

STA. 101+00.00 BEGIN JOB 080614 LOG MILE 2.27

W 92°25' 03"

1.1

N 35*36'25" W 92*24'47"

(5 SPANS @ 40') BR. END STA. 116+55.50

LONGITUDE W 92°25' 18'

ARKANSAS DEPARTMENT OF TRANSPORTATION CONSTRUCTION PLANS FOR STATE HIGHWAY

PEE DEE CREEK

STR. & APPRS. (CLINTON) (S)

VAN BUREN COUNTY ROUTE 16 SECTION IO

JOB 080614

FED. AID PROJ. NHPP-BFP-007(41)

NOT TO SCALE R 14 W R 13 W 9807 St. Hwy. R 14 W R 13 W

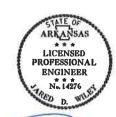
DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	080614	1	82
		PEE DE	E CREE	K STR. & AP	PRS. (CLIN	TON) (S)



ARK. HWY. DIST. NO. 8

DESIGN TRAFFIC DATA	
DESIGN YEAR20	44
2024 ADT48	00
2044 ADT56	00
2044 DHV6	16
DIRECTIONAL DISTRIBUTION O.	60
TRUCKS55 M	5%
DESIGN SPEED55 M	PH

APPROVED



CHIEF ENGINEER - PRECONSTRUCTION

STA. 129+69.00

END JOB 080614

JAN 2 2 2024

DATE REVISED	DATE REVISED	FED.RO. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS			
		6	ARK.	080614	2	82			
-		INDEX OF SHEETS							

ARKANSAS

ARKANS

Digitally signed by Trinity Smith Date: 2023.12.11 14:27:38-06'00'

INDEX OF SHEETS

SHEET NO.	TITLE	BRIDGE NO.	DRWG.NO.
4	TITLE SHEET		
1	INDEX OF SHEETS		
2	STANDARD DRAWNGS AND GENERAL NOTES		
4	GOVERNING SPECIFICATIONS		
5 - 8	TYPICAL SECTIONS OF IMPROVEMENT		
9 - 11	SPECIAL DETAILS		
12 - 20	TEMPORARY EROSION CONTROL DETAILS		
21 - 27	MAINTENANCE OF TRAFFIC DETAILS		
28	PERMANENT PAVEMENT MARKING DETAILS		
29 - 33	QUANTITIES		
34	SCHEDULE OF BRIDGE QUANTITIES	07565	64431
35	SUMMARY OF QUANTITIES AND REVISIONS		_ 51161
36 - 38	SURVEY CONTROL DETAILS		
39 - 42	PLANAND PROFILE SHEETS		
43	LAYOUT OF BRIDGE HIGHWAY 16 OVER PEE DEE CREEK (SHEET 1 OF 2)	7565	64432
44	LAYOUT OF BRIDGE HIGHWAY 16 OVER PEE DEE CREEK (SHEET 2 OF 2)	7565	64433
45	DETAILS OF END BENTS	7565	64434
46	DETAILS OF INTERMEDIATE BENTS (SHEET 1 OF 2)	7565	64435
47	DETAILS OF INTERMEDIATE BENTS (SHEET 2 OF 2)	7565	64436
48	DETAILS OF ELASTOMERIC BEARINGS	7565	64437
49	DETAILS OF 200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT (SHEET 1 OF 5)	7565	64438
50	DETAILS OF 200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT (SHEET 2 OF 5)	7565	_64439
51	DETAILS OF 200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT (SHEET 3 OF 5)	7565	64440
52	DETAILS OF 200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT (SHEET 4 OF 5)	7565	64441
53	DETAILS OF 200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT (SHEET 5 OF 5)	7565	64442
54	DETAILS OF TYPE SPECIAL APPROACH SLABS	7565	_64442A
55 - 82	CROSS SECTIONS		

NOTE: CROSS SECTIONS NOT NORMALLY INCLUDED IN PLANS SOLD TO PROSPECTIVE BIDDERS, BUT MAY BE HAD UPON REQUEST.

ARKANSAS

LICENSED

PROFESSION

N. 11425

W. Tr. D. Strift

12-11-2023

BRIDGE STANDARD DRAWINGS

DRWG.NO.	TITLE	DATE
55000	STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS	02-27-14
55001	STANDARD DETAILS FOR DUMPED RIPRAP AND FILTER BLANKET AND COMPUTING EXCAVATION FOR STRUCTURES	02-27-14
55005	STANDARD DETAILS FOR PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS	03-24-16
55006	STANDARD GENERAL NOTES FOR STEEL BRIDGE STRUCTURES	09-02-15
55007	STANDARD DETAILS FOR STEEL BRIDGE STRUCTURES	02-11-16
55010	STANDARD DETAILS FOR TYPE D BRIDGE NAME PLATE	04-14-23
55020	STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS	03-24-16
55030F	STANDARD DETAILS FOR TYPE F APPROACH GUTTERS	04-08-21
55070	STANDARD DETAILS FOR BRIDGE TRAFFIC RAIL TYPE SSTR36	09-27-22

ROADWAY STANDARD DRAWINGS

DRWG.N	IO. TITLE	DATE
CDP-1	CONCRETE DITCH PAVING	12-08-16
DR-2	DETAILS OF DRIVEWAYS & TURNOUTS	05-19-22
FES-1	FLARED END SECTION	10-18-96
FES-2	FLARED END SECTION	10-18-96
GR-6	GUARDRAIL DETAILS	05-19-22
GR-7	GUARDRAIL DETAILS	11-07-19
GR-8	GUARDRAIL DETAILS	11-07-19
GR-9	GUARDRAIL DETAILS	11-07-19
GR-10	GUARDRAIL DETAILS	11-07-19
GR-11	GUARDRAIL DETAILS	11-07-19
GR-12	GUARDRAIL DETAILS	05-14-20
MB-1	MAILBOX DETAILS	11-18-04
PCC-1	CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING	02-27-14
PCM-1	METAL PIPE CULVERT FILL HEIGHTS & BEDDING	02-27-14
PCP-1	PLASTIC PIPE CULVERT (HIGH DENSITY POLYETHYLENE)	02-27-14
PCP-2	PLASTIC PIPE CULVERT (PVC F949)	02-27-14
PCP-3		
PM-1	PAVEMENT MARKING DETAILS	02-27-20
PU-1	DETAILS OF PIPE UNDERDRAIN	12-08-16
SE-2	TABLES AND METHOD OF SUPERELEVATION FOR TWO-WAY TRAFFIC	11-07-19
TC-1	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	11-07-19
TC-2	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	05-20-21
TC-3	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION	08-12-21
TC-4	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION-TEMPORARY PRECAST BARRIER	11-07-19
TC-5	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION-TEMPORARY PRECAST BARRIER	11-07-19
TEC-1	TEMPORARY EROSION CONTROL DEVICES	11-16-17
TEC-2	TEMPORARY EROSION CONTROL DEVICES	06-02-94
TEC-3		11-03-94
WF-2	WIRE FENCE WATER GAPS	04-20-79
WF-4	WIRE FENCE TYPE C AND D	08-22-02

GENERAL NOTES

- 1. GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- 2. ALL PIPE LINES, POWER, TELEPHONE, AND TELEGRAPH LINES TO BE MOVED OR LOWERED BY THE RESPECTIVE OWNERS AS PER AGREEMENT WITH SUCH OWNERS.
- ANY EQUIPMENT OR APPURTENANCE THAT INTERFERES WITH THE PROPOSED CONSTRUCTION AND WHICH MAY BE THE PROPERTY OF UTILITY SERVICE ORGANIZATIONS SHALL BE MOVED BY THE OWNERS UNLESS OTHERWISE PROVIDED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING U. S. MAILBOXES WITHIN THE PROJECT LIMITS IN SUCH A MANNER THAT THE PUBLIC MAY RECEIVE CONTINUED MAIL SERVICE. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS BID ITEMS.
- 5. ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- 6. ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED AS DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO ENSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A FENCE TO CONTROL LIVESTOCK IN AREAS WHERE PASTURES ARE SEVERED. WIRE FENCE MAY BE CONSTRUCTED INITIALLY, OR IN LIEU THEREOF, THE CONTRACTOR AT HIS OWN EXPENSE, MAY ELECT TO PROVIDE TEMPORARY FENCING SUITABLE TO CONTAIN LIVESTOCK.
- 8. THE SEQUENCE AS SHOWN ON THE MAINTENANCE OF TRAFFIC PLANS IS A GENERAL OUTLINE FOR THE CONSTRUCTION OF THIS PROJECT, AND IN NO WAY IS IT INTENDED TO COVER EVERY ITEM IN THE PROJECT. ITEMS NOT CRITICAL TO THE CONSTRUCTION SEQUENCE MAY BE CONSTRUCTED IN ANY STAGE AS APPROVED BY THE PESIDENT ENGINEER
- 9. ALL FLEXIBLE BASE AND ASPHALTIC PAVEMENTS REMOVED SHALL BE PAID FOR UNDER THE ITEM NO. 210 UNCLASSIFIED EXCAVATION.
- 10. THE EXISTING ASPHALT PAVEMENT TO BE REMOVED FROM THE REMAINING PAVEMENT SHALL BE SEPARATED BY SAWING ALONG A NEAT LINE. AFTER SAWING, THE PAVEMENT TO BE REMOVED SHALL BE CAREFULLY REMOVED IN A MANNER THAT WILL NOT DAMAGE THE PAVEMENT THAT IS TO REMAIN. ANY DAMAGE OF THE ASPHALT PAVEMENT THAT IS TO REMAIN IN PLACE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS		
02/01/24		6	ARK.	080614	4	82		
		GOVERNING SPECS.						



GOVERNING SPECIFICATIONS (1 OF 2)

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS SUPPLEMENTAL SPECIFICATIONS:

NUMBER	TITLE
ERRATA	ERRATA FOR THE BOOK OF STANDARD SPECIFICATIONS
FHWA-1273_	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTICE TO CONTRACTORS
FHWA-1273_	_ SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 140)
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
FHWA-1273_	_ SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
FHWA-1273_	_ SUPPLEMENT - WAGE RATE DETERMINATION
100-3	_ CONTRACTOR'S LICENSE
100-4	_ DEPARTMENT NAME CHANGE
	_ ISSUANCE OF PROPOSALS
	_ CONTACT INFORMATION FOR MOTORIST DAMAGE CLAIMS
105-4	_ MAINTENANCE DURING CONSTRUCTION
107-2	_ RESTRAINING CONDITIONS
108-1	_ LIQUIDATED DAMAGES
108-2	_ WORK ALLOWED PRIOR TO ISSUANCE OF WORK ORDER
110-1	_ PROTECTION OF WATER QUALITY AND WETLANDS
210-1	_ UNCLASSIFIED EXCAVATION
	_ AGGREGATE BASE COURSE
	_ QUALITY CONTROL AND ACCEPTANCE
307-1	_ CEMENT
308-1	_ CEMENT
400-1	_ TACK COATS
400-4	DESIGN AND QUALITY CONTROL OF ASPHALT MIXTURES
	_ PERCENT AIR VOIDS FOR ACHM MIX DESIGNS
	_ LIQUID ANTI-STRIP ADDITIVE
	_ TRACKLESS TACK
404-3	_ DESIGN OF ASPHALT MIXTURES
409-2	_ ASPHALT LABORATORY FACILITY
410-1	_ CONSTRUCTION REQUIREMENTS AND ACCEPTANCE OF ASPHALT CONCRETE PLANT MIX COURSES
410-2	_ DEVICES FOR MEASURING DENSITY FOR ROLLING PATTERNS
	_ EVALUATION OF ACHM SUBLOT REPLACEMENT MATERIAL
416-1	_ RECYCLED ASPHALT PAVEMENT
501-2	_ CEMENT
505-1	_ PORTLAND CEMENT CONCRETE DRIVEWAY
600-2	_ INCIDENTAL CONSTRUCTION
603-1	LANE CLOSURE NOTIFICATION
	RETROREFLECTIVE SHEETING FOR TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES
	_ TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES (MASH)
	_ CONCRETE DITCH PAVING
606-1	_ PIPE CULVERTS FOR SIDE DRAINS
	_ GUARDRAIL TERMINAL (TYPE 2)
	_ GUARDRAIL DELINEATORS
	_ MULCH COVER
800-1	STRUCTURES
802-3	_ CONCRETE FOR STRUCTURES
802-4	_ CEMENT _ REINFORCING STEEL FOR STRUCTURES
	_ STEEL STRUCTURES
	_ INSTALLATION OF ELASTOMERIC BEARINGS
808-2	ELASTOMERIC REARINGS

808-2_____ ELASTOMERIC BEARINGS

GOVERNING SPECIFICATIONS (2 OF 2)

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS SUPPLEMENTAL SPECIFICATIONS:

NUMBER TITLE	
JOB 080614 BIDDING REQUIREMENTS AND CONDITIONS	
JOB 080614 BROADBAND INTERNET SERVICE FOR ASPHALT CONCRETE PLANT	
JOB 080614 BROADBAND INTERNET SERVICE FOR FIELD OFFICE	
JOB 080614 BUY AMERICA - CONSTRUCTION MATERIALS	
JOB 080614 CARGO PREFERENCE ACT REQUIREMENTS	
JOB 080614 CAVE DISCOVERY	
JOB 080614 CLASS C FLY ASH IN PORTLAND CEMENT CONCRETE PAVEMENT AND CLASS S(AE) CONCRETE	
JOB 080614 COLD MILLING - COUNTY PROPERTY	
JOB 080614 CONCRETE BRIDGE DECK CURING AND SURFACE TREATMENT RESTRICTIONS	
JOB 080614 DESIGN AND QUALITY CONTROL OF ASPHALT MIXTURES	
JOB 080614 DESIGN OF ASPHALT MIXTURES - AGGREGATES	
JOB 080614 DIRECT TENSION INDICATORS FOR HIGH STRENGTH BOLT ASSEMBLIES	
JOB 080614 DISADVANTAGED BUSINESS ENTERPRISE BIDDER'S RESPONSIBILITIES	
JOB 080614 ESTABLISHING CONTRACT TIME – WORKING DAY CONTRACT	
JOB 080614 GOALS FOR DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION	
JOB 080614LIQUIDATED DAMAGES PROCEDURE FOR BID LETTINGS	
JOB 080614 MANDATORY ELECTRONIC CONTRACT	
JOB 080614 MANDATORY ELECTRONIC DOCUMENT SUBMITTAL	
JOB 080614 NESTING SITES OF MIGRATORY BIRDS JOB 080614 OFF-SITE RESTRAINING CONITIONS FOR INDIANA AND NORTHERN LONG-EARED BATS	
JOB 080614_ OFF-SITE RESTRAINING CONTIONS FOR INDIANA AND NORTHERN LONG-EARED BATS JOB 080614_ PARTNERING REQUIREMENTS	
JOB 080614 PERCENT AIR VOIDS AND NDESIGN FOR ACHM SURFACE COURSE	
JOB 080614 PLASTIC PIPE	
JOB 080614 PRICE ADJUSTMENT FOR ASPHALT BINDER	
JOB 080614 PRICE ADJUSTMENT FOR FUEL	
JOB 080614 PROHIBITION OF CERTAIN TELECOMMUNICATIONS AND VIDEO SURVEILLANCE SERVICES OR EQUIPM	ENT
JOB 080614 SECTION 404 NATIONWIDE 14 PERMIT REQUIREMENTS	
JOB 080614 SHORING FOR CULVERTS	
JOB 080614 SOIL STABILIZATION	
JOB 080614 SPECIAL CLEARING PUP SEASON REQUIREMENTS	
JOB 080614 STORM WATER POLLUTION PREVENTION PLAN	
JOB 080614 SUBMISSION OF ASPHALT CONCRETE HOT MIX ACCEPTANCE TEST RESULTS	
JOB 080614 TOTAL SOLAR ECLIPSE	
JOB 080614 UTILITY ADJUSTMENTS	
JOB 080614 VALUE ENGINEERING	
JOB 080614 WARM MIX ASPHALT	
JOB 080614WATER POLLUTION CONTROL	
JOB 080614 WORK ALLOWED PRIOR TO ISSUANCE OF WORK ORDER FOR TREE CLEARING	

	_	TYPICA	SECT	IONS OF IMPRO	VEMENT	
		6	ARK.	080614	5	82
DATE REVISED	DATE REVISED	DIST.NO.	STATE	JOB NO.	SHEET NO.	SHEETS

ARKANSAS LICENSED ROFESSION A ENGINEER No. 11425 12-11-2023

CONSTRUCTION VAR. SUBGRADE WIDTH NOTES: REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER. •TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER 34'-0" ACHM SURFACE COURSE (3/8") (220 LBS./SQ. YD.) THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED. VAR. WIDTH ACHM SURFACE COURSE (3/8") •(VAR. LBS./SO.YD.) FOR LEVELING VAR. WIDTH ACHM SURFACE COURSE (3/8")
(220 LBS./SO.YD.) & TACK COAT & TACK COAT (O.17 GAL./SO. YD.) VAR. WIDTH ACHM BINDER COURSE (I") (440 LBS./SO.YD.) & TACK COAT ASPHALT FOR LEVELING OF EXISTING PAVEMENT SHALL BE PLACED ONLY IF AND WHERE DIRECTED BY THE ENGINEER. CALCULATIONS FOR THE AMOUNT OF 30' -0" 30' -0" CALCULATIONS FOR THE AMOUNT OF LEVELING AND/OR LEVELING OPERATIONS SHALL BE PERFORMED BEFORE CONSTRUCTING NOTCH AND WIDENING. CALCULATIONS WILL NOT BE PAID FOR DIRECTLY, BUT PAYMENT WILL BE CONSIDERED INCLUDED IN THE VARIOUS PAY ITEMS. NOTE: ON ALL SUPERELEVATED CURVES AND THRU SUPERELEVATED CURVES
AND THRU SUPERELEVATION TRANSITIONS
THE ALGEBRAIC DIFFERENCE BETWEEN
PAVEMENT SLOPE AND SHOULDER SLOPE
SHALL NOT EXCEED 0.08'/'. WITH THE APPROVAL OF THE ENGINEER, THE CONTRACTOR WILL BE ALLOWED TO SUBSTITUTE, AT NO ADDITIONAL COST TO THE DEPARTMENT, THE FIRST LIFT OF ACHM SURFACE COURSE (1/2") IN LIEU OF VAR. VAR. 6' -0" 11'-0" 11' -0" 6' -0" TRAVEL LANE TRAVEL LANE SHLD. SHLD. AGGREGATE BASE COURSE ON THE SHOULDERS. THEORETICAL PROFILE GRADE THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES. POINT OF SUPERELEVATION ROTATION
0.22' BELOW PROFILE GRADE SUPERELEVATION SLOPE MIN. DITCH S.E. SLOPE VAR. NOTCH S.E. SLOPE 12" NOTCH VAR. WIDTH EXISTING
PAVEMENT

AGGREGATE BASE COURSE

(CLASS 7) VAR. COMP. DEPTH VAR. TONS/STA.

HWY, 16 - NOTCH, WIDEN, AND OVERLAY SECTION SUPERELEVATION

VAR. WIDTH

AGG. BASE CRSE. (CL.7) (6" COMP. DEPTH) VAR. TONS/STA.

RETAIN

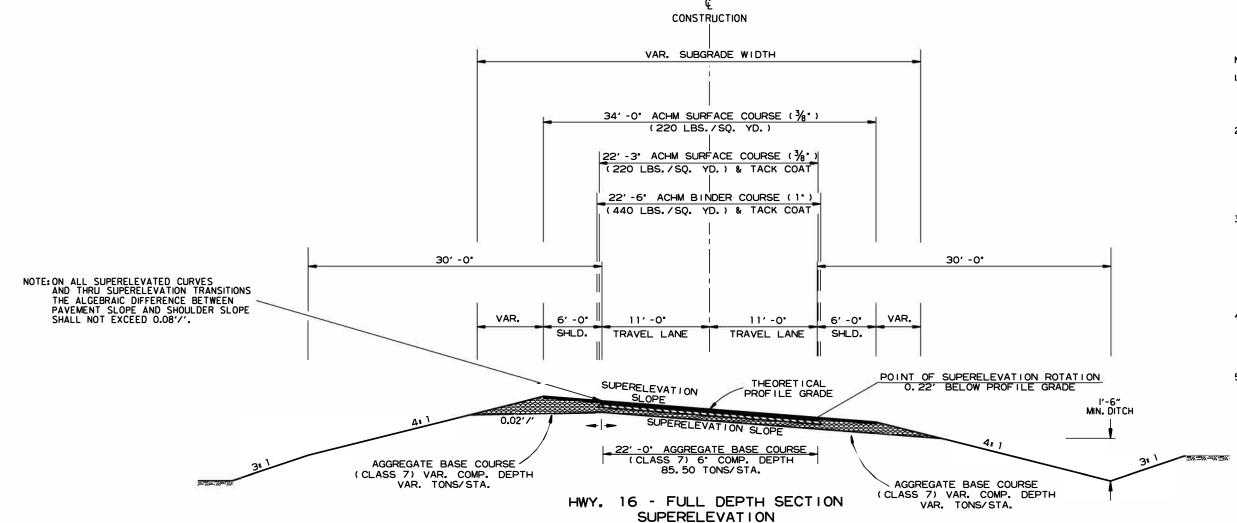
AGGREGATE BASE COURSE (CLASS 7) VAR. COMP. DEPTH

VAR. TONS/STA.

STA. 101+00.00 TO STA. 103+74.62 STA. 128+82.12 TO STA. 129+69.00







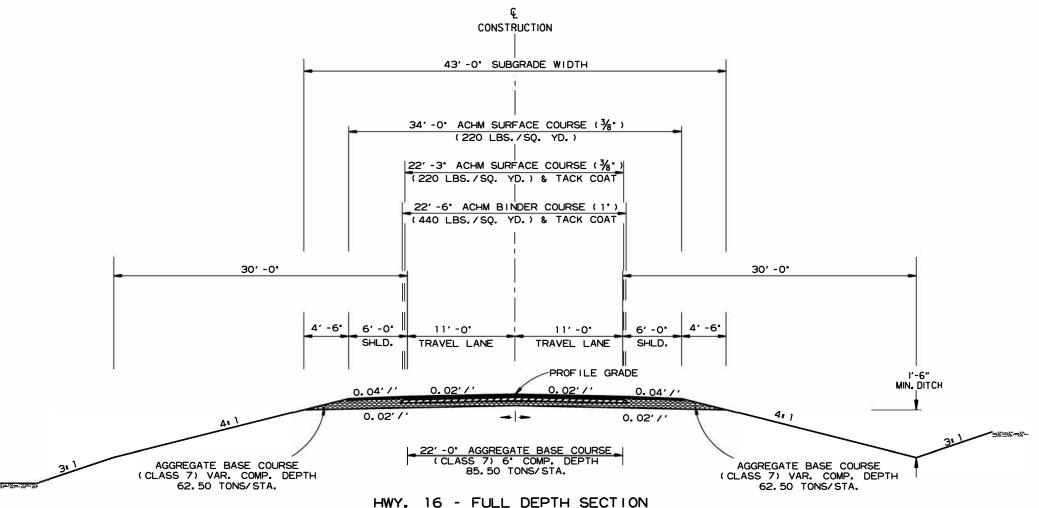
STA. 103+74.62 TO STA. 107+37.59 STA. 122+22.29 TO STA. 128+82.12

NOTES:

- I. REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.
- 2. THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET TOLERANCE INDICATED, PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.
- 3. ASPHALT FOR LEVELING OF EXISTING PAVEMENT SHALL BE PLACED ONLY IF AND WHERE DIRECTED BY THE ENGINEER. CALCULATIONS FOR THE AMOUNT OF LEVELING AND/OR LEVELING OPERATIONS SHALL BE PERFORMED BEFORE CONSTRUCTING NOTCH AND WIDENING. CALCULATIONS WILL NOT BE PAID FOR DIRECTLY, BUT PAYMENT WILL BE CONSIDERED INCLUDED IN THE VARIOUS PAY ITEMS.
- 4. WITH THE APPROVAL OF THE ENGINEER,
 THE CONTRACTOR WILL BE ALLOWED TO
 SUBSTITUTE, AT NO ADDITIONAL COST TO
 THE DEPARTMENT, THE FIRST LIFT OF ACHM
 SURFACE COURSE (1/2") IN LIEU OF
 AGGREGATE BASE COURSE ON THE SHOULDERS.
- 5. THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.

