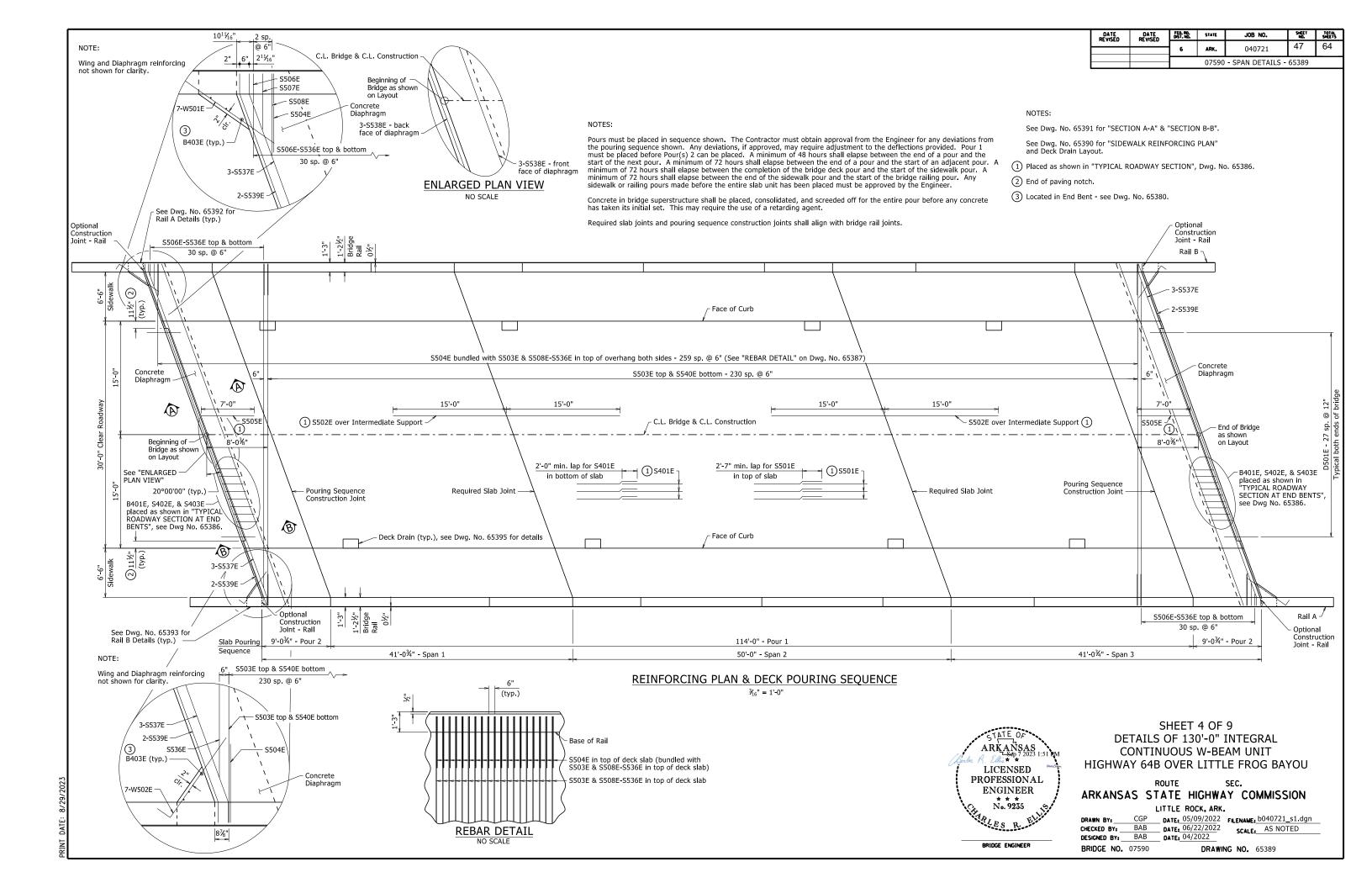
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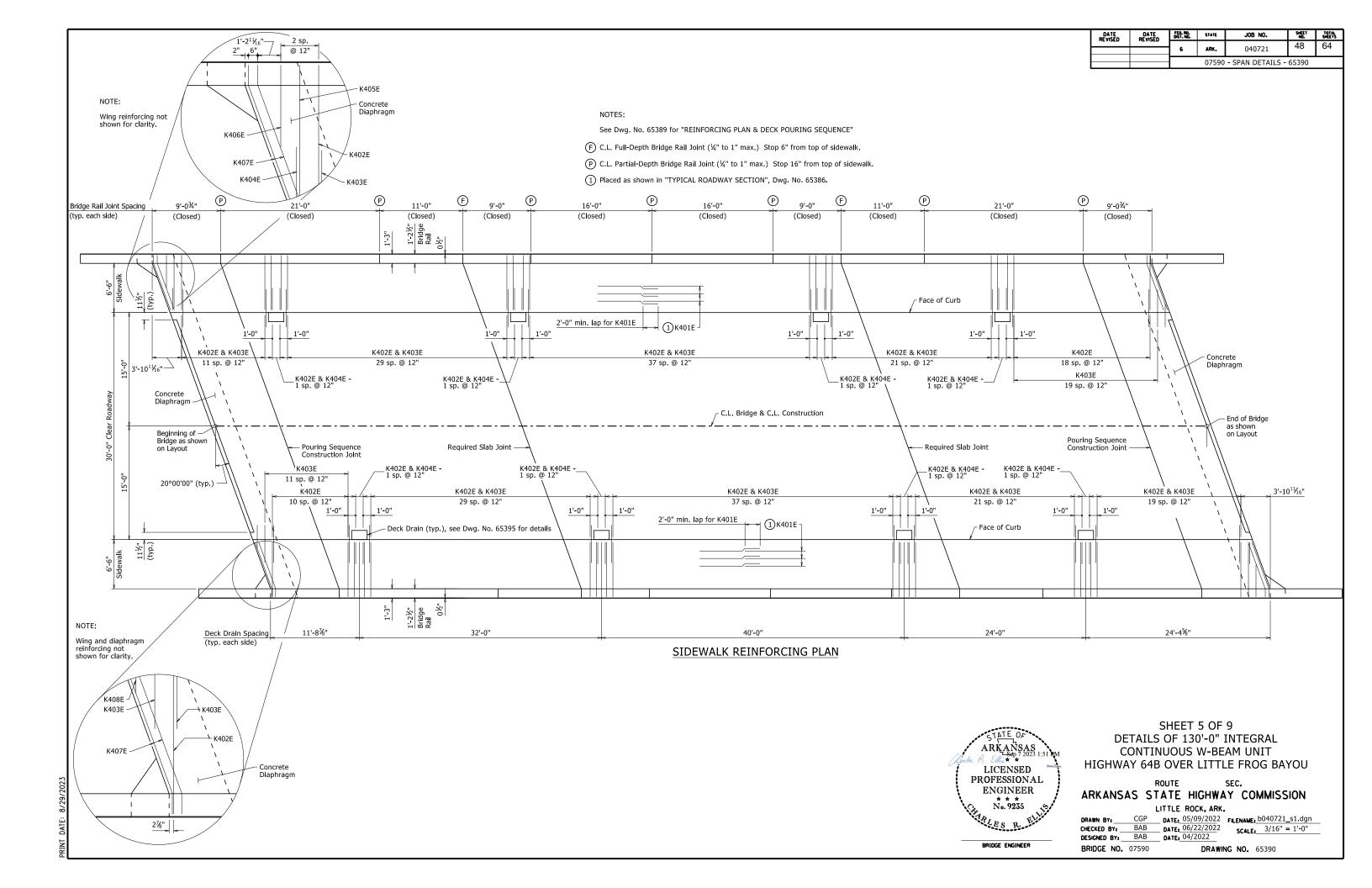
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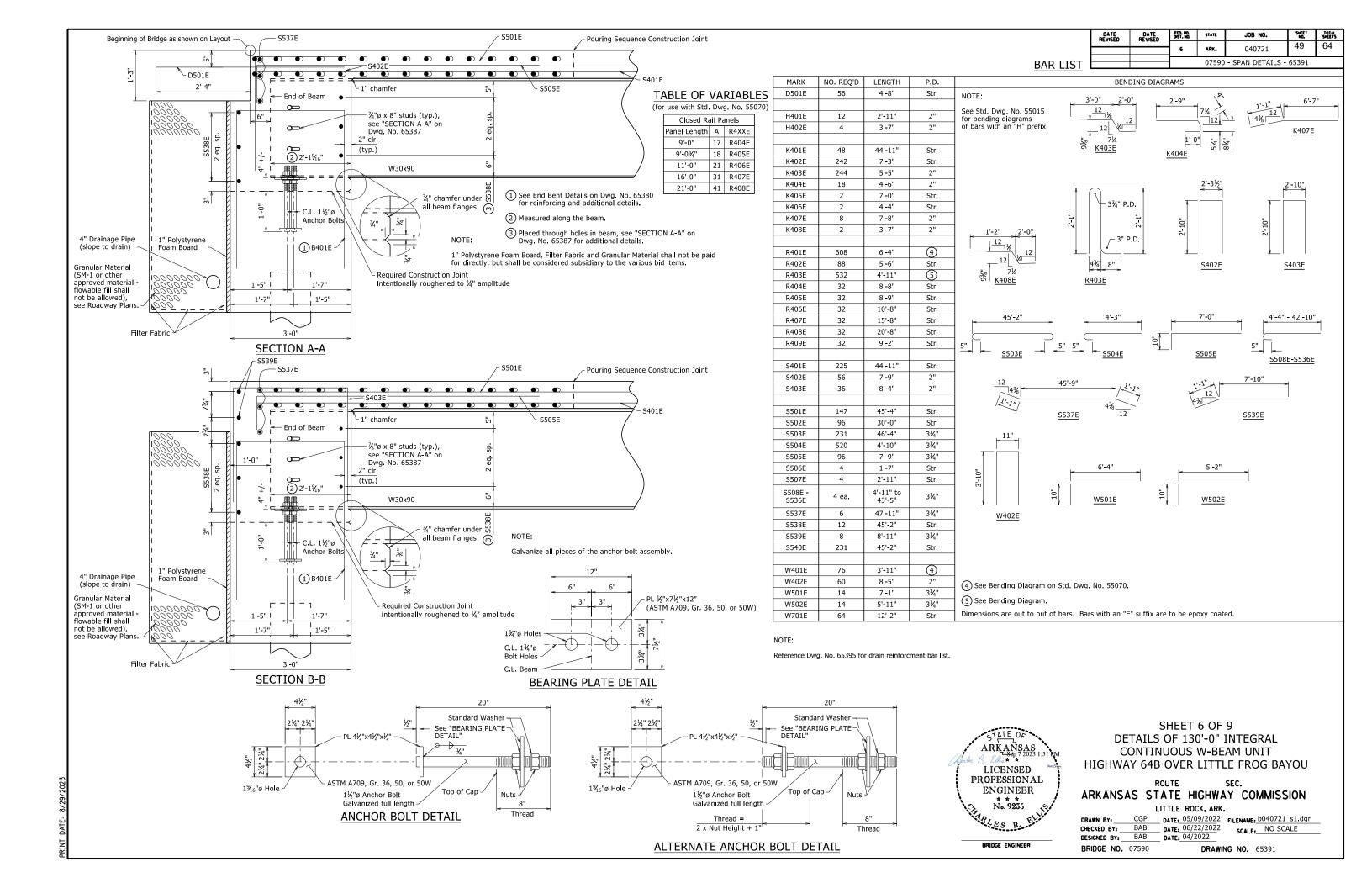


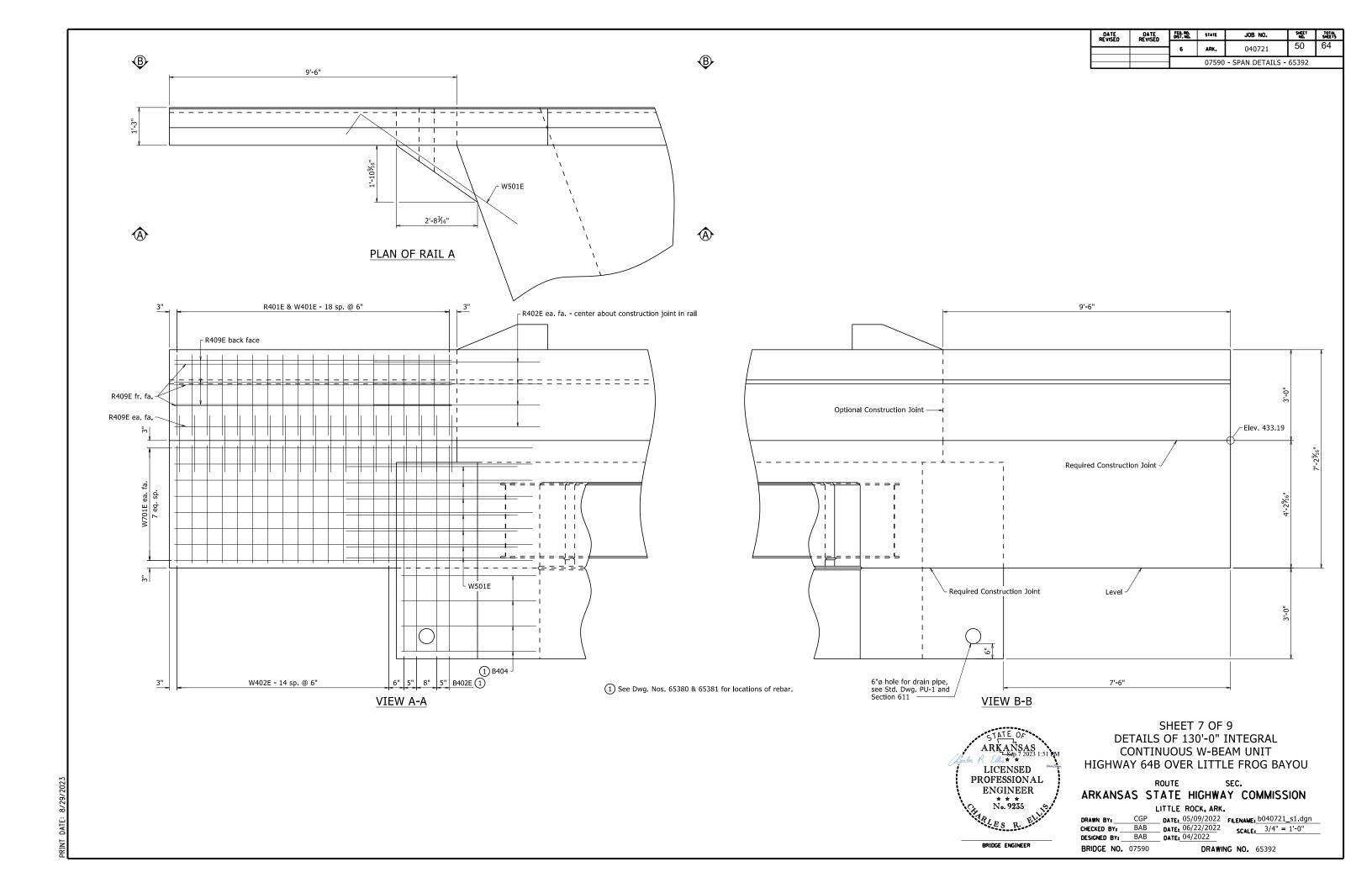












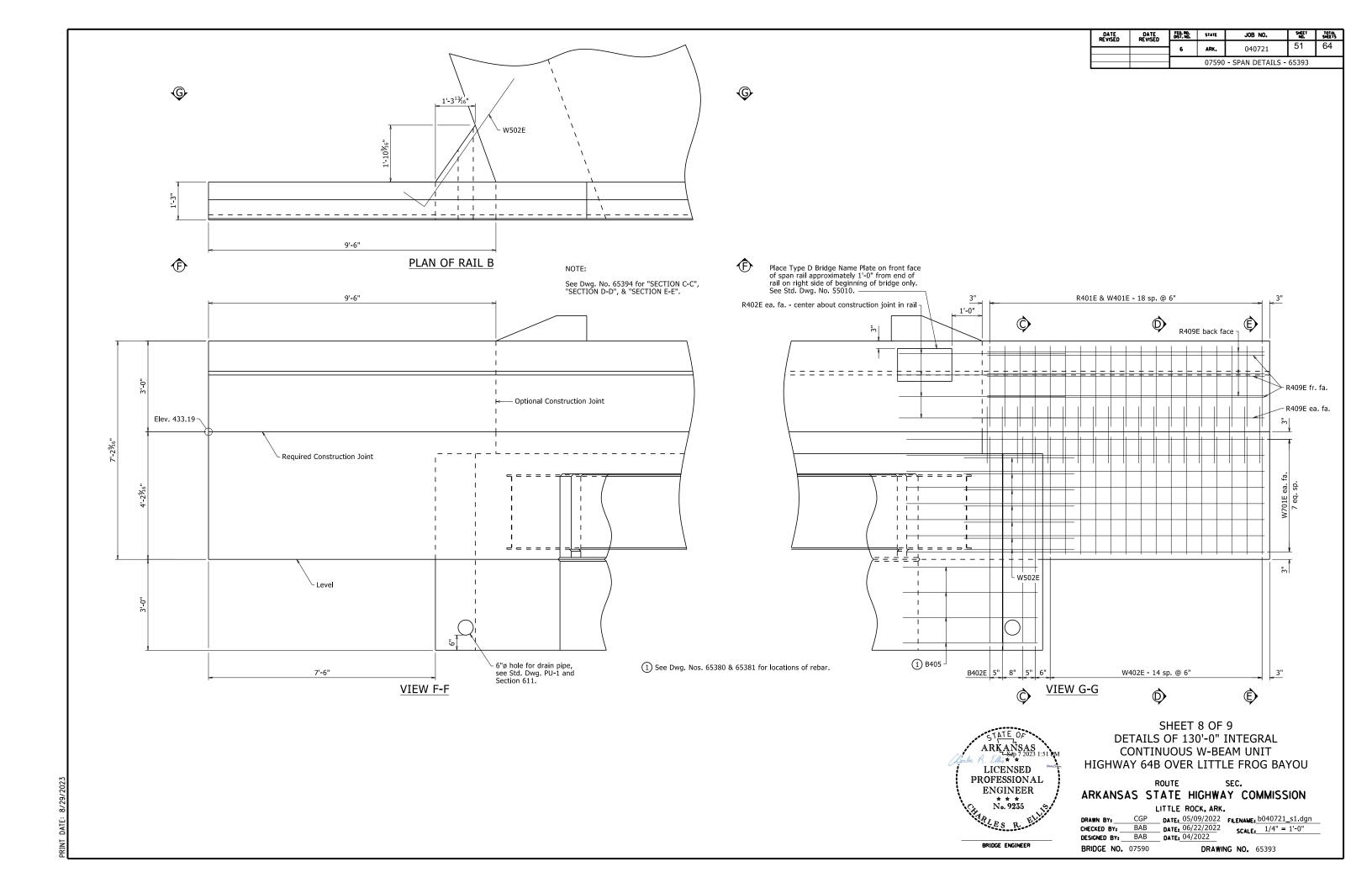


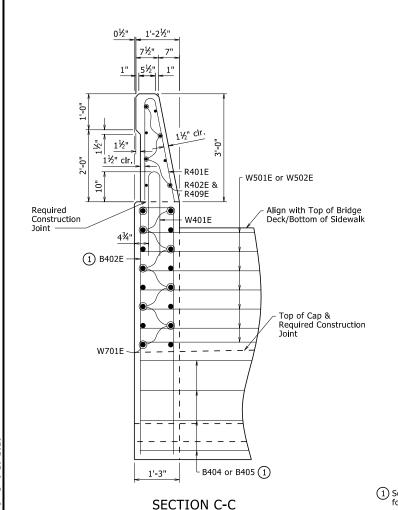
TABLE OF DEAD LOAD DEFLECTIONS (INCHES)

Span	Point of Deflection	Structural Steel	Structural Steel + Slab	Structural Steel + Slab + Bridge Rail + Sidewalk
	0.00	0.000	0.000	0.000
	0.10	0.009	0.080	0.089
	0.20	0.016	0.148	0.164
	0.30	0.021	0.193	0.214
1.	0.40	0.023	0.212	0.235
Span	0.50	0.022	0.202	0.224
S	0.60	0.019	0.168	0.186
	0.70	0.013	0.117	0.130
	0.80	0.007	0.061	0.068
	0.90	0.002	0.015	0.017
	0.00	0.000	0.000	0.000
	0.10	0.005	0.045	0.050
	0.20	0.014	0.130	0.144
	0.30	0.024	0.218	0.241
7	0.40	0.031	0.281	0.311
Span	0.50	0.034	0.304	0.337
Ϋ́	0.60	0.031	0.281	0.311
	0.70	0.024	0.218	0.241
	0.80	0.014	0.130	0.144
	0.90	0.005	0.045	0.050
	0.00	0.000	0.000	0.000
	0.10	0.002	0.015	0.017
	0.20	0.007	0.061	0.068
	0.30	0.013	0.117	0.130
m	0.40	0.019	0.168	0.186
듄	0.50	0.022	0.202	0.224
Span	0.60	0.023	0.212	0.235
	0.70	0.021	0.193	0.214
	0.80	0.016	0.148	0.164
	0.90	0.009	0.080	0.089
	0.00	0.000	0.000	0.000

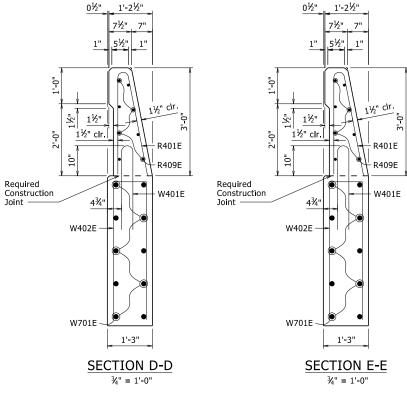
0.0 0.1 0.3 0.4 0.5 0.6 0.8	0.0 0.3 0.3 0.5 0.0 0.0 0.9	0.0 0.1 0.0 0.0 0.0 0.0 0.0
Span 1	Span 2	Span 3

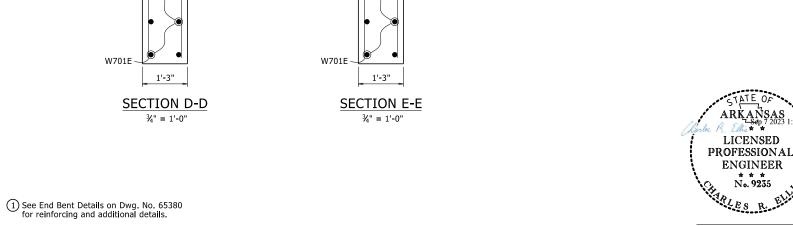
DEAD LOAD DEFLECTION DIAGRAM

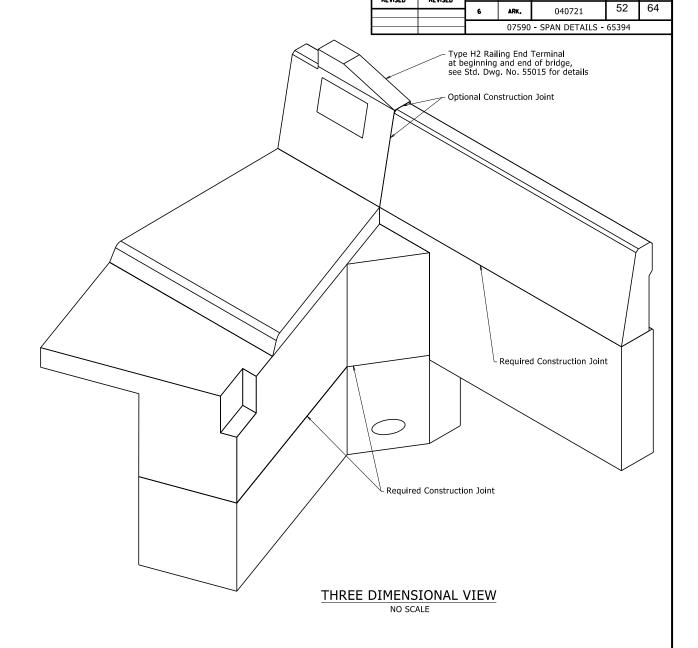
Camber for Dead Load Deflection $+/-\frac{1}{4}$ Inch tolerance. Deflections shown are along centerline of beam from a chord from C.L. Bearing to C.L. Bearing. Negative sign (-) indicates point above chord. Vertical curve corrections not included. Superelevation transition corrections not included.



¾" = 1'-0"







DATE REVISED

FED. RD. STATE

JOB NO.

ARKANSAS Sep 7 2023 1:51 RM PROFESSIONAL

BRIDGE ENGINEER

SHEET 9 OF 9 DETAILS OF 130'-0" INTEGRAL CONTINUOUS W-BEAM UNIT HIGHWAY 64B OVER LITTLE FROG BAYOU

SHEET TOTAL NO. SHEETS

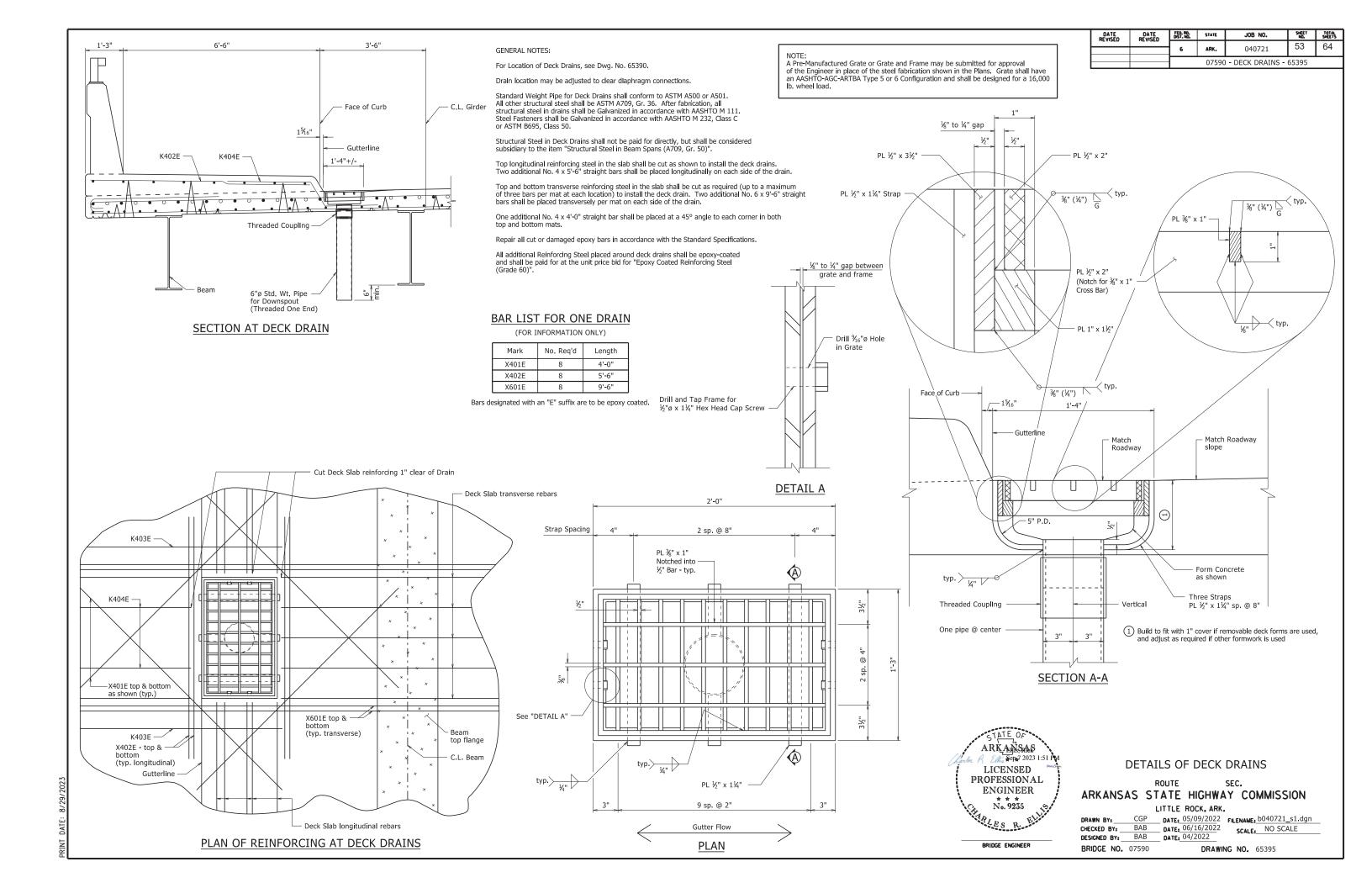
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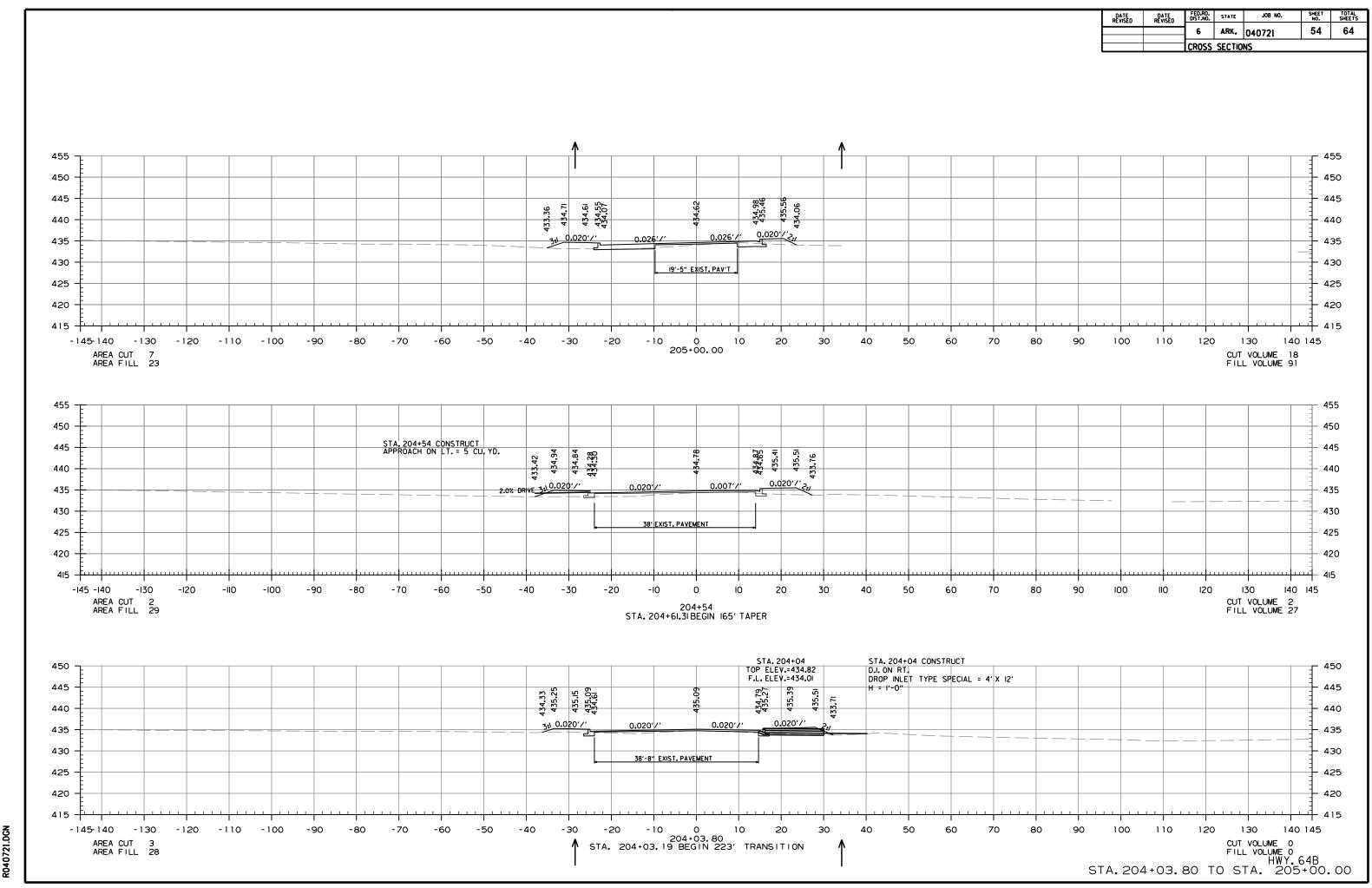
LITTLE ROCK, ARK.

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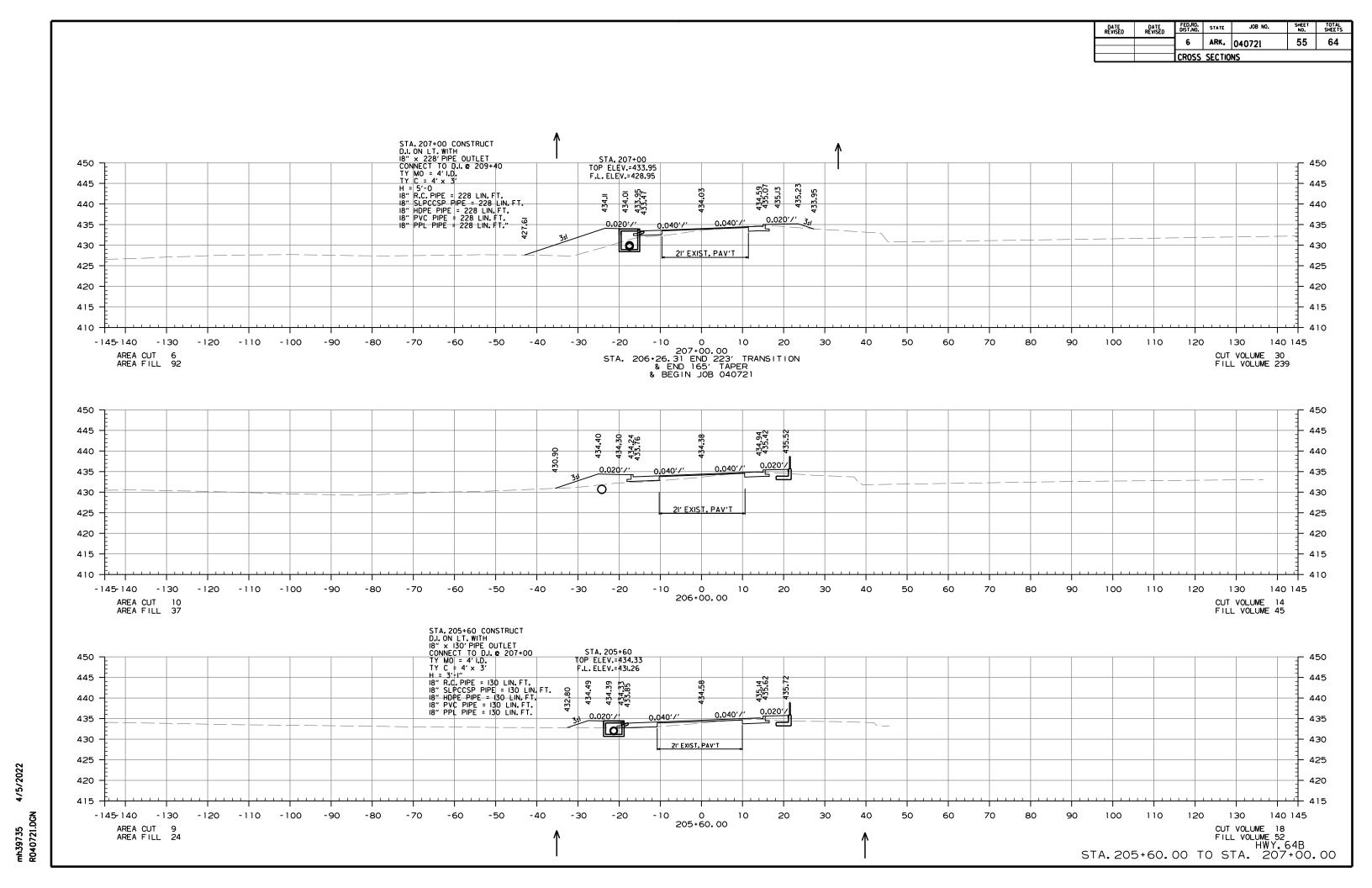
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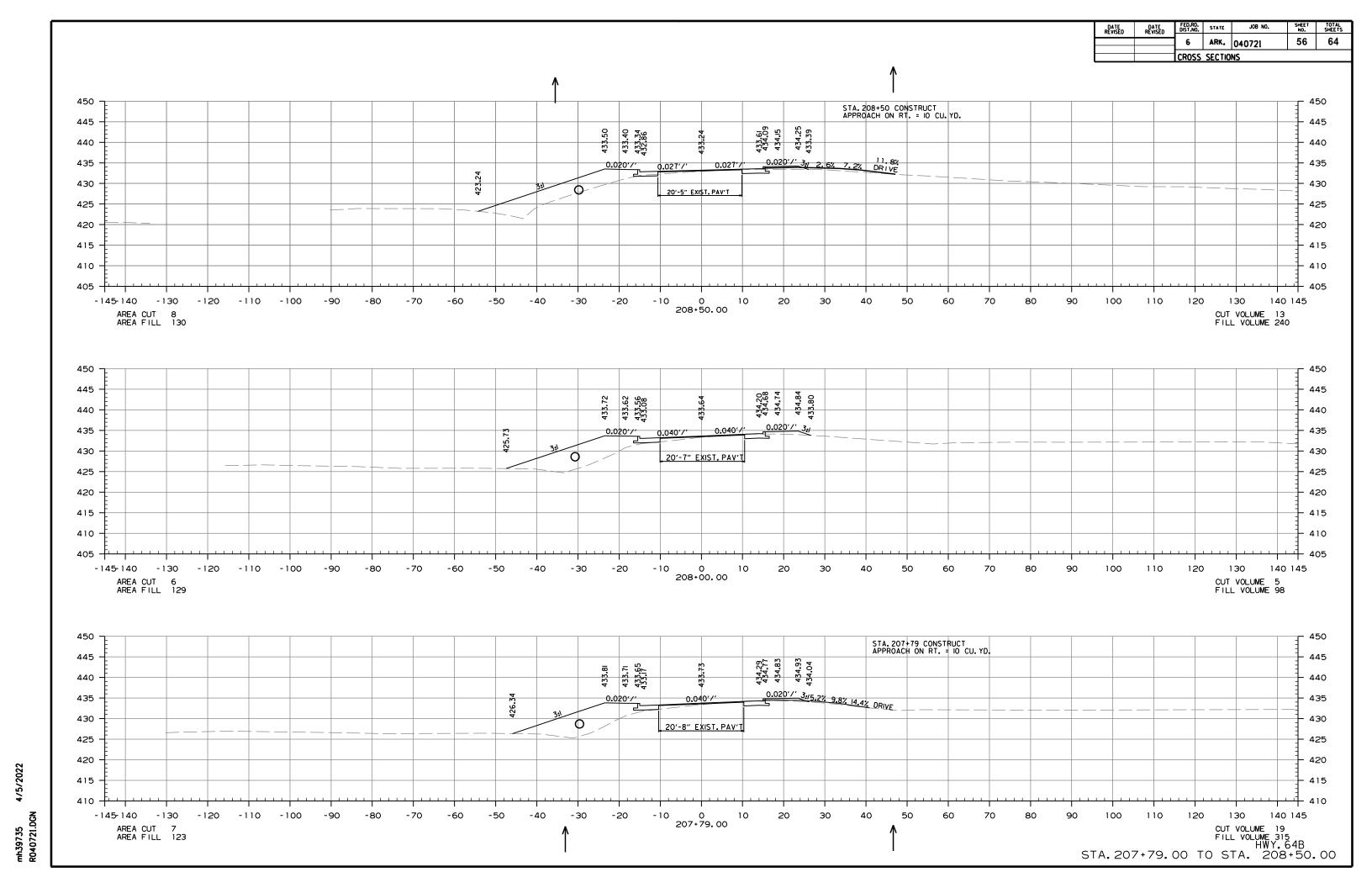
 CHECKED BY:
 CALE: AS NOTED
 SCALE: AS NOTED DESIGNED BY: BAB DATE: 04/2022 **BRIDGE NO.** 07590

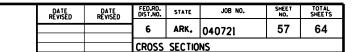


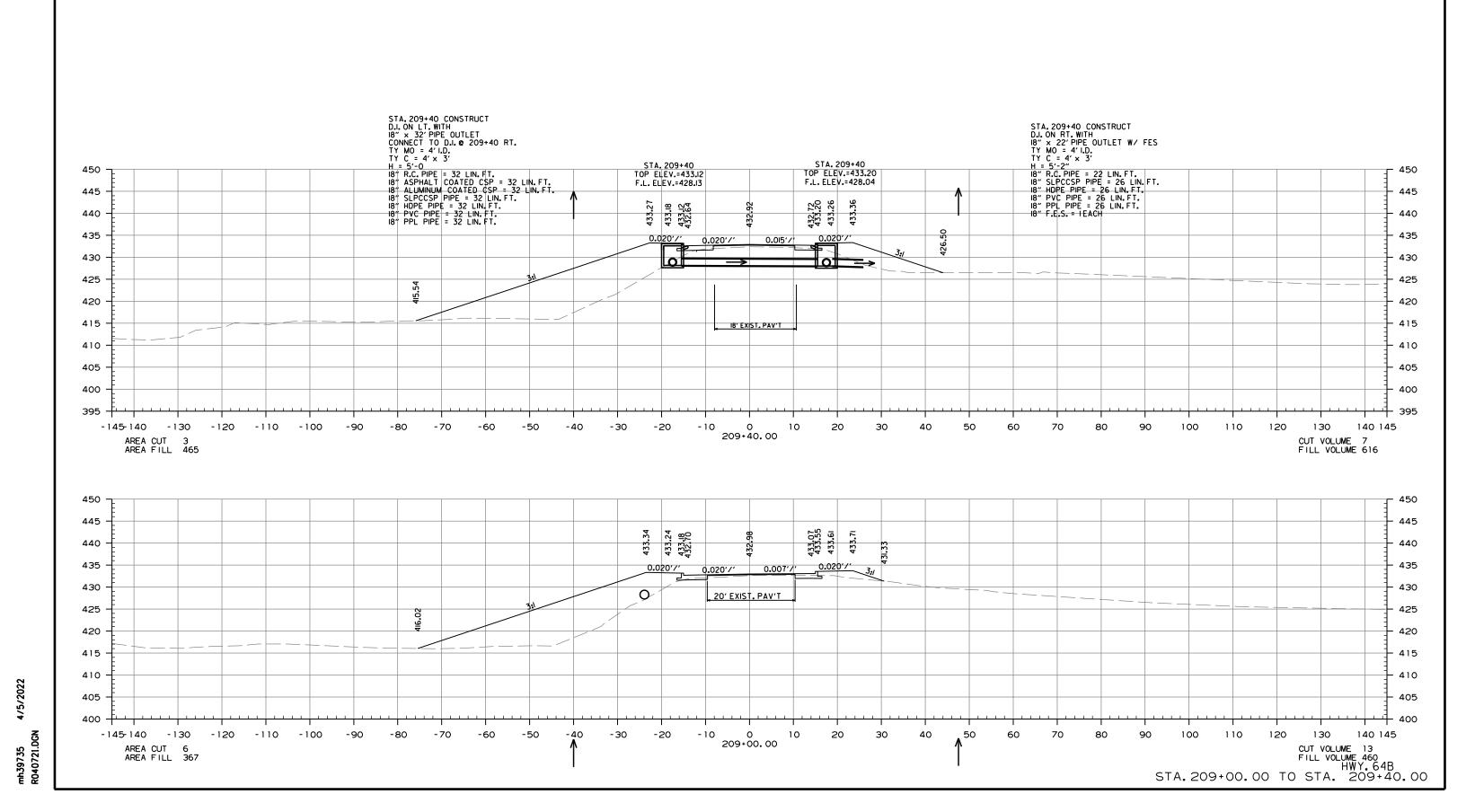


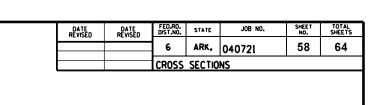
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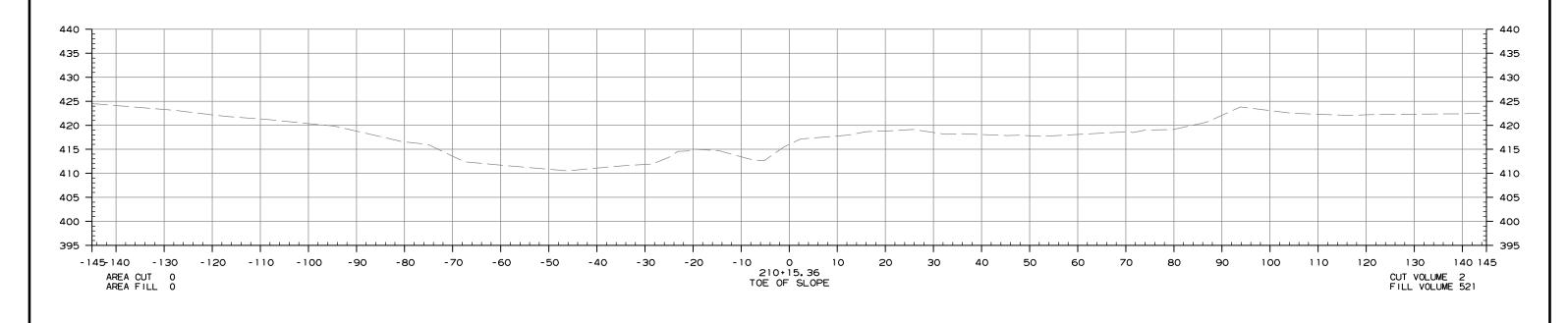


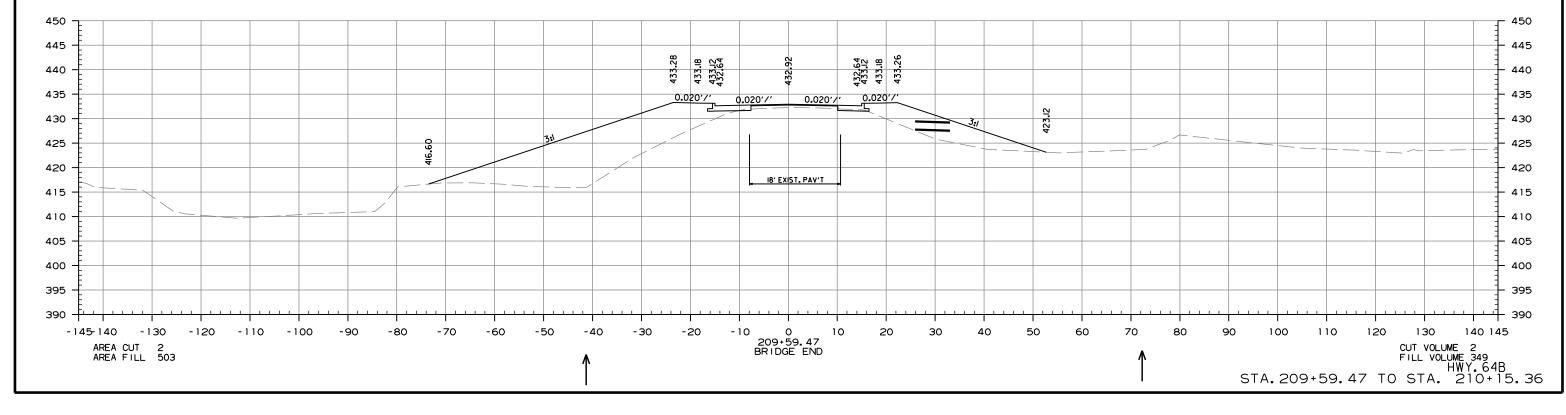




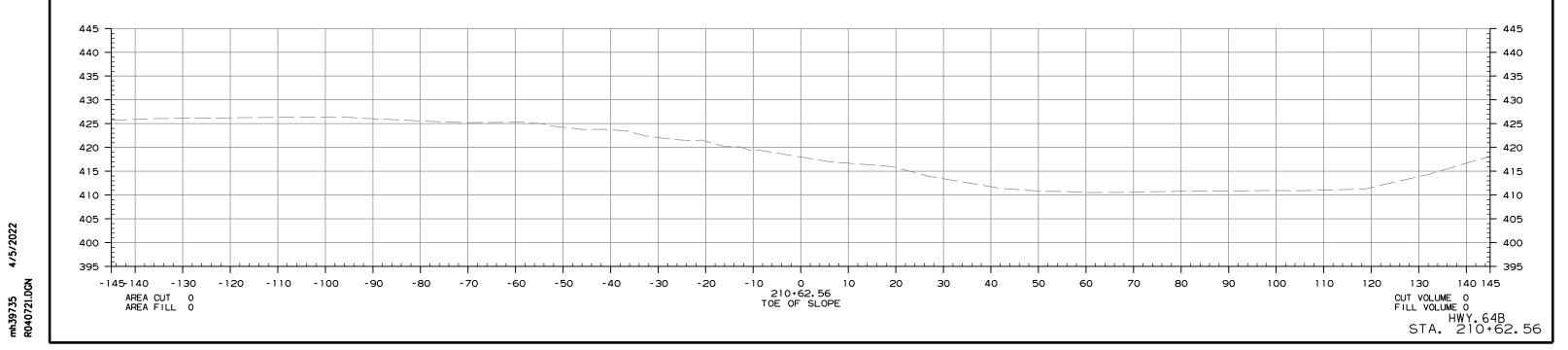


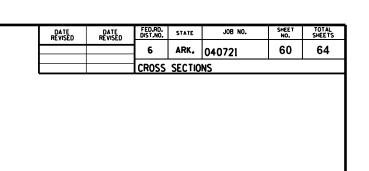


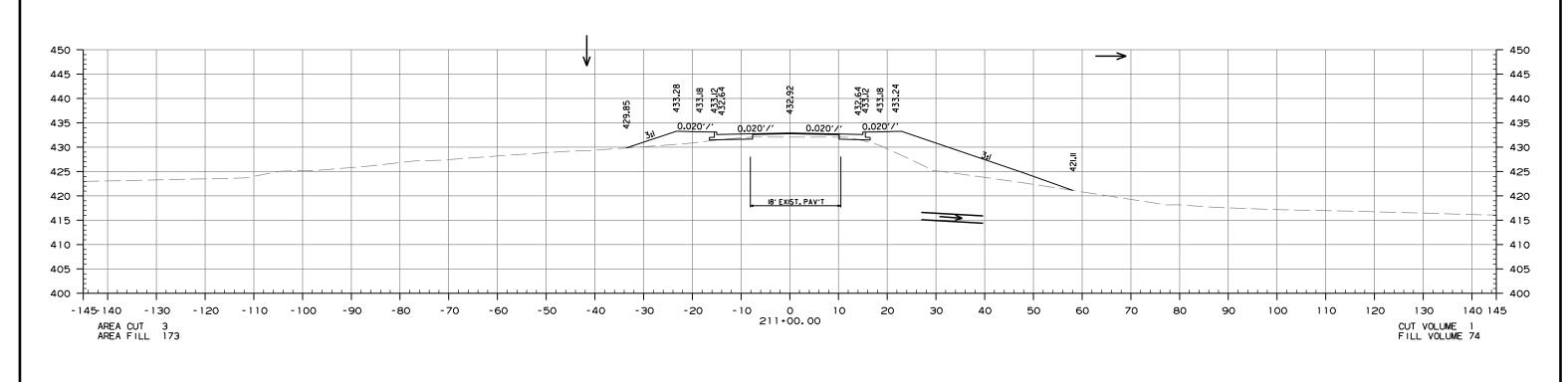


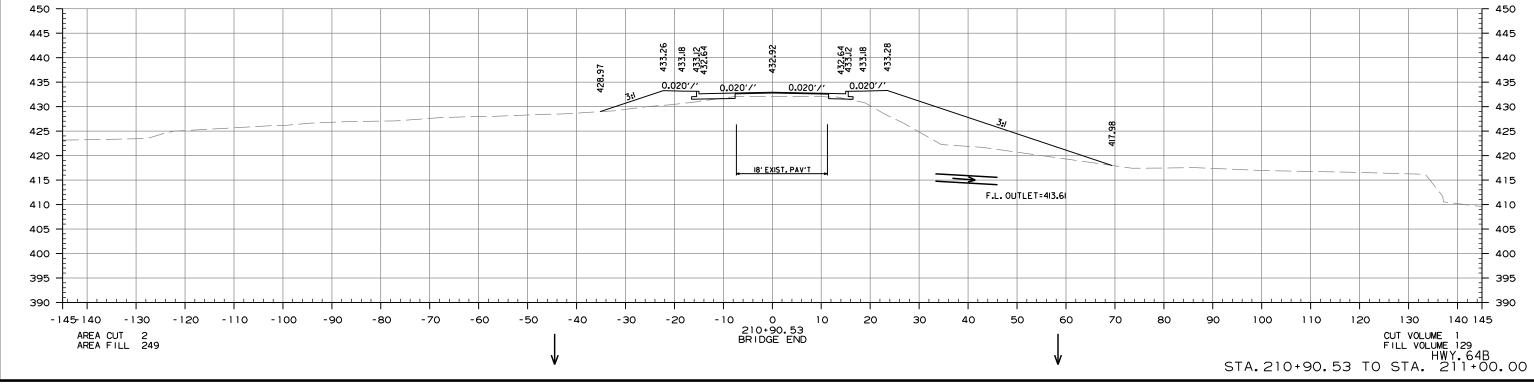


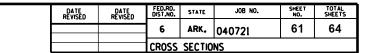
DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	040721	59	64
		CROSS	SECTIO	NS		