-	+	TYPICA	SECT	IONS OF IMPRO	VEMENT	
		6	ARK.	080614	7	82
DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	SHEETS

LICENSED ENGINEER No. 11425 12-11-2023



NOTES:

- REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.
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 SURFACE COURSE (1/2") IN LIEU OF
 AGGREGATE BASE COURSE ON THE SHOULDERS.
- THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.

STA. 107+37.59 TO STA. 114+19.50 STA. 116+90.50 TO STA. 122+22.29

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
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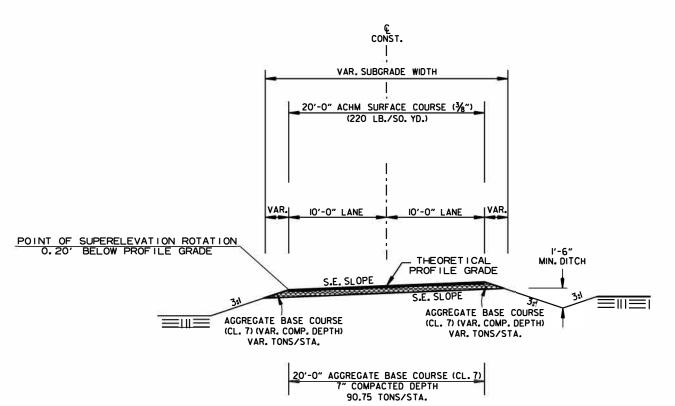
ARKANSAS

LECENSED

PROFESSION
ENGINEER

No. 11425

12-11-2023



COUNTY ROAD 93 - FULL DEPTH SECTION SUPERELEVATION

STA. 200+25.72 TO STA. 204+19.58

NOTES:

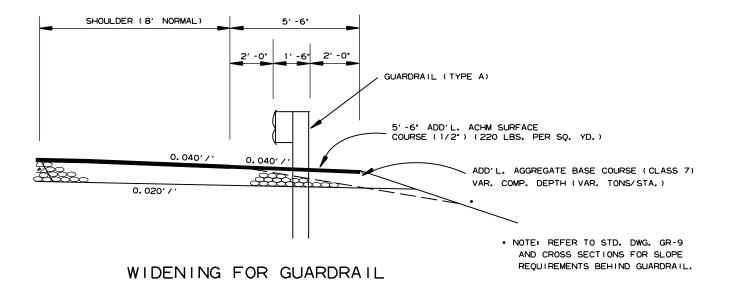
- I. REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.
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- 3. THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT LANE LINES.

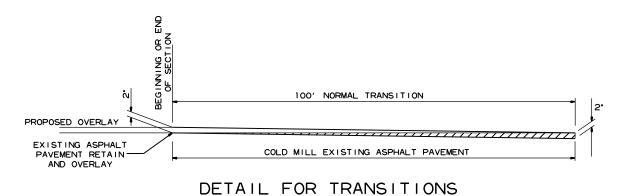


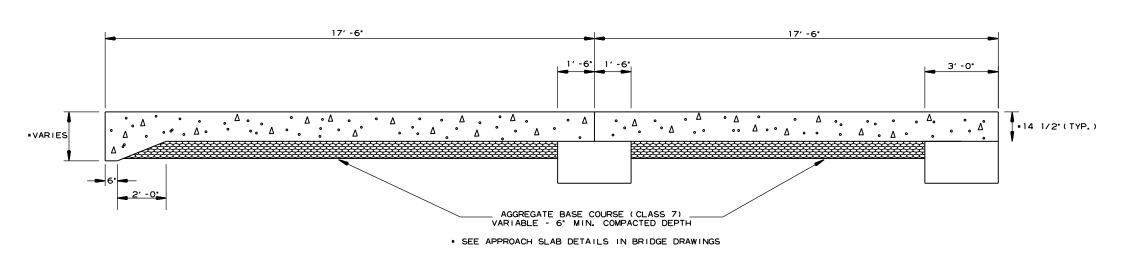
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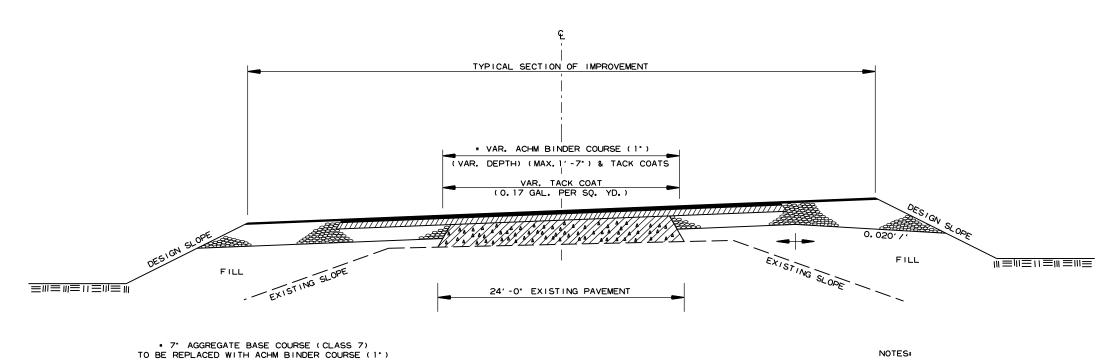






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-		SPECIA	DETA			





METHOD OF RAISING GRADE

NOTES:

- (1) THIS DETAIL TO BE USED ONLY WHERE DIRECTED BY THE ENGINEER.
- (2) QUANTITIES FOR METHOD OF GRADE RAISE USING ASPHALT WERE CALCULATED ON THIS PROJECT AT LOCATIONS WHERE THE DISTANCE BETWEEN THE EXISTING ASPHALT ROADWAY AND THE PROPOSED SUBGRADE WAS ONE FOOT OR LESS.
- (3) IN LOCATIONS WHERE THE DISTANCE BETWEEN THE PROPOSED SUBGRADE AND THE EXISTING ASPHALT ROADWAY IS MORE THAN ONE FOOT, SCARIFICATION OF THE EXISTING ASPHALT ROADWAY WILL BE REQUIRED AS STATED IN SECTION 210, SUBSECTION 210.09, OF THE STANDARD SPECIFICATIONS.

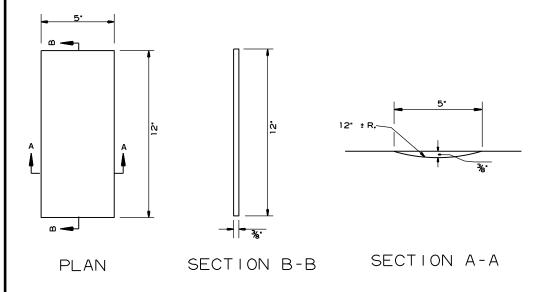
DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	080614	11	82
		SPECIA	L DETA	LS		

TOTAL OF ARKANSAS

DEFENSION
ENGINEER

No. 11425

12-11-2023



TRAVEL LANE—

EDGE LINE

90 4-2

SHOULDER

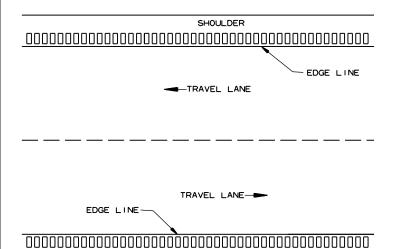
(TYPICAL)

DETAILS OF RUMBLE STRIPS

LOCATION PLAN OF RUMBLE STRIPS

LEFT OR RIGHT SHOULDER

DETAIL FOR RUMBLE STRIP GAP AT DRIVEWAY TURNOUTS



SHOULDER

GENERAL NOTES

- 1. RUMBLE STRIPS SHALL NOT BE INSTALLED ON CURB SECTIONS, BRIDGE DECKS, APPROACH SLABS, INTERSECTING STREETS OR ROADWAYS, RESIDENTIAL OR COMMERCIAL DRIVEWAYS OR ACROSS TRANSVERSE JOINTS OF CONCRETE SHOULDERS.
- 2. RUMBLE STRIPS SHALL NOT BE INSTALLED ON A PAVED SHOULDER THAT IS USED AS A DECELERATION LANE FOR THE LENGTH DEEMED APPROPRIATE BY THE ENGINEER.
- 3. THE 4° OFFSET FROM THE EDGE LINE MAY BE INCREASED TO AVOID LONGITUDINAL JOINTS. IN ALL CASES, THE LATERAL DEVIATION FROM THE PLANNED OFFSET SHOULD BE KEPT TO A MINIMUM.
- 4. RUMBLE STRIPS SHALL BE MEASURED BY THE LINEAR FOOT LONGITUDINALLY ALONG THE SHOULDER. PAYMENT SHALL ONLY INCLUDE THAT PORTION OF THE SHOULDER ON WHICH RUMBLE STRIPS HAVE BEEN CONSTRUCTED. NO MEASUREMENT OR PAYMENT WILL BE MADE FOR GAPS, DRIVEWAYS, TURNOUTS, OR OTHER PUBLIC ROAD INTERSECTIONS WHERE RUMBLE STRIPS HAVE NOT BEEN CONSTRUCTED.
- 5. THE % DEPTH SHALL GENERALLY APPLY FOR THE ENTIRE 12 LENGTH. SOME VARIATION TO SUIT SHOULDER SLOPE BREAKS MAY BE NECESSARY.

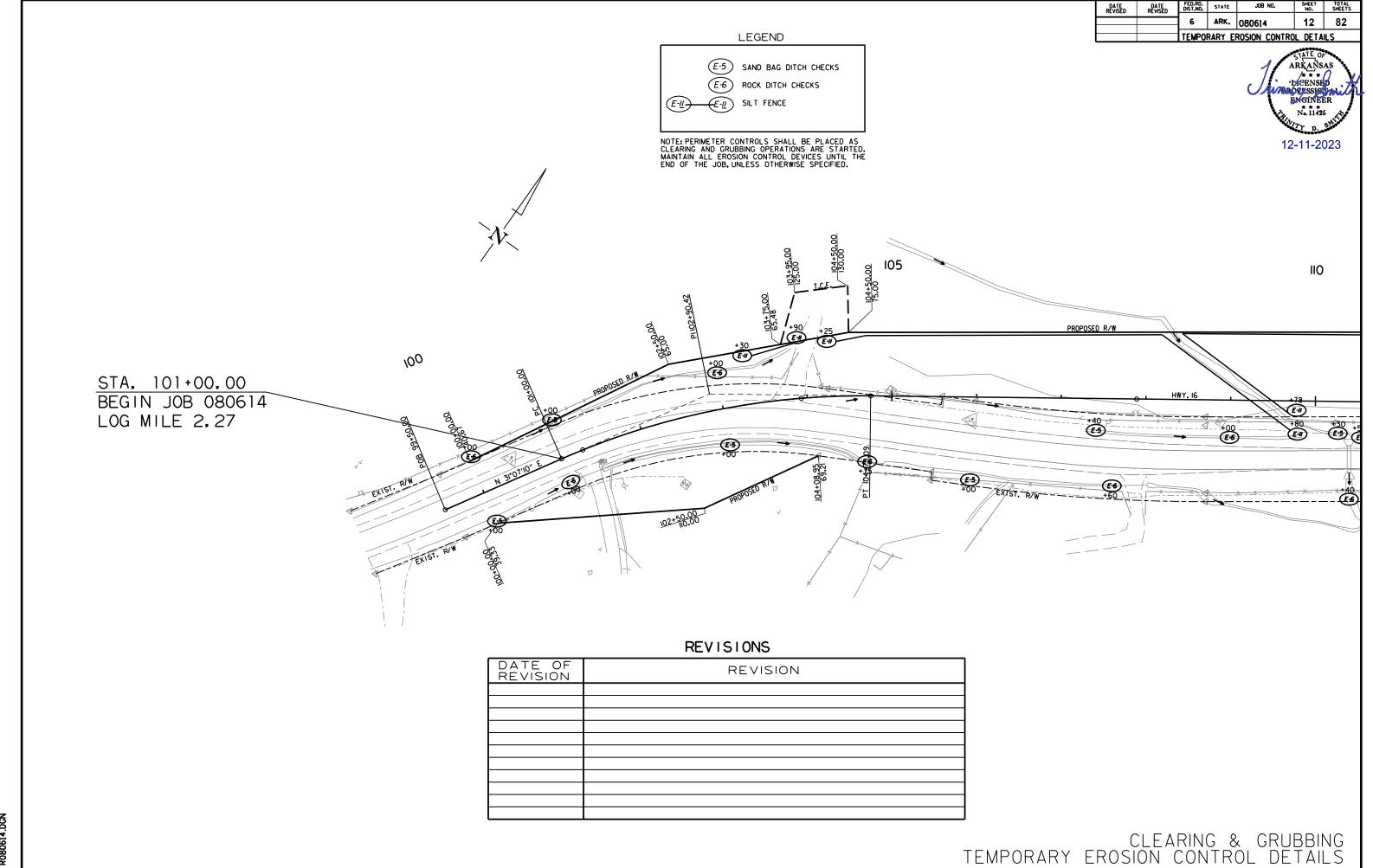
TRAVEL LANE

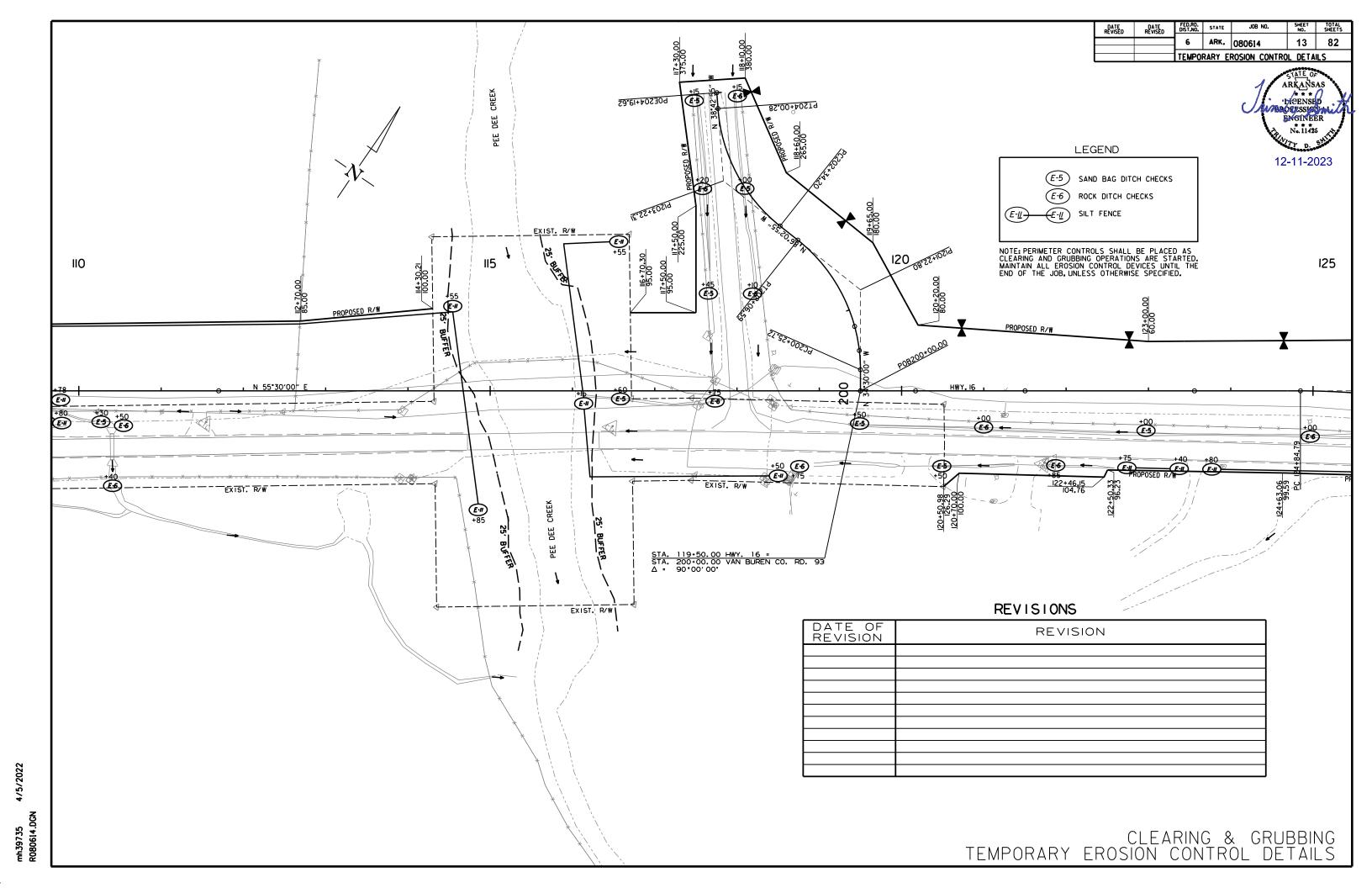
| 12' GAP | 48' RUMBLE STRIP | 12' GAP | SHOULDER

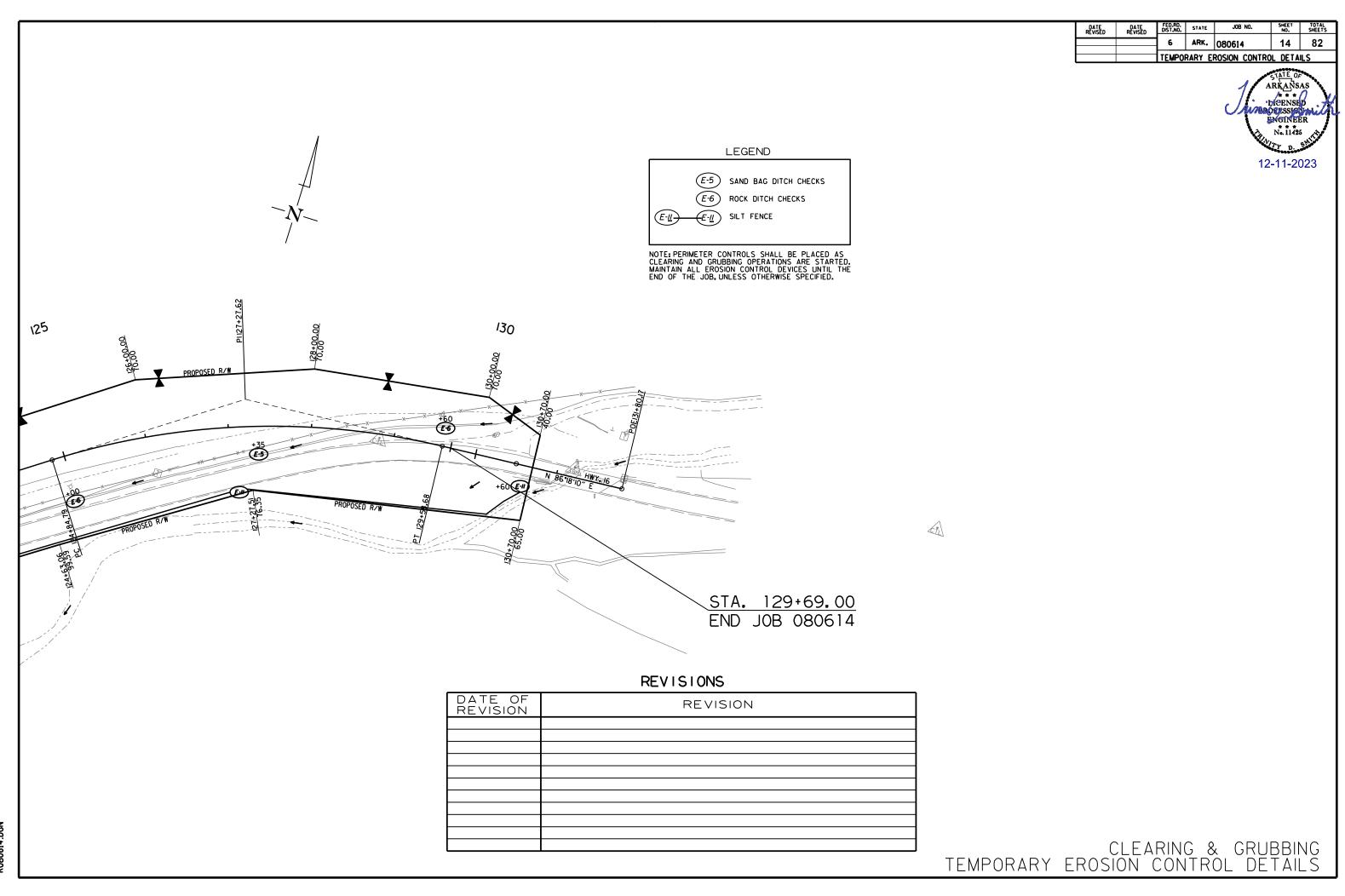
NOTE: GAP PATTERN SHALL BE ADJUSTED BY THE ENGINEER
IN THE FIELD ALLOWING FOR DRIVEWAYS TO SERVE
AS THE GAP.

DETAIL FOR GAP PATTERN RUMBLE STRIP

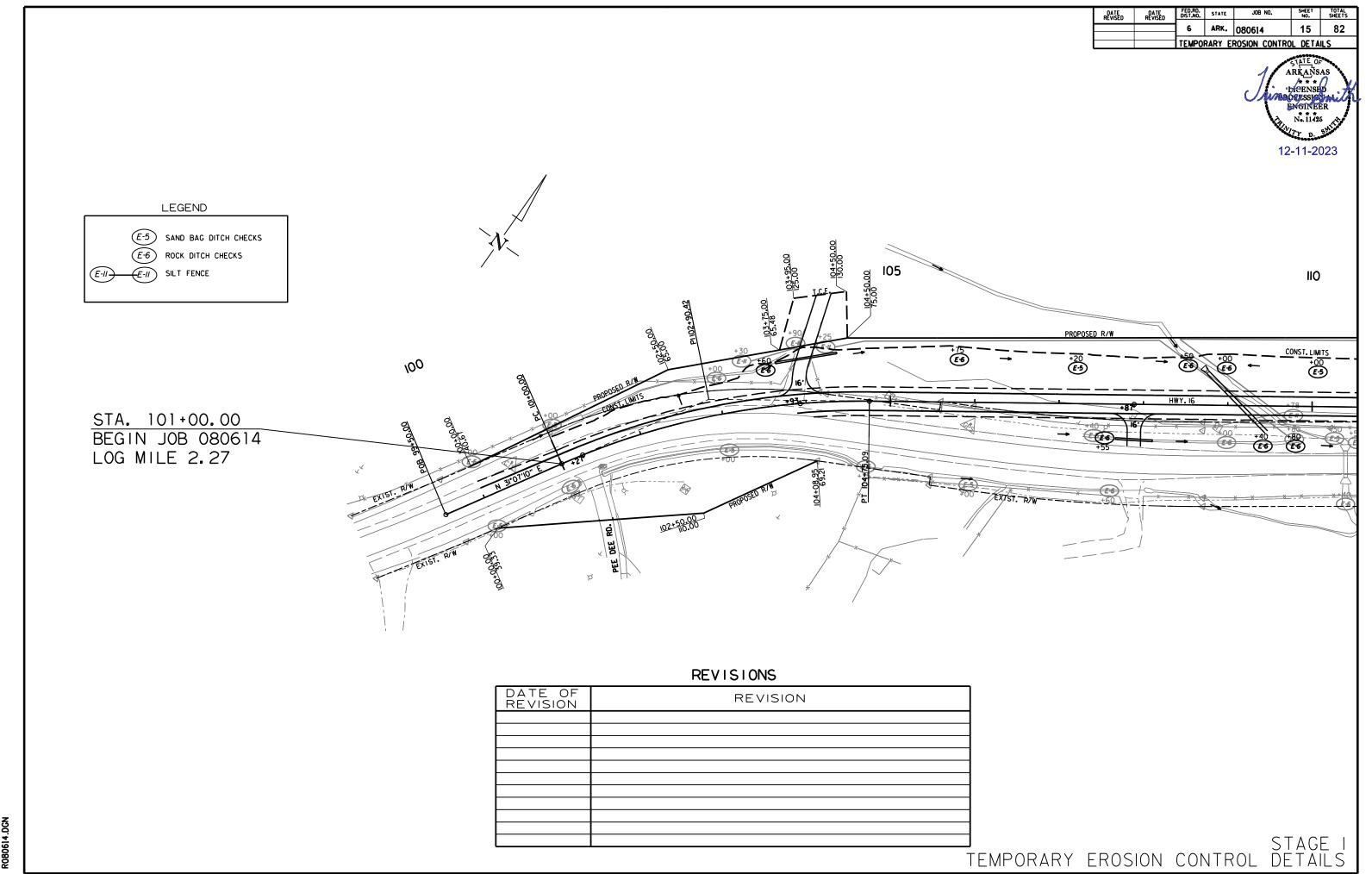
PLAN VIEW

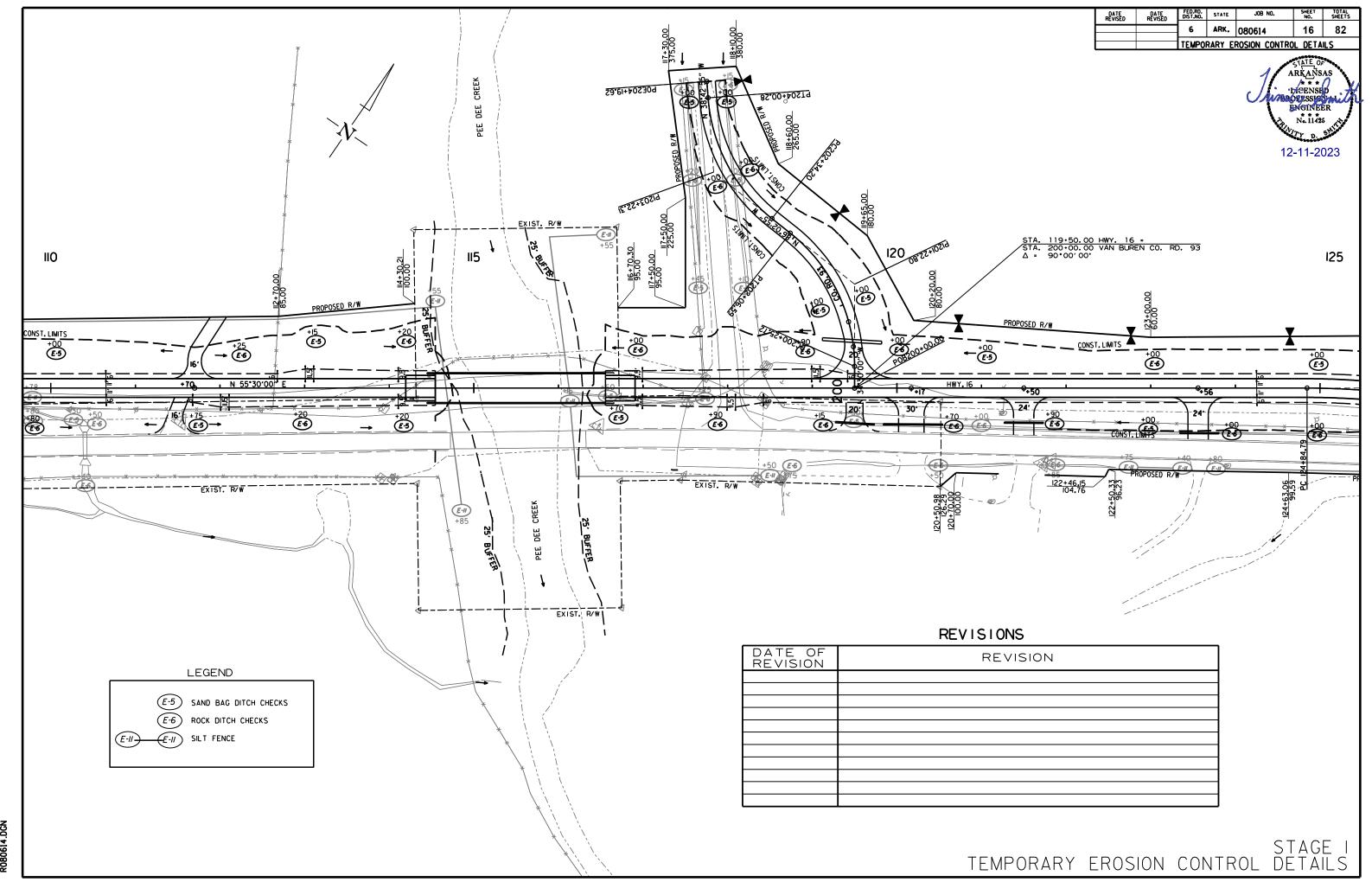


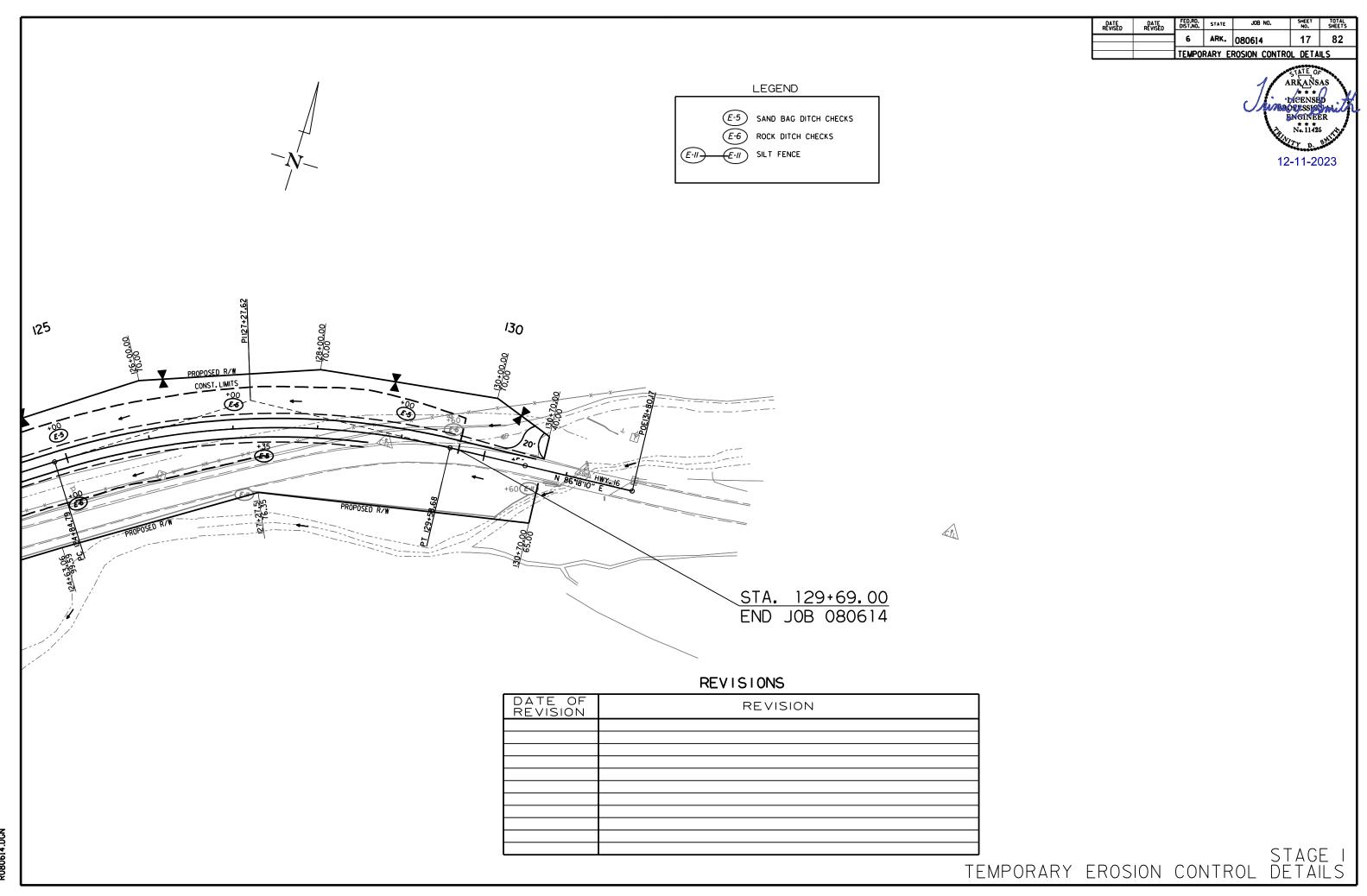




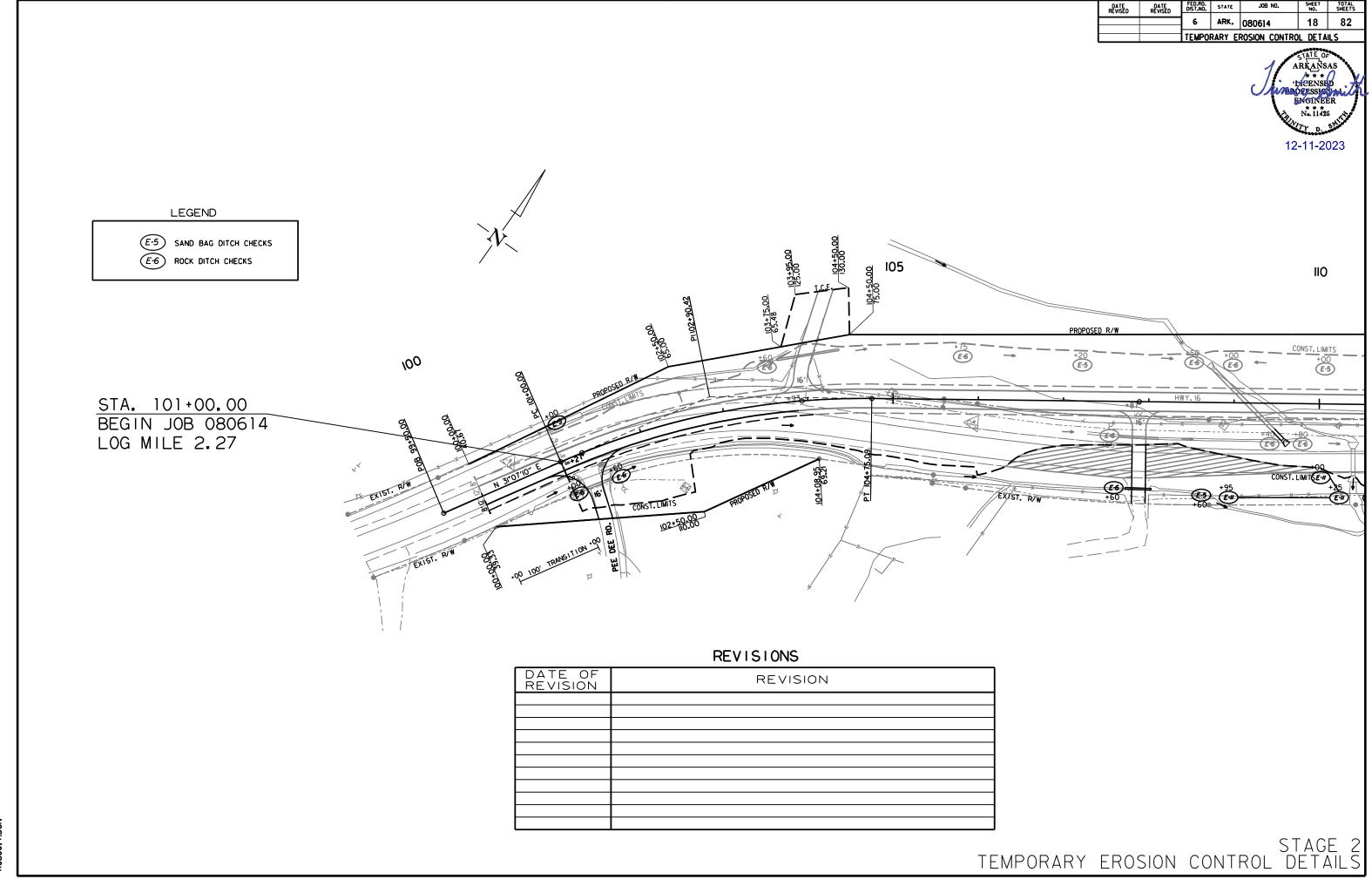
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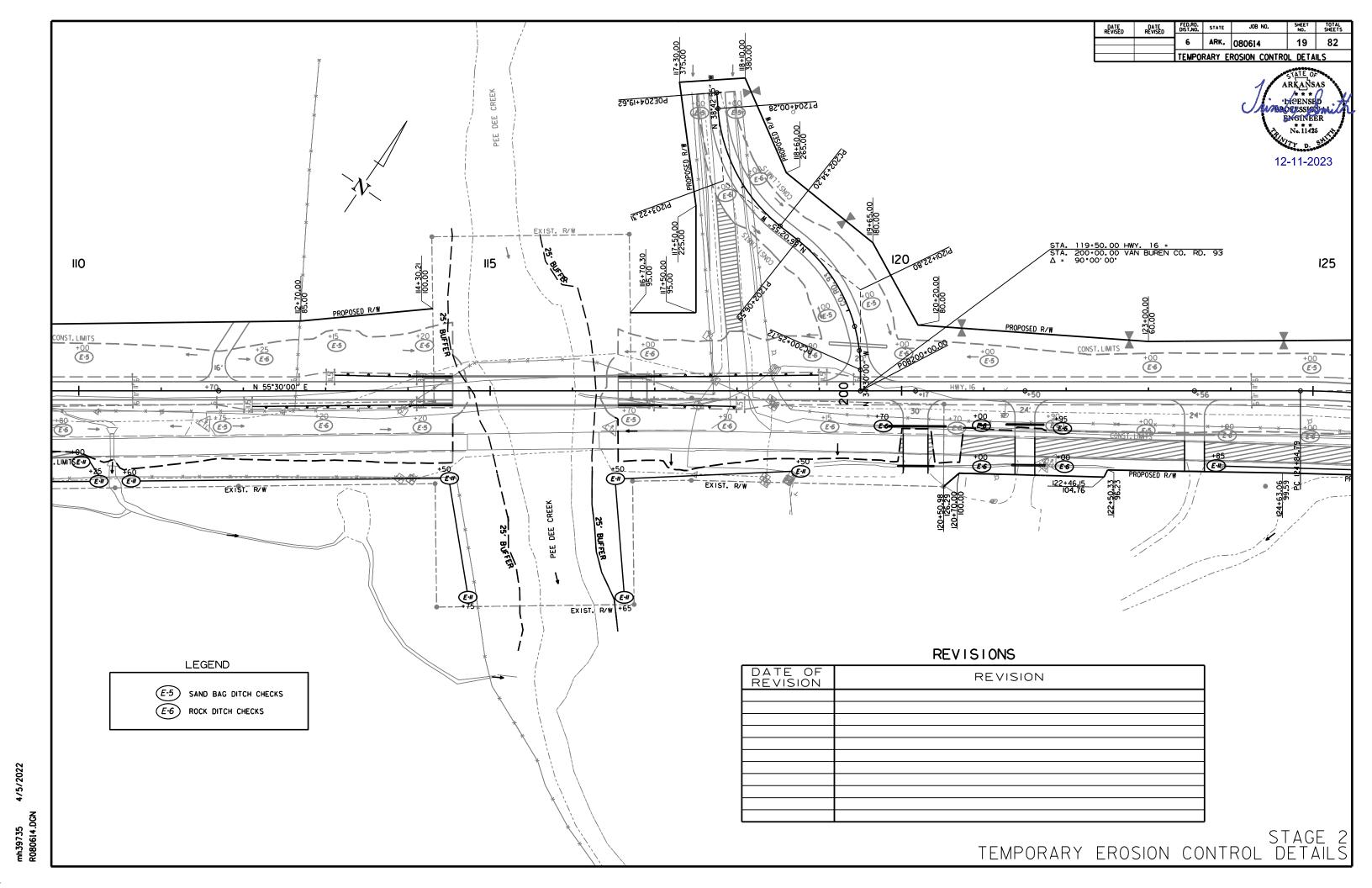


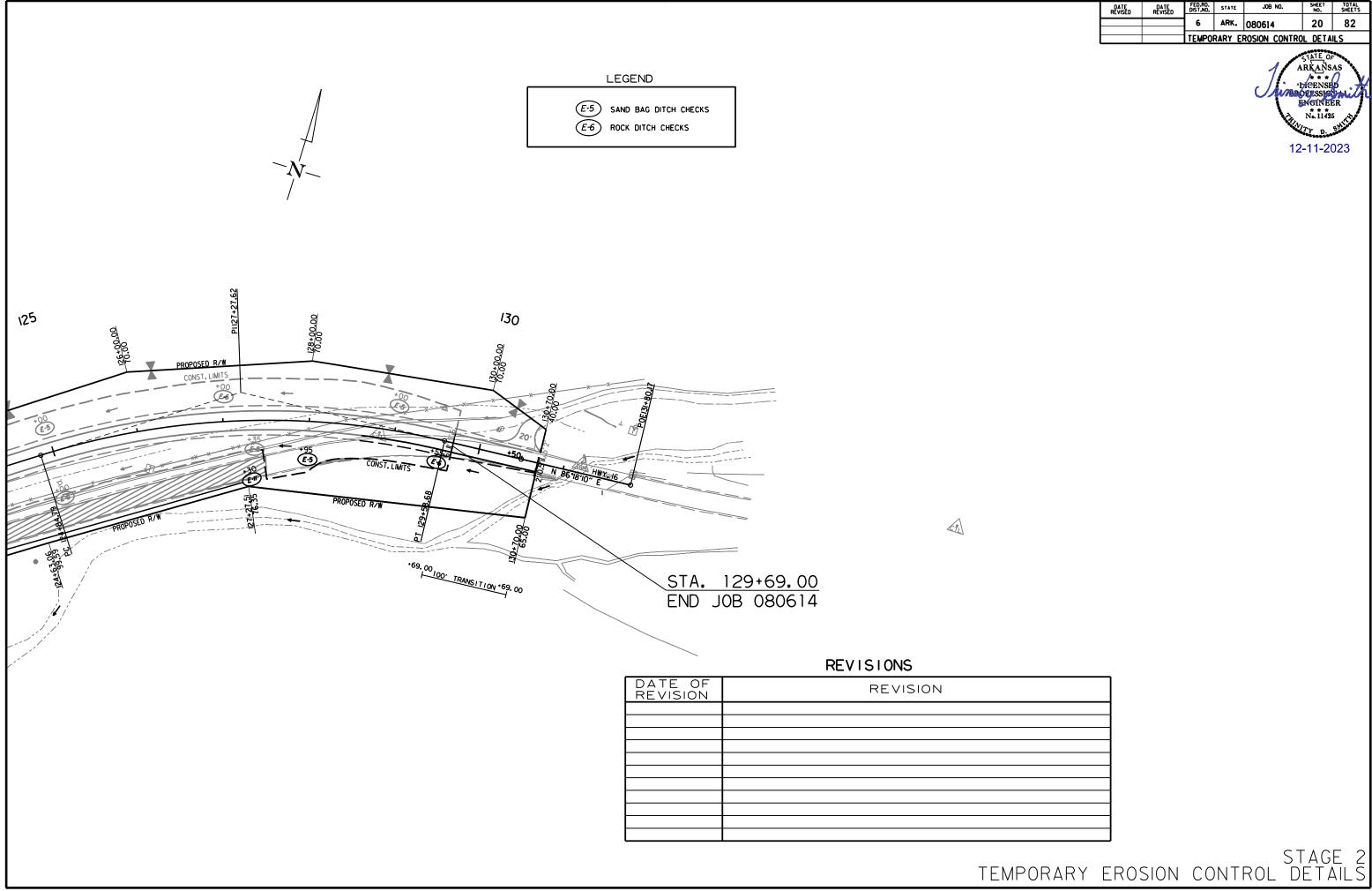


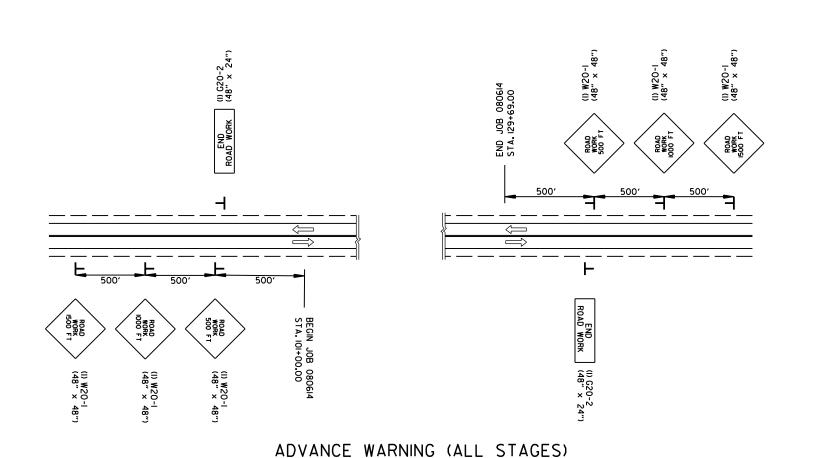


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DATE REVISED DATE DETAILS

DISTUND. STATE JOB NO. SHEET TOTAL SHEETS

ARK. 080614 21 82

MAINTENANCE OF TRAFFIC DETAILS

STATE OF ARKANSAS

DESCRIBED

MARCHESISCO

ENGINEER

No. 11425

MARCHESISCO

MARCHESISCO

NO. 11425

MARCHESISCO

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12-11-2023

STAGE I CONSTRUCTION SEQUENCE

INSTALL ADVANCE WARNING SIGNS AND END ROAD WORK SIGNS AT THE BEGINNING AND END OF JOB AS SHOWN ON THE ADVANCE WARNING DETAIL

USE VERTICAL PANELS AND TRAFFIC DRUMS SPACED 45' O.C. TO DELINEATE THE WORK ZONE. USE TRAFFIC DRUMS TO DELINEATE DRIVEWAYS.

CONSTRUCT HWY.16 LT. AND PROPOSED BRIDGE AS SHOWN IN THE STAGE IMAINTENANCE OF TRAFFIC DETAILS.

APPLY LEVELING COURSE TO EXISTING LANES IF AND WHERE DIRECTED BY THE ENGINEER.

STAGE 2 CONSTRUCTION SEQUENCE

FURNISH AND INSTALL P.C.C.B. AND T.I.A.B. AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

INSTALL CONSTRUCTION PAVEMENT MARKINGS AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

SHIFT TRAFFIC ONTO THE PROPOSED HWY. 16 LANES.

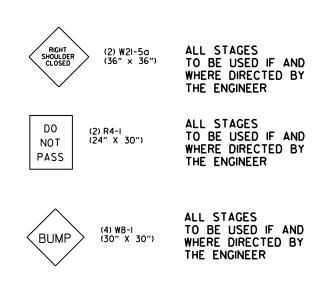
USE VERTICAL PANELS AND TRAFFIC DRUMS SPACED 45' O.C. TO DELINEATE THE WORK ZONE. USE TRAFFIC DRUMS TO DELINEATE DRIVEWAYS.