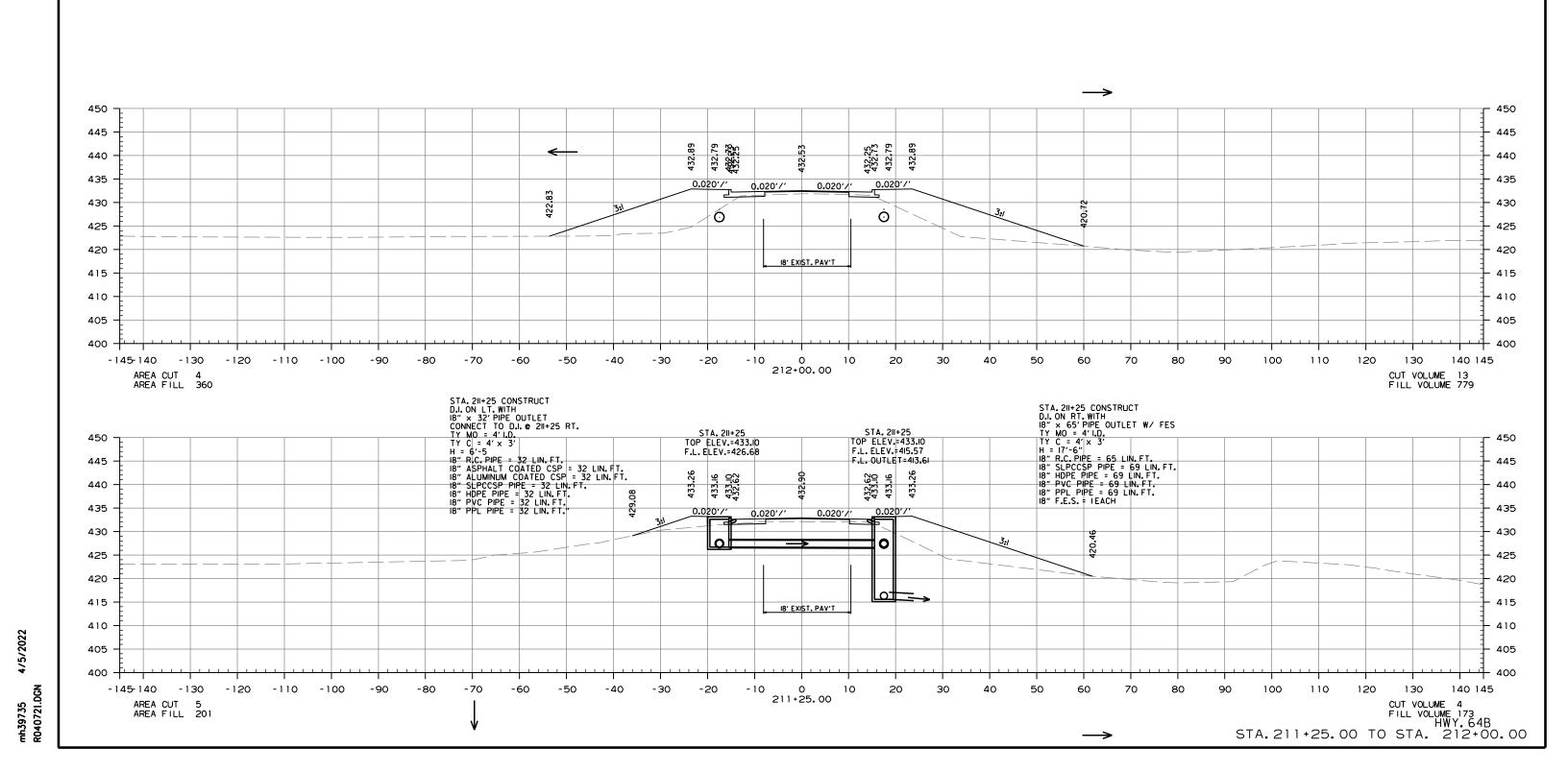


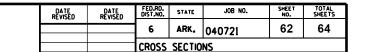


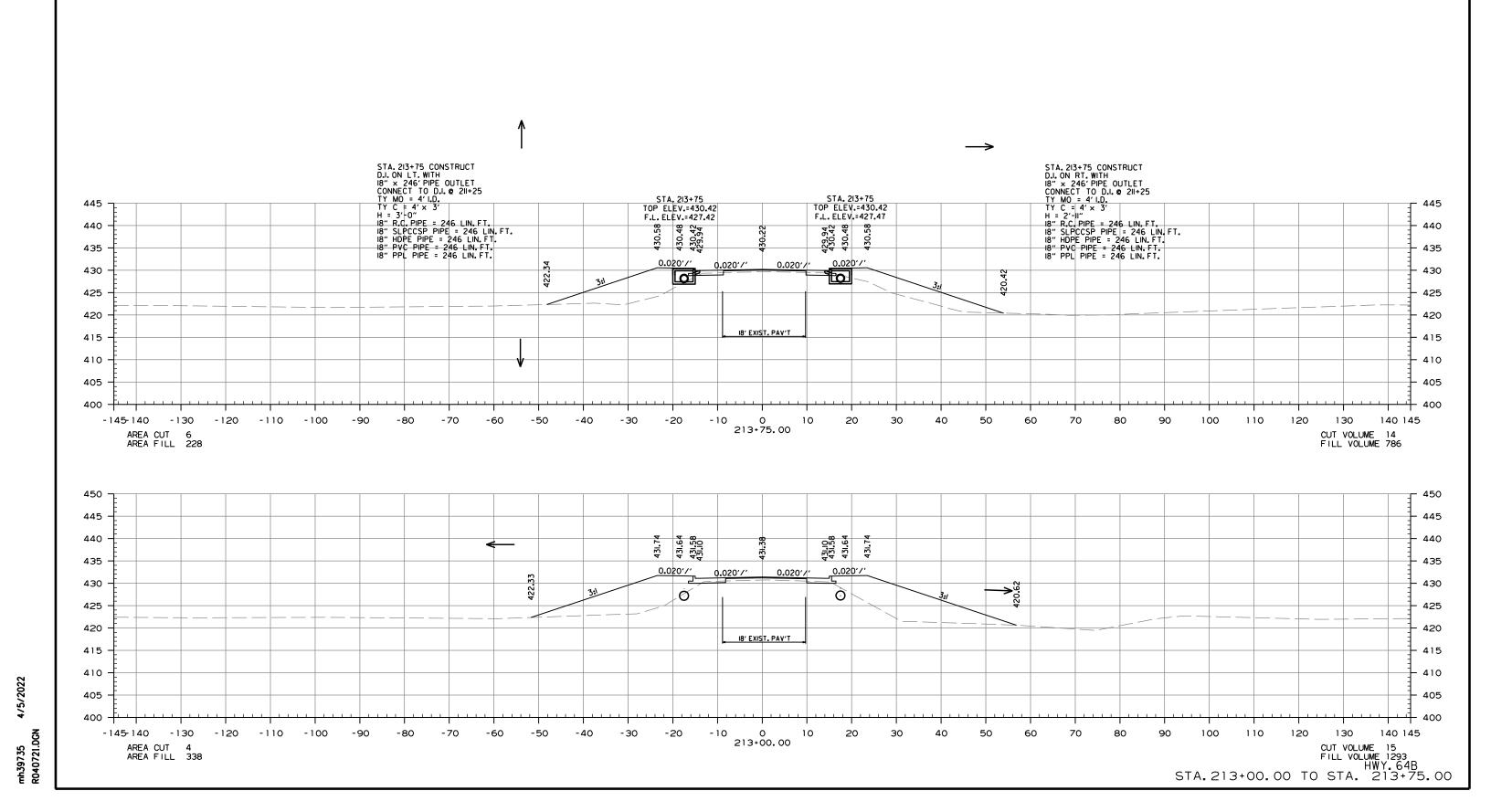


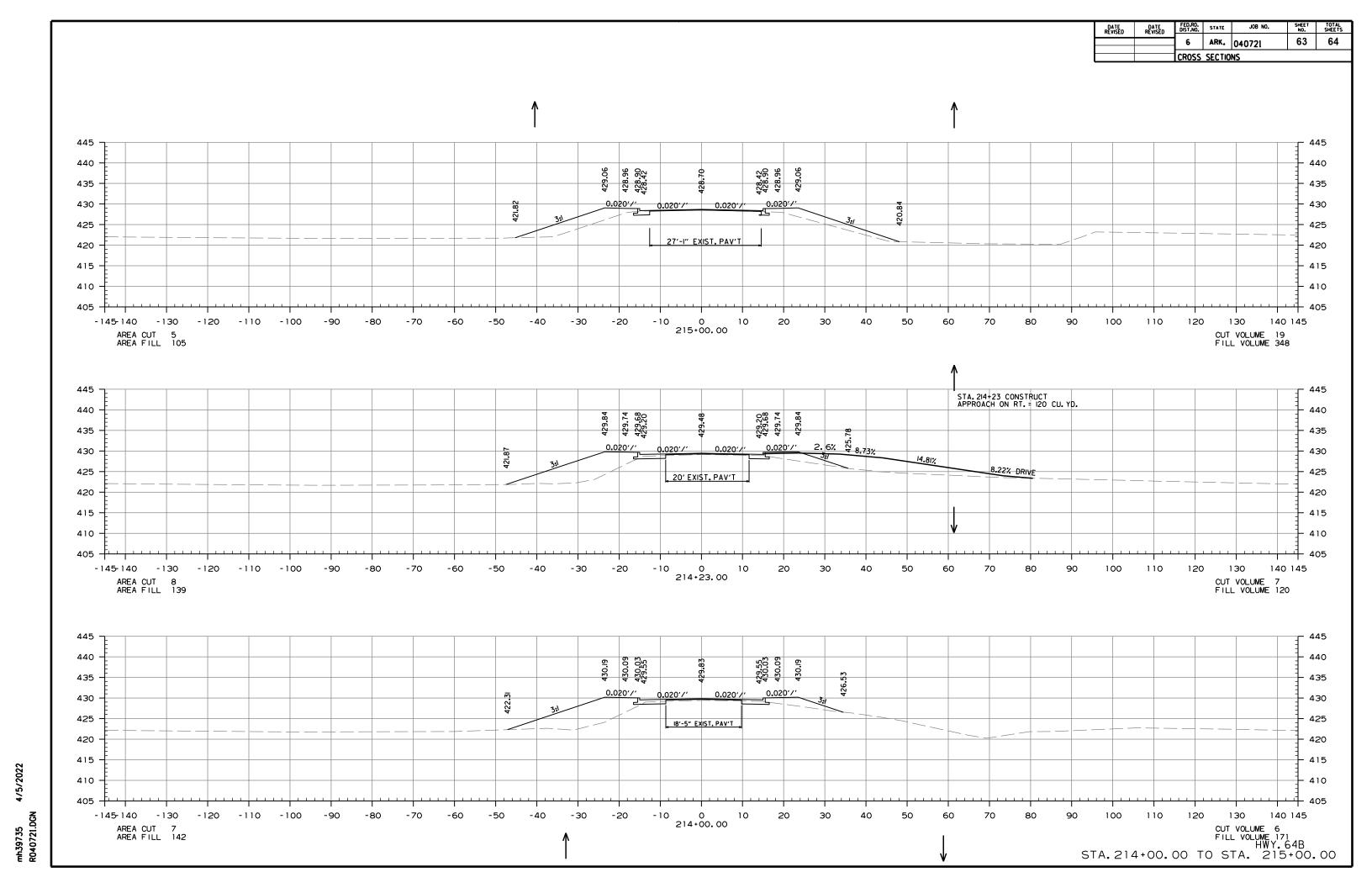






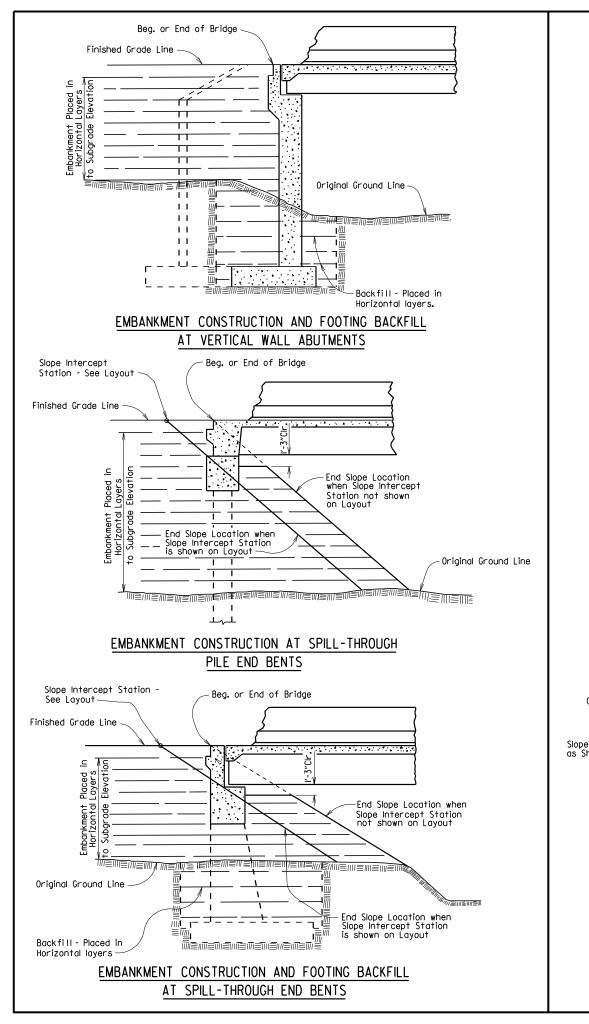


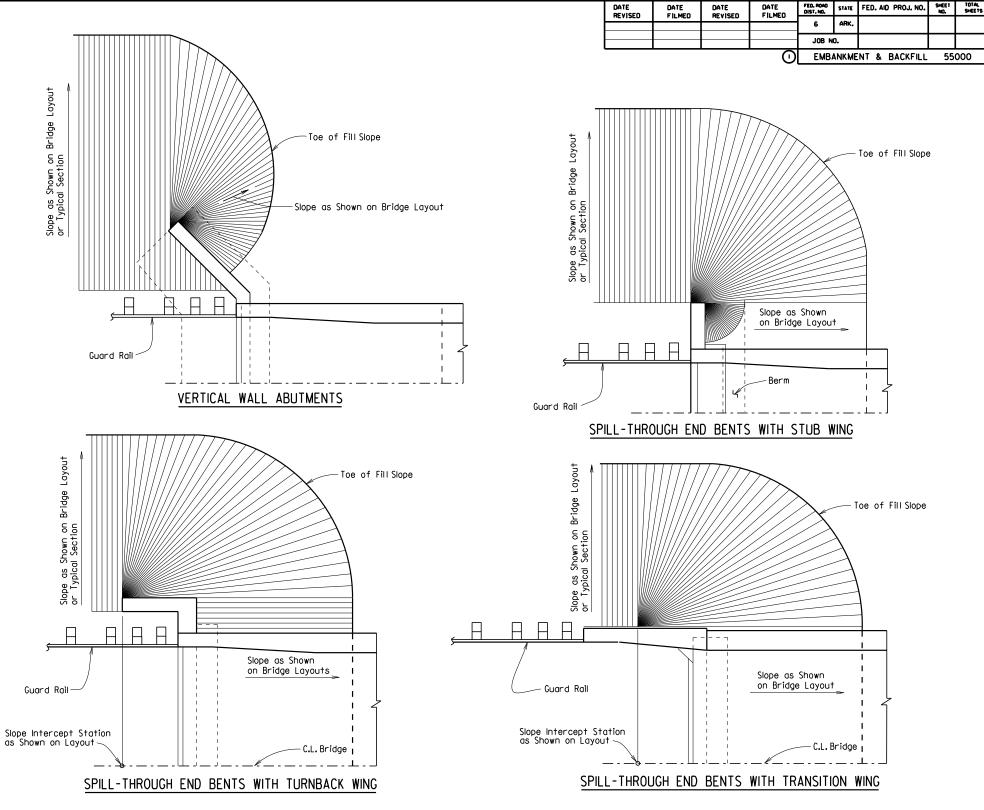




FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED 6 ARK. 040721 64 64 CROSS SECTIONS AREA CUT 0 AREA FILL 0 CUT VOLUME 6 FILL VOLUME 74 STA. 216+85.25 END 177' TRANSITION 445 445 440 440 428.30 427.74 435 435 05 430 0.020'/' 0.02017 0.020'/' 430 425 425 37' EXIST. PAV'T 420 420 415 415 410 410 405 405 400 --145-140 - 130 -120 -110 -100 -90 -70 -50 -40 - 30 -20 20 30 70 100 110 130 -80 -60 -10 10 50 60 216+08.21 END 100' TAPER AREA CUT 5 AREA FILL 52 CUT VOLUME 8 FILL VOLUME 73 STA. 215+75 CONSTRUCT
D.I. ON RT. WITH
18" x 98' PIPE OUTLET
CONNECT TO EXIST. D.I. @ 216+73
TY MO = 4' I.D.
TY C = 4' x 3'
H = 3', 0"
18" R.C. PIPE = 98 LIN. FT.
18" SLPCCSP PIPE = 98 LIN. FT.
18" PVC PIPE = 98 LIN. FT.
18" PVC PIPE = 98 LIN. FT.
18" PPL PIPE = 98 LIN. FT. STA. 215+75 CONSTRUCT
D.I. ON LT. WITH
18" × 16' PIPE OUTLET W/ FES
TY MO = 4' LD.
TY C = 4' × 3'
H = 3'-0"
18" R.C. PIPE = 16 LIN. FT.
18" SLPCCSP PIPE = 20 LIN. FT.
18" HDPE PIPE = 20 LIN. FT.
18" PPL PIPE = 20 LIN. FT.
18" PPL PIPE = 20 LIN. FT.
18" PPL PIPE = 20 LIN. FT. STA. 215+75 TOP ELEV.=428.70 STA. 215+75 TOP ELEV.=428.75 F.L. ELEV.=425.75 445 445 F.L. ELEV = 425.70 440 440 428.76 428.76 428.70 428.22 428.27 428.75 428.81 435 435 430 0.020'/ 430 0.0207 0.02077 425 425 27'-6" EXIST. PAV'T 420 420 F.L. OUTLET=421.86 415 410] 410 405 405 mh39735 R040721.DGN -20 -10 0 10 20 215+75.00 STA. 215+08.21 END JOB 040721 & BEGIN 100' TAPER & BEGIN 177' TRANSITION -145-140 -130 140 145 -120 -110 -100 -90 -80 - 70 -60 -50 -40 - 30 -20 20 30 40 50 60 70 80 90 100 110 120 130 CUT VOLUME 18 FILL VOLUME 239 HWY. 64B STA. 215+75.00 TO STA. 216+08.21 AREA CUT 8 AREA FILL 67

4/5/2022





METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 6 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to Subsections 210.09, 210.10 and 801.08 for construction requirements.

STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

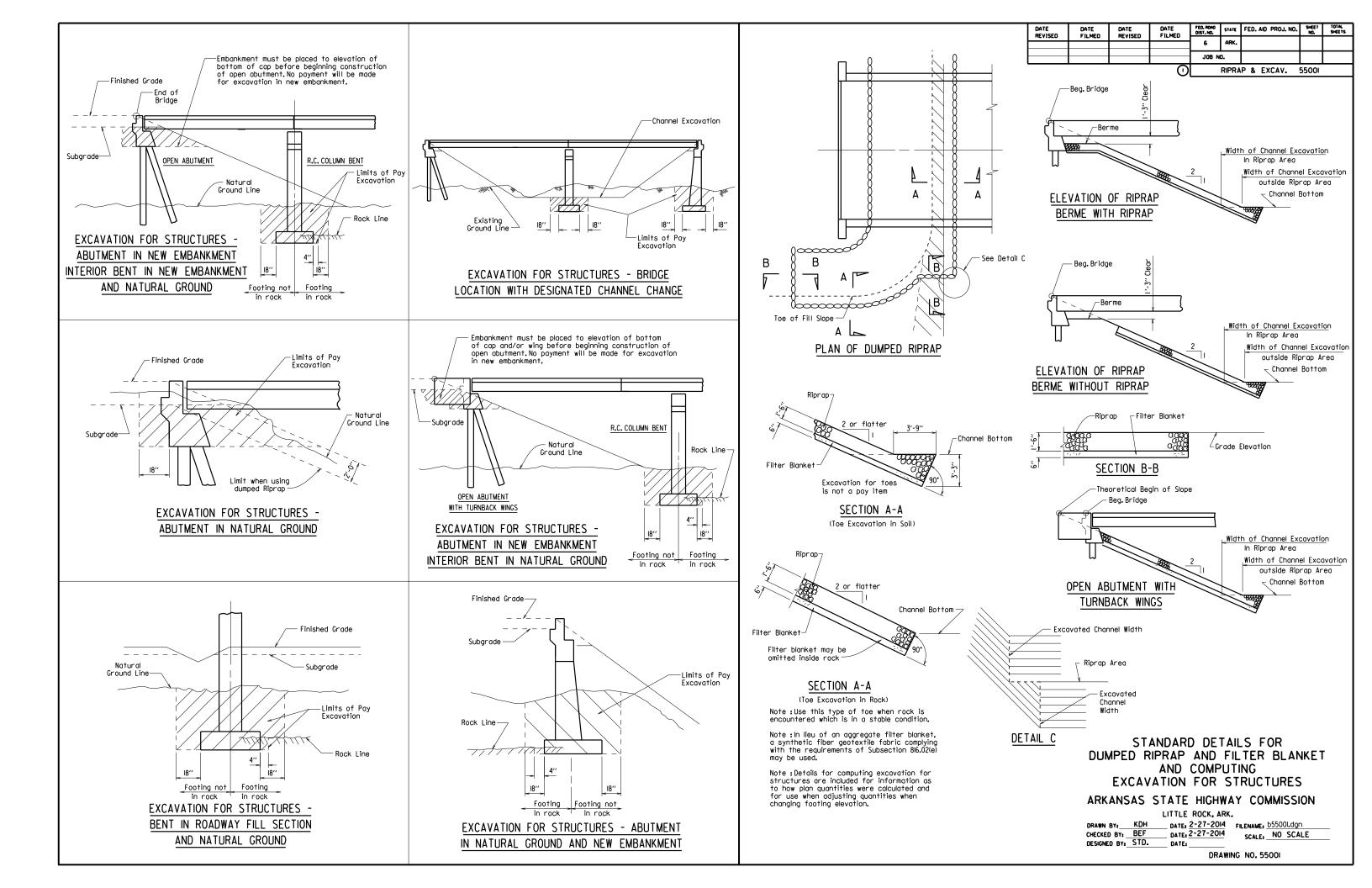
ARKANSAS STATE HIGHWAY COMMISSION

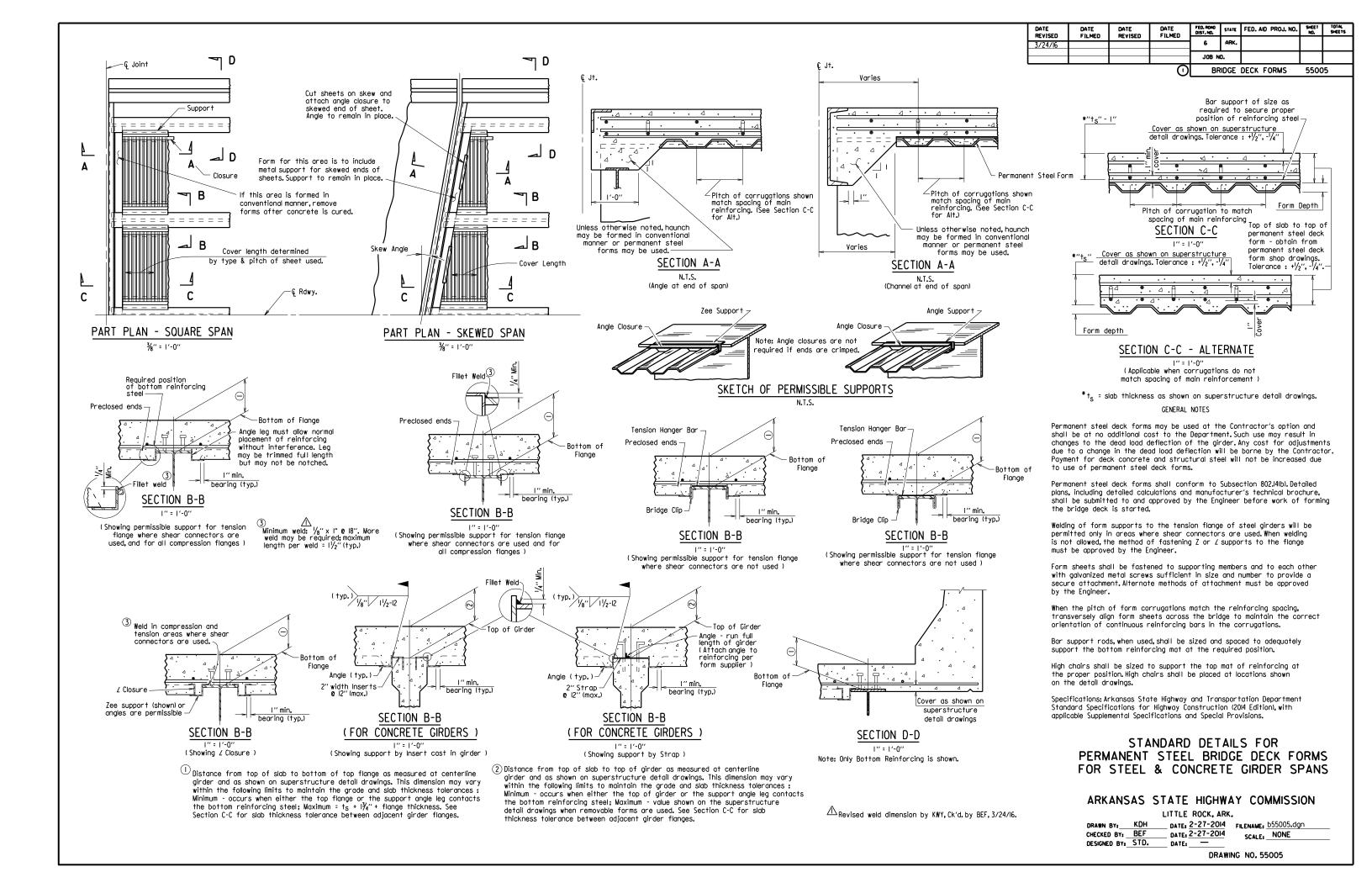
LITTLE ROCK, ARK.

 DRAWN BY:
 KDH
 DATE:
 2-27-2014
 FILENAME:
 b55000.dgn

 CHECKED BY:
 BEF
 DATE:
 2-27-2014
 SCALE:
 NO SCALE

 DESIGNED BY:
 STD.
 DATE:





GENERAL NOTES

These GENERAL NOTES are applicable unless otherwise shown in the Plan Details, Special Provisions, or Supplemental Specifications.

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 Edition) with applicable Supplemental Specifications and Special Provisions. Section and Subsection refer to the Standard Specifications.

DESIGN SPECIFICATIONS: See Bridge Layout(s).

SUPERSTRUCTURE NOTES:

MATERIALS AND STRENGTHS:

Class S(AE) Concrete	fʻc	=	4,000 psi
Reinforcing Steel (Gr. 60, AASHTO M 31 or M 322, Type A)	fy	=	60,000 psi
Structural Steel (AASHTO M 270, Gr. 36)	Fy	=	36,000 psi
Structural Steel (AASHTO M 270, Gr. 50)	Fy	=	50,000 psi
Structural Steel (AASHTO M 270, Gr. 50W)	Fу	=	50,000 psi
Structural Steel (AASHTO M 270, Gr. HPS70W)	Fу	=	70,000 psi

See Plan Details for Grade(s) of Structural Steel required.

CONCRETE:

All concrete shall be Class S(AE) with a minimum 28 day compressive strength f'c = 4,000 psi. Concrete shall be poured in the dry and all exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

The superstructure details shown are for use when removable deck forming is used and are the basis for measurement of Class S(AE) Concrete. See Standard Drawing No.55005 for allowable modifications and for tolerances when Permanent Steel Bridge Deck Forms are used.

Use of a longitudinal screed is not permitted on any span of a bridge deck with horizontal curvature.

The concrete deck (roadway surface) shall be given a tine finish in accordance with Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Sidewalks shall receive a broomed finish as specified for final finishing in Subsection 802.19 for Class 6 Broomed Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour. Sufficient concrete must be placed ahead of the strike-off to fully load the beam or girder. When permitted, the use of a longitudinal strike-off will require that a vertical camber adjustment be made in the strike-off to account for the future dead load deflection due to any railings, median barrier, and sidewalks.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 conforming to AASHTO M 3lor M 322, Type A, with mill test reports and shall be epoxy coated. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Epoxy Coated Reinforcing Steel (Grade 60)".

STRUCTURAL STEEL (COMMON TO W-BEAMS AND PLATE GIRDERS):

Structural steel shall be AASHTO M 270 with grade and payment as specified in the plans. Grade 50W steel shall not be painted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84(e). Grade 36 and Grade 50 steel shall be painted unless otherwise noted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84. Structural steel completely embedded in concrete may be AASHTO M 270, Gr. 36, Gr. 50 or Gr. 50W unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with the specifications, submitted and approval secured before fabrication is begun.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval; however, additional welds used for attaching falsework support devices or screed rail supports to the structural steel that do not exceed the limitations of Subsection 802.13 will not require approval prior to construction. All welding shall conform to Subsection 807.26.

Unless otherwise noted, field connections shall be bolted with 3/4" ø high-strength bolts using 13/6" ø open holes. Holes for $\frac{3}{4}$ " ø high-strength bolts may be $\frac{1}{6}$ " ø if a washer is supplied for use under both the nut and head of the bolt. The use of oversized holes will not be allowed on main members unless otherwise noted. Bolts shall be placed with heads on the outside face of the exterior beam or girder webs and on the bottom of the beam or girder flanges.

All stud shear connectors shall be granular flux filled, solid fluxed, or equal and shall be automatically end welded in accordance with recommendations of the Manufacturer.

When painting is required, all structural steel except galvanized steel and steel completely encased in concrete shall be painted in accordance with Subsection 807.75. The color of paint shall be as specified in the plans.

FILMED FILMED 6 JOB NO. \odot GENERAL NOTES 55006

FED. AID PROJ. NO. SHEET

STRUCTURAL STEEL (W-BEAMS):

All beams and field splice plates, and all diaphragms and connection plates attached to horizontally curved beams are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Beam Spans (M 270, Gr. ...)".

All beams in continuous units and simple spans with field splices shall be blocked in their true position in the shop in groups as specified in Subsection 807.54(b)(2) with the webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All beams in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Flange field splice plates shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

All beam dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for comber.

Bent plate diaphragms for horizontally curved beams shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight beams may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved beams.

Unless otherwise noted, diaphragms shall be installed as beams are erected. All bolts in diaphragms and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

STRUCTURAL STEEL (PLATE GIRDERS):

All references to cross-frames shall include "X" or "K" types.

All girder web and flange plates, all field splice plates, and all diaphragms, cross-frames and connection plates attached to horizontally curved girders are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr. ...)".

All girders in continuous units and simple spans with field splices shall be assembled in the shop as specified in Subsection 807.54(b)(2) and blocked in their true position with webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All girders in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Web and flange plates for main members and flange splice plates for main members shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

Girder webs may be made by shop splicing with minimum lengths of 25 feet for sections. Flange plates longer than 50 feet may be made by shop splicing with minimum lengths of 25 feet for sections. No additional payment will be made for shop welded splices.

All girder dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for camber.

Groove welds in web and flange plates shall be Quality Control (Q.C.) tested by nondestructive testing, as required in Subsection 807.23(b). Fillet welds at flange to web plate connections shall be O.C. tested by the magnetic particle method. All O.C. testing shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr. ...)".

Bent plate diaphragms for horizontally curved girders shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight girders may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved girders.

Unless otherwise noted, cross-frames and diaphragms shall be installed as girders are erected. All bolts in cross-frames, diaphragms, and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

SUBSTRUCTURE NOTES:

CONCRETE:

Unless otherwise noted concrete in caps, columns and footings (except seal footings) shall be Class "S" with a minimum 28 day compressive strength f'c = 3,500 psi and shall be poured in the dry. Seal Concrete for footings shall have a minimum 28 day compressive strength f'c = 2,100 psi.

Concrete in drilled shafts shall be Class "S" as modified by Job SP "Drilled Shaft Foundations".

All exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322. Type A. with mill test reports.

Top reinforcing bars in cap shall be properly placed to avoid interference with anchor bolts or sheet metal sleeves.

STRUCTURAL STEEL:

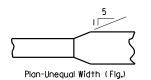
Structural steel in end bents shall be AASHTO M 270 with grade and payment as specified in the

FOR ADDITIONAL INFORMATION AND NOTES, SEE LAYOUT(S) AND PLAN DETAILS.

STANDARD GENERAL NOTES FOR STEEL BRIDGE STRUCTURES

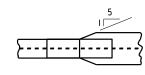
ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARK.

DRAWN BY: A.M.S. DATE: 9-2-2015 FILENAME: b55006.dgn CHECKED BY: B.E.F. DATE: 9-2-2015 SCALE: NO SCALE DESIGNED BY: STD. DATE:

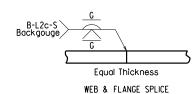


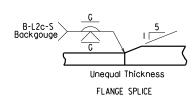
FLANGE SPLICE

Plate Girder Spans (____)".

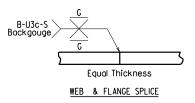


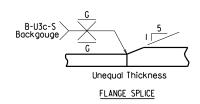
FLANGE SPLICE AT UNEQUAL BOTTOM FLANGE WIDTHS





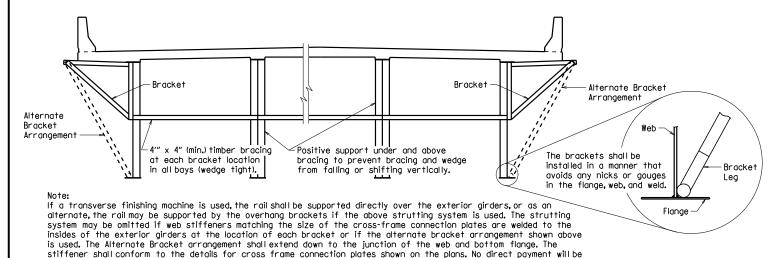
(Use when Base Metal Thickness is Equal to or Less than 2")





(Use when Base Metal Thickness is Greater than 2")

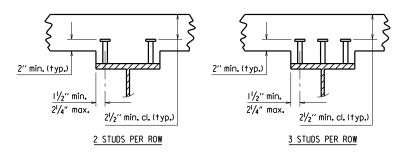
DETAILS OF WELDED SPLICES FOR PLATE GIRDERS



SCREED RAIL SUPPORT FOR PLATE GIRDERS

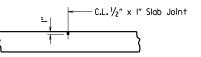
(USE WHEN WEB DEPTHS ARE 48" OR GREATER)

made for brackets, timber bracing, supports, or welded stiffeners. Payment shall be subsidiary to "Structural Steel in



Stud Shear Connectors shall be automatically end welded to the beam or girder flange in accordance with the recommendations of the Manufacturer. See plan details for number and size.

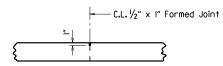
SHEAR CONNECTOR DETAIL



Use Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint Sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. Slab Joints shall extend to the outside edge of the deck slab and shall align with open joints at the front face of the parapet. Slab joints shall be installed before the parapet railing is poured. If slab joints are to be sawed, they shall be sawed as soon as the concrete has sufficiently set to allow sawing of the joint without damage to the slab. Slab joints shall be placed at all pouring sequence construction joints and required slab joint locations. The joint sealer shall extend across the deck from gutterline to gutterline.

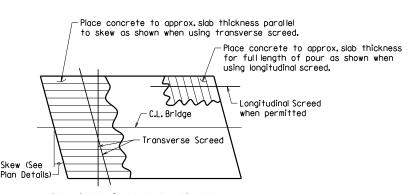
ADDITIONAL NOTES IF SIDEWALKS OR RAISED MEDIANS ARE REQUIRED: Slab Joints shall be installed before the sidewalk or raised median is poured. After installation of the joint in the sidewalk or raised median and prior to pouring the parapet rail, the joint sealer shall be placed extending across the deck slab from gutterline to gutterline and acrosss the top of the sidwalk or raised median to the edge of the slab. No joint sealer shall be placed on the deck slab under the sidewalk or raised median.

TRANSVERSE SLAB JOINT DETAIL



Use $\frac{1}{2}$ " x I" Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. This joint shall be formed. Seal color shall be gray or other color similar to concrete.

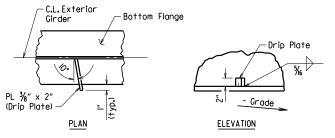
LONGITUDINAL CONSTRUCTION JOINT



Note: At the Contractor's option, the transverse screed may be placed parallel to the skew or perpendicular to C.L. Bridge.

CONCRETE PLACEMENT PROCEDURE

FOR BRIDGES WITH SKEW



Drip Plate to be welded to the outer side of the bottom flange of the exterior girders.

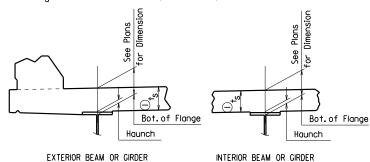
Locate drip plate 5'-0" from C.L. Bearing on high side of each Bent, unless otherwise noted in the plans.

BOTTOM FLANGE DRIP PLATE

(USE WHEN WEB DEPTHS ARE 54" OR GREATER AND UNIT OR SPAN IS NOT IN LEVEL GRADE)

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FEO. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
REVISED	FILMED	REVISED	TIENED	6	ARK.			
				JOB N	0.			
			<u> </u>		STE	EL BRIDGE STRUCT	URES	55007

 $t_{\rm S}$ = slab thickness. See "Typical Roadway Section" in the plans.