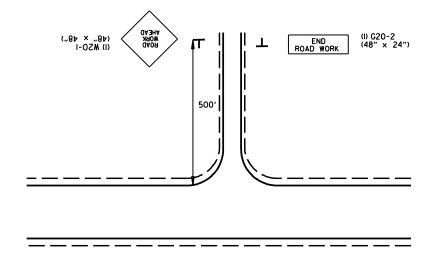
CONSTRUCT HWY.16 RT.AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS. REMOVE EXISTING BRIDGE STRUCTURE.

APPLY FINAL 2" LIFT OF ACHM SURFACE COURSE AND INSTALL PERMANENT PAVEMENT MARKINGS AS SHOWN IN THE PERMANENT PAVEMENT MARKING DETAILS.



DRIVEWAY/TRAFFIC DRUM DETAIL

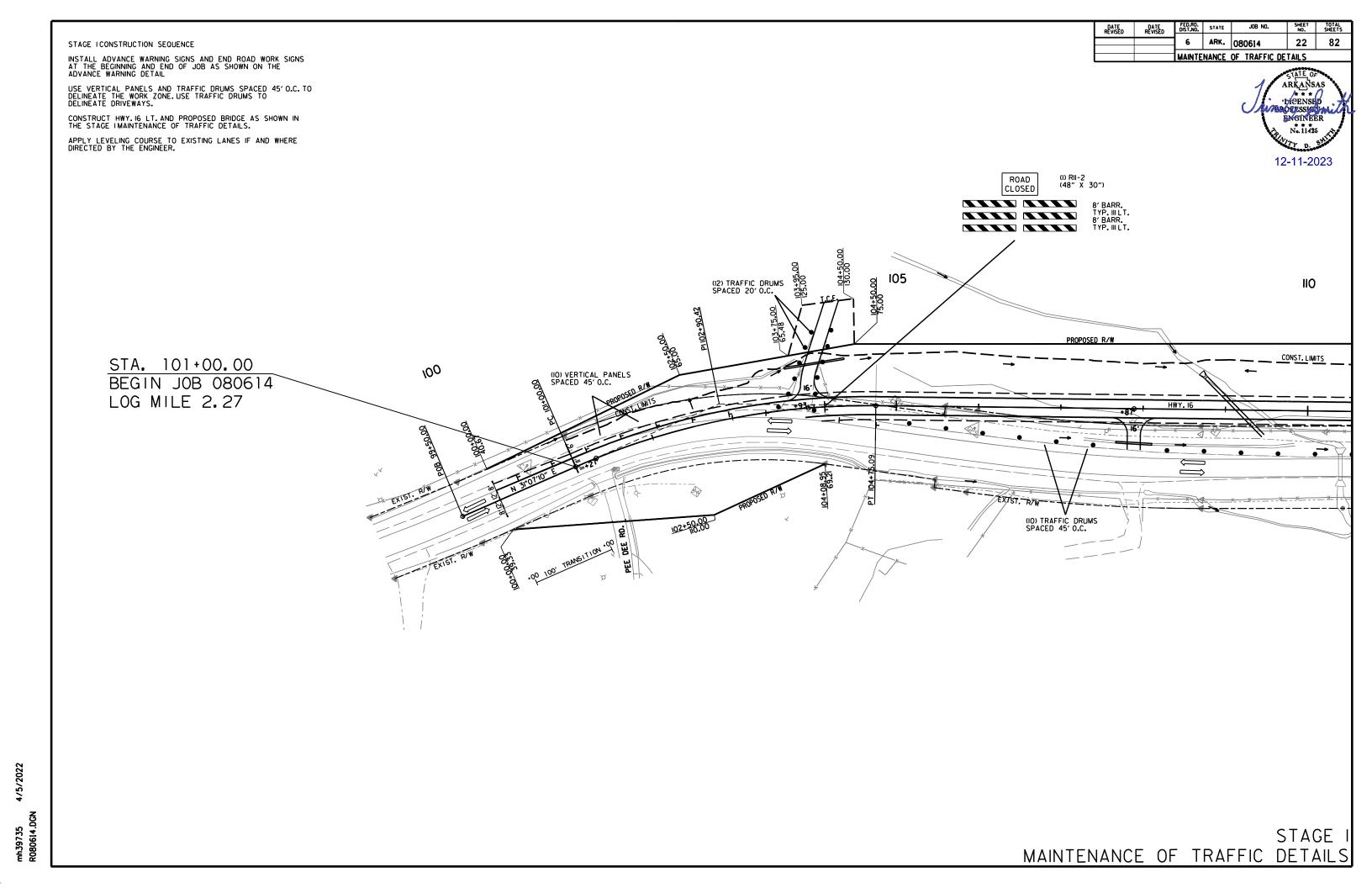


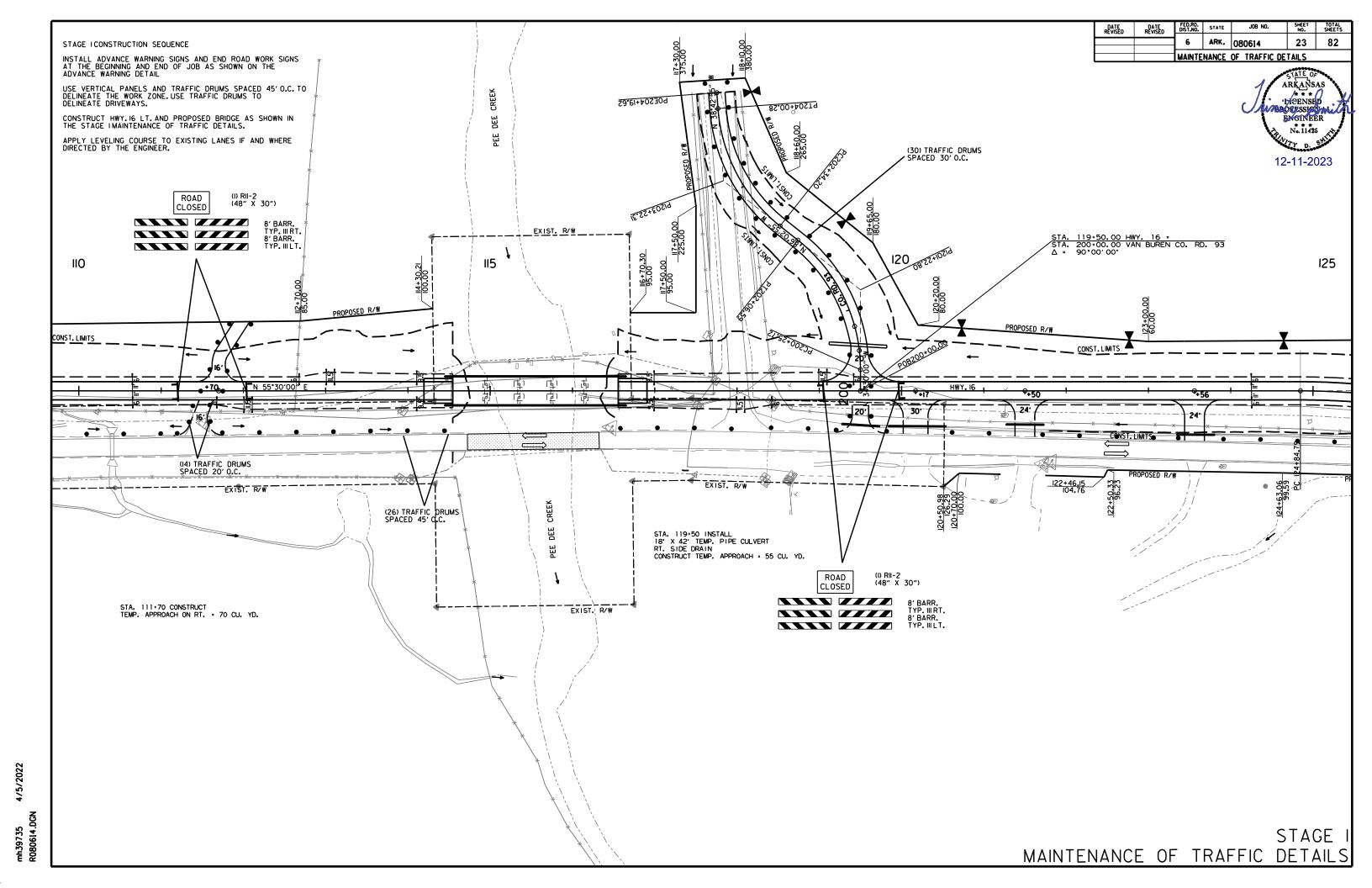


ADVANCE WARNING SIGNS

CO. RD. 93

ADVANCE WARNING SIGNS MAINTENANCE OF TRAFFIC DETAILS





STAGE I CONSTRUCTION SEQUENCE

INSTALL ADVANCE WARNING SIGNS AND END ROAD WORK SIGNS AT THE BEGINNING AND END OF JOB AS SHOWN ON THE ADVANCE WARNING DETAIL

USE VERTICAL PANELS AND TRAFFIC DRUMS SPACED 45' O.C. TO DELINEATE THE WORK ZONE. USE TRAFFIC DRUMS TO DELINEATE DRIVEWAYS.

CONSTRUCT HWY.16 LT. AND PROPOSED BRIDGE AS SHOWN IN THE STAGE IMAINTENANCE OF TRAFFIC DETAILS.

APPLY LEVELING COURSE TO EXISTING LANES IF AND WHERE DIRECTED BY THE ENGINEER.

FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED ARK. 080614 24 82 MAINTENANCE OF TRAFFIC DETAILS

ARKANSAS

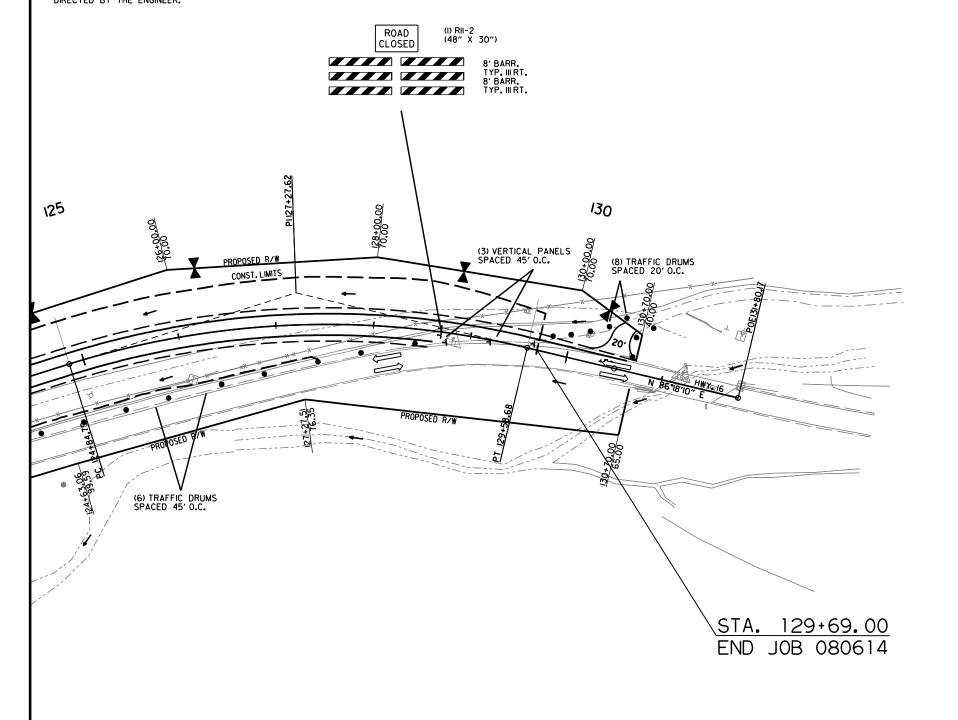
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No. 11425

12-11-2023





STAGE 2 CONSTRUCTION SEQUENCE

FURNISH AND INSTALL P.C.C.B. AND T.L.A.B. AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

INSTALL CONSTRUCTION PAVEMENT MARKINGS AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

SHIFT TRAFFIC ONTO THE PROPOSED HWY. 16 LANES.

USE VERTICAL PANELS AND TRAFFIC DRUMS SPACED 45' O.C. TO DELINEATE THE WORK ZONE. USE TRAFFIC DRUMS TO DELINEATE DRIVEWAYS.

CONSTRUCT HWY. 16 RT. AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS. REMOVE EXISTING BRIDGE STRUCTURE.

APPLY FINAL 2" LIFT OF ACHM SURFACE COURSE AND INSTALL PERMANENT PAVEMENT MARKINGS AS SHOWN IN THE PERMANENT PAVEMENT MARKING DETAILS.

DATE REVISED FED.AR. STATE JOB NO. SHEET TOTAL SHEETS

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MAINTENANCE OF TRAFFIC DETAILS

ARKANSAS

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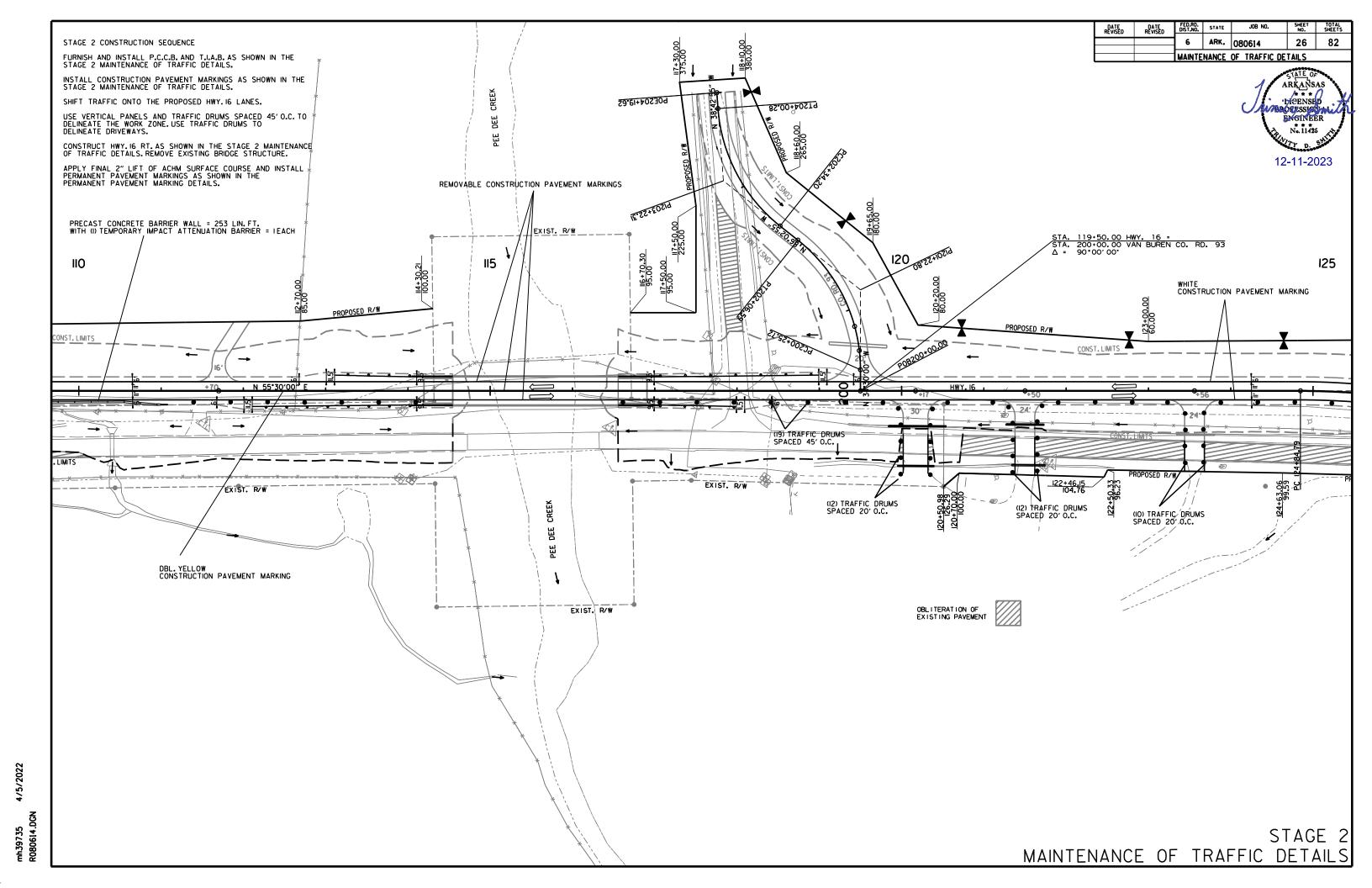
ENGINEER

No. 11425

12-11-2023

OBLITERATION OF EXISTING PAVEMENT

PRECAST CONCRETE BARRIER WALL = 253 LIN.FT.
WITH (1) TEMPORARY IMPACT ATTENUATION BARRIER = IEACH 105 IIO DBL. YELLOW CONSTRUCTION PAVEMENT MARKING PROPOSED R/W STA. 101+00.00 BEGIN JOB 080614 CONST. LIMITS 100 LOG MILE 2.27 EXIST. RIW (7) TRAFFIC DRUMS SPACED 45' O.C. (7) VERTICAL PANELS SPACED 45' O.C. SPACED 45' O.C. 7 .00 100. TRANSITION .00 B (12) TRAFFIC DRUMS SPACED 20' O.C. WHITE CONSTRUCTION PAVEMENT MARKING ROAD CLOSED (I) RII-2 (48" X 30")



STAGE 2 CONSTRUCTION SEQUENCE

FURNISH AND INSTALL P.C.C.B. AND T.I.A.B. AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

INSTALL CONSTRUCTION PAVEMENT MARKINGS AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

SHIFT TRAFFIC ONTO THE PROPOSED HWY. 16 LANES.

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CONSTRUCT HWY.16 RT. AS SHOWN IN THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS. REMOVE EXISTING BRIDGE STRUCTURE.

APPLY FINAL 2" LIFT OF ACHM SURFACE COURSE AND INSTALL PERMANENT PAVEMENT MARKINGS AS SHOWN IN THE PERMANENT PAVEMENT MARKING DETAILS.

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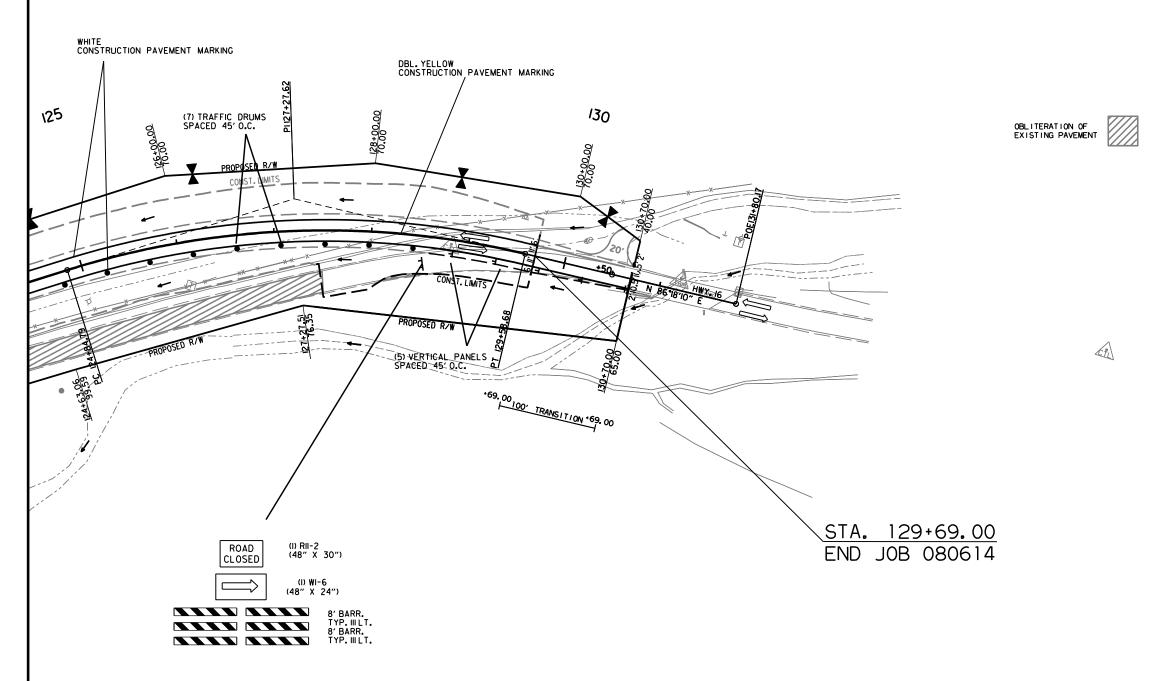
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12-11-2023



PERMANENT PAVEMENT MARKINGS

THERMOPLASTIC PAVEMENT MARKINGS WHITE (6") = 6138 LIN.FT. THERMOPLASTIC PAVEMENT MARKINGS YELLOW (6") = 6138 LIN.FT.

REFLECTORIZED PAINT PAVEMENT MARKINGS YELLOW (6") = 820 LIN. FT.

RAISED PAVEMENT MARKERS (TYPE II)(YELLOW/YELLOW)(80' O.C.) = 39 EACH

•THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT. THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING. CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
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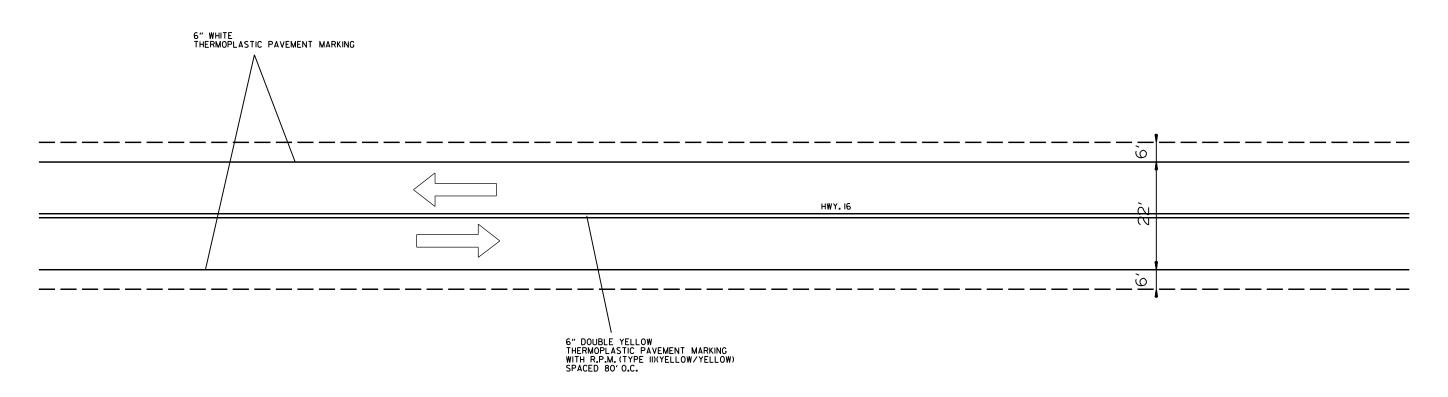
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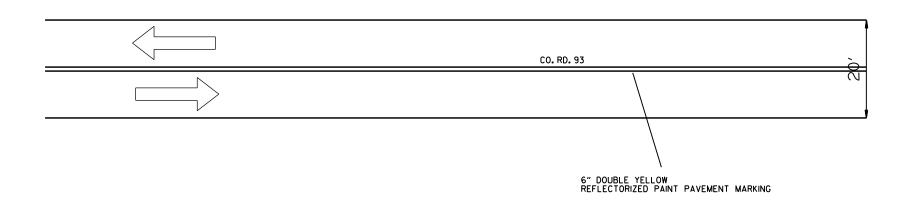
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12-11-2023



6" WHITE
THERMOPLASTIC PAVEMENT MARKING



DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
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ADVANCE WARNING SIGNS AND DEVICES

SIGN NUMBER	DESCRIPTION	SIGN SIZE	STAGE 1	STAGE 2	MAXIMUM NUMBER REQUIRED	TOTAL SIGN	S REQUIRED	VERTICAL PANELS	TRAFFIC DRUMS	BARRICADES (TYPE III)		FURNISHING & INSTALLING PRECAST CONC. BARRIER	TEMPORARY IMPACT ATTENUATION BARRIER	TEMP. IMPACT ATTEN.BARR. (REPAIR)
				L	KEGOIKED				<u> </u>					
			LIN. FT	EACH		NO.	SQ. FT.	EA	СН		LIN. F	т.	EA	СН
W20-1	ROAD WORK 1500 FT.	48"x48"	3	3	3	3	48.0							
W20-1	ROAD WORK 1000 FT.	48"x48"	3	3	3	3	48.0							
W20-1	ROAD WORK 500 FT.	48"x48"	3	3	3	3	48.0							
W20-1	ROAD WORK AHEAD	48"x48"	3	3	3	3	48.0							
WI-6	ARROW	48"x24"		2	2	2	16.0							
G20-2	END ROAD WORK	48"x24"	3	3	3	3	24.0							
R11-2	ROAD CLOSED	48"x30"	6	2	6	6	60.0							
R2-1	SPEED LIMIT 45 MPH	24"x30"	2	2	2	2	10.0							
R4-1	DO NOT PASS	24"x30"	2	2	2	2	10.0							
W21-5A	RIGHT SHOULDER CLOSED	36"x36"	2	2	2	2	18.0							
W8-1	BUMP	30"x30"	2	2	2	2	12.5							
SPECIAL	WORK WITH US SIGN (USE CAUTION, SLOW DOWN)	120"x60"	2	2	2	2	100.0							
	VERTICAL PANELS		13	12	13			13						
	TRAFFIC DRUMS		106	89	106				106					
	TYPE III BARRICADE-RT. (8')		6	2	6					48				
	TYPE III BARRICADE-LT. (8')		6	2	6						48			
	FURNISHING AND INSTALLING PRECAST CONCRETE BARRIER			253	253						_	253		
	TEMPORARY IMPACT ATTENUATION BARRIER			1	1	1				1			1	
	TEMPORARY IMPACT ATTENUATION BARRIER (REPAIR)			1	1									1
						-					_			
TOTALS:	·		•			<u> </u>	442.5	13	106	48	48	253	1	1

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

CONSTRUCTION PAVEMENT MARKINGS AND PERMANENT PAVEMENT MARKINGS

CONSTRUCTION		CIVI WAKKIN	IGS AND PERINA	NENI PAVEMEN	I WAKKINGS			
DESCRIPTION	STAGE 2	STAGE 2 END OF JOB	CONSTRUCTION PAVEMENT MARKINGS	REMOVABLE CONSTRUCTION PAVEMENT	RAISED PAVEMENT MARKERS	THERMOPLASTIC PAVEMENT MARKING		REFLECTORIZED PAINT PAVEMENT MARKING 6"
				MARKINGS	(YELLOW/YELLOW)	WHITE	YELLOW	YELLOW
	LIN. F	T EACH	LIN. FT.	LIN. FT.	EACH	LIN. FT.		
CONSTRUCTION PAVEMENT MARKINGS	11192		11192					
REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	1084			1084				
RAISED PAVEMENT MARKERS TYPE II (YELLOW/YELLOW)		39			39			
THERVOPLASTIC PAVEMENT MARKING WHITE (6")		6138				6138		
THERNOPLASTIC PAVEMENT MARKING YELLOW (6")		6138					6138	
REFLECTORIZED PAINT PAVEMENT MARKING YELLOW (6")		820			<u> </u>			820
					<u> </u>			
TOTALS:		-	11192	1084	39	6138	6138	820

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

NOTE: THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT.
THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING.
CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

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12-11-2023

CLEARING AND GRUBBING

STATION STATION LOCATION	LOCATION	CLEARING	GRUBBING				
			STATION				
101+00	130+69	HWY. 16	30	30			
TOTALS:			30	30			

REMOVAL AND DISPOSAL OF FENCE

STATION	STATION	LOCATION	FENCE
			LIN. FT.
105+28	114+27	HWY. 16 LT.	899
112+64	112+69	HWY. 16 RT.	5
114+27	117+31	HWY. 16 RT.	304
117+31	117+72	HWY. 16 RT.	41
200+00	202+36	CO. RD. 293 LT.	236
200+00	202+84	CO. RD. 293 LT.	284
202+84	204+20	CO. RD. 293 RT.	136
TOTAL:		I .	1905

REMOVAL AND DISPOSAL OF CUI VERTS

REMOVAL AND DISPOSAL OF COLVERTS							
STATION	DESCRIPTION	PIPE CULVERTS	BOX CULVERTS				
		EACH	EACH				
101+27	HWY. 16 RT.	1					
101+41	HWY. 16 RT.	1					
107+87	HWY. 16 RT.	1					
110+40	HWY. 16 RT.		1				
118+03	HWY. 16 LT.	1					
120+17	HWY. 16 RT.	1					
121+50	HWY. 16 RT.	1					
TOTAL:		6	1				

NOTE: QUANTITIES SHOWN ABOVE SHALL INCLUDE REMOVAL & DISPOSAL OF ALL HEADWALLS AND FLARED END SECTIONS IF APPLICABLE.

SOIL STABILIZATION

			SOIL
STATION	STATION	LOCATION / DESCRIPTION	STABILIZATION
			TON
ENTIRE	PROJECT	TO BE USED IF AND WHERE	100
		DIRECTED BY THE ENGINEER	
TOTAL:		_	100

QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

FARTHWORK

		LAKINWOKK		
			UNCLASSIFIED	COMPACTED
STATION	STATION	LOCATION / DESCRIPTION	EXCAVATION	EMBANKMENT
			CU.	YD.
ENTIRE	PROJECT	STAGE 1-MAIN LANES	4080	27247
ENTIRE	PROJECT	STAGE 2-MAIN LANES	4572	263
ENTIRE	PROJECT	APPROACHES	50	2635
200+11.00	204+20.00	CO. RD. 93	464	2133
ENTIRE	PROJECT	ADD'L FOR BRIDGE EXCAVATION	760	
ENITRE	PROJECT	OBLITERATION OF EXIST. PAVEMENT	1426	
TOTALS:	•		11352	32278

NOTE: EARTHWORK QUANTITIES SHOWN ABOVE SHALL BE PAID AS PLAN QUANTITY.

EROSION CONTROL MATTING

STATION	STATION	LOCATION	LENGTH	CLASS 3
			LIN. FT.	SQ. YD.
104+00.00	105+71.75	HWY. 16 LT.	171.75	152.67
108+00.00	108+72.00	HWY. 16 LT.	72.00	64.00
111+70.00	114+54.50	HWY. 16 LT.	284.50	252.89
128+55.00	129+69.00	HWY. 16 LT.	114.00	101.33
TOTAL:	570.89			

NOTE: AVERAGE WIDTH = 8'-0"

GUARDRAIL

STATION	STATION	TION LOCATION		INAIL	GUARDRAIL (TYPE A)	THRIE BEAM GUARDRAIL TERMINAL	GUARDRAIL TERMINAL (TYPE 2)
					LIN. FT.		EACH
113+11.35	114+55.10	LT. SIDE			75	1	1
112+10.75	114+55.10	RT. SIDE			150	1	1
116+54.90	118+99.05	LT. SIDE	-		150	1	1
116+54.90	117+98.65	RT. SIDE	·		75	1	1
TOTALS:					450	4	4

4" PIPE UNDERDRAIN

	T I II E GIBERBITAIN										
STATION	STATION	LOCATIONS	4" PIPE UNDERDRAINS	UNDERDRAIN OUTLET PROTECTORS							
			LIN. FT.	EACH							
ENTIRE	PROJECT	TO BE USED IF AND WHERE	1500	6							
		DIRECTED BY THE ENGINEER									
TOTALS:			1500	6							

* NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

SOIL LOG

3012 200																																				
STATION	LATITUDE		LATITUDE		LATITUDE		LATITUDE		LATITUDE		LATITUDE								LATITUDE		LATITUDE		LONGITUDE		LONGITUDE		LONGITUDE		LONGITUDE		LOCATION	DEPTH	LIQUID	PLASTICITY	AASHTO	COLOR
	DEG	MIN	SEC	DEG	MIN	SEC		FEET	LIMIT	INDEX	CLASSIFICATION																									
101+00	35	36	8.90	92	25	17.50	HWY. 16 6' RT.					BROWN																								
101+00	35	36	8.90	92	25	17.50	HWY. 16 22' RT.	0.5	23	9	A-2-4(0)	BROWN																								
117+00	35	36	18.70	92	25	1.30	HWY. 16 80' LT.	0.5	ND	NP	A-2-4(0)	BROWN																								
131+00	35	36	24.40	92	24	46.80	HWY. 16 30' RT.	0.5	ND	NP	A-1-2	BROWN																								

SOIL CHARACTERISTICS TABULATED ABOVE ARE REPRESENTATIVE AT THE LOCATION OF THE SAMPLE, AND FROM SURFACE INDICATIONS ARE TYPICAL FOR THE LIMITS SHOWN. THESE DATA ARE SHOWN FOR INFORMATION CNLY. THE STATE WILL NOT BE RESPONSIBLE FOR VARIATIONS IN THE SOIL CHARACTERISTICS AND/OR EXTENT OF SAME DIFFERING FROM THE ABOVE TABULATIONS.

Z- AUGER REFUSAL

NP - NON-PLASTIC ND - NOT DETERMINABLE

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DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS				
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					STATE OF					

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FENCING

I LINGING									
STATION	STATION	LOCATION	WIRE FENCE						
	STATION	LOCATION	(TYPE D-1)						
			LIN. FT.						
120+20	130+15	HWY. 16 LT	1033						
200+65	204+20	CO. RD. 93 RT.	375						
TOTALS:	1408								

COLD MILLING ASPHALT PAVEMENT

	COLD MILLING ASPHALT PAVEMENT											
STATION	STATION	LOCATION	AVG. WIDTH	COLD MILLING ASPHALT PAVEMENT								
			FEET	SQ. YD.								
100+00.00	101+00.00	MAIN LANES	37.00	411.11								
129+69.00	130+69.00	MAIN LANES	21.00	233.33								
TOTAL:		644.44										
	1411 1 1110 0 0 0 0 0	OVER ELECTION										

NOTE: COLD MILLING STOCKPILE LOCATION VAN BUREN COUNTY SHOP, 438 OLD HWY. 9, CLINTON, AR 72031

APPROACH GUTTERS AND SLABS

STATION	STATION	LOCATION	APPROACH GUTTER (TYPE F)	APPROACH SLABS (TYPE SPECIAL)	REINFORCING STEEL-RDWY. (GR. 60)	AGGREGATE BASE CRS. (CLASS 7)
			CU.YD.	CU.YD.	POUND	TON
114+19.50	114+54.50	HWY. 16 LT. SIDE	4.20		210	
114+19.50	114+54.50	HWY. 16 RT. SIDE	4.20		210	
114+19.50	114+54.50	HWY. 16		59.30	7140	19.60
116+55.50	116+90.50	HWY. 16 LT. SIDE	4.20		210	
116+55.50	116+90.50	HWY. 16 RT. SIDE	4.20		210	
116+55.50	116+90.50	HWY. 16		59.30	7140	19.60
TOTALS:			16.80	118.60	15120	39.20

NOTE: USE T =14" FOR 6' SHOULDER.

BENCH MARKS

BENOTIMATO									
STATION	LOCATION	BENCH MARKS							
		EACH							
114+55	LT SIDE OF BRIDGE	1							
TOTAL:		1							

NOTE: SHOWN FOR INFORMATION ONLY, BENCH MARKS SHALL BE FURNISHED AND PLACED BY STATE FORCES.

DRIVEWAYS & TURNOUTS

STATION	SIDE	LOCATION	WIDTH	PORTLAND CEMENT CONCRETE DRIVEWAY	ACHM S COURSE (1/ PER SQ. YD		AGGREGATE BASE COURSE (CLASS 7)		SIDE DRAINS			STANDARD DRAWINGS
			FEET	SQ. YD.	SQ. YD.	TON	TON	18"		. FT.	42"	-
101+27	RT	HWY. 16 - PEE DEE RD.	16	OQ. 1D.	165.73	18.23	67.67		<u> </u>	42	Г	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
103+93	LT	HWY. 16	16		30.43	3.35	93.67		74	72		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
107+87	RT	HWY. 16	16		30.43	3.35	76.03	 	42			PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
107+87	RT	HWY. 16	16		30.10	0.00	7 0.00		28			PCC-1. PCM-1. PCP-1. PCP-2. PCP-3
111+70	LT	HWY. 16	16		30.43	3.35	75.10					1 00 1,1 0111 1,1 01 1,1 01 2,1 01 0
111+70	RT	HWY. 16 - TEMP. ACCESS	16				30.80					
119+50	LT	HWY. 16 - CO. RD. 93	20		938.04	108.18	383.03				70	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
119+50	RT	HWY. 16 - TEMP. ACCESS	20				16.80	46				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
120+17	RT	HWY. 16	30		52.21	5.74	150.98	70				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
120+17	RT	HWY. 16	30					44				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
121+50	RT	HWY. 16	24		299.18	32.91	122.17	46				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
121+50	RT	HWY. 16	24					42				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
123+56	RT	HWY. 16	24	220.80				46				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
130+50	LT	HWY. 16	20		36.65	4.03	20.80					
* ENTIRE PRO	ENTIRE PROJECT TEMPORARY DRIVES						100.00					
TOTALS:				220.80	1583.10	179.1 ₄	1137.05	294	144	42	70	

BASIS OF ESTIMATE: ACHM SURFACE COURSE (1/2").....94.4% MIN. AGGR......5.6% ASPHALT BINDER MAXIMUM NUMBER OF GYRATIONS = 115 FOR PG 64-22

* QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED. NOTE: FOR C.M. PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED. **MAILBOXES**

	MAILBOXES	MAILBOX SUPPORTS					
LOCATION	WAILBUXES	(SINGLE)	(DOUBLE)				
	EACH						
ENTIRE PROJECT	4	2 1					
TOTALS:	4	2	1				

ACHM PATCHING OF EXISTING ROADWAY

DESCRIPTION	TON
ENTIRE PROJECT - TO BE USED IF AND WHERE	15
DIRECTED BY THE ENGINEER	
TOTAL:	15

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC

IVIAINTENANCE OF TRAI	· FIC	
LOCATION NTIRE PROJECT - TO BE USED IF AND WHERE	TON	TACK COAT
		GALLON
ENTIRE PROJECT - TO BE USED IF AND WHERE	15	30
DIRECTED BY THE ENGINEER		
TOTALS:	15	30

BASIS OF ESTIMATE:

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC...25 TON/MILE TACK COAT FOR MAINTENANCE OF TRAFFIC.....

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	080614	32	82
		OUANT	TIES			

ARKANSAS

DICENSED

NOTINEER

No. 11425

No. 11425

12-11-2023

RUMBLE STRIPS IN ASPHALT SHOULDERS

STATION	STATION	LOCATION	* RUMBLE STRIPS IN ASPHALT SHOULDERS
			LIN.FT.
101+00	114+20	RIGHT OF MAIN LANES	1320
116+91	129+69	RIGHT OF MAIN LANES	1278
101+00	114+20	LEF⊺ OF MAIN LANES	1320
116+91	129+69	LEF⊺ OF MAIN LANES	1278
TOTAL:		·	5196

^{*} QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.
TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

CONCRETE DITCH PAVING

STATION	STATION	LOCATION	LENGTH	"W"	(TYPE B)	SOLID	WATER
STATION	STATION	ECCATION	LIN. FT.	FEET	SQ. YD.	SODDING SQ. YD.	M. GAL.
103+30.00	104+00.00	HWY. 16 LT.	70.00	6.32	49.16	31.11	0.39
108+35.00	109+57.00	HWY. 16 RT.	122.00	6.32	85.67	54.22	0.68
110+40.00	111+70.00	HWY. 16 RT.	130.00	6.32	91.29	57.78	0.73
116+50.00	117+00.00	HWY. 16 RT.	50.00	6.32	35.11	22.22	0.28
TOTALS:					261.23	165.33	2.08

BASIS OF ESTIMATE:

WATER......12.6 GAL. / SQ YD. OF SOLID SODDING.

STRUCTURES

			PIPE CULVERT ALTERNATES	FLARED END SECTION ALTERNATES FOR PIPE		WATER	
STATION	DESCRIPTION	ALT. 1 (CLASS V)	ALT. 2, 3, 4, 5, 6, AND 7 (WITH CLASS V ALT. 1)	CULVERT ALTERNATES	SODDING	WATER	STD. DWG. NOS.
		48"	48"	48"			
			LIN. FT.	EACH	SQ.YD.	M.GAL.	
109+15	CONST. 48" x 120' PIPE CULVERT	110	114	114 2			FES-1, FES-2, PCC-1, PCM-1, PCP-1. PCP-2, PCP-3
TOTALS:		110	114	2	29	0.37	

BASIS OF ESTIMATE:

WATER......12.6 GAL. / SQ. YD. OF SOLID SODDING

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED.

NOTE: FOR C.M. PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.

SELECTED PIPE BEDDING

SELECTED FIFE BEDDI	NG
LOCATION	SELECTED PIPE BEDDING
	CU.YD.
ENTIRE PROJECT TO BE USED IF	
AND WHERE DIRECTED BY THE	50
ENGINEER	
TOTAL:	50
NOTE: QUANTITY ESTIMATED	

NOTE: QUANTITY ESTIMATED.
SEE SECTION 104.03 OF THE STD. SPECS.

EROSION CONTROL

				PERMAN	NENT EROSION	N CONTROL					TEMF	PORARY EROSIG	ON CONTROL			
STATION	STATION	LOCATION	SEEDING	LIME	MULCH COVER	WATER	SECOND SEEDING	TEMPORARY SEEDING	MULCH COVER	WATER	SAND BAG DITCH CHECKS	ROCK DITCH CHECKS	SILT FENCE	SEDIMENT BASIN	OBLITERATION OF SEDIMENT	*SEDIMENT REMOVAL &
							APPLICATION				(E-5)	(E-6)	(E-11)	(E-14)	BASIN	DISPOSAL
			ACRE	TON	ACRE	M.GAL.	ACRE	ACRE	ACRE	M.GAL.	BAG	CU.YD.	LIN. FT.	CU.YD.	CU.YD.	CU. YD.
ENTIRE	PROJECT	CLEARING AND GRUBBING						10.63	10.63	216.9	308	51	4050			181
ENTIRE	PROJECT	STAGE 1						6.39	6.39	130.4	330	69				38
ENTIRE	PROJECT	STAGE 2	5.23	10.46	5.23	533.5	5.23	4.24	4.24	86.5	66	27	1435			65
*ENTIRE PRO	DJECT TO BE (JSED IF AND WHERE DIRECTED BY THE ENGINEER.	2.62	5.24	2.62	267.2	2.62	5.32	5.32	108.5	176	37	1371	300	300	351
TOTALS:	ı	ı	7.85	15.70	7.85	800.7	7.85	26.58	26.58	542.3	880	184	6856	300	300	635

BASIS OF ESTIMATE:

 LIME
 2 TONS / ACRE OF SEEDING

 WATER.
 102.0 M.G. / ACRE OF SEEDING

 WATER.
 20.4 M.G. / ACRE OF TEMPORARY SEEDING

 WATER.
 12.6 GAL. / SQ. YD. OF SOLID SODDING

SAND BAG DITCH CHECKS......22 BAGS / LOCATION ROCK DITCH CHECKS......3 CU.YD./LOCATION

NOTE: THE TEMPORARY EROSION CONTROL DEVICES SHOWN ABOVE AND ON THE PLANS SHALL BE INSTALLED IN SUCH A SEQUENCE AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM PERMIT.

*QUANTITIES ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
		6	ARK.	080614	33	82
		OUANT	TIES			

BASE AND SURFACING

										BASE	E AND SUF	REACING													
			LENGTH		ATE BASE (CLASS 7)				TACK COAT					ACHM BINDE	R COURSE (1	")				ACHMS	JRFACE COUI	RSE (3/8")			
STATION	STATION	LOCATION	LENGTH	TON / STATION	TON	(0.05 TOTAL WID.	GAL. PER SC SQ.YD.	GALLON	(0.17 TOTAL WID.	GAL. PER SO SQ.YD.	i	TOTAL GALLONS	AVG. WID.	SQ.YD.	POUND /	PG 64-22	AVG. WID.	SQ.YD.	POUND / SQ.YD.	PG 64-22	AVG. WID.	SQ.YD.	POUND /	PG 64-22	TOTAL PG 64-22
			FEET	STATION		FEET	SQ.TD.	GALLON	FEET	SQ.1D.	GALLON	GALLONS	FEET	1	SQ.YD.	TON	FEET		SQ.TD.	TON	FEET		SQ.YD.	TON	TON
	LANES								,																
100+00.00		HWY. 16 TRANSITION	100.00						37.00	411.11	69.89	69.89									37.00	411.11	220.00	45.22	45.22
101+00.00	103+74.62	HWY. 16 NOTCH AND WIDEN	274.62	VAR.	416.35	VAR.	429.04	21.45	-			21.45	VAR.	216.43	440.00	47.61	VAR.	212.61	220.00	23.39	34.00	1037.45	220.00	114.12	137.51
103+74.62		HWY. 16 FULL DEPTH	1044.88	210.50	2199.47	44.75	5195.38	259.77				259.77	22.50	2612.20	440.00	574.68	22.25	2583.18	220.00	284.15	34.00	3947.32	220.00	434.21	718.36
116+90.50		HWY. 16 FULL DEPTH	1191.62	210.50	2508.36	44.75	5925.00	293.25	-			296.25	22.50	2979.05	440.00	655.39	22.25	2945.95	220.00	324.05	34.00	4501.68	220.00	495.18	819.23
128+82.12			86.88	VAR.	156.94	VAR.	279.86	13.99	04.00	200.00	00.07	13.99	VAR.	140.53	440.00	30.92	VAR.	139.33	220.00	15.33	34.00	328.21	220.00	36.10	51.43
129+69.00	130+69.00 TIONAL FOR	HWY. 16 TRANSITION	100.00	VAR.	133.80	VAR.	54.43	2.72	21.00	233.33	39.67	42.39	VAR.	27.91	440.00	6.14	VAR.	26.52	220.00	2.92	29.50	327.78	220.00	36.06	38.98
			074.00		1	40.00	110101	70.00	T 0400 T	700.00	104.40	107.70	0400	700.00	T	1 101 11			1	1		700.00	T 1/4B	00.50	1 00.50
101+00.00	103+74.62		274.62	-		48.00	1464.64	73.23	24.00	732.32	124.49 54.40	197.72	24.00	732.32	VAR.	161.11 70.40					24.00	732.32	VAR. VAR.	80.56	80.56
103+74.62 128+02.12	104+94.62 128+82.12		120.00 8C.00			48.00	640.00	32.00 18.67	24.00	320.00 186.67	31.73	86.40 50.40	24.00	320.00	VAR.	70.40	21.00	186.67	VAR	20.52	24.00	320.00_	VAR.	35.20 20.56	35.20 41.09
128+02.12	129+69.00		86.88	-		42.00 42.00	373.34 405.44	20.27	21.00	202.72	31.73	54.73		 	<u> </u>	 	21.00	202.72	VAR.	20.53	21.00	186.67	VAR.	22.30	44.60
120+02.12	129+69.00	HVVT. 10	80.00	+		42.00	405.44	20.27	21.00	202.72	34.40	54.73		+			21.00	202.72	VAR.	22.30	21.00	202.72	VAR.	22.30	44.60
ADDI	TIONAL FOR	R GUARDRAIL WIDENING						· .			<u> </u>						1		1	1	<u> </u>		1		
111+67.75		HWY. 16 WIDENING TAPER RT.	33.00	15.75	5.20			T	1 7					_	1	T .					2.75	10.08	220.00	1.11	1.11
112+00.75		HWY. 16 WIDENING RT.	10.00	31.50	3.15				+					_							5.50	6.11	220.00	0.67	0.67
112+10.75		HWY. 16 WIDENING RT.	200.00	26.25	52.50				+					 		 					4.50	100.00	220.00	11.00	11.00
114+10.75		HWY. 16 WIDENING RT.	43.75	21.00	9.19			_	+					 							3.50	17.01	220.00	1.87	1.87
114.10.70	114104.00	TIVVI: 10 VVIDEINING IXI:	40.70	21.00	0.10			,	+					 							3.30	17.01	220.00	1.07	1.07
112+67.75	113+00 75	HWY. 16 WIDENING LT.	33.00	15.75	5.20																2.75	10.08	220.00	1.11	1.11
113+00.75		HWY. 16 WIDENING LT.	1C.00	31.50	3.15																5.50	6.11	220.00	0.67	0.67
113+10.75		HWY. 16 WIDENING LT.	100.00	26.25	26.25																4.50	50.00	220.00	5.50	5.50
114+10.75	114+54.50	HWY. 16 WIDENING TAPER LT.	43.75	21.00	9.19				1 1												3.50	17.01	220.00	1.87	1.87
116+55.50	116+99.25	HWY. 16 WIDENING TAPER RT.	43.75	21.00	9.19																3.50	17.01	220.00	1.87	1.87
116+99.25	117+99.25	HWY. 16 WIDENING RT.	100.00	26.25	26.25																4.50	50.00	220.00	5.50	5.50
117+99.25	118+09.25	HWY. 16 WIDENING RT.	1C.00	31.50	3.15																5.50	6.11	220.00	0.67	0.67
118+09.25	118+42.25	HWY. 16 WIDENING RT.	33.00	15.75	5.20																2.75	10.08	220.00	1.11	1.11
																								1	
116+55.50		HWY. 16 WIDENING LT.	43.75	21.00	9.19																3.50	17.01	220.00	1.87	1.87
116+99.25			200.00	26.25	52.50																4.50	100.00	220.00	11.00	11.00
118+99.25		HWY. 16 WIDENING LT.	1C.00	31.50	3.15																5.50	6.11	220.00	0.67	0.67
119+09.25	119+42.25	HWY. 16 WIDENING TAPER LT.	33.00	15.75	5.20																2.75	10.08	220.00	1.11	1.11
									<u> </u>																
														<u> </u>							<u> </u>				
		SUPERELEVATION	1	1					т т																
101+00.00	102+87.54		187.54	30.50	57.20									 											
102+87.54	104+68.09	HWY. 16	180.55	64.00	115.55				1		1		-	 		 	-	-	+	-			+		+
104+68.09	107+37.59	HVVY, 16	269.50	32.00	86.24			<u> </u>	 		1		-	 	 	 	-	-	+	+	_		+		+
122122.22	105 170 00	LIMAN 46	250.00	40.75	140.60	+		 	+		 	+	-	+	<u> </u>	 	+	 	+	+	 		+		+
122+22.29	125+72.29		350.00	40.75	142.63	1			+		1	-		+	<u> </u>	 	+	-	+	+	 	<u> </u>	1	—	+
125+72.29	127+34.50 129+69.00	HWY. 16 HWY. 16	162.21	81.50	132.20				+		1	1		+	-	 		-	+	+	 		1		+
127+34.50	129709.00	TIVVI. IO	234.50	38.75	90.87				+		1	1	 	+	1	 	+	 	1	1			+		+
TOTALS:				-	6267.27		14767.13	738.35	+	2086.15	354.64	1092.99	 	7028.44	 	1546.25	+	6296.98	+	692.67	 	12428.06	+	1367.11	2059.78
TOTALS.					0201.21		14/0/.13	135.33		2000.10	J 334.04	1032.39		1 / 020.44		1046.20		0∠30.38		03Z.0/		12428.06		1307.11	1 2008.70