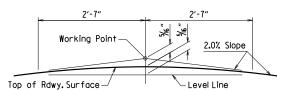
Tolerance when removable deck forming is used is + ½",- ¼". Haunch forming is required and shall be adjusted to maintain slab thickness tolerance.

NOTES:

Haunch dimension may vary within the following limits to maintain the grade and slab thickness tolerance: Minimum occurs when top flange contacts bottom reinforcing steel; Maximum = top flange thickness plus 1½" unless otherwise noted in the plans. No increase in concrete and structural steel quantities will be made to maintain tolerances.

Tolerances shown are applicable only when removable deck forming is used. See Std. Dwg. No. 55005 for tolerances when permanent steel deck forms are used. Payment for concrete shall be based on removable deck forming.

ADJUSTMENT FOR SLAB THICKNESS TOLERANCE



NOTE: Working Point matches Theoretical Roadway Grade.

ROUNDING DETAIL BRIDGES IN NORMAL CROWN

WELD TABLE

Material Thickness of Thicker Part Joined (Inches)	Minimum Size of Fillet Weld (Inches)	Single Pass Weld Must
To ¾" Inclusive	1/4"	Be
0ver ¾"	5/6 ′′	Used

NOTE: When a fillet weld size, as shown on the plans, is larger than the minimum, the first pass shall be that specified for minimum size of fillet weld.

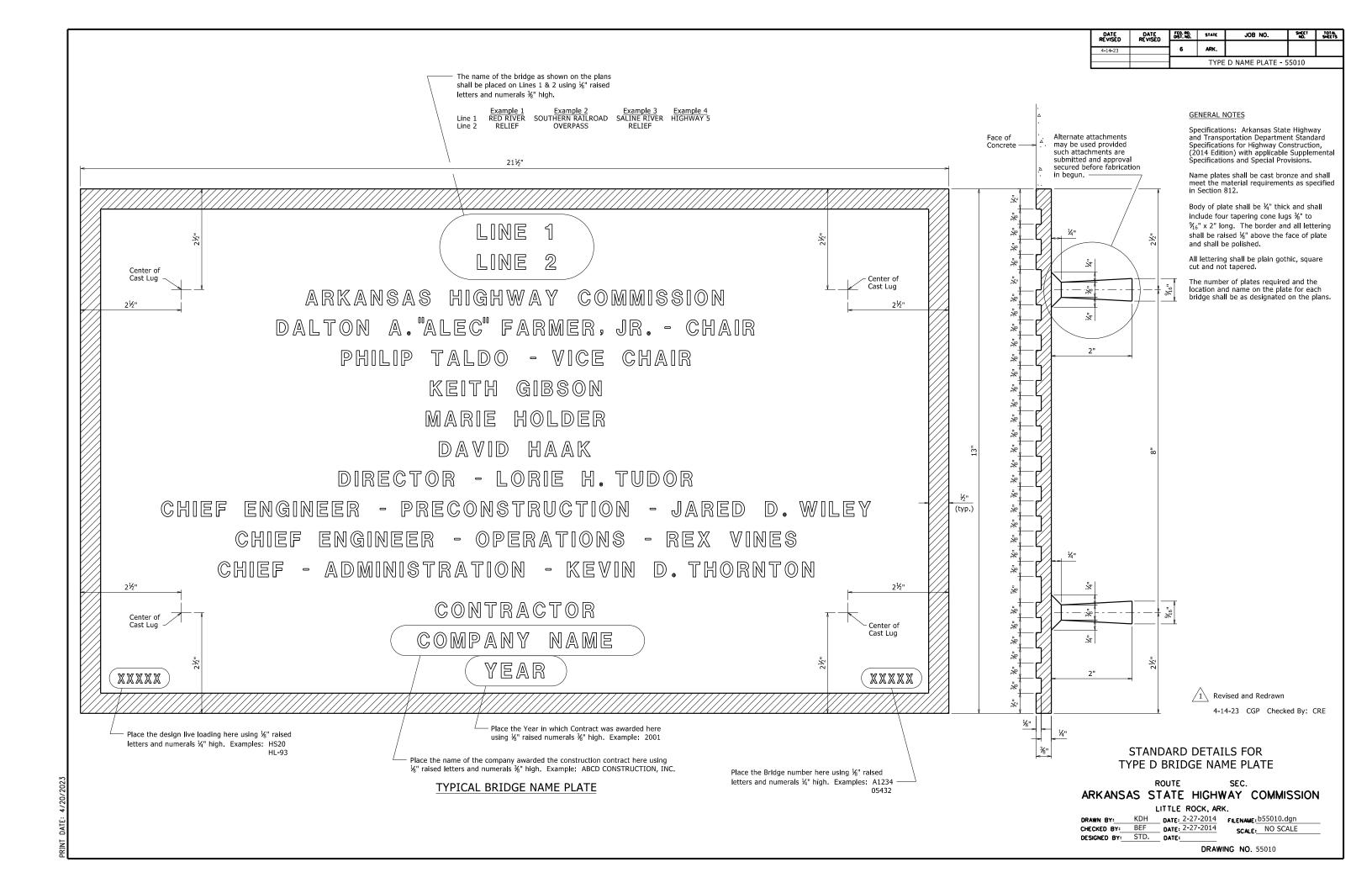
SECTION AND SUBSECTION REFER TO THE ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2014 EDITION).

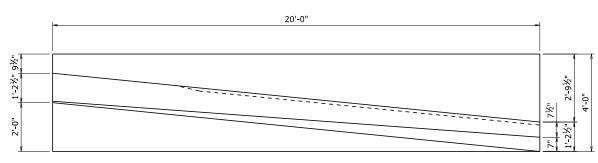
THESE DETAILS ARE APPLICABLE UNLESS OTHERWISE SHOWN IN THE PLAN DETAILS, SPECIAL PROVISIONS, OR SUPPLEMENTAL SPECIFICATIONS.

STANDARD DETAILS FOR STEEL BRIDGE STRUCTURES

ARKANSAS STATE HIGHWAY COMMISSION

		LITTLE ROCK, AR	N _e
DRAWN BY:	JYP	DATE: 2/11/2016	FILENAME: b55007.dgn
CHECKED BY:	AMS	DATE: 2/11/2016	SCALE: No Scale
DESIGNED BY.	STD.	DATE: -	50422

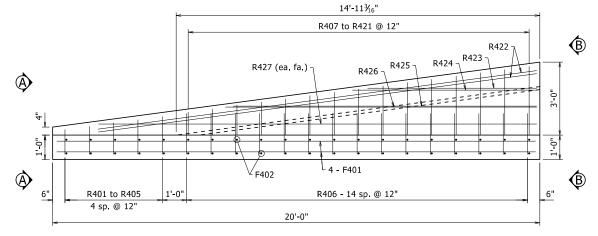




PLAN OF TRANSITIONAL APPROACH RAILING

Railings on each side of roadway are opposite hand to each other

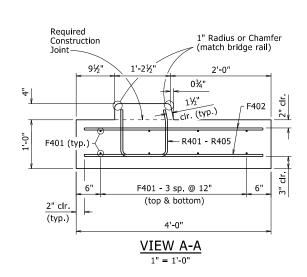
½" = 1'-0"

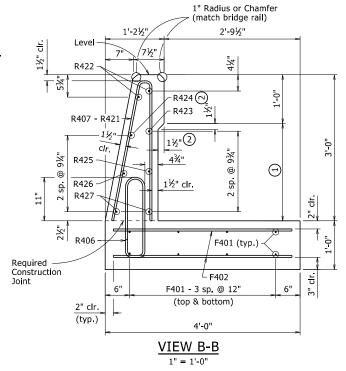


ELEVATION OF TRANSITIONAL APPROACH RAILING

½" = 1'-0"

- 1) Recess height varies as shown from 2'-0" to 0".
- (2) Eliminate recess when formliner with architectural finish is used. See Plans for additional information.





GENERAL NOTES

Transitional Approach Railing Type SSTR36 shall be placed at locations shown in plans.

All concrete shall be Class "S" with a minimum 28 day compressive strength f'c = 3,500 psi and shall be poured in the dry. All exposed corners to be chamfered 1" unless otherwise noted

All reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A, with mill test reports.

All longitudinal lines within the limits of horizontal curves shall be on curves concentric to C.L. Construction. Adjustment to longitudinal bar lengths may be required. Transverse reinforcing shall be placed on radial lines to C.L. Construction.

Unless otherwise required in the plans, curing and finishing shall be in accordance with Subsection 806.05(c) and the surface finish type and areas of application shall match that used on the adjacent bridge railing or barrier wall. See Subsection 802.19(3) for Class 3 Textured Coating Finish or Subsection 803.03(a) or 803.03(b) for Class 1 or 2 Protective Surface Treatment, respectively. Surface finishes shall not be paid for directly, but shall be considered incidental to the unit price bid for "Transitional Approach Railing."

When alternate surface and/or architectural finishes are specified in the plans, no direct payment will be made, and the alternate finish shall be considered incidental to the unit price bid for "Transitional Approach Railing". See plan details for additional information when architectural finishes are specified.

Transitional Approach Railing Type SSTR36 shall be paid for at the contract unit price bid for "Transitional Approach Railing". See Section 806 for additional information.

Scales shown are for 22"x34" drawings. When using 11"x17" drawings, reduce scale by one half.

DATE FILMED

R406

Concrete terminal where

shown in plans.

JOB NO.

FED. AID PROJ. NO. SHEET

TRANSITIONAL RAIL - 55013A

-Bridge Rail (shown) or

Concrete Barrier Wall

BAR LIST - ONE TRANSITIONAL APPROACH RAILING MARK NO. REQ'D LENGTH P.D. BENDING DIAGRAMS F401 19'-8" Str. Var. 7¾" to 3½" F402 40 3'-8" Str. 2'-10" -R401 2" 1 ea. 3'-11" to R405 11½" ' to 2'-101/4" R406 2" 15 4'-5" R401 to R405 2'-5" -R407 3¾" P.D. 1 ea. 2" 5'-9" to R421 R422 2 18'-2" Str. R407 to R421 R423 6'-11" Str. R424 7'-6" 1 Str. R425 12'-6" Str. 1

Dimensions are out to out of bars.

FOR INFORMATION ONLY SCHEDULE OF QUANTITIES PER RAIL UNIT

R426

R427

2

12'-9"

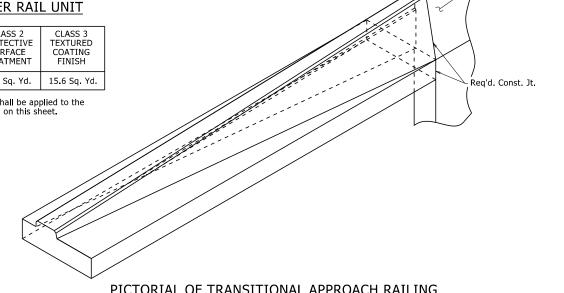
17'-11"

Str.

Str.

CLASS "S" CONCRETE	REINFORCING STEEL (GRADE 60)	CLASS 1 PROTECTIVE SURFACE TREATMENT	CLASS 2 PROTECTIVE SURFACE TREATMENT	CLASS 3 TEXTURED COATING FINISH
4.1 Cu. Yds.	374 Lbs.	0.2 Gal.	8.1 Sq. Yd.	15.6 Sq. Yd.

Only one of the above three surface treatments shall be applied to the transitional approach railing. See "General Notes" on this sheet.



PICTORIAL OF TRANSITIONAL APPROACH RAILING

Sidewalk not shown for clarity

No Scale

SECTION AND SUBSECTION REFER TO THE ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2014 EDITION).

THESE DETAILS ARE APPLICABLE UNLESS OTHERWISE SHOWN IN THE PLAN DETAILS, SPECIAL PROVISIONS, OR SUPPLEMENTAL SPECIFICATIONS.

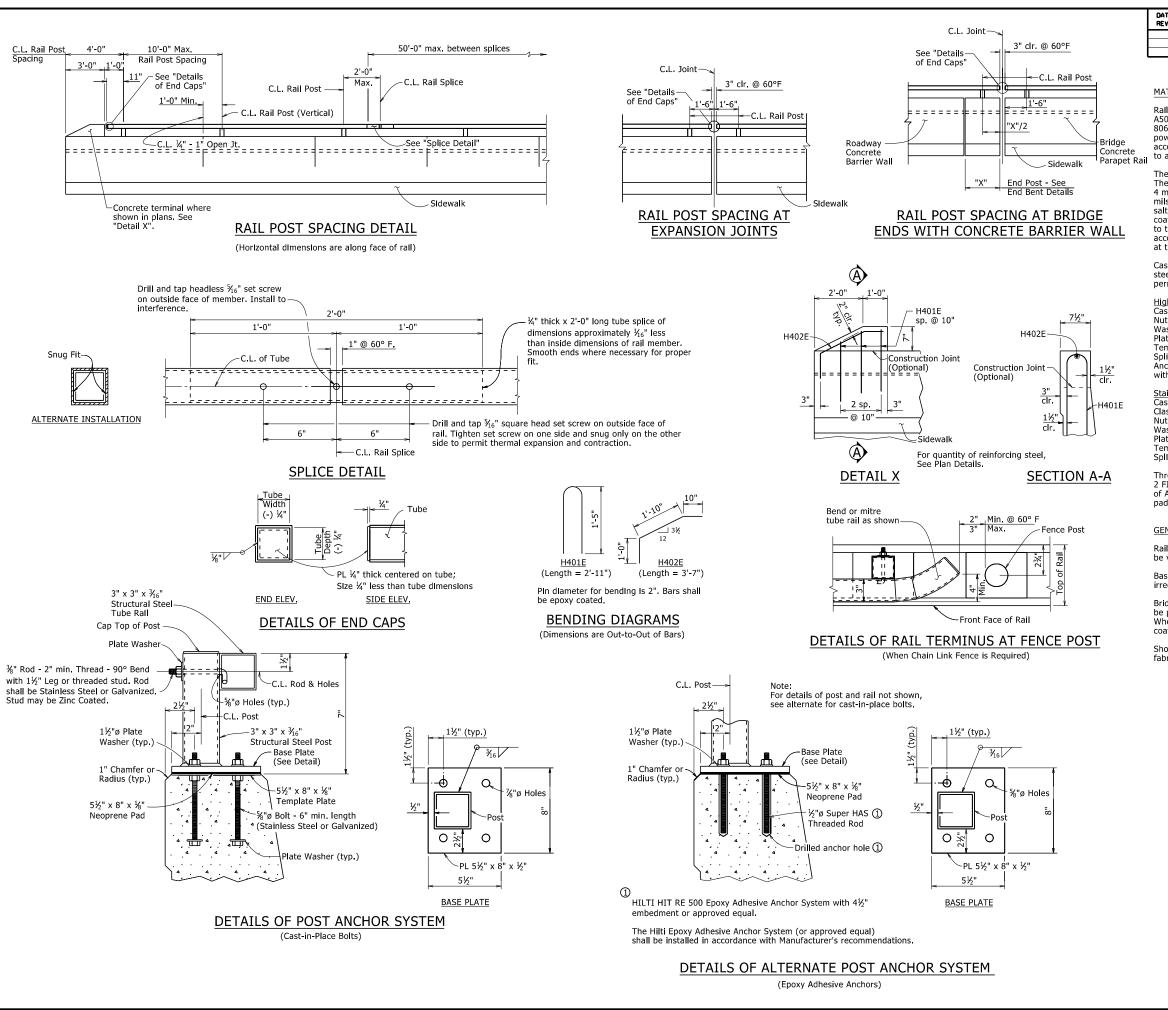
STANDARD DETAILS FOR TRANSITIONAL APPROACH RAILING TYPE SSTR36

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

DRAWN BY: KWY DATE: 4/8/2021 FILENAME: b55013a.dgn SCALE: As Shown CHECKED BY: BHS DATE: 4/8/2021 DESIGNED BY: STD.

DRAWING NO. 55013A



FED. AID PROJ. NO. FILMED JOB NO. TYPE H2 RAILING - 55015

MATERIALS:

Rall tubing, posts, end caps, and base plates shall conform to ASTM A709, Grade 36 or ASTM A500-Grade B, and shall be galvanized after fabrication in accordance with Subsection 806.02(c). When required elsewhere in the plans, steel rail members shall receive a powder coating process after galvanizing. Galvanized surfaces shall be prepared in accordance with Subsection 807.87 and the manufacturer's recommendations prior to application of the powder coating process.

The powder coating process shall be a two coat system applied using electrostatic spray. The base coat shall be a thermosetting epoxy powder with a minimum thickness of 2 to 4 mils. The top coat shall be tough polyester powder with a minimum thickness of 2 to 4 mils. The color shall be as shown in the plans. Coated galvanized framework shall have a salt spray resistance of 3,000 hours using ASTM B117 without loss of adhesion. The powde coating process shall be in accordance with manufacturer's recommendations. Any damage to the powder coated finish shall be repaired with a compatible touch-up system in accordance with the manufacturer's recommendations and to the satisfaction of the Engineer at the Contractor's expense.

Cast-in-place anchor bolts, nuts, washers, and set screws shall be galvanized high-strength steel or stainless steel. Mixing of galvanized and stainless steel fasteners will not be

High-Strength Steel:

Cast-in-place anchor bolts shall conform to ASTM F3125, Grade A325, Type 1. Nuts shall conform to ASTM A563, Grade DH or AASHTO M 292, Grade 2H.

Washers shall conform to ASTM F436. Plate Washers shall conform to ASTM A709, Grade 36.

Template Plates shall conform to ASTM A709, Grade 36.

Splice Set Screws shall conform to ASTM A307, Grade A.
Anchor bolts, nuts, washers, plate washers, and set screws shall be galvanized in accordance with AASHTO M 232, Class C or ASTM B695, Class 50.

Stainless Steel:

Cast-in-place anchor bolts shall conform to ASTM A193, Grade B8, Class 2 or A320, Grade B8, Class 2 with a minimum yield strength of 80,000 psi. Nuts shall conform to ASTM A194, Grade 8.

Washers shall conform to ASTM A240. Type 302 Plate Washers shall conform to ASTM A240, Type 302.

Template Plates shall conform to ASTM A240, Type 302. Splice Set Screws shall conform to ASTM A193, Grade B8, Class 1 or A320, Grade B8, Class 1

Threads on bolts, screws, and nuts shall conform to American Standard Coarse Series, Class 2 FIT, ASA Specification B1.1. Plate washers shall have dimensions meeting the requirements of ANSI/ASME B18.22.1, Type A plain washer (Wide Series) unless otherwise noted. Neopren pads shall conform to the requirements of Subsection 807.15(b).

GENERAL NOTES FOR BRIDGE RAILING:

Rail layout shall conform to vertical and horizontal alignment of bridge. All posts shall be vertical. Rall sections shall be fabricated to attach to at least three posts.

Base plates shall not be placed upon areas that are improperly finished, deformed or

Bridge railing, including posts, templates, and base plates, fasteners, and neoprene pads shall be paid for at the contract unit price bid per linear foot for "Metal Bridge Railing (Type H2)". When required elsewhere in the plans, powdered coating finish and repair of powdered coating finish shall be considered subsidiary to the item "Metal Bridge Railing (Type H2)"

Shop drawings showing details of railing shall be submitted and approval secured prior to

SECTION AND SUBSECTION REFER TO THE ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR

THESE DETAILS ARE APPLICABLE UNLESS OTHERWISE SHOWN IN THE PLAN DETAILS, SPECIAL PROVISIONS, OR SUPPLEMENTAL SPECIFICATIONS.

> STANDARD DETAILS FOR TYPE H2 RAILING

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

_ DATE: 6/25/2020 FILENAME: b55015.dgn K1T SCALE: No Scale CHECKED BY: KWY DATE: 6/25/2020 DESIGNED BY: STD.

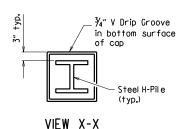
GENERAL NOTES FOR STEEL H-PILES:

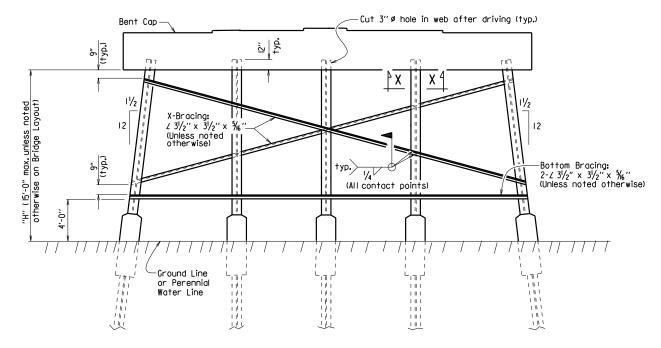
Steel H-Piles shall conform to AASHTO M 270, Grade 36 or greater.

See Bridge Layout and Bent Details for pile size, estimated length, spacing, pile anchorage (if required) and for driving information.

Steel H-Piles that extend above the ground and are not protected by pile encasement shall be painted in accordance with Subsection 805.02.

Brackets, lugs, cap plates, pile tips, driving points, pile painting, splicing and welding shall not be paid for directly, but shall be considered subsidiary to the item "Steel Piling".





Notes:

All bracing shall be cut and welded in the field. Each brace shall be furnished in one piece, Payment shall be made under Item 807.

Unless noted otherwise, omit X-Bracing when "H" is less than 8 feet.

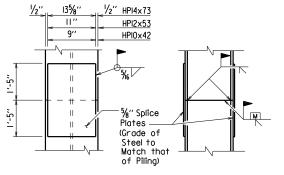
Omit X-Bracing and Bottom Bracing when "H" is 5 feet or less.

When required on the Bridge Layout sheet, pile encasements shall be constructed. See Notes and Details for H-Pile Encasements.

Omit all bracing (and V-groove in cap) when pile encasement is extended to bottom of bent cap.

TYPICAL DETAILS OF H-PILE TRESTLE INTERMEDIATE BENT

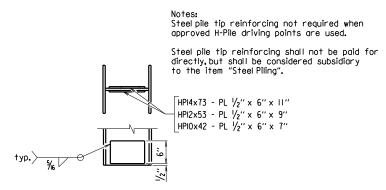
(Shown with Partial Height Encasement)



The Contractor may for his own convenience and at his own expense provide as many as three splices per pile. Minimum spacing between splices shall be 5 feet.

TYPICAL SPLICE DETAILS

H-pile splicers manufactured by Associated Pile and Fitting Corporation, LB Foster Piling, Skyline Steel or equivalent may be used in lieu of the "Typical Splice Details" shown. H-pile splicers shall match the same grade of steel specified for the piling and shall be welded to the pile with a %" fillet weld around the entire perimeter of the splice. Flanges shall be welded with a complete penetration groove weld complying with AASHTO/AWS Joint Designation B-U4a or B-U4b. All welding shall conform to Subsection 807.26 of the AHTD Standard Specifications for Highway Construction (2014 Edition).



REINFORCING DETAIL FOR STEEL H-PILE TIP

GENERAL NOTES FOR H-PILE ENCASEMENTS:

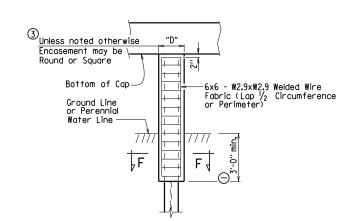
See Bridge Layout for additional notes, any pile encasement restrictions and required location of pile encasements.

All concrete shall be Class S with a minimum 28-day compressive strength, f'c = 3,500 psi. If concrete cannot be placed in the dry, Seal Concrete may be used from top to bottom of encasement.

Reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A.

Welded Wire Fabric shall conform to AASHTO M 55 or M 221. Galvanized Corrugated Steel Pipe shall conform to AASHTO M 36 and M 218.

Concrete, welded wire fabric or reinforcing steel and galvanized pipe shall not be paid for directly, but shall be considered subsidiary to the item "Pile Encasement".



PILE ENCASEMENT DETAIL FOR STEEL H-PILES (4) (Shown with Encasement to Bottom of Cap)

Round

Encasement

DATE FILMED

 \odot

6

JOB NO.

REVISED

3/24/16

FILMED

SECTION F-F

*Measured out-to-out of bar.

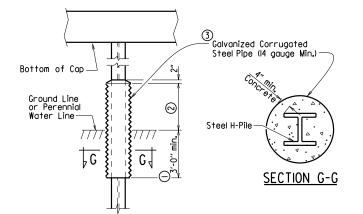
FED. AID PROJ. NO.

STEEL H-PILES

55020

TABLE OF VARIABLES FOR PILE ENCASEMENT

	"[
Pile Size	Square Encsmt.	Round Encsmt.	"L"*
HPI0×42	l'-7"	2'-0"	l'-4"
HPI2x53	l'-8"	2'-2"	l'-5"
HPI4x73	l'-l1"	2′-6″	l'-8"



- () Unless otherwise noted on Bridge Layout.
- $^{\circ}$ 3'-0" minimum or as shown on Bridge Layout.
- The state of the pile. Encasement dimensions shall be sized to maintain a minimum concrete cover of 4" from the H-Pile. Reinforcement shall be sized to provide a minimum concrete cover of $1\frac{1}{2}$ " and a minimum clearance of $1\frac{1}{4}$ " from the pile.
- Alternate pile encasement, when not extended to bottom of cap, shall have 2" concrete taper for water runoff as shown in the Partial Height Encasement detail.

ALTERNATE PILE ENCASEMENT DETAIL FOR STEEL H-PILES

(Shown with Partial Height Encasement)

Added alternate method of splicing H-piles and revised pile encasement note. 3/24/2016 AMS

This document was originally issued and sealed by Charles R. Ellis, PE No. 9235, on March 24, 2016.

This copy is not a signed and sealed document.

REGISTERED PROFESSIONAL ENGINEER
No. 9235

BRIDGE ENGINEER

STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS

ARKANSAS STATE HIGHWAY COMMISSION

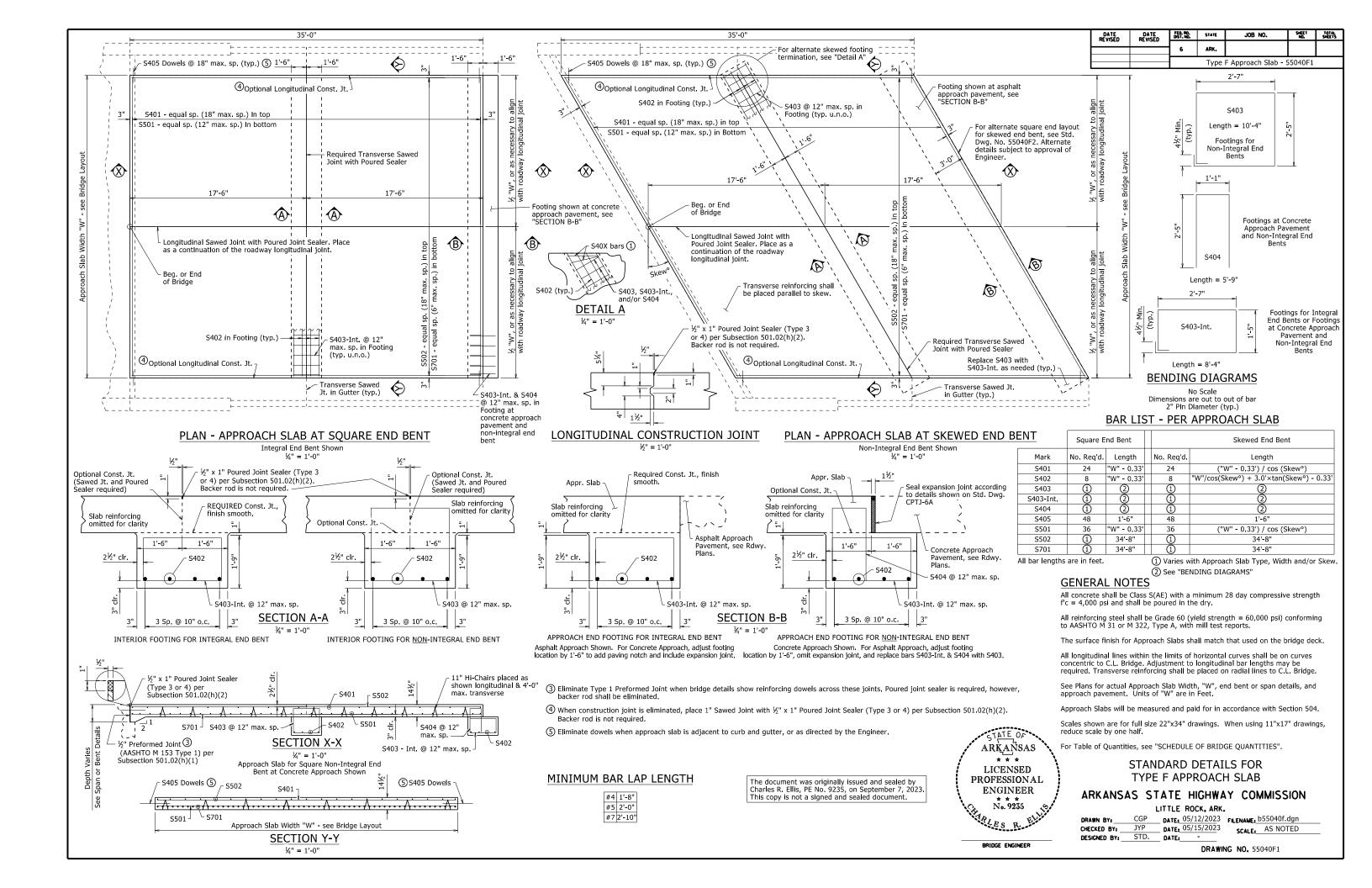
LITTLE ROCK, ARK.