DATE REVISED	DATE REVISED	FED. RO. DIST. NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS
11211323	11211323	6	ARK.	080614	34	82
			07565 -	QUANTITIES	- 64431	

SCHEDULE OF BRIDGE QUANTITIES - JOB NO. 080614

			ITEM NO.	205	801	SP, SS, & 802	SP, SS, & 802	SP & 803	SS & 804	SS & 804	SS & 805	SS & 805	SP, SS, & 807	SS & 807	SS & 808	812	SS & 816	SS & 816
RIDGE NO.	AME PLATE TITLE	UNIT OF STRUCTURE	ПЕМ	EXISTING BRIDGE	UNCLASSIFIED EXCAVATION FOR STRUCTURES - BRIDGE	CLASS S CONCRETE - BRIDGE	CLASS S(AE) CONCRETE - BRIDGE	CLASS 2 PROTECTIVE SURFACE TREATMENT	REINFORCING STEEL - BRIDGE (GRADE 60)	EPOXY COATED REINFORCING STEEL (GRADE 60)	② STEEL PILING (HP 14X73)	PREBORING	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50W)	PAINTING STRUCTURAL STEEL	ELASTOMERIC BEARINGS	BRIDGE NAME PLATE (TYPE D)	FILTER BLANKET	FOUNDATION PROTECTION RIPRAP
В	ż		UNIT	LUMP SUM	CU. YD.	CU. YD.	CU. YD.	SQ. YD.	LB.	LB.	LIN. FT.	LIN. FT.	LB.	TON	CU. IN.	EACH	SQ. YD.	TON
07565	IWAY 16 OVER E DEE CREEK	BENT NO. 1 BENT NO. 2 BENT NO. 3 BENT NO. 4 BENT NO. 5 BENT NO. 6			101 104 111 97	24.12 32.33 37.89 38.26 31.98 24.12			2,595 5,782 6,308 6,361 5,729 2,595	384	92	40			1,178.2 1,178.3 1,178.3 1,178.2		550	2,096
	HIGHW	200'-0" INTEGRAL CONTINUOUS W-BEAM UNIT SITE NO. 1 (EXISTING BR. NO. 02173)		1			230.50	941.0		66,712			125,930	2.1		1		
		TOTALS FOR JOB NO. 080614			① 413	188.70	230.50	941.0	29,370	67,480	196	80	125,930	2.1	4,713	1	1,191	4,605

① Includes approximately 92 cu. yds. of rock excavation.

JIM POOL DESIGN SECTION SUPERVISOR



SCHEDULE OF BRIDGE QUANTITIES

ARKANSAS

PEE DEE CREEK STR. & APPRS. (CLINTON) (S)

VAN BUREN COUNTY

ROUTE 16 SEC. 10
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

 DRAWN BY:
 JCG
 DATE:
 11/2/2021
 FILENAME:
 b080614_q1.dgn

 CHECKED BY:
 DKS
 DATE:
 12/9/2021
 SCALE:
 No Scale

 DESIGNED BY:
 DATE:
 No Scale

BRIDGE ENGINEER BRIDGE NO. 07565

DRAWING NO. 64431

PRINT DATF: 2/1/2023

② All steel piling shall be Grade 50 and are required to have approved driving points which will not be paid for directly, but will be considered subsidiary to the item "Steel Piling (HP 14x73)". All piles shall conform to Standard Drawing No. 55020.

HIMMADVA	QUANTITIES	(BOY 1 OF 2	١,
DUNINARTO	- QUANTITIES	(DUX TUF 2	-)

ITEM NUMBER	ITEM	QUANTITY	UNIT				
201	CLEARING	30	STATIO				
201	GRUBBING						
202	REMOVAL AND DISPOSAL OF FENCE	1905	LIN. F				
202	REMOVAL AND DISPOSAL OF PIPE CULVERTS	6	EAC				
202 SP, SS, & 210	REMOVAL AND DISPOSAL OF BOX CULVERTS UNCLASSIFIED EXCAVATION	1 11252	EAC CU. Y				
SP, SS, & 210 SP & 210	COMPACTED EMBANKMENT	11352 32278	CU. Y				
SP & 210	COUNTAGE LE PRIDATIVILIENT SOLL STABILIZATION	100	TON				
	SOIL STABILIZATION AGGREGATE BASE COURSE (CLASS 7)	7444	1OT				
SS & 401	TACK COAT	1123	GAL				
	MINERAL AGGREGATE IN ACHM BINDER COURSE (1")	1476	TOT				
	ASPHALT BINDER (PG 64-22) IN ACHM BINDER COURSE (1")	70	TOT				
	MINERAL AGGREGATE IN ACHM SURFACE COURSE (1/2")	2114	TOT				
	ASPHALT BINDER (PG 64-22) IN ACHM SURFACE COURSÉ (1/2")	125	TOT				
SP & 412	COLD MILLING ASPHALT PAVEMENT	644	SQ. Y				
SP, SS, & 414	ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC	15	1OT				
SP, SS, & 415	ACHM PATCHING OF EXISTING ROADWAY	15	1OT				
SP, SS, & 504	APPROACH SLABS	118.60	CU. Y				
	APPROACH GUTTERS	16.80	CU. Y				
	PORTLAND CEMENT CONCRETE DRIVEWAY	220.80	SQ. Y				
	MOBILIZATION	1.00	LUMP S				
SP & 602	FURNISHING FIELD OFFICE	1	EAC				
	MAINTENANCE OF TRAFFIC	1.00	LUMP S				
	SIGNS	443	SQ. F				
	BARRICADES TO A STATE OF THE ST	96	LIN. F				
SS & 604	TRAFFIC DRUMS	106	EAC				
SS & 604	FURNISHING AND INSTALLING PRECAST CONCRETE BARRIER	253	LIN. F				
604	CONSTRUCTION PAVEMENT MARKINGS	11192	LIN. F				
604	REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	1084	LIN. F				
SS & 604	VERTICAL PANELS	13	EAC				
	CONCRETE DITCH PAVING (TYPE B) 48" REINFORCED CONCRETE PIPE CULVERTS (CLASS V) (ALTERNATE NO. 1)	261 110	SQ. Y				
SS & 606	48" ASPHALT COATED CORRUGATED STEEL PIPE CULVERTS (14 GAUGE) (ALTERNATE NO. 2)	110	LIN. F				
SS & 606	48° ALUMNUM COATED CORRUGATED STEEL PIPE CULVERTS (14 GAUGE) (ALTERNATE NO. 2) (ALTERNATE NO. 2)	114	LIN. F				
SS & 606	48° POLYMER PRECOATED METALLIC COATED CORRUGATED STEEL PIPE CULVERT (14 GAUGE) (ALTERNATE NO. 3)	114	LIN. F				
	48" POLYPROPYLENE PIPE (ALTERNATE NO. 5)	114	LIN. F				
	48° HIGH DENSITY POLYETHYLENE PIPE (ALTERNATE NO. 9)	114	LIN. F				
	48" PVC PIPE (ALTERNATE NO. 7)	114	LIN. F				
	18" SIDE DRAIN	294	LIN. F				
	24" SIDE DRAIN	144	LIN. F				
	36" SIDE DRAIN	42	LIN. F				
	42" SIDE DRAIN	70	LIN. F				
SS & 606	48" FLARED END SECTIONS FOR REINFORCED CONCRETE PIPE CULVERTS (ALTERNATE NO. 1)	2	EAC				
SS & 606	48" FLARED END SECTIONS FOR CORRUGATED STEEL PIPE CULVERTS (ALTERNATE NO. 2)	2	EAC				
SS & 606	SELECTED PIPE BEDDING	50	CU. Y				
SS & 611	UNDERDRAIN OUTLET PROTECTORS	6	EAC				
SS & 611	4" PIPE UNDERDRAINS	1500	LIN. F				
SS & 617	GUARDRAIL (TYPE A)	450	LIN. F				
SS & 617	GUARDRAIL TERMINAL (TYPE 2)	4	EAC				
SS & 617	THRIE BEAM GUARDRAIL TERMINAL	4	EAC				
SS & 619	MIRE FENCE (TYPE D-1)	1408	LIN. F				
620	LIME	16	1OT				
620	SEEDING	7.85	ACR				
SS & 620	MULCH COVER	34.43	ACR				
620	WATER TEMPORADY SEEDING	1345.5	M. GA				
621 621	TEMPORARY SEEDING SILT FENCE	26.58 6856	LIN. F				
621	SAND BAG DITCH CHECKS	880	BAC				
621	SEDIMENT BASIN	300	CU. Y				
621	SEDIMENT DASIN OBLITERATION OF SEDIMENT BASIN	300	CU. Y				
621	UBLITERATION OF SEDIMENT BASIN SEDIMENT REMOVAL AND DISPOSAL	635	CU. Y				
621	BOOK DITCH CHECKS	184	CU. Y				
623	SECOND SEEDING APPLICATION	7.85	ACR				
624	SOLID SODDING	194	SQ. Y				
626	EROSION CONTROL MATTING (CLASS 3)	571	SQ. Y				
635	ROADWAY CONSTRUCTION CONTROL	1.00	LUMP S				
637	MAILBOXES	4	EAC				
637	MAILBOX SUPPORTS (SINGLE)	2	EAC				
637	MAILBOX SUPPORTS (DOUBLE)	1	EAC				
642	RUMBLE STRIPS IN ASPHALT SHOULDERS	5196	LIN. F				
718	REFLECTORIZED PAINT PAVEMENT MARKING YELLOW (6")	820	LIN. F				
719	THERMOPLASTIC PAVEMENT MARKING WHITE (6")	6138	LIN. F				
719	THERMOPLASTIC PAVEMENT MARKING YELLOW (6")	6138	LIN. F				
721	RAISED PAVEMENT MARKERS (TYPE II)	39	EAC				
SS & 731	TEMPORARY IMPACT ATTENUATION BARRIER	1	EAC				
SS & 731	TEMPORARY IMPACT ATTENUATION BARRIER (REPAIR)	1	EAC				
SS & 804	REINFORCING STEEL-ROADWAY (GRADE 60)	15120	POUN				

DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS	
02/01/24		6	ARK.	080614	35	82	
		SUMMARY OF QUANTITIES & REVISIONS					

SUMMARY OF QUANTITIES (BOX 2 OF 2)

ITEM NUMBER	ІТЕМ		UNIT		
	STRUCTURES OVER 20' SPAN				
205	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1)	1	LUMP SUM		
636	BRIDGE CONSTRUCTION CONTROL	1.00	LUMP SUM		
801	UNCLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE	413	CU. YD.		
SP, SS, & 802	CLASS S CONCRETE-BRIDGE	188.70	CU. YD.		
SP, SS, & 802	CLASS S(AE) CONCRETE-BRIDGE	230.50	CU. YD.		
SP & 803	CLASS 2 PROTECTIVE SURFACE TREATMENT	941.0	SQ. YD.		
SS & 804	REINFORCING STEEL-BRIDGE (GRADE 60)	29370	POUND		
SS & 804	EPOXY COATED REINFORCING STEEL (GRADE 60)	67480	POUND		
SS & 805	STEEL PILING (HP 14X73)	196	LIN. FT.		
SS & 805	PREBORING	80	LIN. FT.		
SP, SS, & 807	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50W)	125930	POUND		
SS & 807	PAINTING STRUCTURAL STEEL	2.1	TON		
SS & 808	ELASTOMERIC BEARINGS	4713	CU. IN.		
812	BRIDGE NAME PLATE (TYPE D)	1	EACH		
SS & 816	FILTER BLANKET	1191	SQ. YD.		
SS & 816	FOUNDATION PROTECTION RIPRAP	4605	TON		

REVISIONS

DATE	REVISION	SHEET NUMBER		
2/1/2024	REMOVED THE FLEXIBLE BEGINNING OF WORK SPECIAL PROVISION	4 & 35		

ARKANSAS

DICENSED

LOCAL SERVICE

DICENSED

LOCAL SERVICE

N. 11425

LOCAL SERVICE

LOCAL SERVI

12-11-2023

SURVEY CONTROL COORDINATES

Project Name: s080614

Date: 5/21/2020

Coordinate System: ARKANSAS STATE PLANE - NORTH ZONE BASED ON GPS CONTROL, PROJECTED TO GROUND.

Units: U.S. SURVEY FOOT

Point. Name	Northing	Easting	Elev Fe	at.ure	e Description	
1	462222. 7871	1187072.2368	526.567	CTL	ARDOT STD, MON, STAMPED PN: 1	
2	462879 . 9275	1187958.2993	502 . 199	CTL	ARDOT STD. MON. STAMPED PN:2	
3	463155.4338	1188368.3183	504.143	CTL	ARDOT STD. MON. STAMPED PN:3	
4	463424.1902	1188832.1807	502.691	CTL	ARDOT STD. MON. STAMPED PN:4	
5	463804.8582	1189400.5136	511.998	CTL	ARDOT STD. MON. STAMPED PN:5	
6	463898.1885	1190072.0431	517.607	CTL	ARDOT STD. MON. STAMPED PN:6	
100	461568.0905	1186530.5120	596.283	GPS	ARDOT GPS #710020	
101	462573, 1055	1187490.6003	506.632	GPS	ARDOT GPS #710020A	
900	463839.7904	1189635.3320	515.372	TBM	CHSO. IN NW COR OF BR	
901	463416.1186	1188826.0484	503.249	TBM	CHSQ, IN E CU HEADWALL 18.6' S OF C/L	

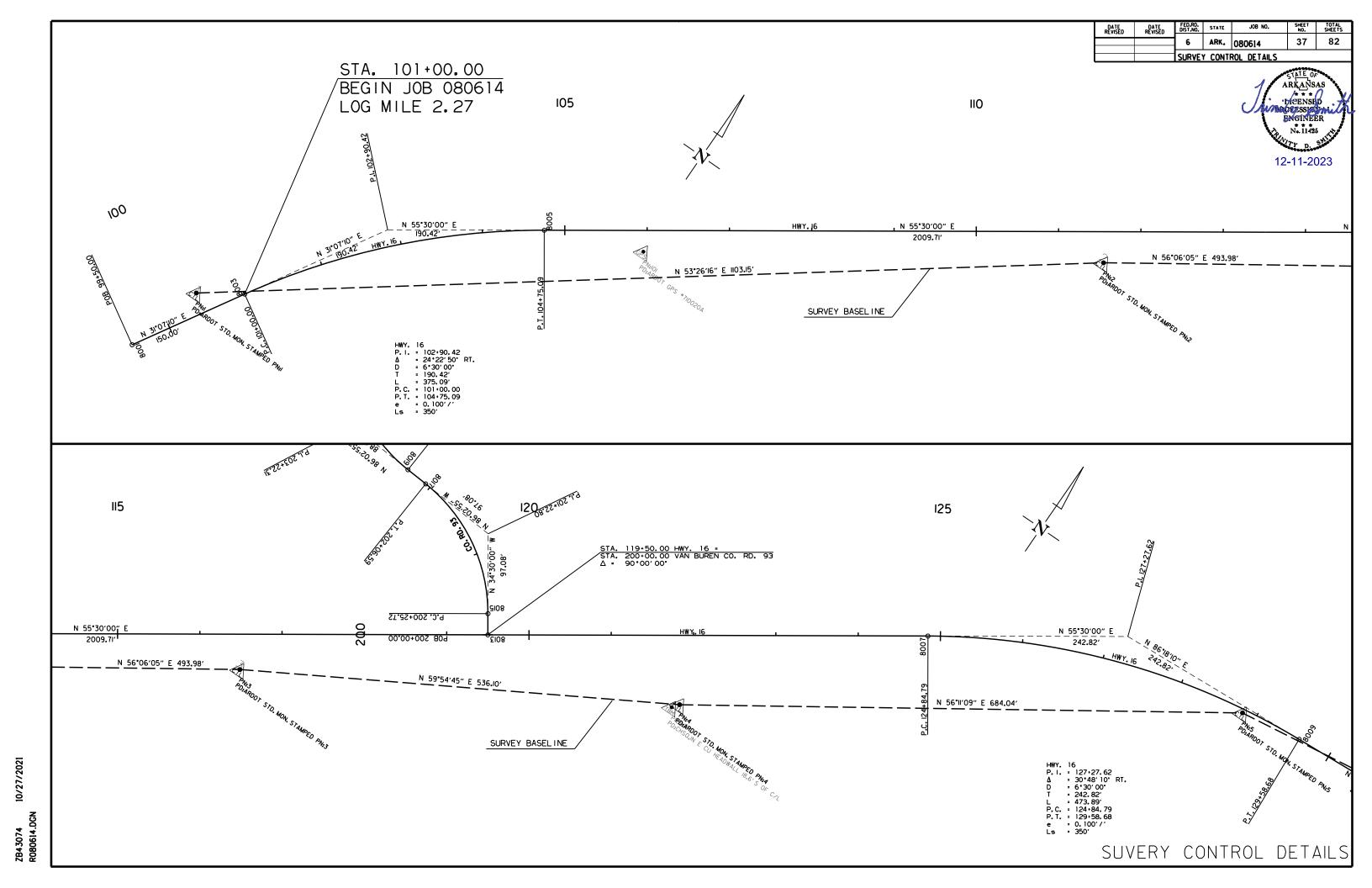
*Note - Rebar and Cap - Standard - 5/8" Rebar with 2" Aluminum Cap stamped
*(standard markings common to all caps), or as indicated
(other markings indicated in the point description of the individual point).

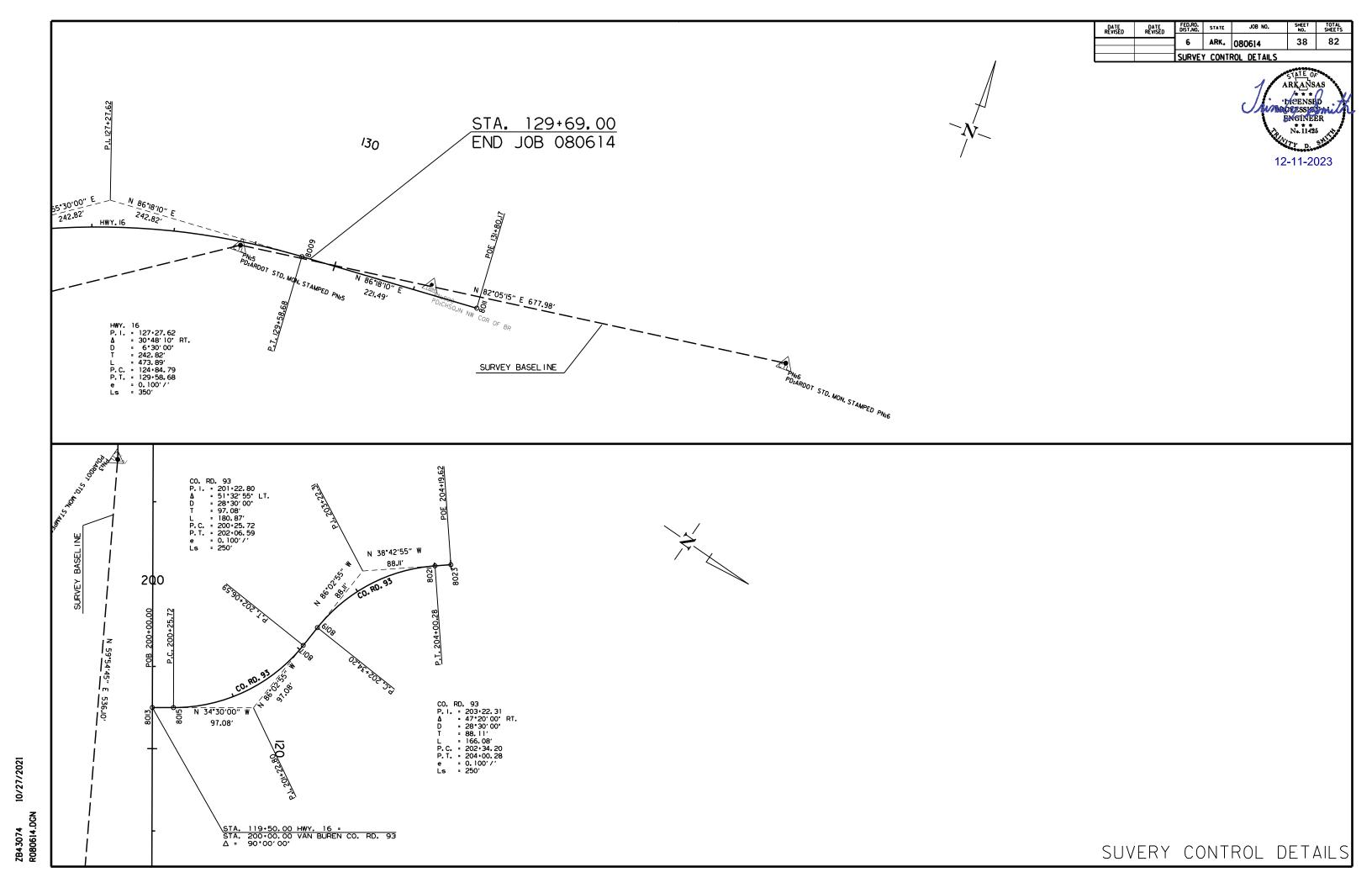
USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT
A PROJECT CAF OF 0.999915565550 HAS BEEN USED TO COMPUTE THE ABOVE GROUND COORDINATES.
THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS.
GRID DISTANCE = GROUND DISTANCE X CAF.
GRID COORDINATES ARE STORED UNDER FILE NAME \$080614GI.CTL
VERTICAL DATUM: NAVD 88 POSITIONAL ACCURACY THIRD ORDER, UNLESS SPECIFIED OTHERWISE
AT A SPECIFIC POINT.

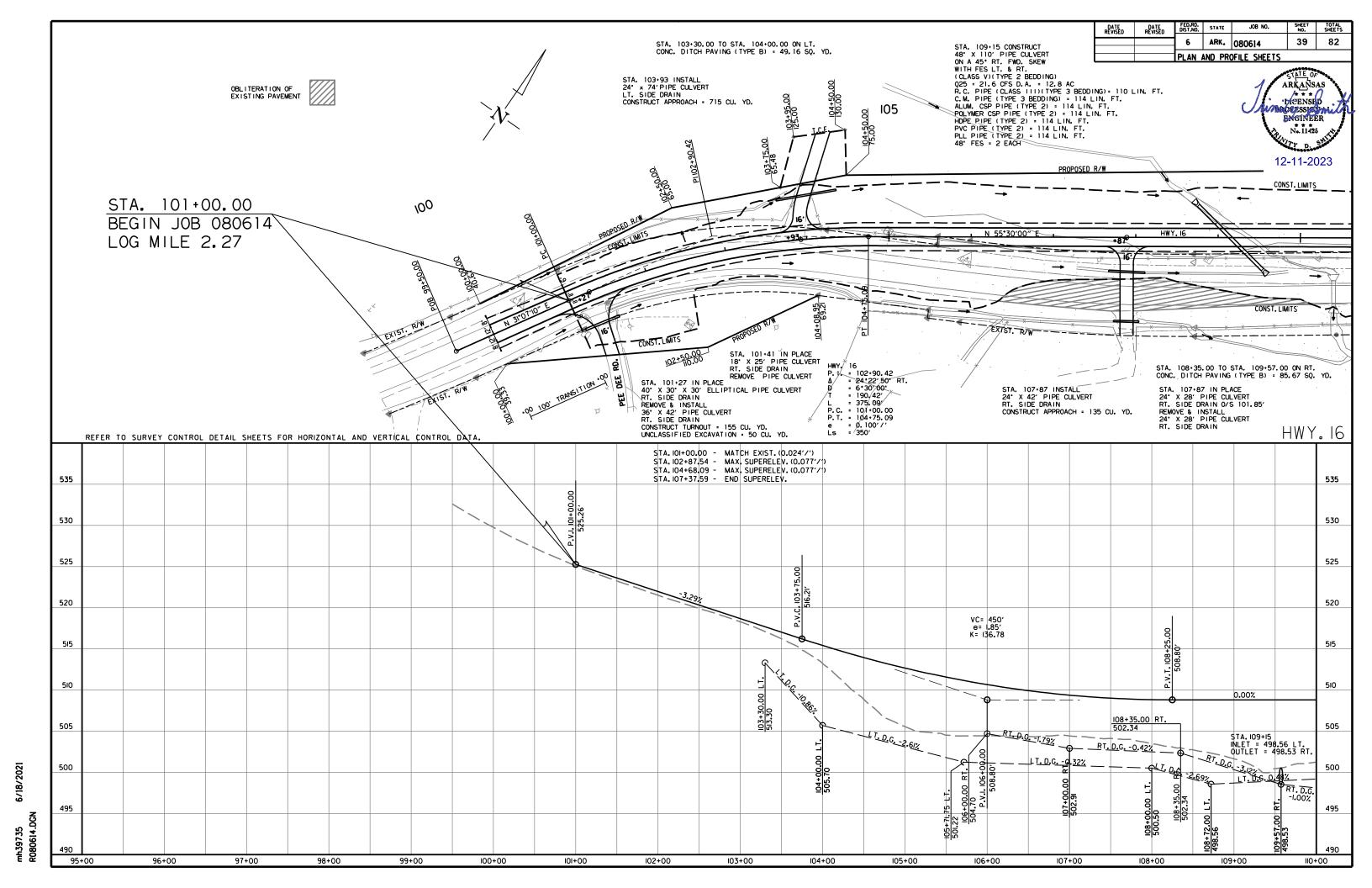
REFERENCE POINTS (1500 SERIES) ARE TO BE USED TO ESTABLISH CONTROL IF THE PRIMARY CONTROL POINTS LISTED ABOVE HAVE BEEN DESTROYED. REFERENCE POINTS ARE NOT TO BE USED FOR VERTICAL CONTROL

BASIS OF BEARING:
ARKANSAS STATE PLANE GRID BEARINGS - 0301-NORTH ZONE
DETERMINED FROM GPS CONTROL POINTS: 710020 - 710020A
CONVERGENCE ANGLE: 00°14'34.31" LEFT AT LAT N35°36'17.91" LON W92°25'02.51"
GRID AZIMUTH = ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

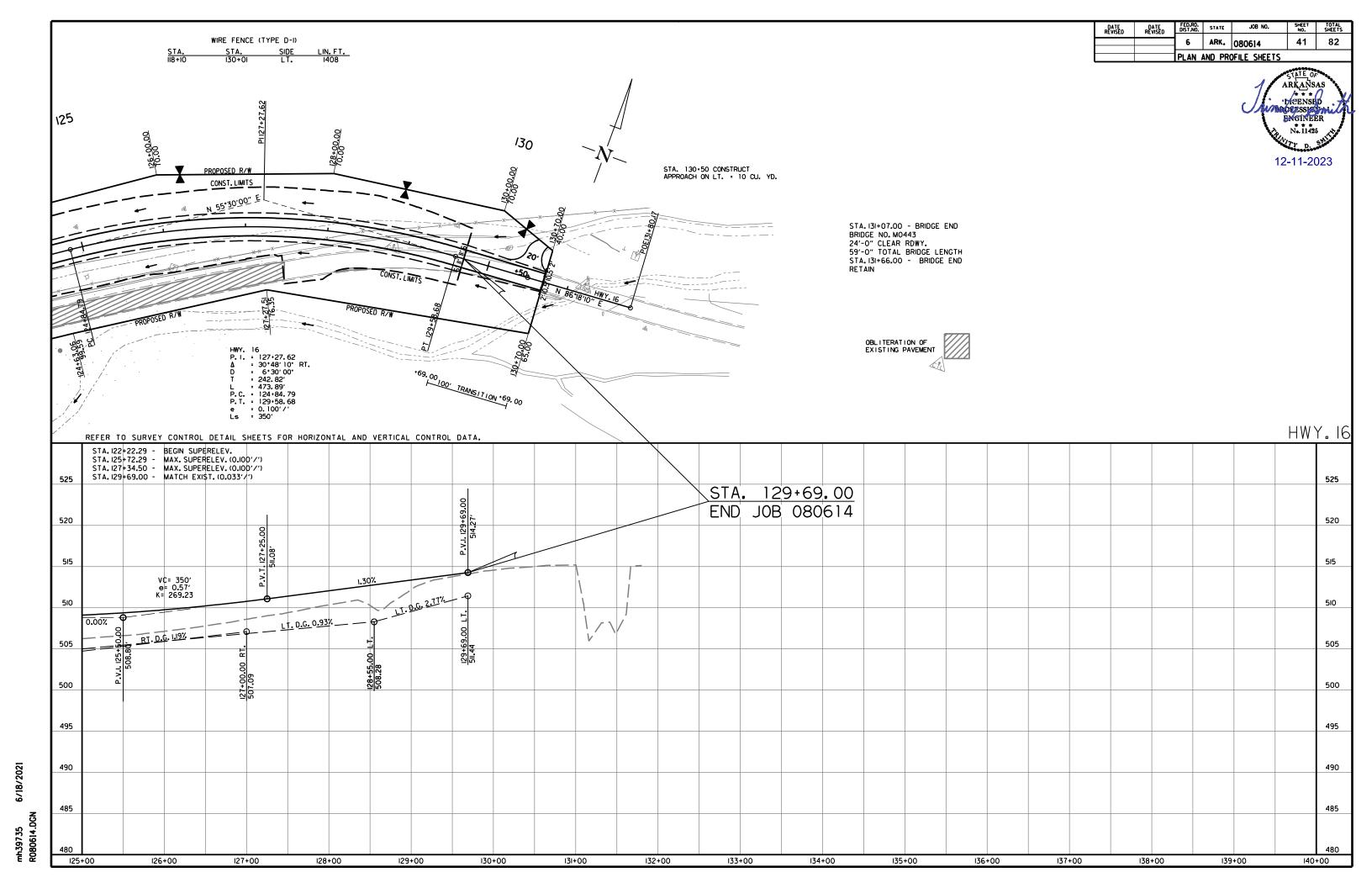
HWY. 16				CO. RD. 93					
POINT NO.	TYPE	STATION	NORTHING	EASTING	POINT NO.	TYPE	STATION	NORTHING	EASTING
8001	POB	99+50.00	462126.7536	1187043.8589	8013	POB	200+00.00	463361.4471	1188592.2482
8003	PC	101+00.00	462255.1674	1187121.3825	8015	PC	200+25.72	463382.6448	1188577.6794
8005	PT	104+75.09	462526.0462	1187376.7326	8017	PT	202+06.59	463469.3357	1188425.8523
8007	PC	124+84.79	463664.3577	1189032.9860	8019	PC	202+34.20	463471.2379	1188398.3136
8009	PT	129+58.68	463817.5513	1189475.4173	8021	PT	204+00.28	463546.0583	1188255.3047
8011	POE	131+80.17	463831.8339	1189696.4483	8023	POE	204+19.62	463561.1475	1188243.2093

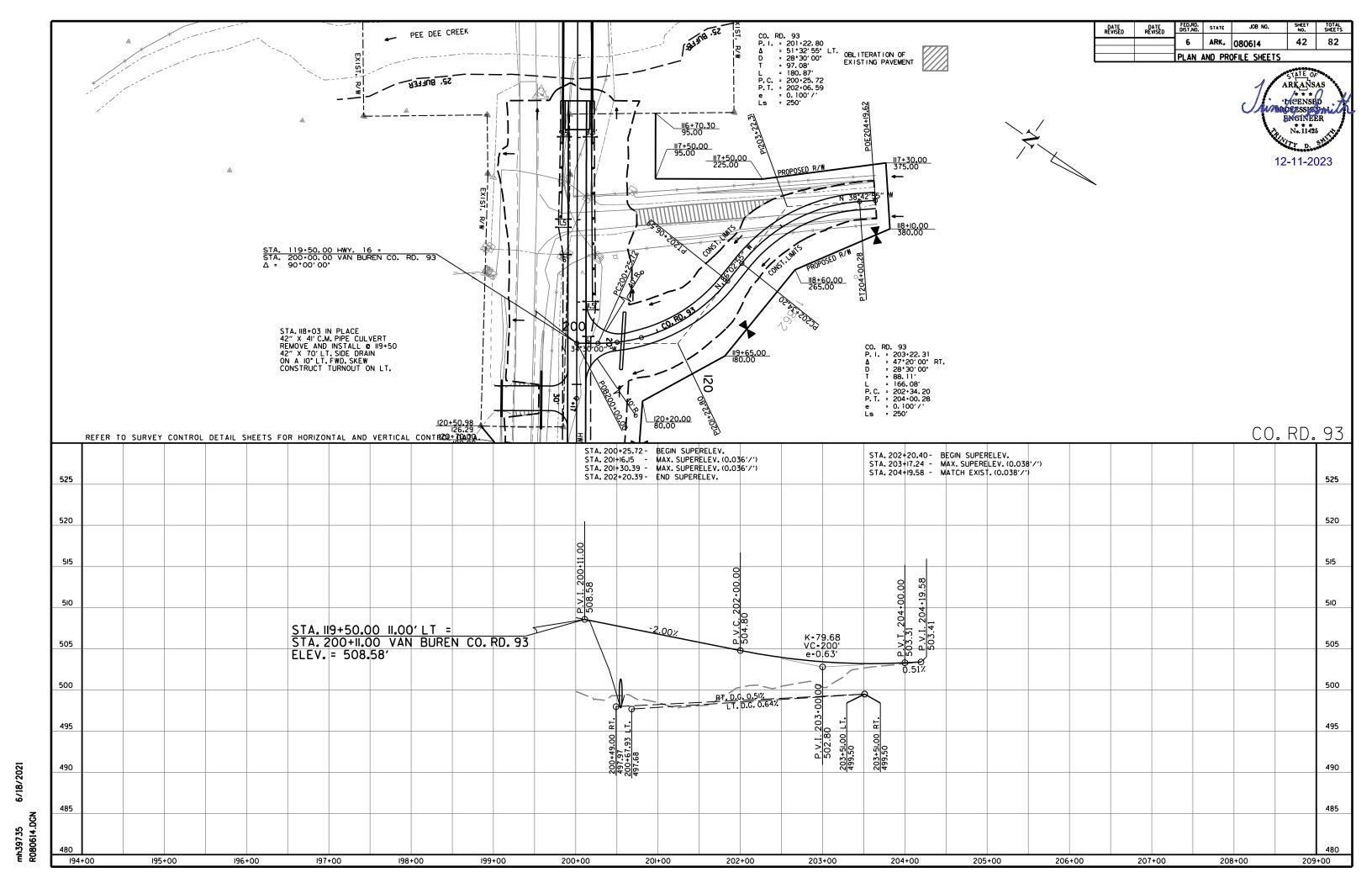


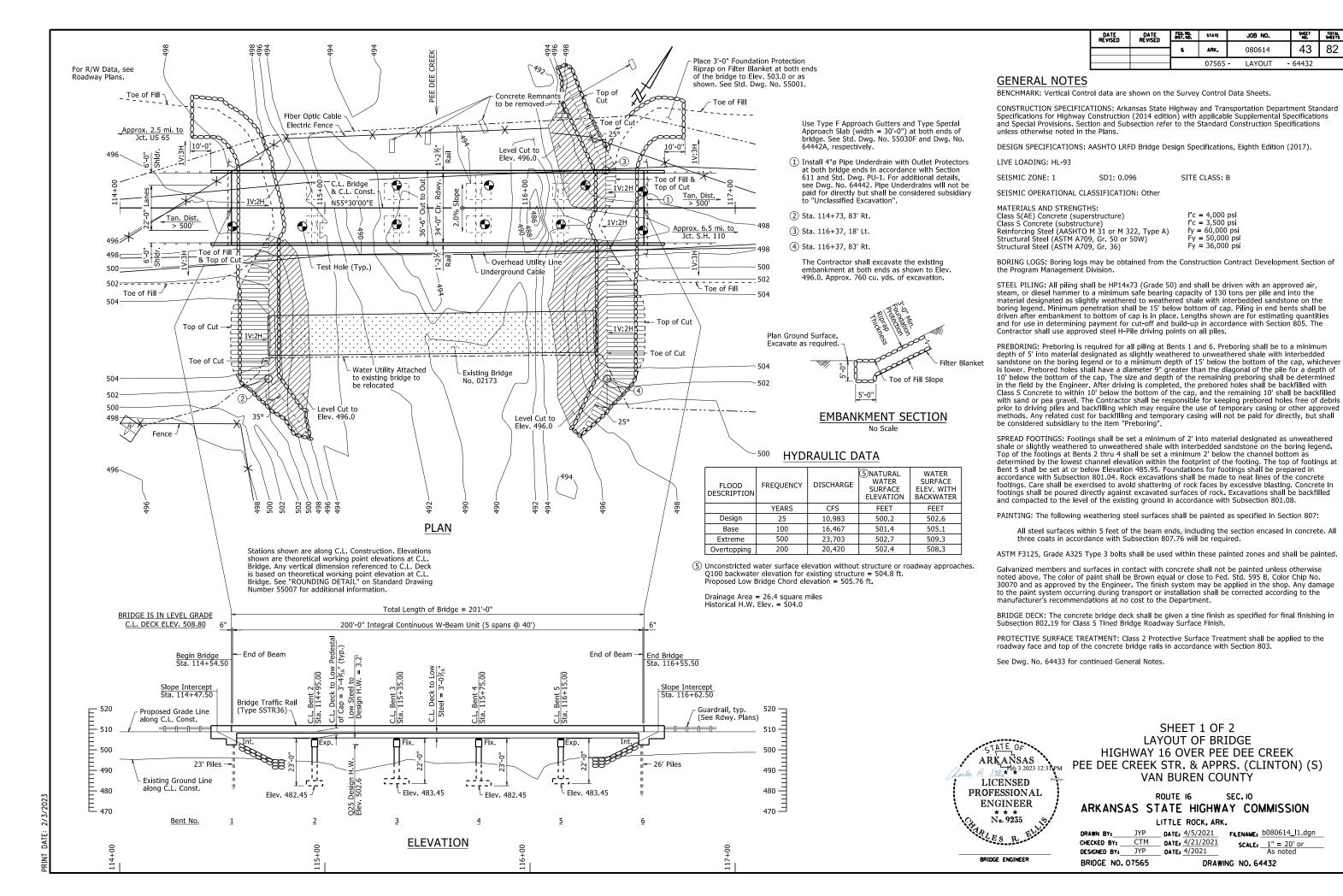




h39735 6/18/2021







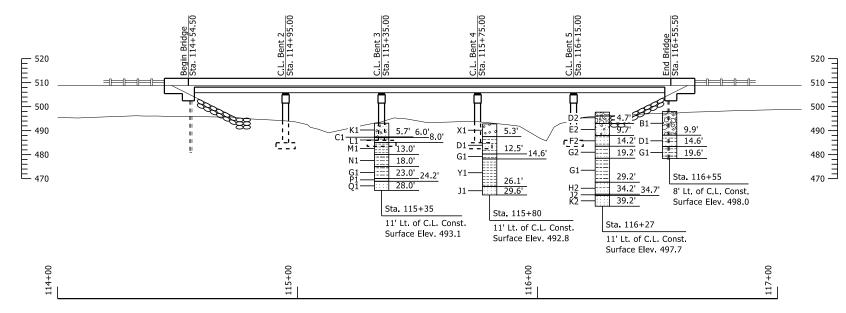
GENERAL NOTES (CONT'D)

	_
DETAIL DRAWINGS:	DRAWING NO(S
End Bents	64434
Intermediate Bents	64435-64436
Elastomeric Bearings	64437
200'-0" Integral Continuous W-Beam Unit	64438-64442
General Notes For Steel Bridge Structures	55006
Details For Steel Bridge Structures	55007
Steel H-Piling	55020
Bridge Traffic Rail	55070

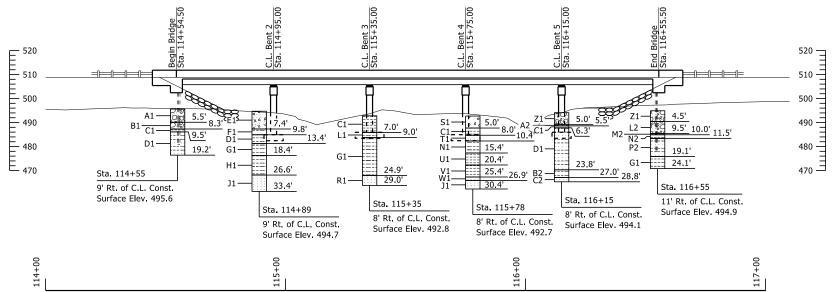
EXISTING BRIDGE: Existing Bridge No. 02173 (Log Mile 2.53) is 25.4' wide (20.0' clear roadway) and 160.0' long and consists of a concrete deck on steel I-Beams (5 spans total) supported by two column concrete abutments on spread footings and two column intermediate bents on spread footings. The existing bridge is located approximately 60' downstream from the proposed new bridge. Plans of the existing structure, if available, may be obtained upon request to the Construction Contract Development Section of the Program Management Division.

REMOVAL AND SALVAGE: After the new bridge is open to traffic, the Contractor shall remove existing Bridge No. 02173, including dumped riprap and concrete remnants, in accordance with Section 205. Removal of dumped riprap and concrete remnants will not be paid for directly but shall be considered subsidiary to the item "Removal of Existing Bridge Structure (Site No. _)". All material from the existing bridge shall become the property of the Contractor except for the bridge name plate which shall remain property of the State. The bridge name plate shall be delivered to Bridge Division.

MAINTENANCE OF TRAFFIC: See Roadway Plans.



ELEVATION OF SOIL BORINGS - LEFT OF C.L. CONST.



FEO. RO. STATE JOB NO. SHEET TOTAL NO. SHEETS DATE REVISED 080614 44 82 ARK. 07565 -LAYOUT - 64433

BORING LEGEND

A1-Moist, Brown Sand with Gravel and Cobbles

B1-Moist, Medium Dense, Brown Sand with Gravel and Cobbles

C1-SHALE - Weathered, Medium Hard, Grav

D1-SHALE WITH INTERBEDDED SANDSTONE - Unweathered, Medium Hard, Gray

E1-Wet, Dense, Brown Sand and Gravel F1-SHALE WITH INTERBEDDED SANDSTONE- Weathered, Gray

G1-SHALE WITH FREQUENT SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Grav

H1-SHALE WITH FREQUENT SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Occasional to Frequent Slickensides, Gray

J1-SANDSTONE - Unweathered, Well Cemented, Gray K1-Molst, Very Dense, Brown and Gray Sand with Gravel

L1-SHALE WITH INTERBEDDED SANDSTONE - Slightly Weathered, Medium Hard, Gray

M1-SHALE - Unweathered, Medium Hard, Gray

N1-SHALE WITH FREQUENT SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Occasional Fractures, Gray P1-SHALE - Unweathered, Medium Hard, Occasional Fractures, Grav

Q1-SANDSTONE - Unweathered, Well Cemented, Occasional Fractures, Light Gray

R1-SANDSTONE WITH OCCASIONAL SHALE PARTINGS AND SEAMS - Unweathered, Well Cemented, Gray

T1-SHALE WITH INTERBEDDED SANDSTONE -Unweathered, Medium Hard, Gray

U1-SHALE WITH OCCASIONAL SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Occasional Fractures, Gray

V1-SHALE WITH OCCASIONAL SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Occasional Fractures and Slickensides, Gray W1-SHALE WITH OCCASIONAL SANDSTONE PARTINGS AND SEAMS - Unweathered, Medium Hard, Occasional Slickensides, Gray

X1-Moist, Very Dense, Brown Gravel (Sandstone Fragments)

Y1-SHALE WITH OCCASIONAL SANDSTONE PARTINGS AND SEAMS - Unweatherd, Medium Hard, Gray

Z1-Sand with Gravel and Cobbles

A2-Wet, Very Dense, Brown Sand and Gravel

B2-SHALE WITH INTERBEDDED SANDSTONE - Unweathered, Medium Hard, Gray

C2-SANDSTONE - Unweathered, Well Cemented, Occasional Shale Partings and Seams, Gray

D2-Dry, Brown Sand with Gravel, Cobbles, and Boulders E2-Moist, Very Dense, Brown Sand with Gravel (Sandstone Fragments)

F2-SANDSTONE WITH INTERBEDDED SHALE - Unweathered, Cemented, Frequent Slickensides, Gray

G2-SHALE WITH INTERBEDDED SANDSTONE - Unweathered, Medium Hard, Occasional Fractures, Gray

H2-SANDSTONE WITH INTERBEDDED SHALE - Unweathered, Well Cemented, Frequent Fractures, Light Gray

J2-SHALE - Unweathered, Medium Hard, Trace Coal, Gray

K2-SANDSTONE - Unweathered, Well Cemented, Light Gray

L2-Molst, Dense, Brown Sand with Gravel and Cobbles M2-Moist, Very Dense, Brown Sand with Gravel and Cobbles

N2-SHALE WITH INTERBEDDED SANDSTONE - Weathered, Medium Hard, Gray

P2-SHALE WITH INTERBEDDED SAN6DSTONE - Unweathered, Medium Hard, Gray

"N" VALUES

Sta. 114+55 - 9' Right of C.L. Const. Sta. 115+80 - 11' Left of C.L. Const. 5.0-5.3 , N=45 (4")

6.0-7.0 , N=13

Sta. 116+15 - 8' Right of C.L. Const. Sta. 114+89 - 9' Right of C.L. Const.