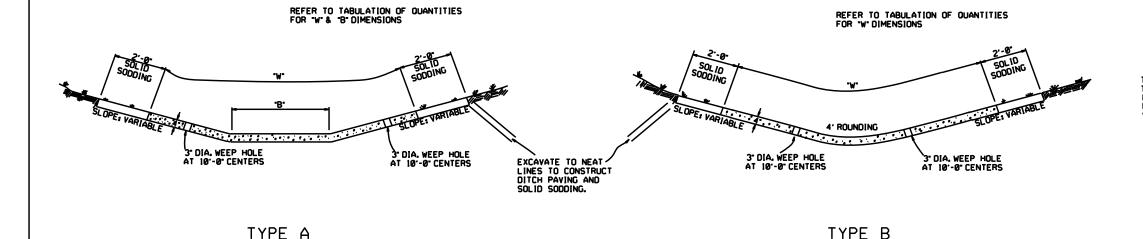
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 2/27/2014
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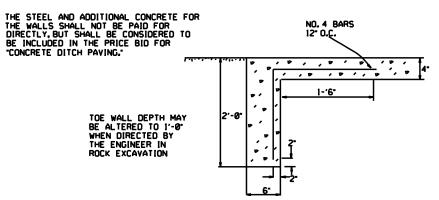
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 2/27/2014
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 NO SCALE

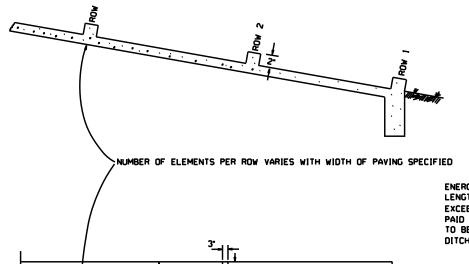
DRAWING NO. 55020







TOE WALL DETAIL FOR CONCRETE DITCH PAVING



ENERGY DISSIPATORS

6'-6"

ENERGY DISSIPATORS TO BE USED FOR THE ENTIRE LENGTH OF DITCH WHEN SLOPE OF DITCH PAVING EXCEEDS 7%. THE DISSIPATORS WILL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID FOR CONCRETE DITCH PAVING.

GENERAL NOTES:

THE FULL WIDTH OF EACH SECTION SHALL BE POURED MONOLITHICALLY.

TOE WALLS TO BE CONSTRUCTED FULL WIDTH AT EACH END OF DITCH PAVING, AND POURED MONOLITHICALLY.

SOLID SOD ALONG DITCH PAVING TO BE PLACED WITHIN 14 DAYS OF DITCH PAVING CONSTRUCTION.

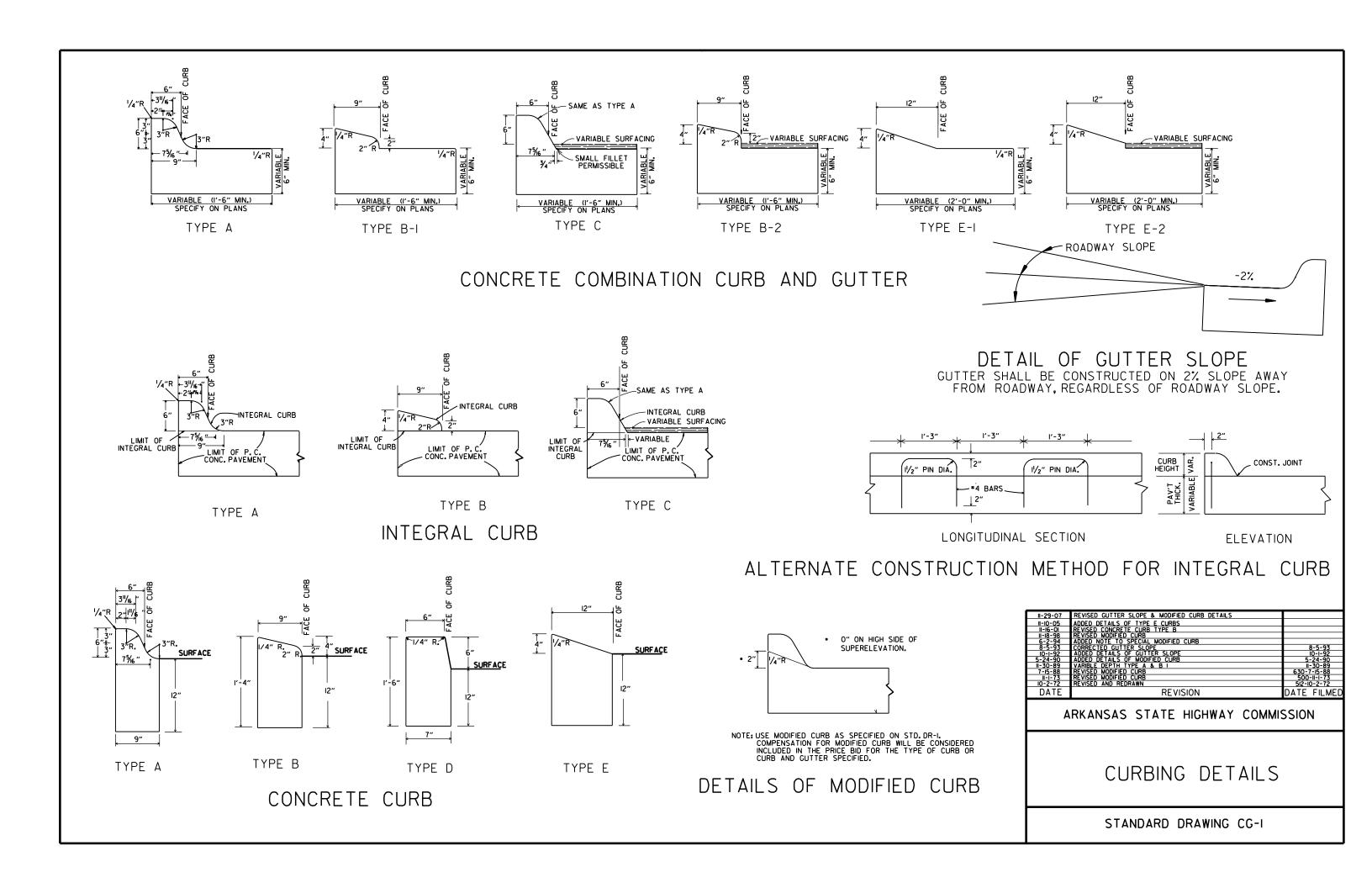
I' WIDE TRANSVERSE EXPANSION JOINTS SHALL BE PLACED IN CONCRETE DITCH PAVING AT 45' INTERVALS. THE SPACE SHALL BE FILLED WITH APPROVED JOINT FILLER COMPLYING WITH AASHTO M213.

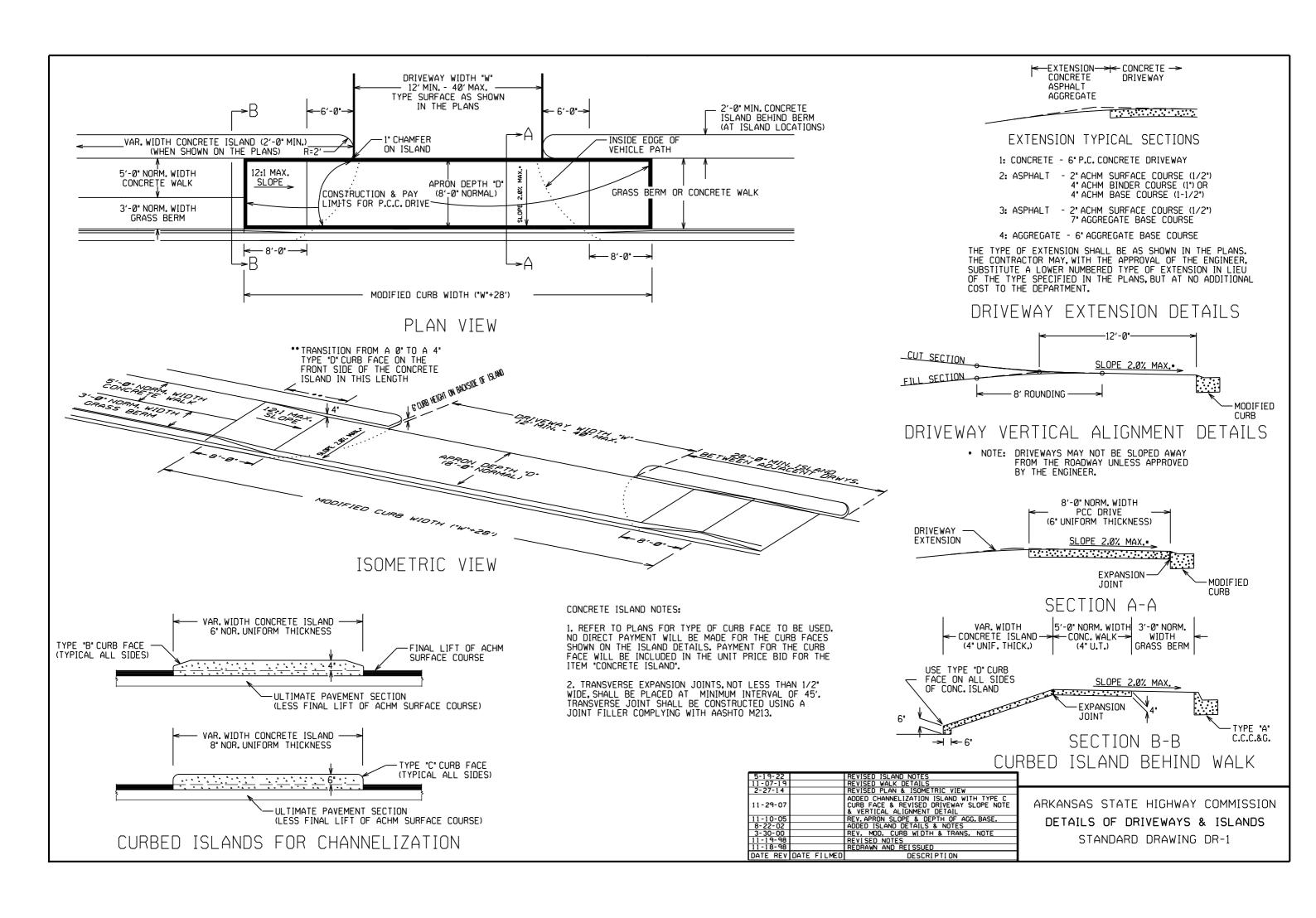
		ı
12-8-16	CORRECTED ENERGY DISSIPATOR DRAWING AND NOTE	
11-17-10	ADDED GENERAL NOTE	
6-2-94	ADDED GENERAL NOTE ABOUT SOLID SODDING	
11-30-8	ELIMINATED MIN. ROWS OF ELEMENTS	1111-30-89
7-15-88	REVISED DISSIPATOR NOTE	1653-7-15-88
4-3-87	REVISED ENERGY DISSIPATOR	1671 - 4 - 3 - 87
1-9-87		1532-1-9-87
11-3-86	ADDED NOTE TO ENERGY DISS.	1599-12-1-86
11-1-84	ENERGY DISSIPATOR DETAILS	1508-11-1-84
_	ADDED	
11-1-84	EXCAVATION DETAILS ADDED	
	TYPED A & B	
10-2-72	REVISED AND REDRAWN	508-10-2-72
_	DATE REVISION	DATE FILM D

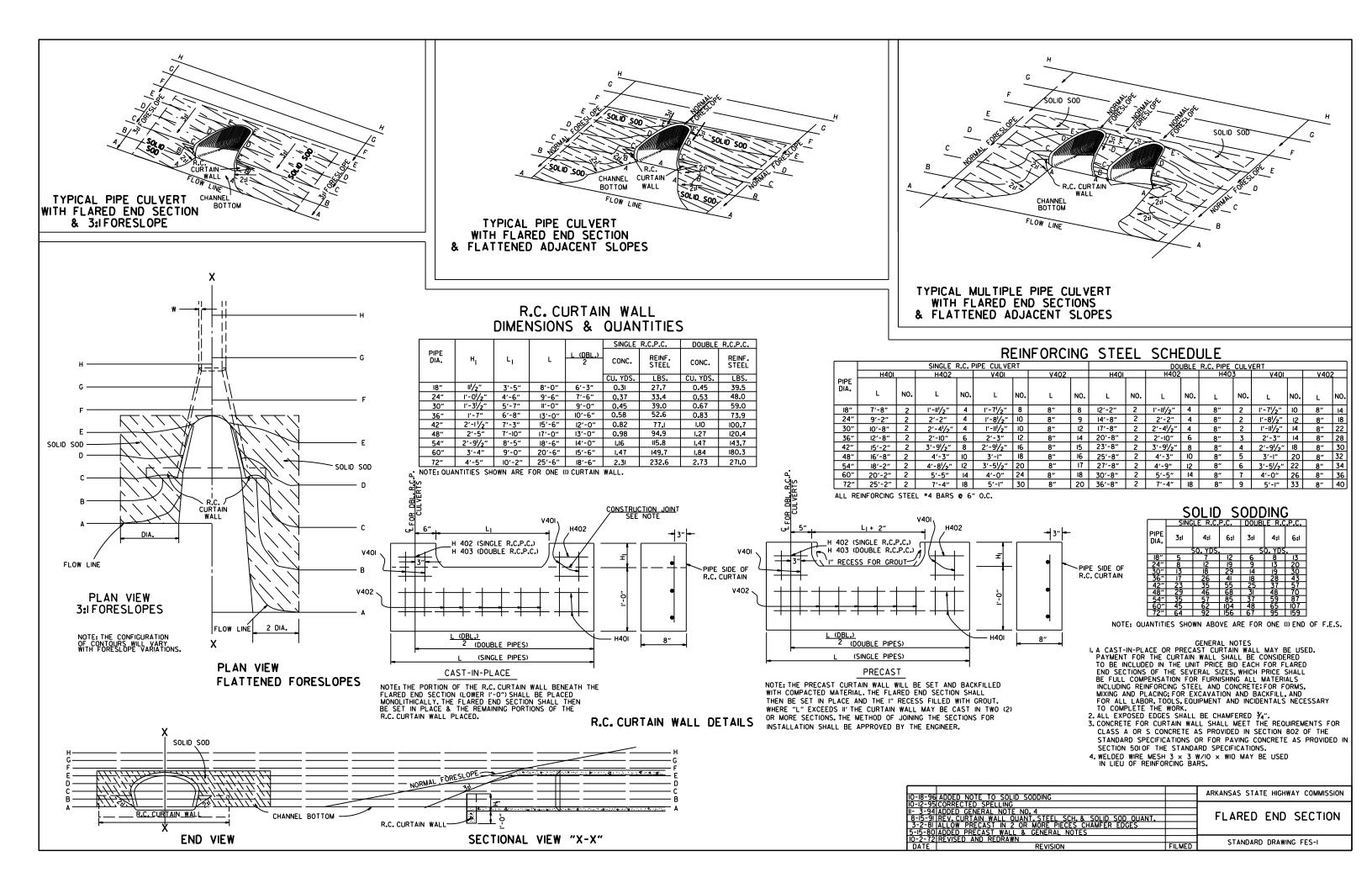
ARKANSAS STATE HIGHWAY COMMISSION

CONCRETE DITCH PAVING

STANDARD DRAWING CDP-1







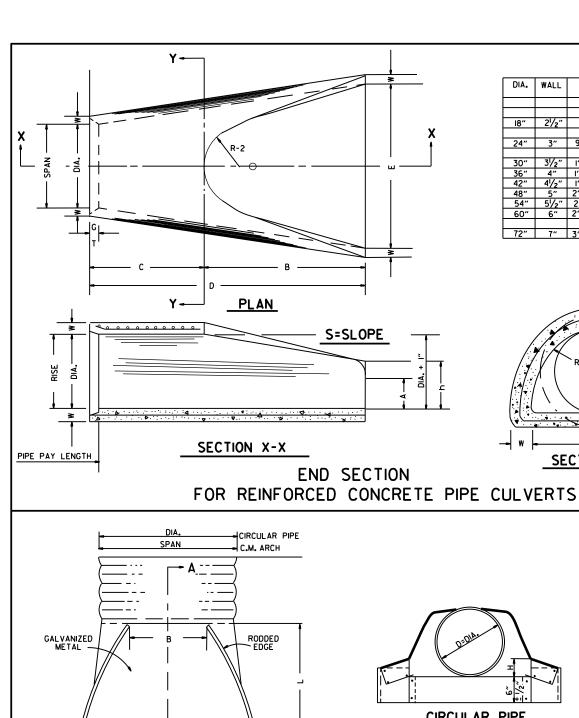
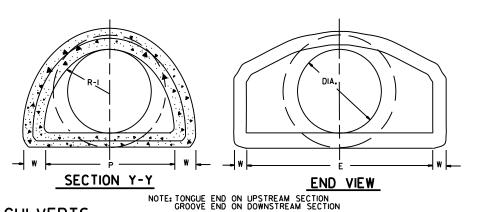


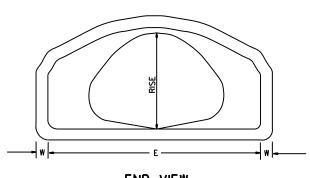
TABLE OF DIMENSIONS 6" 2'-10" 6'-6" 1'-10" 8'-4" 8'-0" 3:1 61" 72 2" 36 6" 24" 4" 9270 3'-5"



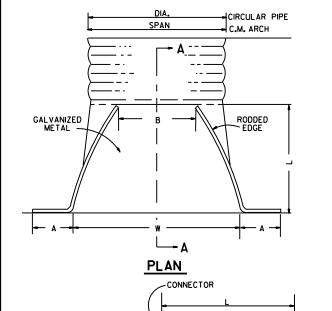
ARCH PIPE

EQUIV.	• SF	PAN	• R	ISE										
1 -2	AASHTO M 206	AHD NOMINAL	AASHTO M 206	AHD NOMINAL	w	A	В	С	D	E	Р	R2	G-T	s
		INCHES												
15	18	18	II	II	2"	4"	2'-0"	4'-0"	6′-0″	3′-0"	29"	12"	11/2"	21/2:1
18	22	22	131/2	14	21/2"	5"	2'-0"	4'-1"	6'-1"	3'-6"	321/8"	13"	21/2"	21/2:1
21	26	26	151/2	16	23/4"	7"	2'-3"	3'-10"	6'-1"	4'-0"	341/8"	14"	21/2"	21/2:1
24	281/2	29	18	18	3"	9"	2'-3"	3′-10″	6'-1"	5′-0″	36⅓ "	15"	21/2"	21/2:1
30	36 ¹ / ₄	36	221/2	23	31/2"	10"	3′-1"	3'-01/2"	6'-11/2"	6′-0″	47 ¹ 3/6 "	20"	3"	21/2:1
36	43¾	44	26%	27	4"	101/2"	4'-0"	2'-1/2"	6'-11/2"	6'-6"	54¾"	22"	31/2"	21/2:1
42	51/8	51	315/16	31	41/2"	111/2"	4'-7"	1-101/4"	6'-51/4"	7′-2″	591/2"	23"	3¾"	21/2:1
48	581/2	59	36	36	5"	1'-3"	5′-3″	2′-10¾"	8'-13/4"	7'-10"	70%"	24"	41/4"	21/2:1
54	65	65	40	40	51/2"	1'-7"	5'-3"	2'-11"	8'-2"	8′-6″	721/16"	24"	43/4"	21/4:1
60	73	73	45	45	6"	1'-10"	5′-6″	2′-8″	8'-2"	9′-0″	7713/16 "	24"	5"	21/4:1

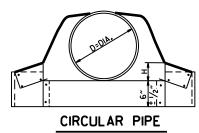
THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PER CENT FROM THE VALUES SPECIFIED BY AASHTO M 206.

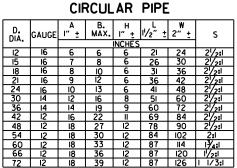


END VIEW CONCRETE ARCH PIPE



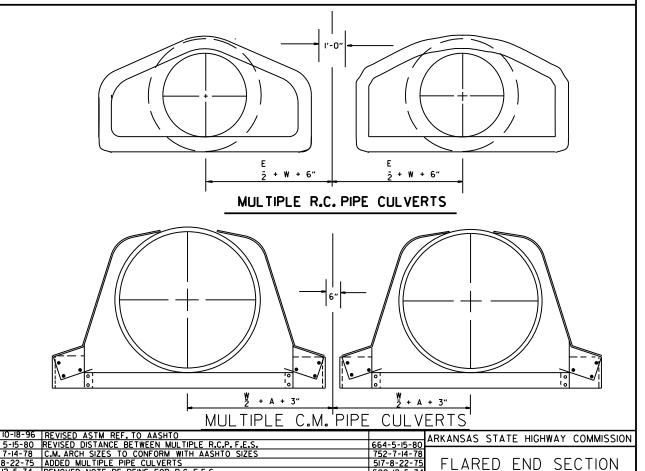
PIPE PAY LENGTH



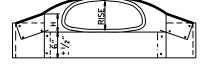


	15	16	工	7	8	6	26		30	21/		
	18	16		8	10	6	31		36	21/2	ادد	
	21	16		9	12	6	36	Ŀ	42	21/2	ادو	
	24	16		10	13	6	41	Ŀ	48	21/2	2:1	
	30	14	Ш	12	16	8	51		60	21/2	2:1	
	36	14	L	14	19	9	60		72	21/2		
	42	12	I	16	22	II	69		84	21/2	ادر	
	48	12		18	27	12	78		90	21/2	ائح	
	54	12		18	30	12	84	Π	02	2:1		
	60	12	\mathbf{I}	18	33	12	87		114	13/4	:	
	66	12		18	36	12	87	D	120	1/2:	:I	
	72	12	ı	18	39	12	87	\Box	26	1 1/3	3:1	
			C.	м.	ARC	н Р	IPE_					
s	SPAN	RISE	и И" <u>+</u>			l½″	± 2"	ţ	s	,	GA	UGE
				INICHE								

EQUIV.	SPAN	RISE	_			l	w 2″ <u>+</u>	s	GAUGE
				INCHE:	5				
15"	17	13	7	9	6	19	30	21/2:1	16
.81	21	15	7	ō	6	23	36	21/2:1	16
21"	24	18	8	12	6	28	42	21/2:1	16
24"	28	20	9	14	6	32	48	21/2:1	16
30"	35	24	10	16	6	39	60	21/2:1	14
36"	42	29	12	18	8	46	75	21/2:1	14
42"	49	33	13	21	9	53	85	21/2:1	12
48"	57	38	18	26	12	63	90	21/2:1	12
54"	64	43	18	30	12	70	102	21/4:1	12
60"	71	47	18	33	12	77	114	21/4:1	12



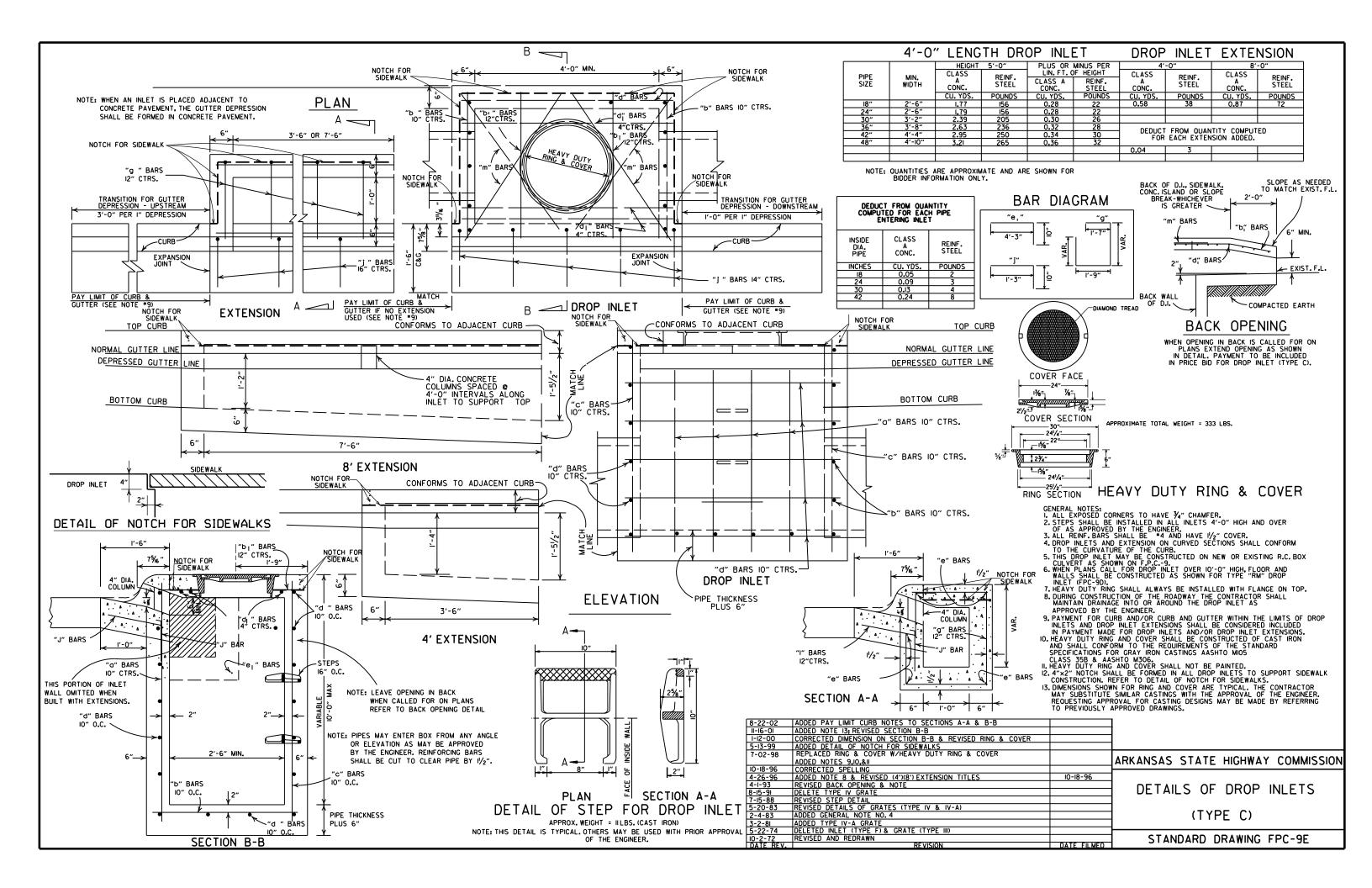
STANDARD DRAWING FES-2

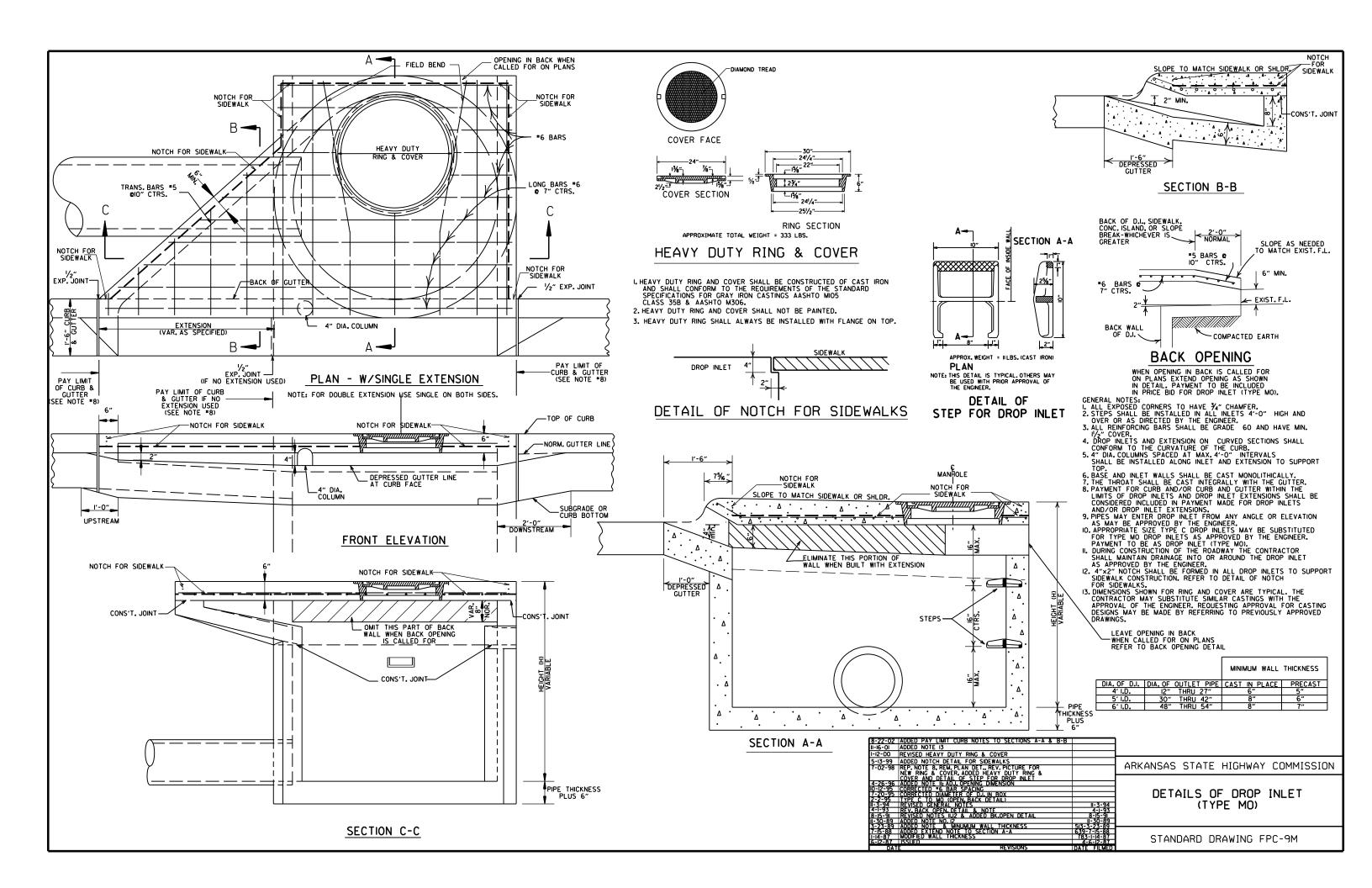


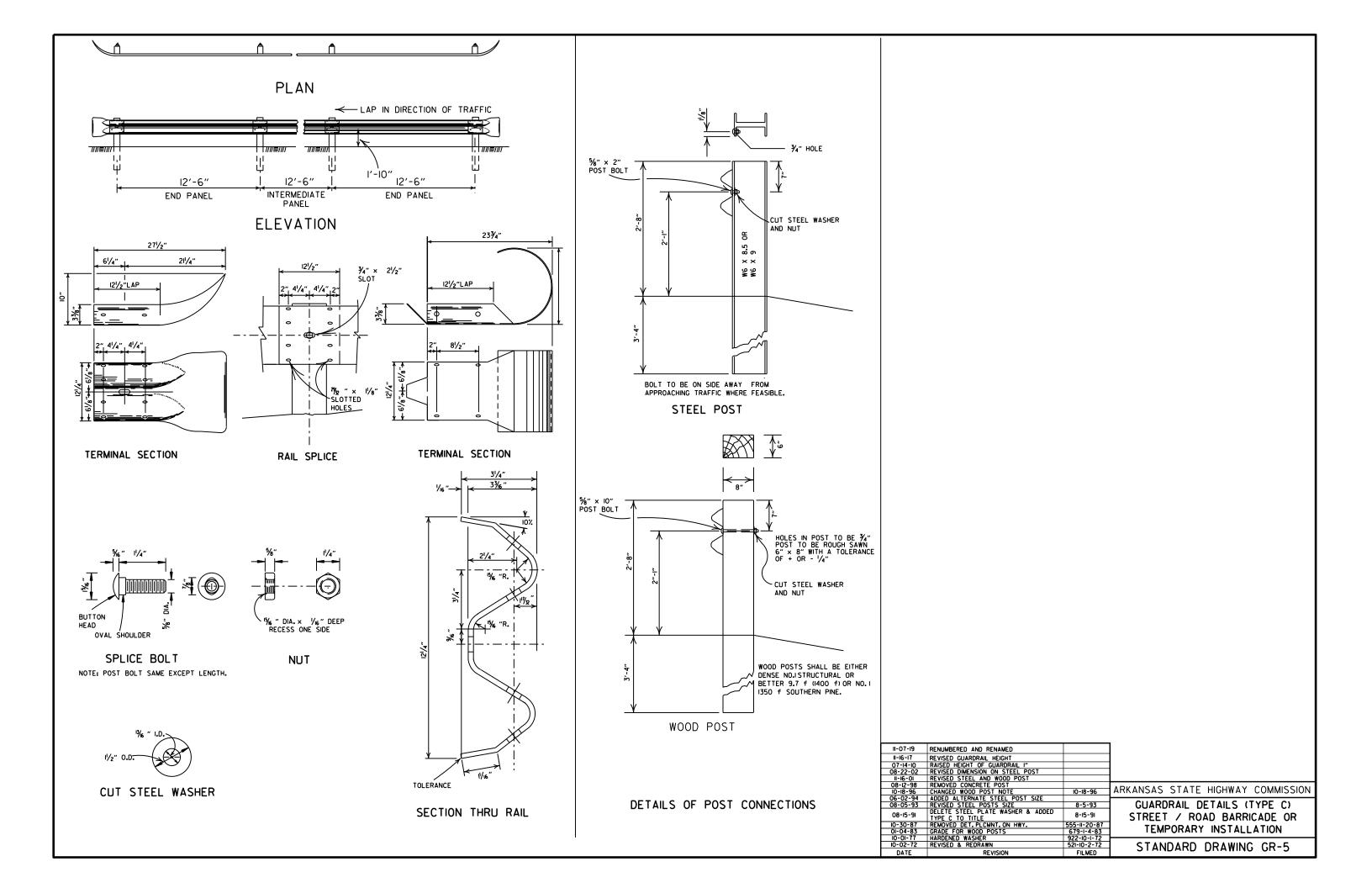
C.M. ARCH PIPE

SECTION A-A NOTE: ALTERNATE CONNECTIONS TO THE PIPE CULVERTS, IN ACCORDANCE WITH MANUFACTURER'S STANDARD PRACTICES, MAY BE MADE SUBJECT TO THE APPROVAL OF THE ENGINEER.

END SECTIONS FOR CORRUGATED METAL PIPE CULVERTS







REINFORCED CONCRETE ARCH PIPE DIMENSIONS

EQUIV.	SP	AN	RI	SE
DIA.	AASHTO M 206	ARDOT NOMINAL	AASHTO M 206	ARDOT NOMINAL
INCHES		INC	HES	
15 18 21 24 30 36 42 48 54 60 72 84 90 96 108 120 132	18 22 26 28½ 36¼ 43¾ 51½ 65 73 88 102 115 122 138 154 168¾	18 22 26 29 36 44 51 59 65 73 88 102 115 122 138 154 169	11 13½/2 15½/2 18 22½/2 26%/3 31%/6 36 40 45 54 62 77½/2 87½/6 106½/2	11 14 16 18 23 27 31 36 40 45 54 62 72 77 87 97

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE DIMENSIONS

Г	-11-	DIME	NOTONO		
	EQUIV.	AASHTO) М 207		
	DIA.	SPAN	RISE		
	INCHES	INC	HES		
	18	23	14		
	24	30	19		
	27	34	22		
	30	38	24		
	33	42	27		
	36	45	29		
	39	49	32		
	42	53	34		
	48	60	38		
	54	68	43		
	60	76	48		
	66	83	53		
	72	91	58		
	78	98	63		
	84	106	68		
	THE MEACHDED CDAM AND DE				

HE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN
± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE. 5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(I).

NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE

- LEGEND -

D₁ = NORMAL INSIDE DIAMETER OF PIPE D₀ = OUTSIDE DIAMETER OF PIPE H = FILL COVER HEIGHT OVER PIPE (FEET) MIN. = MINIMUM

= UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

- *SM-3 WILL NOT BE ALLOWED.
- ** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.

MINIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

	CLASS OF PIPE					
	CLASS	III	CLASS IV	CLASS V		
INSTALLATION TYPE	TYPE 1 OR 2	TYPE 3	ALL	ALL		
PIPE ID (IN.)		FEE	Т			
12-15	2	2 . 5	2	1		
18-24	2.5	3	2	1		
27-33	3	4	2	1		
36-42	3 . 5	5	2	1		
48	4.5	5 . 5	2	1		
54-60	5	7	2	1		
66-78	6	8	2	1		
84-108	7.5	8	2	1		

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MINIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS	OF PIPE
INSTALLATION TYPE	CLASS III	CLASS IV
	FE	EΤ
TYPE 2 OR TYPE 3	2.5	1.5

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

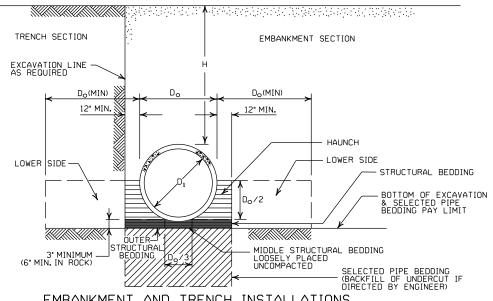
	CLASS OF PIPE						
INSTALLATION TYPE	CLASS III CLASS IV		CLASS V				
1176	FEET						
TYPE 1	21	32	50				
TYPE 2	16	25	39				
TYPE 3	12	20	30				

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MAXIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS	OF PIPE			
INSTALLATION	CLASS III	CLASS IV			
1175	FEET				
TYPE 2	13	21			
TYPE 3	10	16			

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH. IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND RECOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
- 3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

- I. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. ALL PIPE SHALL CONFORM TO SECTION 606.CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO MI70, R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING. HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SOUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
- 9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- IO. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

L				
ſ	2-27-14	REVISED GENERAL NOTE I.		
[12-15-11	REVISED FOR LRFD DESIGN SPECIFICATIONS		
	5-18-00	REVISED TYPE 3 BEDDING & ADDED NOTE		
	3-30-00	REVISED INSTALLATIONS		
	II-06-97	ISSUED		
	DATE	REVISION	DATE	FILMED

ARKANSAS STATE HIGHWAY COMMISSION CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING

STANDARD DRAWING PCC-1



CORRUGATED STEEL PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX.FILL	HEIGHT "	H" ABOVE	TOP OF PI	PE (FEET)
DIAMETER	PIPE TO TOP		METAL	THICKNESS	(INCHES)	
(INCHES)	OF GROUND ''H'' (FEET)	0.064	0.079	0.109	0.138	0.168
	2⅓ RIVET	INCH BY ED, WELDE	½ INCH D, OR HEL	CORRUGATI		
12 15 18 24 30 36 42 48	 	84 67 56 42 34	9I 73 6I 46 36 30 43	59 47 39 67 58	41 70 61	73 64
				BY 1 INC	H CORRUGA	TION
36 42 48 54 60 66 72 78 84 90 96 102 108 114	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	48 41 36 32 29 26 24	60 51 45 40 36 33 30 28 26 24 22	88 72 64 59 53 47 44 41 38 35 33 31 30 28 27	III 90 77 71 64 58 53 49 45 43 40 38 35 34	II8 IO2 85 79 71 659 54 51 45 44 42 37 35

CORRUGATED ALUMINUM PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX. FILL	HEIGHT '	'H'' ABOVE	TOP OF F	PIPE (FEET		
DIAMETER	PIPE TO TOP		METAL THICKNESS IN INCHES					
(INCHES)	OF GROUND "H" (FEET)	0.060	0.075	0.105	0.135	0.164		
		2 ² / ₃	INCH B		CORRUGA			
12 18 24 30 36 42 48 54 60 66	1 2 2 2 2,5 2 2 2 2 2 2 2	45 30 22	45 30 22 18 15	52 39 31 26 43 40 35	41 32 27 43 41 37 33	34 28 44 43 38 34 31 29		

CONSTRUCTION SEQUENCE

- 1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. COMPLETE STRUCTURAL BECKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE, WHITCHEVER IS 155
- NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4,5,6,0R 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

3 SM-3 WILL NOT BE ALLOWED.

EQUIVALENT METAL THICKNESSES AND GAUGES

METAL			
STI	EEL		GAUGE NUMBER
ZINC COATED	UNCOATED	ALUMINUM	
0.064	0.0598	0.060	16
0.079	0.0747	0.075	14
0.109	0.1046	0.105	12
0.138	0.1345	0.135	10
0.168	0.1644	0.164	8

ALUMINUM

2 3 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM

MAX. HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

TYPE 1

(1) MIN. HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

TYPE 1

2.25 2.5

CORRUGATED METAL PIPE ARCHES

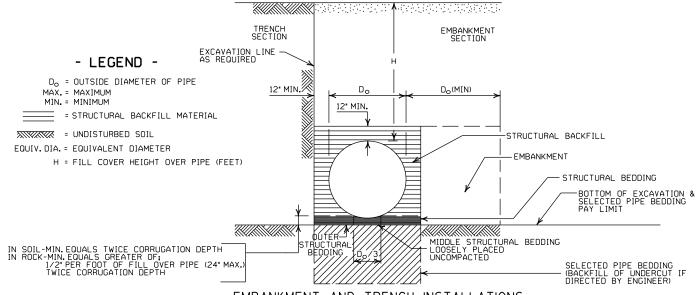
MINUMUM MIN. (1) MIN. HEIGHT OF

MAX. HEIGHT OF

MIN.

EQUIV.	DIMENSION	CORNER	THICKNESS	FILL,"		FILL,"	H'' (FT.)	THICKNESS
DIA.	SPAN X RISE		REQUIRED	INSTAL	LATION	INSTAL	LATION	REQUIRED
(INCHES)	(INCHES)	(INCHES)	INCHES	TYPI	Ε 1	TYPI	Ξ 1	INCHES
			RIV	2 ² / ₃ INCH E	BY ½ INCH (ED. OR HELIC	ORRUGATION	м	
15	17×13	3	0.064	2		I IS		0.060
18	21×15	3	0.064	2		İ		0.060
21	24×18	3	0.064	2.2		l is		0.060
24	28×20	3	0.064	2.		15	j	0.075
30	35×24	3	0.079	3		12		0.075
36	42×29	31/2	0.079	3		12		0.105
42	49×33	4	0.079	3		12		0.105
48	57×38	5	0.109	3		13		0.135
54	64×43	6 7	0.109	3		14		0.135
60 66	71×47 77×52	8	0.138 0.168	3		15		0.164
72	83×57	9	0.168] 3		15		
12	03831		3 INCH	RY 1 INCH	OR 5 INCH E	RY 1 INCH CO	ORRUGATION	1
			RIVE	TED, WELDE	D, OR HELIC	AL LOCK-SE	AM	
				INSTAL	LATION	INSTAL	LATION	1
				TYPE 2	TYPE 1	TYPE 2	TYPE 1	2
36	40×3I	5	0.079	3	2	12	15	
42	46×36	6	0.079	3	2	13	15	
48	53×4I	[[0.079	3	2	13	15	
54	60×46	8 9	0.079	3 3	2	13 13	15	
60 66	66×5I 73×55	12	0.079 0.079	3	2	15	15 15	
72	81x59	14	0.079	3	2	15	15	
78	87×63	14	0.079	3	2	15	15	
84	95×67	i6	0.109	3	2	15	15	
90	103×71	16	0.109	ž	l 2	15	15	
96	112×75	18	0.109	3	2	15	i5	
102	117×79	18	0.109	3	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	15	15	
108	128×83	18	0.138	3	2	15	15]

- ① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.
- ② WHERE THE STANDARD 2 2/3'x ½ CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3*x 1*OR 5*x 1*CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS GAUGED FOR A FILL HEIGHT CONDITION EQUAL TO OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. INSTALLATION TYPE FOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
- 3. INSTALALTION TYPE ISHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 23" X 1/2"
- 4. INSTALLATION TYPE IOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X I" OR 5" X I" CORRUGATION.

GENERAL NOTES

- I. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL),
 BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE.

 IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

2-27-14 REVISED GENERAL NOTE I. 12-15-11 REVISED FOR LRFD DESIGN SPECT 3-30-00 REVISED INSTALLATIONS II-06-97 ISSUED REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION METAL PIPE CULVERT

FILL HEIGHTS & BEDDING

STANDARD DRAWING PCM-1



INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	*SELECTED MATERIALS (CLASS SM-I, SM-2 OR SM-4)

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF HOPE PIPE.

MULTIPLE INSTALLATION OF HIGH DENSITY POLYETHYLENE PIPES

BETWEEN PIPES
1'-6"
2'-0"
2'-6"
3'-0"
3'-6"
4'-0"

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"	
18"	4'-6"	4′-6″	
24"	5′-0″	6′-0″	
30"	5′-6″	7′-6″	
36"	6′-0″	9'-0"	
42"	7′-0″	10'-6"	
48"	8'-0"	12'-0"	

(DNOTE:
18" MIN. (18" - 30" DIAMETERS)
24" MIN. (36" - 48" DIAMETERS)
MINIMUM COVER VALUES, "H"
SHALL INCLUDE A MINIMUM 12"
OF PAVEMENT AND/OR BASE.

MINIMUM COVER FOR CONSTRUCTION LOADS

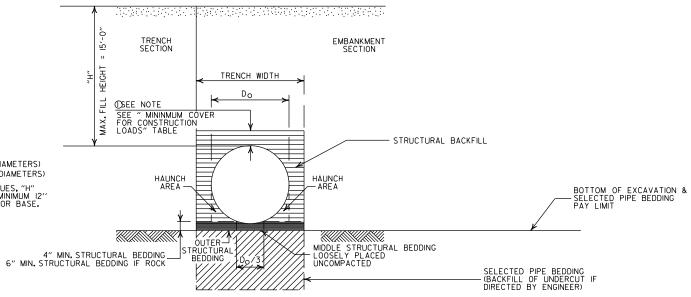
	Ø MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS				
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	110.0-175.0 (KIPS)	
36" OR LESS	2'-0"	2'-6"	3'-0"	3'-0"	
42" OR GREATER	3'-0"	3′-0″	3′-6″	4'-0"	

☑ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE

MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M294, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE OUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE, IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. HIGH DENSITY POLYETHYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR HDPE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

H = FILL HEIGHT (FT.)
B = OUTSIDE DIAMETER OF PIPE
MAX. = MAXIMUM
MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-I4 REVISED GENERAL NOTE I. 12-I5-II REVISED GENERAL NOTES & MINIMUM COVER NOTE II-17-I0 ISSUED DATE REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT
(HIGH DENSITY POLYETHYLENE)

STANDARD DRAWING PCP-1

INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	<pre>*SELECTED MATERIALS (CLASS SM-I, SM-2, OR SM-4)</pre>

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INNCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF PVC PIPE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)				
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0'			
18"	4'-6"	4'-6"			
24"	5′-0″	6′-0″			
30"	5′-6″	7′-6″			
36"	6'-0"	9'-0"			

MULTIPLE INSTALLATION OF PVC PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2′-6″
36"	3'-0"
	J -0

MAXIMUM FILL HEIGHT BASED ON STRUCTURAL BACKFILL

PIPE DIAMETER	"H"
18"	45'-0"
24"	45'-0"
30"	40′-0″
36"	40'-0"

① NOTE: 12" MIN. (18" - 36" DIAMETERS) MINIMUM COVER VALUE, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

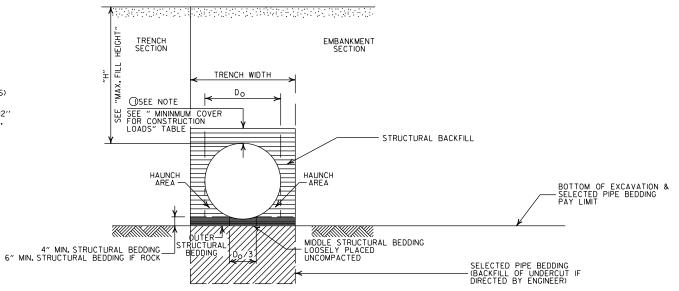
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	110.0-175.0 (KIPS)
18" THRU 36"	2'-0"	2'-6"	3'-0"	3'-0"

 ${}^{\textcircled{O}}$ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I.PIPE SHALL CONFORM TO ASTM F949, CELL CLASS 12454. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. PVC PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR PVC PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.)

 D_{O} = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM

MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I. | 12-15-|| REV GENERAL NOTES & MINIMUM COVER NOTE; DELETED SM3 MATERIAL | 11-17-10 | ISSUED REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (PVC F949)

STANDARD DRAWING PCP-2



INSTALLATION TYPE	**MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE I	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	*SELECTED MATERIALS (CLASS SM-1, SM-2 OR SM-4) OR TYPE INSTALLATION MATERIAL

*SM3 WILL NOT BE ALLOWED.

** STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF POLYPROPYLENE PIPE.

MULTIPLE INSTALLATION OF POLYPROPYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2'-6"
36"	3′-0″
42"	3′-6″
48"	4′-0″
60"	5′-0″

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"	
18"	4'-6"	4'-6"	
24"	5′-0″	6′-0″	
30"	5′-6″	7′-6″	
36"	6′-0″	9'-0"	
42"	7′-0″	10'-6"	
48"	8'-0"	12'-0"	
60"	10'-0"	15'-0"	

12" MIN. (18" - 42" DIAMETERS) 24" MIN. (60" DIAMETER) MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12'' OF PAVEMENT AND/OR BASE.

MINIMUM COVER FOR CONSTRUCTION LOADS

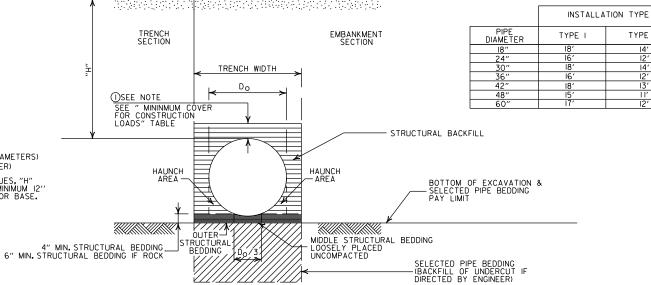
	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	110.0-150.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3'-0"	3'-0"
42" OR GREATER	3'-0"	3'-0"	3′-6″	4'-0"

②MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M330, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SIXTH EDITION (2012) WITH 2013 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. POLYPROPYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR POLYPROPYLENE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN SECTION 26.4.2.4 AND 30.4.2 OF THE AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS 3RD EDITION (2010) WITH 2012 INTERIMS. JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"



EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.) Do = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM MIN. = MINIMUM

MAXIMUM HEIGHT OF FILL "H"

TYPE 2

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

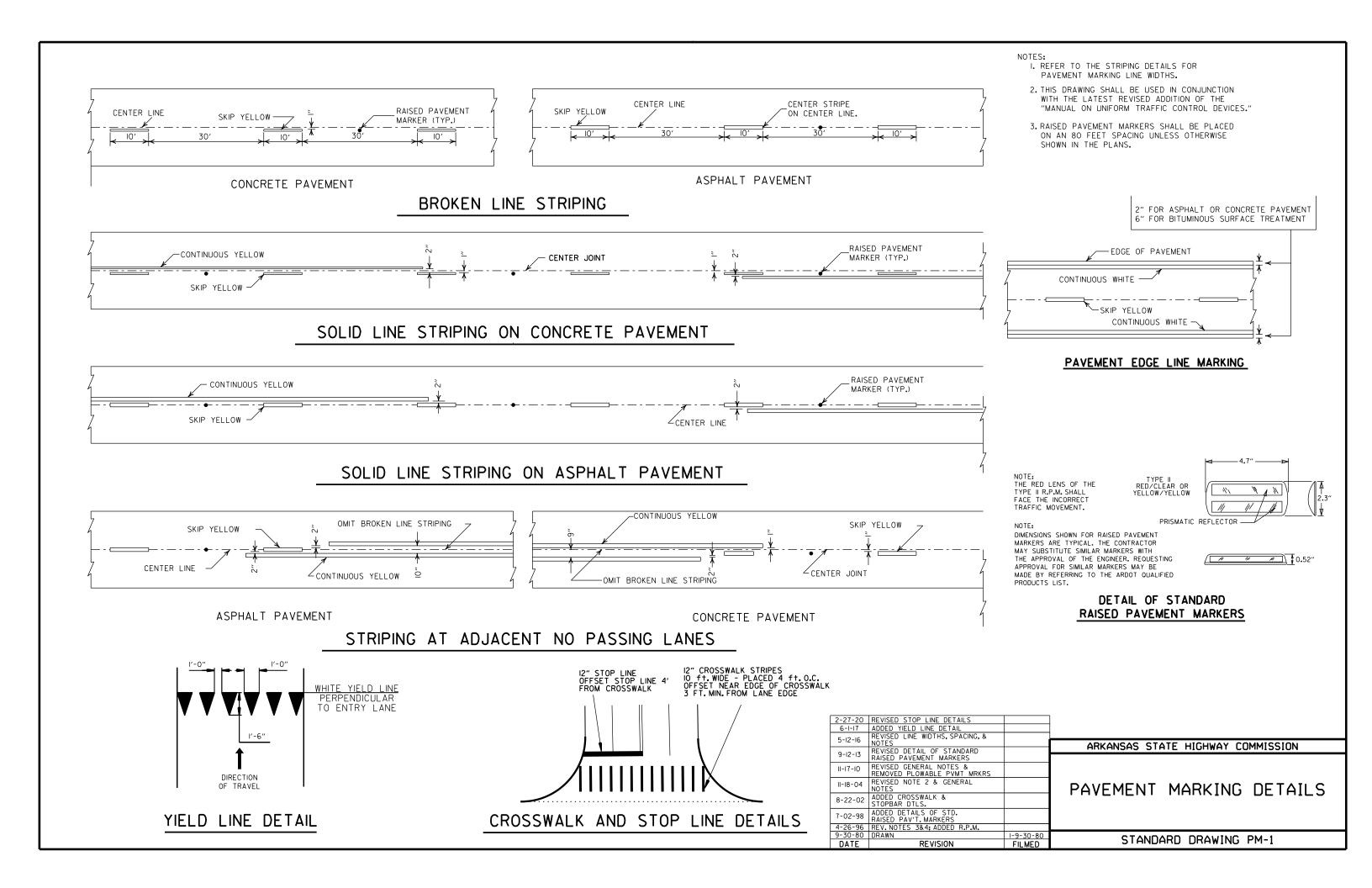
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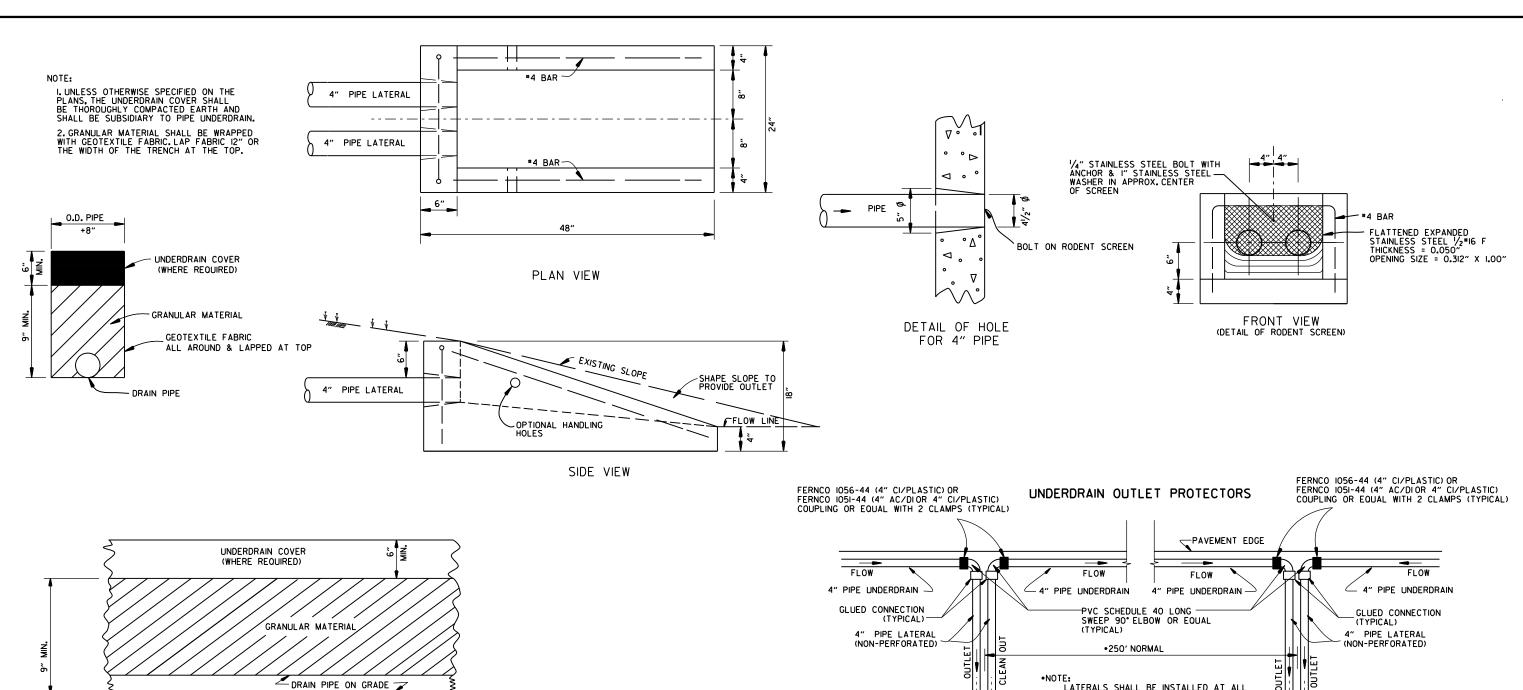
ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (POLYPROPYLENE)

STANDARD DRAWING PCP-3







DETAILS OF PIPE UNDERDRAIN

NOTES FOR PIPE UNDERDRAINS

I. GEOTEXTILE FABRIC SHALL MEET THE REQUIREMENTS OF SECTION 625 FOR TYPE I. PAYMENT FOR GEOTEXTILE FABRIC AND GRANULAR FILTER MATERIAL SHALL BE INCLUDED IN THE PRICE BID PER LIN. FT. FOR "4" PIPE UNDERDRAINS" IN ACCORDANCE WITH SECTION 611 OF THE STANDARD SPECIFICATIONS.

2.4" NON-PERFORATED SCHEDULE 40 PVC PIPE LATERALS WITH OUTLET PROTECTORS SHALL BE INSTALLED AS SHOWN HEREON, LATERALS WILL BE MEASURED AND PAID FOR BY THE UNIT IN ACCORDANCE WITH SECTION GII OF THE STANDARD SPECIFICATIONS.

3. EXISTING 4" PIPE UNDERDRAINS MAY BE CONNECTED TO PROPOSED DROP INLETS OR EXTENDED WHERE DIRECTED BY THE ENGINEER. PAYMENT FOR CONNECTING TO DROP INLETS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR "4" PIPE UNDERDRAINS."

4. THE LOCATION OF ALL LATERALS SHALL BE MARKED WITH 4" X 12" PERMANENT PAVEMENT MARKING TAPE (TYPE III WHITE) AT THE OUTSIDE EDGE OF THE SHOULDER, PLACED TRANSVERSE TO TRAFFIC. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

5. PAYMENT FOR THE RODENT SCREEN SHALL BE INCLUDED IN THE PRICE BID PER EACH FOR "UNDERDRAIN OUTLET PROTECTORS."

6. ANY EXISTING UNDERDRAINS THAT INTERFERE WITH INSTALLATION OF THE NEW UNDERDRAIN SYSTEM SHALL BE REMOVED AND DISPOSED OF AS DIRECTED BY THE ENGINEER. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS. EXISTING UNDERDRAIN OUTLET PROTECTORS SHALL BE REMOVED UNDER THE ITEM "REMOVAL AND DISPOSAL OF UNDERDRAIN OUTLET PROTECTORS."

7. AT LOCATIONS WHERE A SINGLE LATERAL IS USED THE CONTRACTOR SHALL HAVE THE FOLLOWING OPTIONS: I. INSTALL OUTLET PROTECTOR AS SHOWN ON STANDARD DRAWING PU-LAND GROUT THE UNUSED HOLE OR 2. INSTALL AN OUTLET PROTECTOR WITH A SINGLE HOLE.

FERNCO 1051-44 (4" AC/DIOR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)	DINDERDRAIN OUTLET FROTECTORS	COUPLING OR EQUAL WITH 2 CLAMPS (TYPIC
FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (TYPICAL) 4" PIPE LATERAL (NON-PERFORATED) ON GRADIENT	PAVEMENT EDGE FLOW FLO	FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (TYPICAL) 4" PIPE LATERAL (NON-PERFORATED) AT SAGS

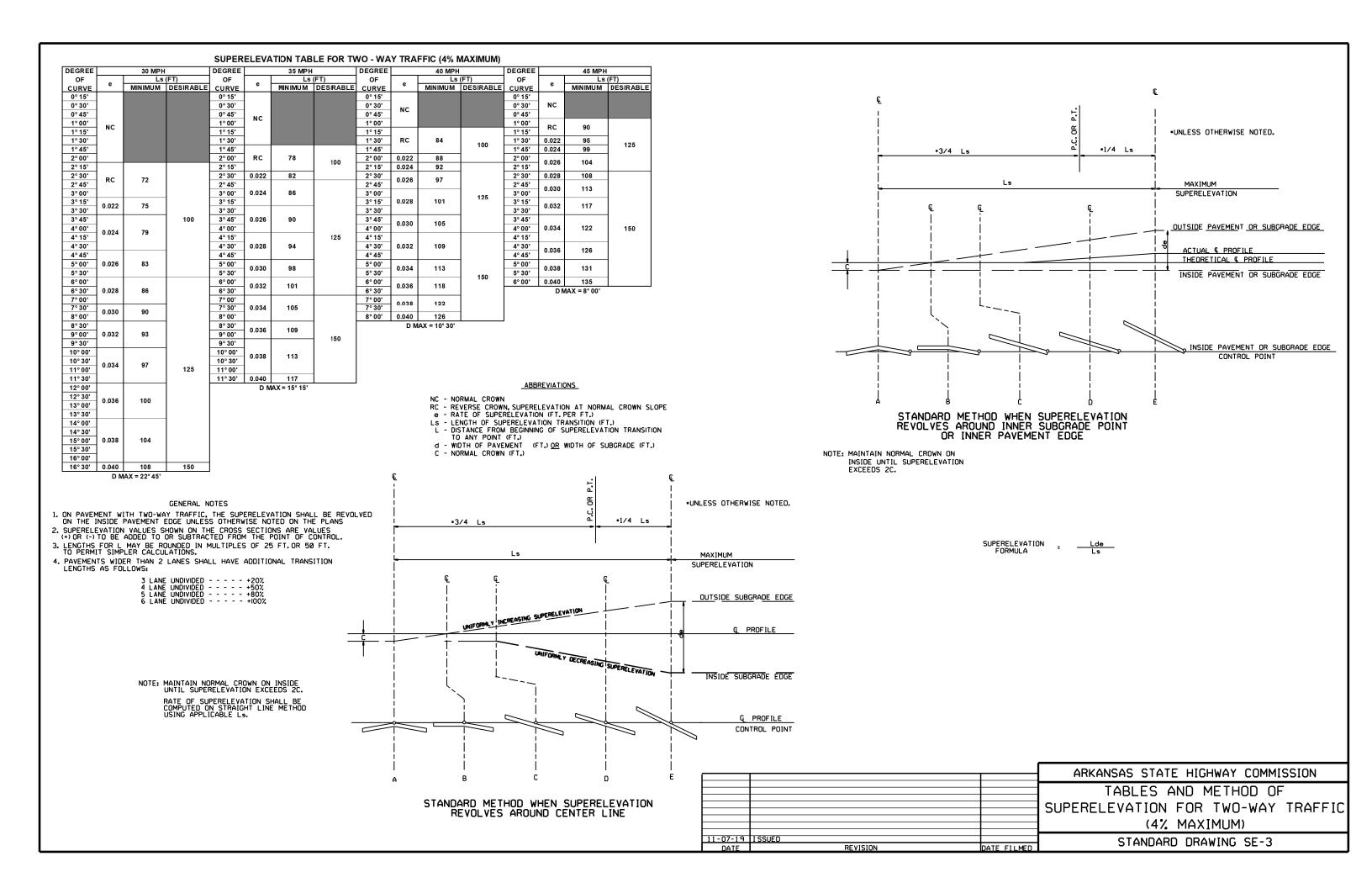
DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.

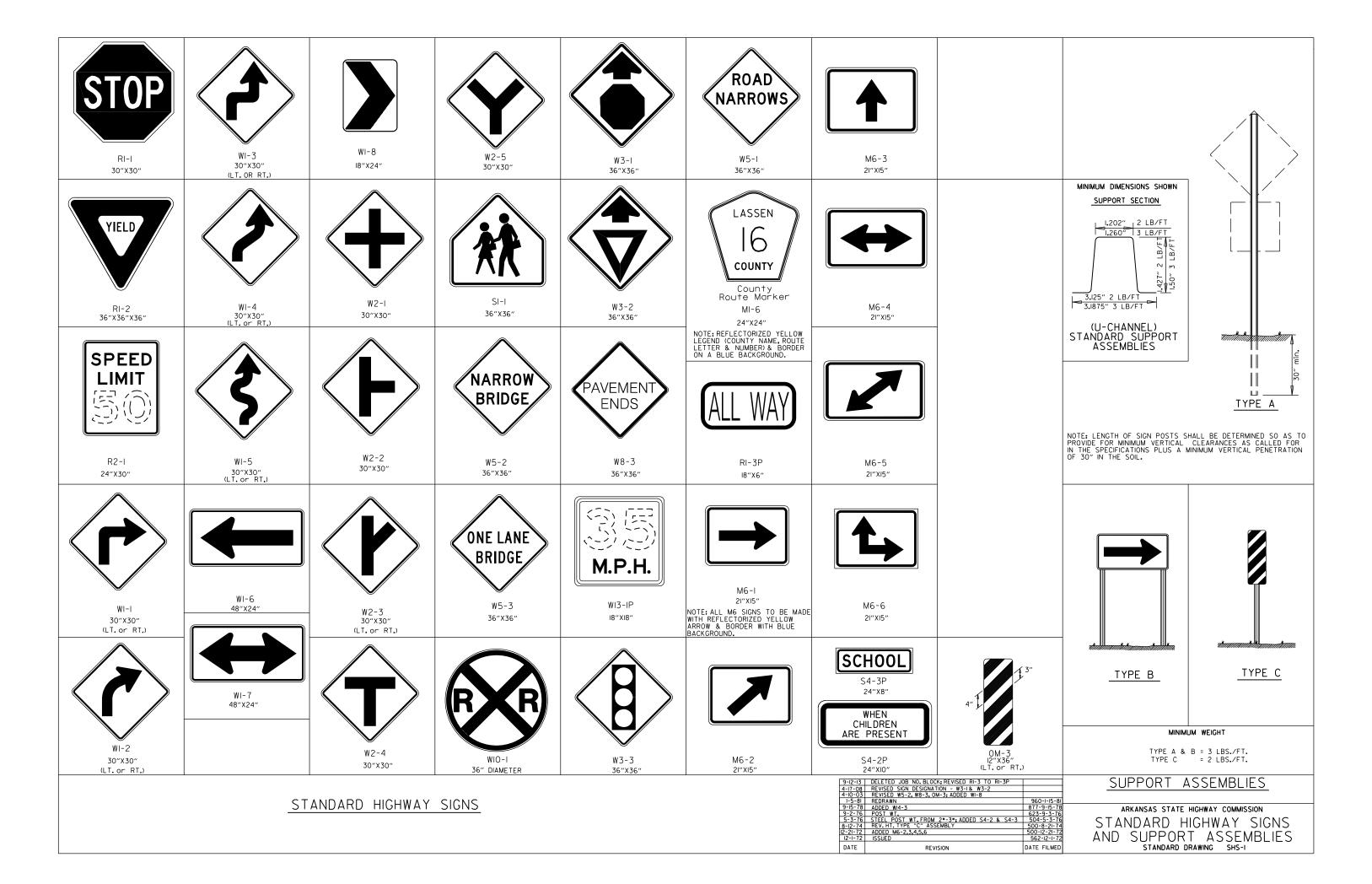
12-8-16	ADDED NOTES FOR PIPE UNDERDRAINS, REVISED RODENT SCREEN DETAIL AND NOTES, REMOVED NOTE IFOR GRANULAR MATERIAL, ADDED NOTE FOR GEOTEXTILE FABRIC		
4-10-03	REVISED NOTE 3		l
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS		l
11-18-98	REVISED NOTE		l
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC		l
4-26-96	ADDED LATERAL NOTE; 51/2" TO 5"		l
II-22-95	REVISED LATERALS		
7-20-95	REVISED LATERALS & ADDED NOTE		
II- 3-94	REVISED FOR DUAL LATERALS	II- 3-94	l
10- 1-92	SUBSTITUTED GEOTEXTILE	10- 1-92	
8-15-91	ADDED POLYEDTHYLENE PIPE	8-15-91	ł
II- 8-90	DELETED ALTERNATE NOTE	II- 8-90	ł
I-25-90	ADDED 4" SNAP ADAPTER	I-25-90	
11-30-89	DEL. (SUBGRADE); ADDED (WHERE REQUIRED)	II-30-89	
7-15-88	ISSUED P.L.M.	647-7-15-88	
DATE	REVISION	DATE FILMED	

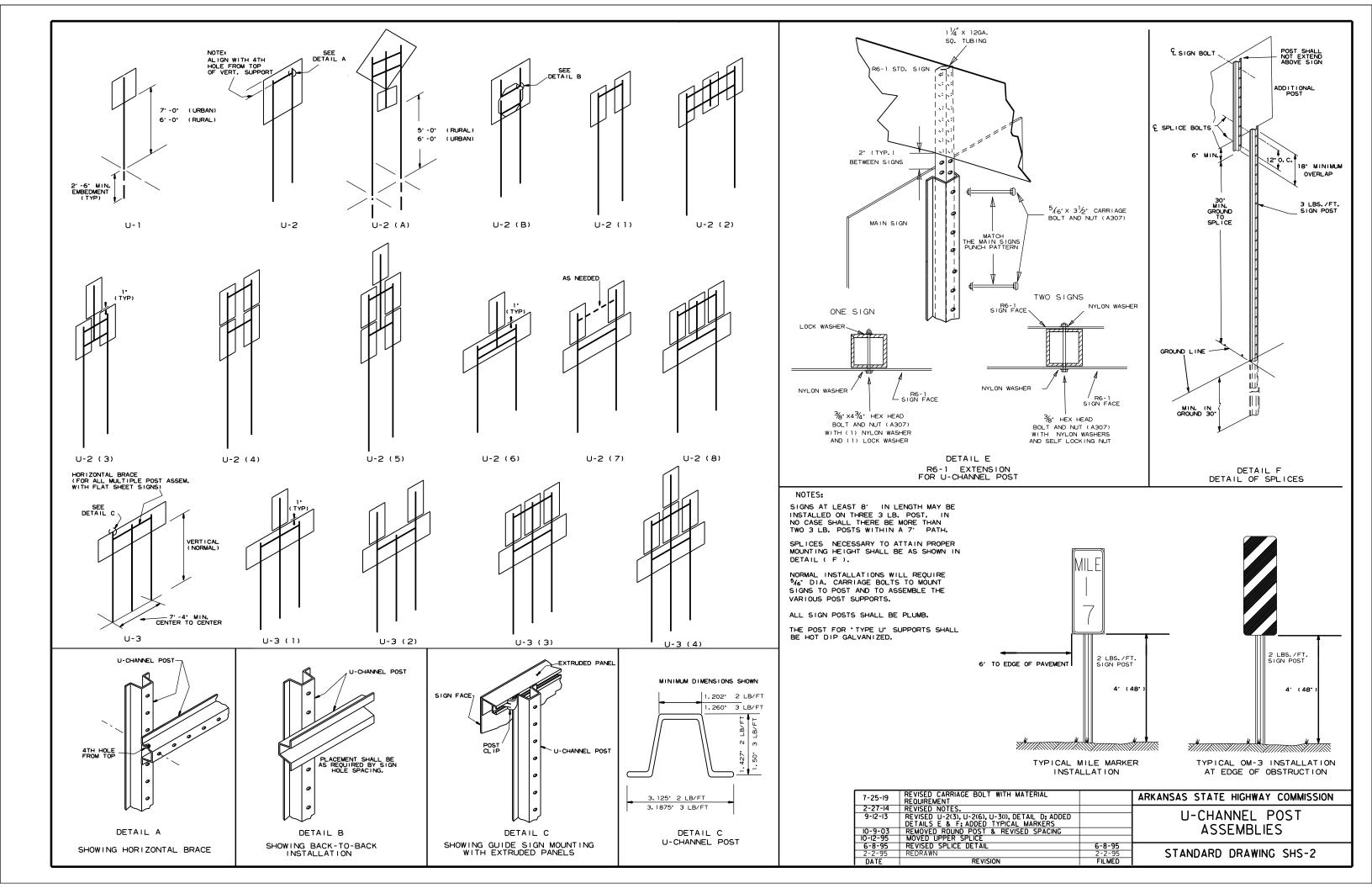
ARKANSAS STATE HIGHWAY COMMISSION

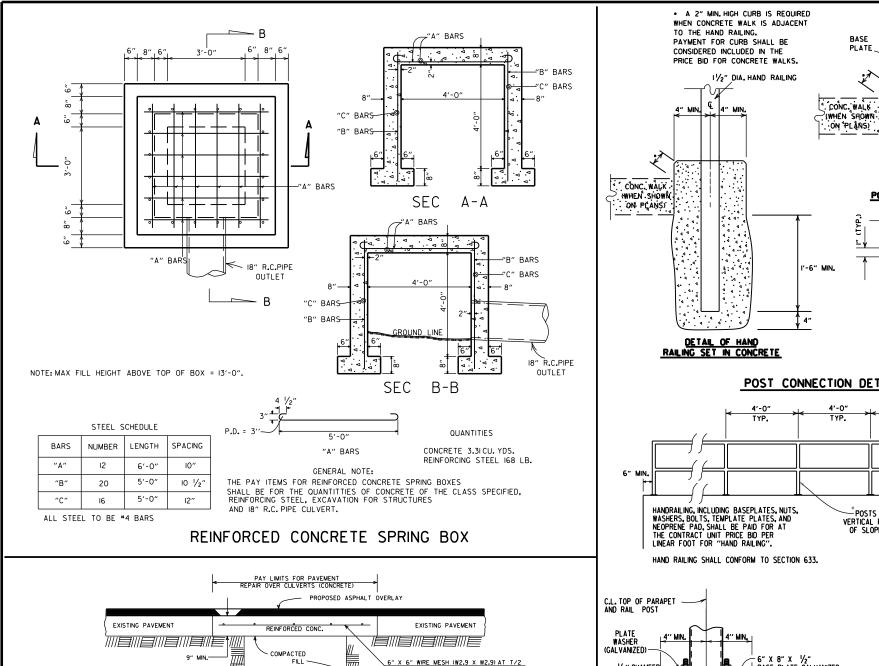
DETAILS OF PIPE UNDERDRAIN

STANDARD DRAWING PU-I









D+2C

PAVEMENT REPAIR OVER CULVERTS (CONCRETE)

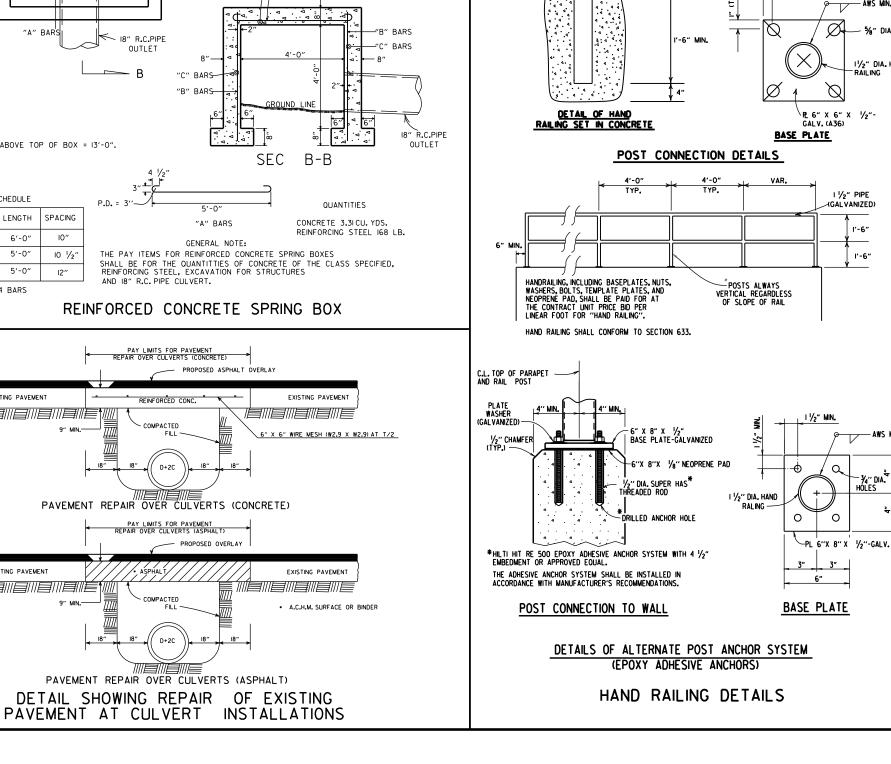
EXISTING PAVEMENT

````**`** 

PAY LIMITS FOR PAVEMENT
REPAIR OVER CULVERTS (ASPHALT)

PAVEMENT REPAIR OVER CULVERTS (ASPHALT)

- PROPOSED OVERLAY



€ 4" MIN.

POST CONNECTION TO WALL

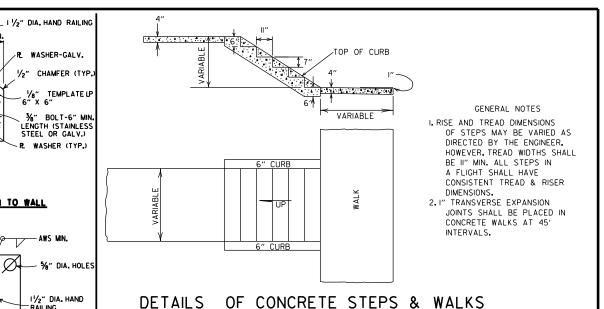
— AWS MIN.

1'-6"

- AWS MIN.

HÓLES

4" MIN



10-25-18 PAVEMENT AT CULVERT INSTALLATIONS 9-12-13 REVISED REINFORCED CONCRETE SPRING BOX 7-26-12 REMOVED RETAINING WALL DETAILS & REVISED HAND RAILING DETAILS
4-17-08 REV.JOINT & FOOTING STEP DETAILS
II-29-07 REVISED RETAINING WALL DRAINAGE 5-25-06 REVISED PVMT REPAIR OVER CULVERTS (CONC); REVISED REINFORCED CONC SPRING BOX REVISED PIPE RAILING DETAILS TO HAND RAILING DETAILS 4-IO-O3 REVISED RETAINING WALL DR 8-22-O2 ADDED HAND RAILING DETAIL REVISED PVMT REPAIR OVER CULVERTS (CONC); CORRECTED SPELLING IN GENERAL NOTES
ADDED GENERAL NOTES TO II-I8-98 ADDED GENERAL NOTES TO
CONCRETE STEPS & WALKS
7-02-98 ENLARGED PIPE
4-03-97 ADDED NOTE TO STEEL BAR SCHED.
IO-I8-96 CORRECTED SPELLING
4-26-96 ADD WEEP HOLE;REV. JOINT SPACING IN RET. WALL
6-2-94 CHANGED CONST. TO CONTRACTION JOINT
IO-I-92 CHANGED MESH FABRIC TO WIRE MESH
B-15-91 DELETED HOWL MODIFICATION DETAIL
II-8-90 DELETED COLD MIX FROM CULV'T. REPAIR
II-30-89 REV. RETAINING WALL STEEL SCHEDULE
II-17-88 V. BARS BEHIND ARROW
7-I5-88 REV. PAVEMENT REPAIR
ADDED HDWL. MODS, DEL, PIPE UNDERDRAINS 10-I-92 8-I5-9I II-8-90 II-30-89 665-II-I7-88 649-7-I5-88 ADDED HDWL. MODS, DEL. PIPE UNDERDRAINS

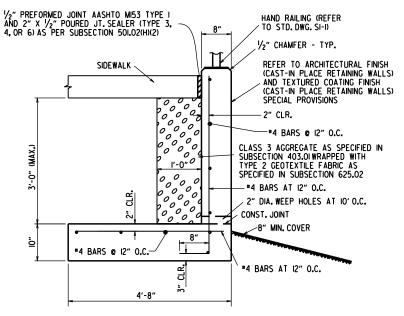
II-I-84 REV. TRENCH FOR PIPE UNDERDRAIN 510-11-1-84 I-4-83 ELIMINATED CONC. CLASS & ADDED CHAMFER NOTE 682-1-4-83 3-2-81 SPELLING OF "UNDERDRAIN 72I-3-2-8I 674-4-20-79 9I9-2-2-76 568-4-I0-75-853 3-2-81 SPELLING OF UNDERDRAIN 4-20-79 REV. UNDERDRAIN DET& PAVEMENT REPAIR 2-2-76 I2"MIN. GRAN. MAT'L. OVER PIPE 4-10-75 REM. SPECS. FOR GRAN. MAT'L. 5-22-74 GRANULAR MAT'L. TO BE SB-3 I0-2-72 REVISED AND REDRAWN DATE DATE FILMED

REVISED DETAIL SHOWING REPAIR OF EXISTING

ARKANSAS STATE HIGHWAY COMMISSION

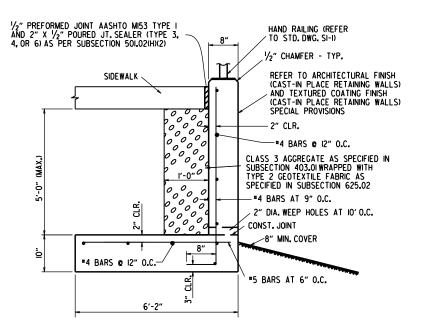
DETAILS OF SPECIAL ITEMS

STANDARD DRAWING SI - I



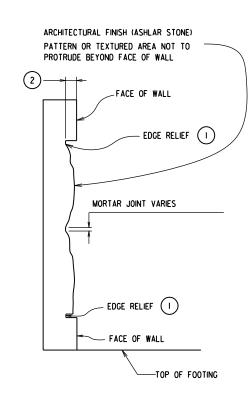
## CONCRETE WALK (TYPE SPECIAL) DETAIL MAX HEIGHT 3'-0"

N.T.S.



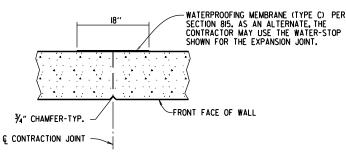
## CONCRETE WALK (TYPE SPECIAL) DETAIL MAX HEIGHT 5'-0"

N.T.S.



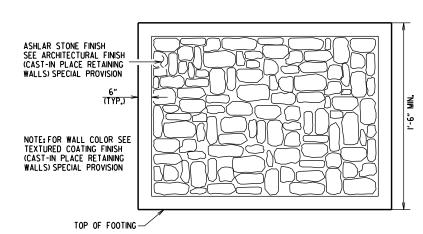
## ARCHITECTURAL FINISH DETAILS N.T.S.

- 1 PROVIDE EDGE RELIEF AROUND PERIMETER OF TEXTURE. EDGE RELIEF DIMENSIONS SHALL MATCH MANUFACTURERS EDGE DISTANCE.
- 2 DEPTH OF ASHLAR STONE PATTERN APPROX. 1%". SEE SP "ARCHITECTURAL FINISH (CAST-IN PLACE RETAINING WALLS)".

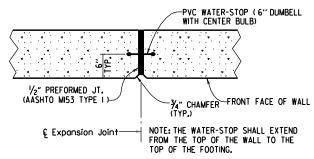


#### TYPICAL CONTRACTION JOINT DETAIL

NOTE: 20'-0" MAX, SPACING BETWEEN CONTRACTION JOINTS, HORIZONTAL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONTRACTION JOINTS.



ASHLAR STONE FINISH DETAIL



#### TYPICAL EXPANSION JOINT DETAIL

N.T.S.

NOTE: 60'-0" MAX. SPACING BETWEEN EXPANSION JOINTS. HORIZONTAL REINFORCING SHALL STOP 2" FROM EXPANSION JOINT.

#### NOTES:

WALL PATTERN SHALL BE APPLIED TO THE EXPOSED SURFACES OF WALL IN ACCORDANCE WITH SP "ARCHITECTURAL FINISH (CAST-IN PLACE RETAINING WALLS)" AND AS SHOWN IN THE PLANS, CARE SHALL BE TAKEN WITH FORM LINER HANDLING AND INSTALLATION TO ENSURE AESTHETIC OUALITY OF THE WALL TEXTURING IS MAINTAINED. WHERE FORM LINER PANELS REQUIRE MODIFICATION TO CONFORM TO THE LOCATION, DIMENSIONS AND LINES SHOWN IN THE PLANS, THE CONTRACTOR SHALL PROVIDE EDGE RELIEF MATCHING THAT OF THE UNALTERED FORM LINER. PAYMENT FOR WALL TEXTURING SHALL BE IN ACCORDANCE WITH SP "ARCHITECTURAL FINISH (CAST-IN PLACE RETAINING WALLS)".

NO ADJUSTMENTS WILL BE MADE IN CONCRETE VOLUME DUE TO THE USE OF "ARCHITECTURAL FINISM", CLASS "S" CONCRETE SHALL BE MEASURED IN ACCORDANCE WITH SUBSECTION 802.24(A),CARE SHALL BE TAKEN IN PLACING CONCRETE TO AVOID SEGREGATION AND TO ELIMINATE FLOW LINES.

CLASS 3 TEXTURED COATING FINISH SHALL BE APPLIED TO WALL SURFACES AS SPECIFIED IN SP "TEXTURED COATING FINISH (CAST-IN PLACE RETAINING WALLS)" AND IN ACCORDANCE WITH SUBSECTION 802.19(B)(3).

#### GENERAL NOTES

CONSTRUCTION SPECIFICATIONS: ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION) WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.

DESIGN SPECIFICATIONS: AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SIXTH EDITION (2012).

LIVE LOAD: LIVE LOAD SURCHARGE IS NOT INCLUDED IN THE DESIGN OF THESE WALLS, VEHICULAR LIVE LOAD SHALL NOT BE ALLOWED WITHIN A DISTANCE EQUAL TO ONE-HALF THE HEIGHT OF THE WALL.

CONCRETE: CONCRETE SHALL BE POURED IN THE DRY AND ALL EXPOSED CORNERS TO BE CHAMFERED 1/5". ALL CONCRETE SHALL BE CLASS S WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH F'C = 3,500 PSI, A CLASS 2 SURFACE FINISH SHALL BE USED ALL SURFACES OF THE CONCRETE UNLESS OTHERWISE NOTED, REFER TO ARCHITECTURAL FINISH (CAST-IN PLACE RETAINING WALLS) AND TEXTURED COATING FINISH (CAST-IN PLACE RETAINING WALLS) SPECIAL PROVISIONS.

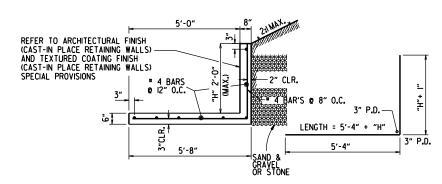
REINFORCING STEEL: ALL REINFORCING STEEL SHALL CONFORM TO AASHTO M3I ORM53, GRADE 60.

FOUNDATIONS FOR FOOTINGS SHALL BE PREPARED IN ACCORDANCE WITH SUBSECTION 801.04. BACKFILL FOR RETAINING WALLS SHALL BE IN ACCORDANCE WITH SUBSECTION 801.08.

WATERPROOF MEMBRANE (TYPE C), WATERSTOPS, PREFORMED JOINTS, PREFORMED JOINT FILLER, WEEP HOLES, CLASS 3 AGGREGATE, REINF, STEEL, CONCRETE, & GEOTEXTILE FABRIC SHALL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED SUBSIDIARY TO CONCRETE WALKS (TYPE SPECIAL).

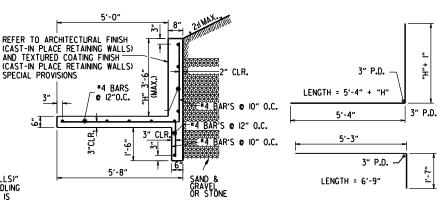
JOINTS IN THE WALL SHALL MATCH TYPE AND SPACING OF THE JOINTS IN THE WALK.

THESE DETAILS ARE NOT INTENDED FOR USE ALONG STREAMS OR DITCHES WITHOUT CONSIDERATION FOR SCOUR.



## CONCRETE WALK (TYPE SPECIAL) DETAILS MAX HEIGHT 2'-0"

N.T.S.



## CONCRETE WALK (TYPE SPECIAL) DETAILS MAX HEIGHT 3'-6"

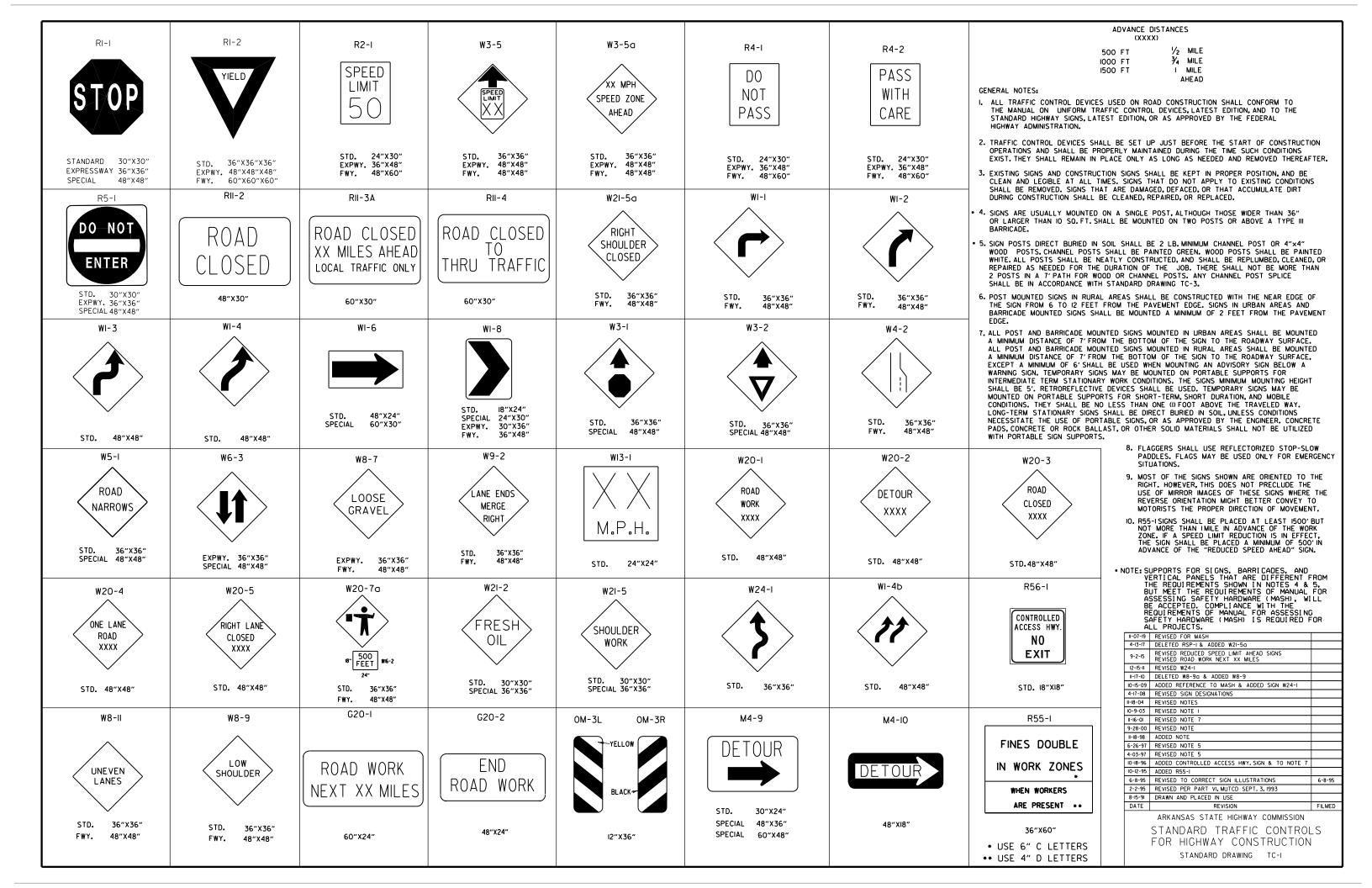
N.T.S.

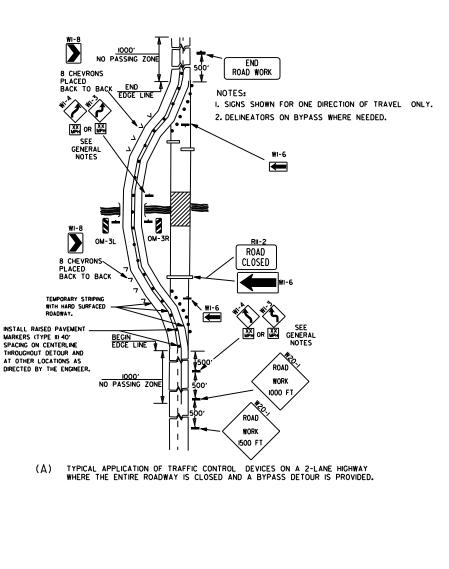
ARKANSAS STATE HIGHWAY COMMISSION

CONCRETE WALK (TYPE SPECIAL)

STANDARD DRAWING SI - 3

II-05-20 REVISED GENERAL NOTES
5-14-20 DRAWING ISSUED
DATE REVISION DATE FILMED





(DETOUR)

DETOUR

DETOUR

1

DETOUR

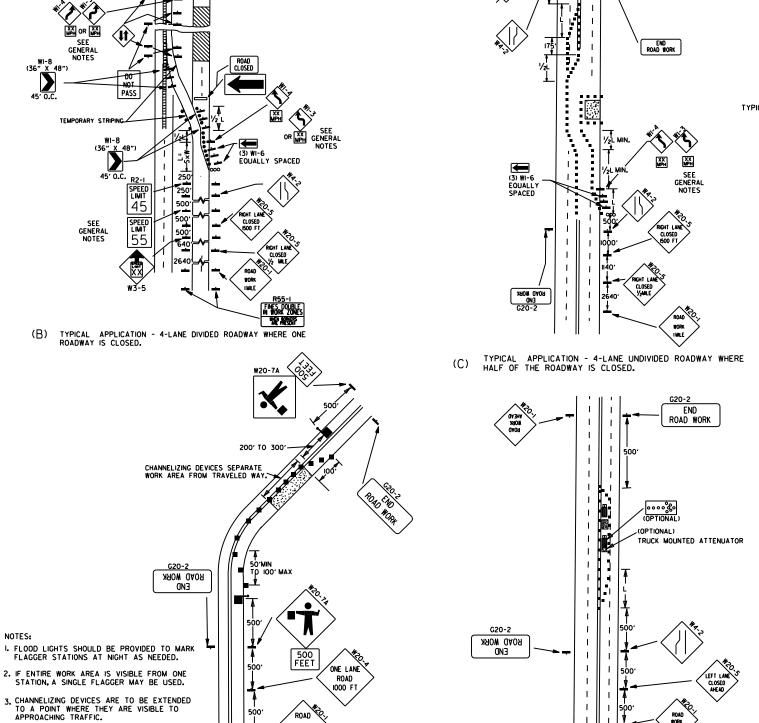
()) TYPICAL APPLICATION - ROADWAY CLOSED BEYOND DETOUR POINT.

₩EST 4

I. REGULATORY TRAFFIC CONTROL DEVICES TO BE MODIFIED AS NEEDED FOR THE DURATION OF

2.STREET NAMES MAY BE USED WHEN DESIRABLE FOR DIRECTING DETOURED TRAFFIC.

NOTES:



(F) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WITH INSIDE LANE CLOSED.

WORK

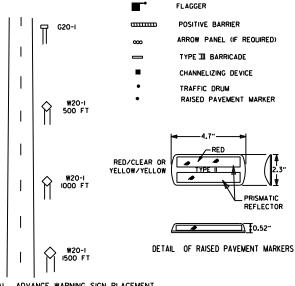
(E) TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES ON 2-LANE HIGHWAY WHERE ONE LANE IS CLOSED AND FLAGGING IS PROVIDED.

I, COMPLETE SIGNING SHOWN ONLY IN CROSSOVER DIRECTION. 2. TWO WAY TRAFFIC SEPARATED WITH POSITIVE BARRIER.

R4-7a RIGHT

4. AUTOMATED FLAGGER ASSISTANCE DEVICE

(AFAD) OPTIONAL. REFER TO MUTCD.



KEY:

TYPICAL ADVANCE WARNING SIGN PLACEMENT

TAPER FORMULAES

L=SXW FOR SPEEDS OF 45MPH OR MORE.

L= WS FOR SPEEDS OF 40MPH OR LESS.

WHERE:

G20-2

L= MINIMUM LENGTH OF TAPER.

S= NUMERICAL VALUE OF POSTED SPEED LIMIT PRIOR TO WORK OR 85TH PERCENTILE SPEED.

W= WIDTH OF OFFSET.

GENERAL NOTES:

I. THE MAINTENANCE DIVISION SHALL CONDUCT A BALL BANK STUDY TO DETERMINE THE ADVISORY SPEED LIMIT PRIOR TO OPENING TO TRAFFIC. THE ADVISORY SPEED WILL BE POSTED ON WI-3 OR WI-4 CURVE WARNING SIGNS. USE WI-4 WHEN SPEED IS GREATER THAN 30MPH AND WI-3 WHEN 30MPH OR LESS

2. WHEN THE EXISTING SPEED LIMIT IS 55MPH AND THE PLANS REQUIRE A SPEED LIMIT OF 45MPH, THE R2-K55) SHALL BE OMITTED AND THE W3-5 SHALL BE INSTALLED AT THAT LOCATION, ADDITIONAL R2-145MPH SPEED LIMIT SIGNS SHALL BE INSTALLED AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK AREA A R2-KXX)
SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

3. WHEN THE EXISTING SPEED LIMIT IS 65MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 55MPH, THE R2-K45) SHALL BE OMITTED.
ADDITIONAL R2-155MPH SPEED LIMIT SIGNS SHALL BE INSTALLED
AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK
AREA A R2-KXX SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

4. THE MAXIMUM SPACING BETWEEN CHANNELIZING DEVICES IN A TAPER
SHOULD BE APPROXIMATELY EQUAL IN FEET TO THE SPEED LIMIT.
BEYOND THE TAPER, MAXIMUM SPACING SHALL BE TWO TIMES
THE SPEED LIMIT, OR AS DIRECTED BY THE ENGINEER.

5. WARNING LIGHTS AND/OR FLAGS MAY BE MOUNTED
TO SIGNS OR CHANNELIZING DEVICES AT NIGHT AS NEEDED.

CONFUSION IN THE MINDS OF VEHICLE OPERATORS SHALL BE REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

7. TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE DELINEATED BY AFFIXING CONSPICUITY MATERIAL IN A CONTINUOUS LINE ON THE FACE OF THE TRAILER, WHEN PLACED ON ON A DAJACENT TO THE SHOULDER AND NOT BEHIND A POSITIVE BARRIER, THESE DEVICES SHALL BE DELINEATED BY PLACING FIVE 15) TRAFFIC DRUMS, EQUALLY SPACED ALONG THE TRAFFIC SIDE OF THE DEVICE, PAYMENT FOR TRAFFIC DRUMS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR VARIOUS TRAILER MOUNTED DEVICES.

B. DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE ARDOT QUALIFIED PRODUCTS LIST.

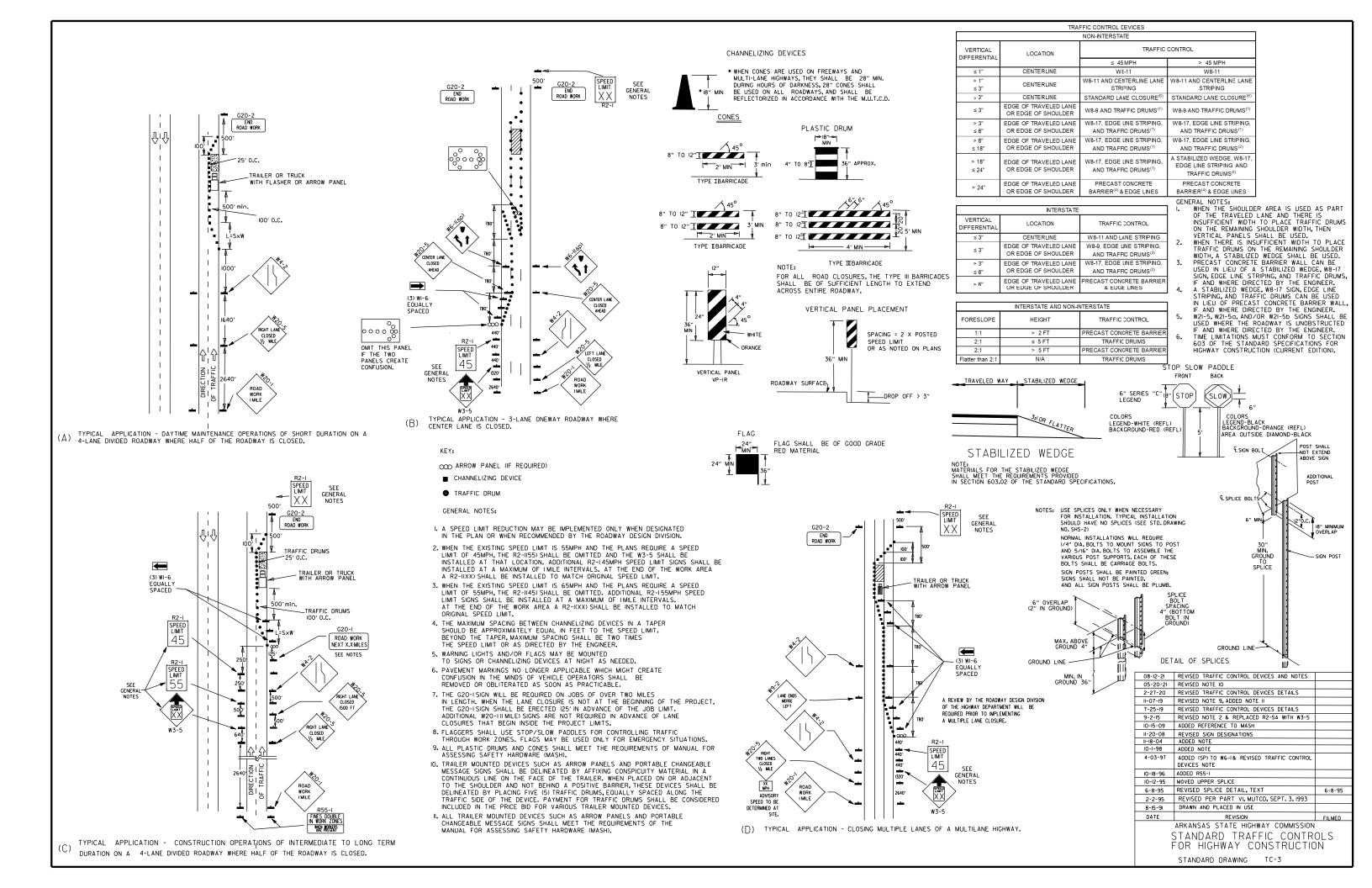
9. ALL TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL MEET THE REQUIREMENTS OF THE MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).

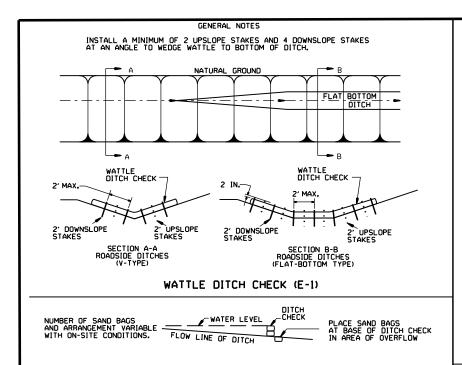
05-20-21	REVISED NOTE 7	
II-07-I9	REVISED NOTE I. ADDED NOTE 9	
9-2-15	REVISED NOTE 2, ADDED NOTE 8, REVISED DRAWING (A) & REPLACED R2-5A WITH W3-5	
9-12-13	REVISED DETAIL OF RAISED PAVEMENT MARKERS	
3-11-10	ADDED (AFAD)	
II-20-08	REVISED SIGN DESIGNATIONS	
11-18-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-I	
4-26-96	CORRECTED (a) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON WI-4A	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	E REVISION FILM	

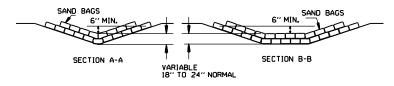
ARKANSAS STATE HIGHWAY COMMISSION

STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION

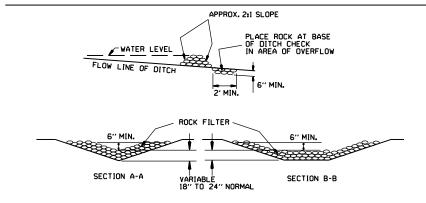
STANDARD DRAWING TC-2







#### SAND BAG DITCH CHECK (E-5)

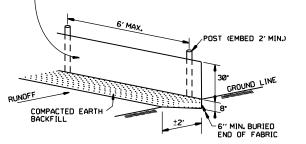


ROCK DITCH CHECK (E-6)

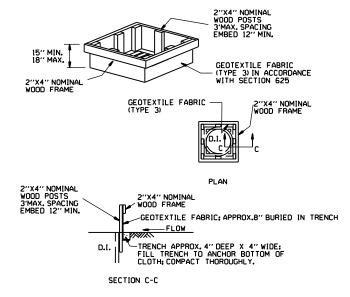
#### GENERAL NOTES

GEOTEXTILE FABRIC (TYPE 4) IN ACCORDANCE WITH SECTION 625

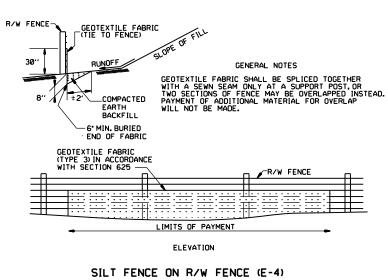
GEOTEXTILE FABRIC SHALL BE SPLICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.



SILT FENCE (E-11)



#### DROP INLET SILT FENCE (E-7)

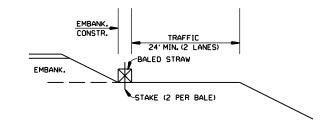


#### GENERAL NOTES

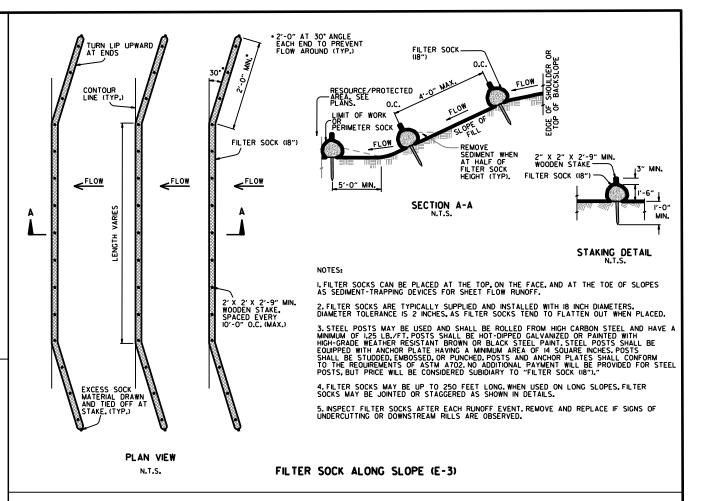
1. STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.

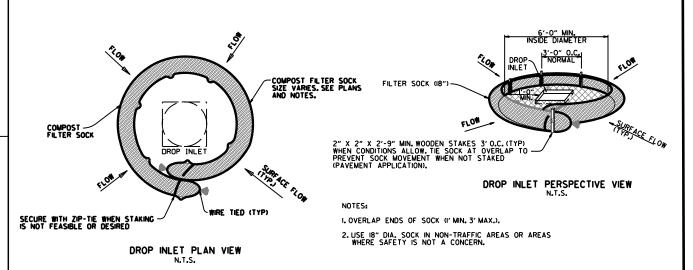
#### 2. NO GAPS SHALL BE LEFT BETWEEN BALES.

3, BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.



BALED STRAW FILTER BARRIER (E-2)





#### COMPOST FILTER SOCK DROP INLET PROTECTION (E-I3)

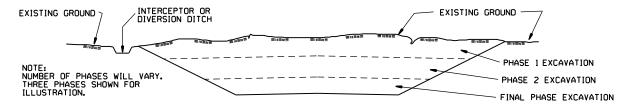
11-16-17	ADDED FILTER SOCK E-3 AND E-13		
12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ARKANSAS STATE HIGHWAY COMMISSION
11-18-98	ADDED NOTES		ARRANSAS STATE HIGHWAT COMMISSION
07-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)	7 00 05	TELIDOD LDV. EDOCION
07-20-95	REVISED SILT FENCE E-4 AND E-II	7-20-95	TEMPORARY EROSION
07-15-94	REV. E-4 & E-II MIN. 13" BURIED END OF FABRIC		
06-02-94	REVISED E-1,4.7 & II; DELETED E-2 & 3	6-2-94	CONTROL DEVICES
04-01-93	REDRAWN		CONTINUE DEVICES
10-01-92	REDRAWN		
08-02-76	ISSUED R.D.M.	298-7-28-76	STANDARD DRAWING TEC-I
DATE	REVISION	FILMED	STANDARD DRAWING TECT

#### CLEARING AND GRUBBING

CONSTRUCTION SEQUENCE

2. PERFORM CLEARING AND GRUBBING OPERATION.

#### EXCAVATION



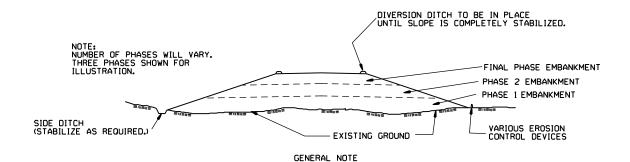
#### GENERAL NOTE

ALL CUT SLOPES SHALL BE ORESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

#### CONSTRUCTION SEQUENCE

- 1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
- 2. PERFORM PHASE 1 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
- 3. PERFORM PHASE 2 EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING.
- 4. PERFORM FINAL PHASE OF EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING. STABILIZE DITCHES, CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

#### **EMBANKMENT**



ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

#### CONSTRUCTION SEQUENCE

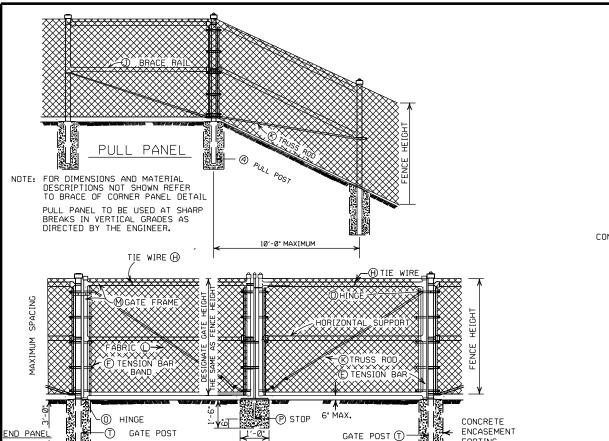
1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.

2. PLACE PHASE 1 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

			ARKANSAS STATE HIGHWAY COMMISSION
			TEMPORARY EROSION
			CONTROL DEVICES
11-03-94	CORRECTED SPELLING		
6-2-94	Drawn & Issued	6-2-94	STANDARD DRAWING TEC-3
DATE	REVISION	FILMED	SIMPHIO DIVENTINO IEC 2



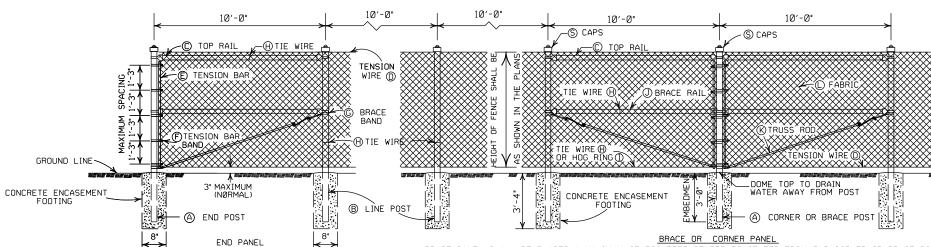
GATE WIDTH SHALL BE AS SHOWN IN THE PLANS

DOUBLE SWING GATE

1'-0"

DIA.

## POST SPACING DETAIL



8 SLATS

BRACE PANEL SHALL BE PLACED A MAXIMUM OF 500 FEET CENTER TO CENTER FROM END, CORNER OR BRACE POSTS. ANY BREAKS IN HORIZONTAL ALIGNMENT OF MORE THAN 30° SHALL BE CONSIDERED A CORNER.

#### GENERAL NOTES:

- (C) CHAIN LINK FENCE BEING PLACED ON PRIVATE PROPERTY SHALL INCLUDE A TOP RAIL. ALL LABOR, MATERIALS, EQUIPMENT, TOOLS, AND INCIDENTALS NECESSARY TO COMPLETE THE WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER LIN. FT. OF
- (D) TENSION WIRE: SHALL BE SECURED TO ALL TERMINAL, PULL, BRACE OR CORNER POSTS WITH TENSION BAR BANDS.
- (J) <u>BRACE RAIL</u>: BRACE RAILS SHALL BE PROVIDED AT ALL TERMINAL, PULL, BRACE OR CORNER POSTS HALFWAY BETWEEN THE TOP RAIL AND GROUND LEVEL WHEN TOPRAIL IS SPECIFIED AND TWELVE INCHES (12\*) DOWN FROM TOP OF FABRIC WHEN TOP TENSION WIRE IS SPECIFIED. BRACE RAIL SHALL EXTEND FROM SUCH POST TO THE FIRST ADJACENT

11/8" X 1/4" REDWOOD SLATS(LENGTH TO MATCH HEIGHT OF FENCE) (L) FABRIC: SHALL CONFORM TO THE SPECIFICATIONS. DETAIL OF REDWOOD SLAT INSTALLATION

(WHERE APPLICABLE)

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HEIGHT	A		Œ	В	©			0		E		F			G	
OF	END, PULL CORNER OF		LINE POSTS		TOP RAIL			TENSION WIRE		TENSION BAR		TENSION BAR BAND			BRACE BAND	
FENCE FABRIC	BRACE POS	т 🗆	SIZE	TIE SPACING	SIZE	TIE SPACING	MIN. LENGTH	SIZE	TIE SPACING	SIZE	LENGTH	SIZE	BOL T SIZE	SPACING	SIZE	BOLT SIZE
6' AND LESS	2½" O.D.	2	" O.D.	1 TIE EVERY 1'-2"	1 <b>%"</b> O.D.	1 TIE EVERY	10'-0"	7 GAUGE	1 TIE EVERY	MIN. OF	MIN. OF 2" LESS THAN	MIN. OF	5/ my 11/	1 BAND AT TOP AND BOTTOM " 15" MAX.	OF	
OVER 6' TO 12' INCL.	3 'O.D.	21/2	½° 0.D.	OF		2′-0"		COIL SPRING WIRE	1'-0"	3/16 " X 3/4"	FÄBRIC HEIGHT	34" X 0.074	%6 "X 11/4"	INTERVAL BETWEEN BANDS	34" X 0.105	%6" × 1¼"
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HEIGHT	H	I		J	K		L		M	(N	0		T			
OF FENCE	TIE	10G	BR	RACE RAIL	TRUSS		FABRIC		FRAME	HORIZONT SUPPORT	AL HINGE TPE		GATE P	DST		
FABRIC	1,1,1,0	ING	SIZ	E SPACIN	ROD	I .	MESH SELVA	AGE SIZE	TIE SPACING	SIZE SPAC	ING SWING	GATE W 12' AND	IDTH GA LESS 1	TE WIDTH OVER 2'TO 24'INCL.	]	
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6′ AND LESS	MIN. OF 12 GA. STEEL	SAME GAUGE	1%" O.D.	1 TIE	MIN. OF 5/6" ROUND WITH	9 GA.	. 2"	KNUCK -ING AND/OR	2" O.D.	1 TIE	2" O.D.	1 TIE	OFFSET	3" O.D.	4" O.D.	
OVER 6' TO 12' INCL.	OR 9 GA. ALUM.	AS FABRIC	1 /8 0.0.	2'-0"	TIGHTNERS AND FITTINGS			TWIST -ING		EVERY 1'-0"		EVERY 1'-0"		4" O.D.	4 0.0.	
															DF 2½"FOR FENCE AMETER OF 3" FOR	
AN OUTSIDE	DIAMETE	R OF 3	1/4" FOR F	FNCF H	FIGHTS OF E	' TO 1	12' 64	TE POST	S WHEE	RE GATE	WIDTH	IS 12'	AND LESS	SHALL HAVE	AN OUTSIDE DIA	METER OF 31

F 6'AND LESS, EIGHT OF 6'AND LESS; 3½ FOR FENCE HEIGHT AN OUISIDE DIAMETER OF 3½ FOR FENCE HEIGHTS OF 6'TO 12'. GATE POSTS WHERE GATE WIDTH IS 12'AND LESS SHALL HAVE AN OUISIDE DIAMETER OF 3½ FOR FENCE HEIG OF 6'AND LESS, ALUMINUM TENSION WIRE SHALL BE 0.192'IN DIAMETER, MINIMUM THICKNESS OF MATERIAL FROM WHICH EXPANSION SLEEVES SHALL BE MADE WILL BE 0.078'. POSTS AND RAILS MAY HAVE ANY CROSS-SECTIONAL SHAPE THAT WILL MEET THE SPECIFICATIONS.

OTHER DETAILS APPLY TO BOTH STEEL AND ALUMINUM FENCE.

FOOTING

END PANEL

ALL MISCELLANEOUS FITTINGS AND HARDWARE SHALL MEET THE REQUIREMENTS AND PRODUCTION TOLERANCES AS SET FORTH IN THE SPECIFICATIONS.

9 GAUGE ALUMINUM WIRE SHALL BE ACCEPTABLE FOR TIEING FABRIC TO TUBULAR AND ROLL FORMED MEMBERS OF STEEL FENCE.

POSTS AND RATES

10313 HIND THEES													
	GRADE	E 1 AND ALUMI	NUM ALL	GRADE 2									
SIZE 0.D.	O.D. INCHES	WALL THICKNESS		PER R FT. ALUMINUM	O.D. INCHES	WALL THICKNESS	LBS.PER LINEAR FT.						
1%	1.660	0.140	2.27	<b>0.</b> 786	1.660	0.111	1.84						
2	1.900	0.145	2.72	0.940	1.900	0.120	2.28						
21/2	2.375	0.154	3.65	1.264	2.375	0.130	3.11						
3	2.875	0.203	5.79	2.004	2.875	0.160	4.64						
31/2	3.500	Ø <b>.</b> 216	7.58	2.621	3.500	0.160	5.71						
4	4.000	0.226	9.11	3.151	4.000	0.160	6.56						

TOLERANCES ON DIMENSIONS AND WEIGHTS ACCORDING TO AASHTO M 181

- (O) HINGES: SHALL BE OF HEAVY PATTERN, OF ADEQUATE STRENGTH FOR GATE, AND WITH LARGE BEARING SURFACES FOR CLAMPING IN POSITION. THE HINGE SHALL BE OF THE PROPER TYPE TO ALLOW FOR THE DESIGNATED DEGREE OF SWING, THE HINGE SHALL NOT TWIST OR TURN UNDER THE ACTION OF THE GATE, THE GATES SHALL BE CAPABLE OF BEING OPENED AND CLOSED EASILY BY ONE PERSON.
- (P) <u>LATCHES AND STOPS</u>: SHALL BE PROVIDED FOR ALL GATES. GATES SHALL HAVE A DROP BAR LATCH. LATCHES SHALL BE ARRANGED FOR LOCKING. THE STOP FOR DROP BAR LATCHES SHALL BE SET IN CONCRETE AND ENGAGE THE PLUNGER OF THE BAR LATCH.
- (S) CAPS: ALL POSTS, EXCEPT ROLL FORMED POSTS AND 'T' POSTS SHALL BE CAPPED OVER THE EXTERIOR OF THE POST, AND SHALL CONFORM TO ASTM F626.

CONCRETE REQUIRED FOR THE EMBEDMENT OF ALL POSTS SHALL NOT BE PAID FOR DIRECTLY BUT SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR CHAIN LINK FENCE.

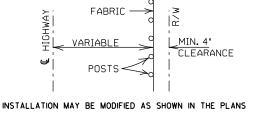
POSTS SHALL BE SPACED EQUIDISTANT ON A MAXIMUM OF 10' CENTERS.

EXCAVATION FOR POSTS: IN OTHER THAN ROCK SHALL BE OF THE DIMENSIONS INDICATED. IF ROCK IS ENCOUNTERED BEFORE REACHING THE REQUIRED DEPTH, THE EXCAVATION SHALL BE CONTINUED TO THE DEPTH INDICATED OR 1'-6" INTO THE ROCK, WHICHEVER IS LESS, AND SHALL BE A MINIMUM OF 8 INCHES IN DIAMETER.

ARKANSAS STATE HIGHWAY COMMISSION

CHAIN LINK FENCE

STANDARD DRAWING WF-3



TYPICAL INSTALLATION DIAGRAM

11-17-10 REVISED TRUSS ROD 12-10-09 REVISED POSTS & RAILS TABLE 5-21-09 ADDED TABLE & GEN.NOTE (C) 8-22-02 REVISED NOTES, REMOVED TABLE, & REMOVED FENCE ALTERNATE

4-3-97 REVISED BRACE RAIL NOTE

10-18-96 REVISED AASHTO & ASTM REF.

11-3-94 REVISED NOTE (L)

10-1-92 DELETED ALTERNATE POST

8-15-91 DELETED ROLL FORMED POST
DETAIL & ADDED NOTE

11-30-89 DELETED CLASS CONCRETE

11-17-88 REVISED O.D. SIZES 11-17-88 REVISED 0,D, SIZES 10-30-87 CENERAL REVISIONS 4-20-79 REVISED AND REDRAWN 10-2-72 REVISED AND REDRAWN 695-4-20-79 530-10-2-72