5.3-6.3 , N=34

9.8-9.8, N=36 (0")

Sta. 116+27 - 11' Left of C.L. Const.

Sta. 115+35 - 11' Left of C.L. Const.

5.2-6.1 , N=74 (11") 9.7-10.1 , N=16 (5")

4.3-4.8 , N=25 (0") 5.7-6.0 , N=85 (4")

Sta. 116+55 - 8' Left of C.L. Const.

Sta. 116+55 - 11' Right of C.L. Const.

5.5-5.7 , N=28 (4")

Sta. 115+35 - 8' Right of C.L. Const.

 $5.0-6.0 \cdot N=17$ 9.9-9.9 , N=15 (0")

5.0-5.0 , N=60 (0")

Sta. 115+78 - 8' Right of C.L. Const.

5.5-6.0 , N=60 (6")

5.0-6.0 , N=33

10.0-10.0 , N=15 (0")

ARKAÑSAS LICENSED PROFESSIONAL **ENGINEER** No. 9235 BRIDGE ENGINEER

SHEET 2 OF 2 LAYOUT OF BRIDGE HIGHWAY 16 OVER PEE DEE CREEK PEE DEE CREEK STR. & APPRS. (CLINTON) (S) VAN BUREN COUNTY

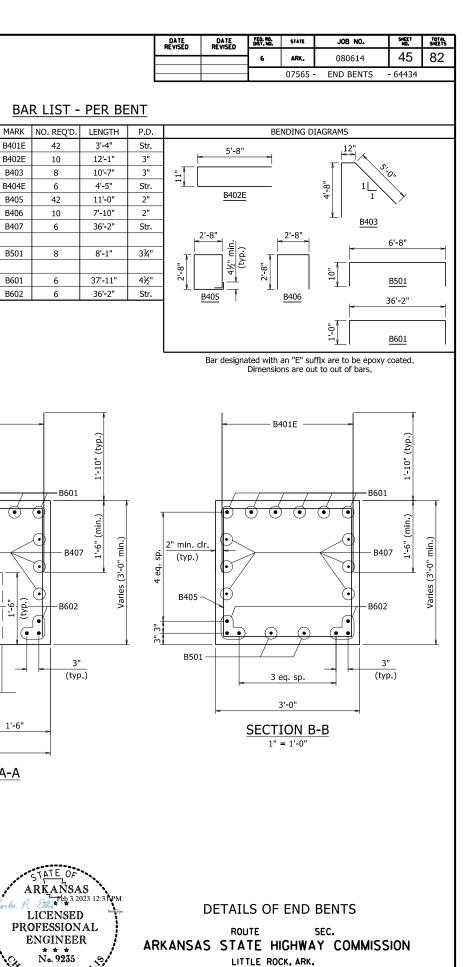
ROUTE 16 ARKANSAS STATE HIGHWAY COMMISSION

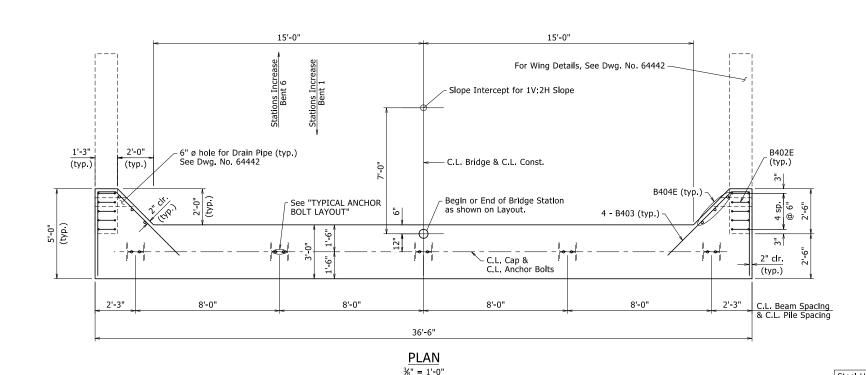
LITTLE ROCK, ARK.

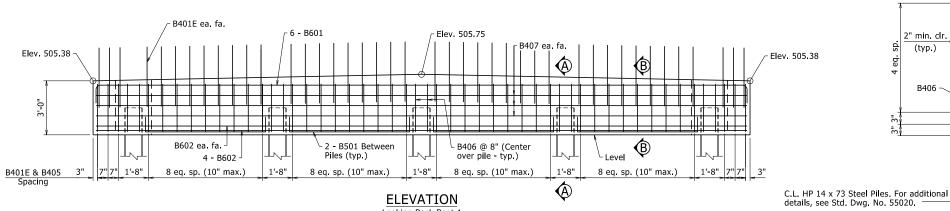
___ DATE: 4/5/2021 FILENAME: b080614_l1.dgn CHECKED BY: CTM DATE: 4/21/2021 SCALE: __1" = 20'

DESIGNED BY: JYP DATE: 4/2021 BRIDGE NO. 07565 **DRAWING NO. 64433**

ELEVATION OF SOIL BORINGS - RIGHT OF C.L. CONST.







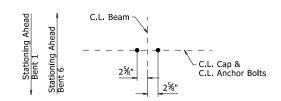
Looking Back Bent 1 Looking Ahead Bent 6 ¾" = 1'-0"

General Notes: B401E bars shall have a 1'-6" embedment into the cap. B402E & B404E bars shall have a 2'-10" embedment into the cap.

Granular backfill and pipe underdrain required behind cap. See Dwg. No. 64442.

See Std. Dwg. No. 55006 for additional General Notes.

For additional information, see Layout.



For details of anchor bolts, see Dwg. No. 64440.

TYPICAL ANCHOR BOLT LAYOUT

No Scale



B401E

B402E

B403

B404E

B405

B406

B407

B501

B601

B602

- B401F

2 - 1½"ø x 20" Anchor Bolts For Details, See Dwg. No. 64440

2" min. clr.

(typ.)

B406

1'-6"

3'-0"

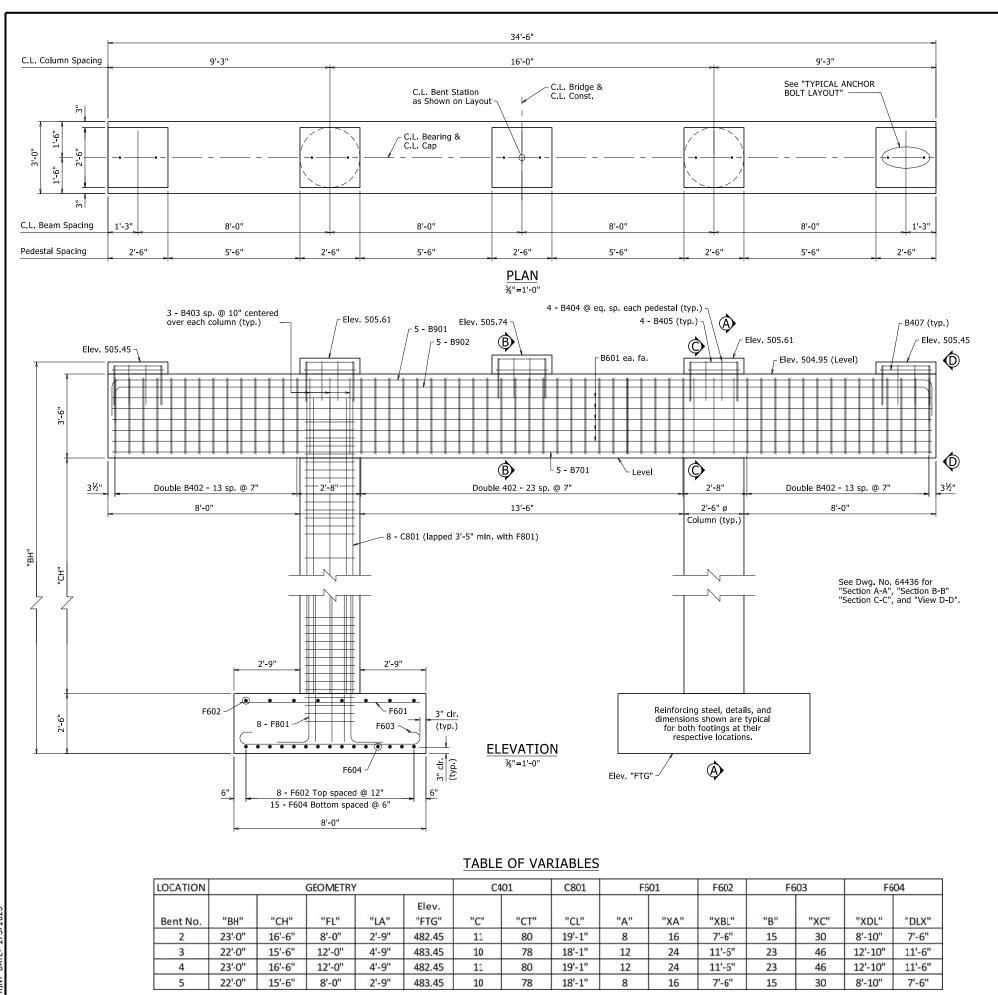
SECTION A-A 1" = 1'-0"

1'-6"

Steel H-Piles shall conform to AASHTO M 270, Grade 50.

JCG DATE: 7/2021 FILENAME: b080614_b1.dgn CHECKED BY: DKS DATE: 9/22/2021 SCALE: As Shown DESIGNED BY: JCG DATE: 7/2021

BRIDGE NO. 07565 DRAWING NO. 64434

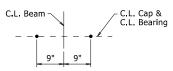


SHEET TOTAL NO. SHEETS DATE REVISED FED. RD. DIST. NO. STATE JOB NO. 82 080614 46 ARK. 6 07565 - INTERMEDIATE BENTS - 64435

BAR LIST - PER BENT

MARK	NO. REQ'D.	LENGTH	P.D.	"X"	BENDING DIAGRAMS
B401	6	3'-0"	Str.	-	, 1'-9" ,
B402	104	10'-2"	2"	-	
B403	6	8'-10"	2"	-	(typ.)
B404	20	5'-11"	2"	2'-1"	31-2" (ty (ty)
B405	20	5'-10"	2"	2'-0"] "
B406	6	2'-6"	Str.	-	B402 B403 B404 & B405
B407	5	9'-0"	2"	-	
					2'-2" < -
B601	10	34'-2"	Str.	-	75" (typ.) (b)
					(typ.)
B701	5	34'-2"	Str.	-	·\$
					1050
B901	5	36'-10"	9"	34'-2"	B407 135° C
B902	5	36'-6"	9"	33'-10"	"X" () :1
C401	"CT"	7' - 8"	3"	-	B901 & B902 C401
C801	16	"CL"	Str.	-	B901 & B902 C401
					10.77
F601	"XA"	7'-6"	Str.	-	
F602	16	"XBL"	Str.	-	
F603	"XC"	8'-10"	4½"	7'-6"	F801 F801
F604	30	"XDL"	4½"	"DLX"	6" <u>F603 & F604</u> 6" → <u>F801</u>
F801	16	8'-11"	6"	-	

Dimensions are out-to-out of bars



For Details of Elastomeric Bearings, see Dwg. No. 64437

TYPICAL ANCHOR BOLT LAYOUT

No Scale



SHEET 1 OF 2 DETAILS OF INTERMEDIATE BENTS

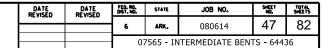
ROUTE SEC. ARKANSAS STATE HIGHWAY COMMISSION

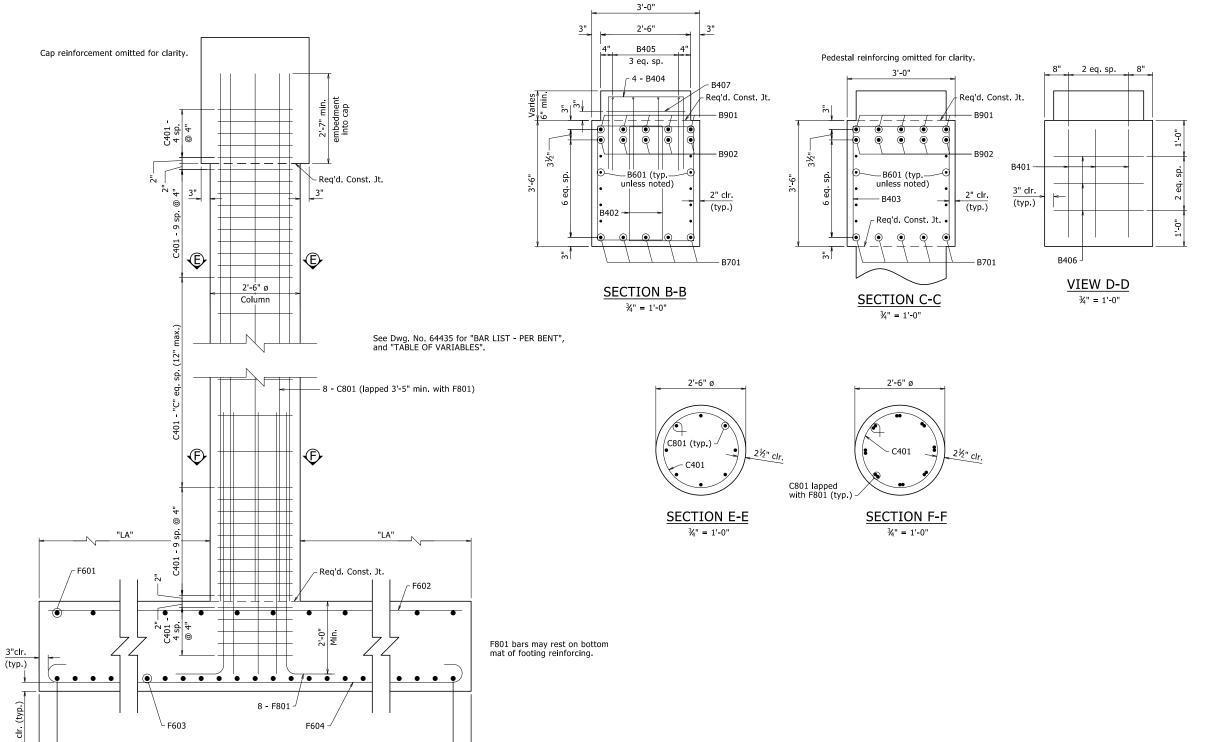
> LITTLE ROCK, ARK. JCG DATE: 7/2021

FILENAME: b080614_b2.dgn CHECKED BY: DKS DATE: 9/22/2021 SCALE: As Shown DESIGNED BY: JCG DATE: 7/2021

DRAWING NO. 64435

BRIDGE NO. 07565







SHEET 2 OF 2 DETAILS OF INTERMEDIATE BENTS

ROUTE ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARK.

 DRAWN BY:
 JCG
 DATE: 7/2021

 CHECKED BY:
 DKS
 DATE: 9/22/2021

 DESIGNED BY:
 JCG
 DATE: 7/2021
 ___ FILENAME: b080614_b2.dgn

SCALE: As Shown

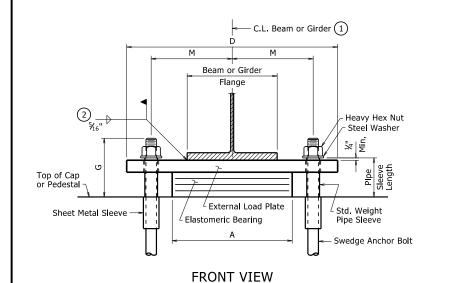
BRIDGE NO. 07565 DRAWING NO. 64436

"A" - F601 Top spaced @ 12" "B" - F603 Bottom spaced @ 6"

"FL"

SECTION A-A

¾" = 1'-0"



1 C.L. Elastomeric Pad shall be aligned with C.L. Beam or Girder.

Thickness under Dead Load Stations Increase 2" (Min.) Steel PL @ C.L. Bearing T_b (External Load Plate — C.L. Bearing Thickness @ Back Station Edge) Top of Cap

The direction of bevel of the external load plate may not be accurately depicted with respect to Ta and Tb values shown in the 'Table of Fabricator Variables."

T_ (External Load Plate Thickness @ Ahead

Station Edge)

(2) Unless otherwise approved by the Engineer, welding of the external load plate at expansion bearings to the beam or girder will be allowed only when: 1) the approximate average air temperature during the 24 hour period immediately preceding welding is between 40°F and 80°F; and 2) the slots in the external load plate are positioned to center on the anchor bolts; and 3) no horizontal deformation of the elastomeric pad is evident. If welding at other temperatures is required, the Engineer will provide adjustment data

Care shall be taken to ensure that the external load plate is in full and complete contact with the beam or girder flange before welding begins.

Prior to erection of the beams or girders,

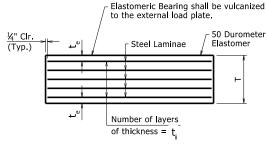
orientation of the bearings with respect

the Contractor shall verify the

to T and Th.

Slot or Hole in External Load Plate PLAN VIEW

(3) Maximum Design Load = Service 1 Limit State



SIDE VIEW

t_o = Thickness of elastomer cover on top and bottom of pad

t, = Thickness of elastomer between steel laminae

N = Number of elastomer layers of thickness t

ELASTOMERIC BEARING

Sheet metal sleeve Steel Washer Pipe Sleeve Top of Cap Swedged

DATE REVISED

4½" Thread

ANCHOR BOLT DETAIL

FEO. RO. STATE

ARK.

JOB NO.

080614

07565 - ELASTO BRGS - 64437

SHEET TOTAL SHEETS 82

48

Anchor Bolts may be cast in place or drilled and grouted into place. If Anchor Bolts are to be cast in place, the Galvanized Sheet Metal Sleeves will not be required.

If Anchor Bolts are to be drilled and grouted in place, the Galvanized Sheet Metal Sleeves shall be cast in place as shown. Sleeves shall be dry packed with styrofoam, urethane foam, or approved equal prior to pouring of concrete. After pouring of the cap and prior to erection of Structural Steel, the dry pack shall be removed and holes for the anchor bolts shall be accurately drilled into the concrete. Bolts placed in drilled holes shall be accurately set and fixed using a QPL-approved epoxy or non-shrink grout that completely fills the holes. Galvanized Sheet Metal Sleeves shall meet the requirements of ÁSTM A653, CS Type B or approved equivalent, be of minimum 16 gauge thickness, and be galvanized according to ASTM B695, Class 50. Sheet Metal Sleeves will not be paid for directly, but will be considered subsidiary to the item "Structural Steel in Beam Spans (A709, Gr. 50W)."

GENERAL NOTES

Elastomeric Bearings shall conform to Section 808 and shall be paid for at the unit price bid for "Elastomeric Bearings".

External load plates shall conform to ASTM A709, Grade 50W. Pipe sleeves shall be ASTM A500, Grade B, and shall be galvanized to conform to AASHTO M 232, Class C or ASTM B695, Class 50.

External load plates shall be completely fabricated (including bevel and bolt holes) and shall be cleaned before vulcanizing to the elastomeric bearing. The surface in contact with the elastomeric bearing shall be cleaned in accordance with Subsection 808.03. Other surfaces shall be blast cleaned in accordance with Subsection 807.84(b) for painted steel and 807.84(e) for unpainted

Anchor Bolts, Washers and Nuts shall conform to Subsection 807.07. The anchor bolt grade of steel shall be as specified in the "Table of Fabricator Variables". Indentations shall be circular with rounded bottoms and staggered as shown in the details.

Pipe Sleeves, Anchor Bolts, Washers and Nuts shall be paid for at the unit price bid for "Structural Steel in Beam Spans (A709, Gr. 50W)." External load plates will not be measured and paid for separately, but will be considered incidental to the unit price bid for "Elastomeric Bearings."

Bearings shall be seated in accordance with Subsection 808.08. This work and materials are considered subsidiary to the item "Elastomeric Bearings" and will not be paid for directly.

ARKAÑSAS Feb 3 2023 12:31 PM LICENSED **PROFESSIONAL ENGINEER** No. 9235 BRIDGE ENGINEER

DETAILS OF ELASTOMERIC BEARINGS

ROUTE SEC. ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK. JCG

BRIDGE NO. 07565

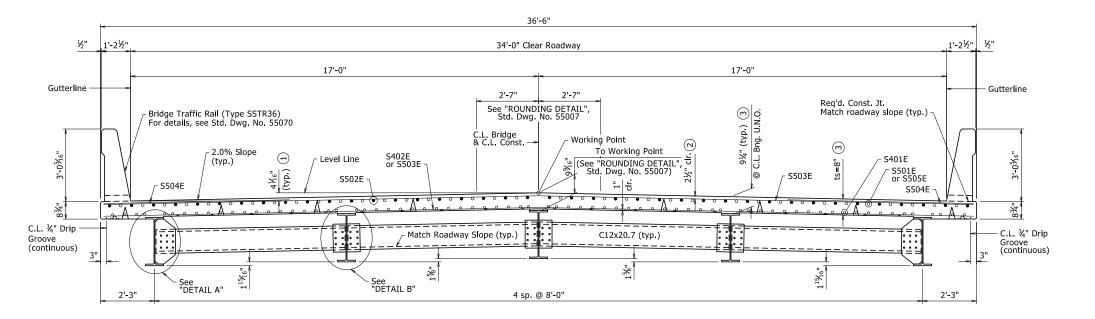
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DRAWING NO. 64437

TABLE OF FABRICATOR VARIABLES

$[\mathfrak{G}]$	Maximum	Design Load	d = Servic	e 1 Limit St	ate						ELAS	ГОМЕЯ	RIC PAD				EXTE	RNAL L	OAD	PLATE					ANCHOR BOL	Т	
- 명	LC	OCATION	BEARING	NO. of	3 MAXIMUM								NO. & THICKNESS OF										ANCHOR	BOLT	DIDE SLEEVE SIZE (Ø V	SHEET METAL	STEEL
BRIDGE NO.	BENT NO (S).	BEAM OR GIRDER NO.	TYPE	BEARINGS EACH BENT	DESIGN LOAD (KIPS)	G	Н	A	В	N	t _i	t _e	STEEL LAMINAE	Т	С	D	E	F	K	М	Ta	T _b	(ø x L)	GRADE	PIPE SLEEVE SIZE (Ø X L)	SLEEVE SIZE (Ø x L)	WASHER SIZE (O.D.)
	2 & 5	ALL	EXP	5	137	6 3/4"	3 3/4"	13"	10"	2	1/2"	1/4"	3 @ 12 ga.	1 13/16"	11"	24"	3 3/4"	2 5/8"	1/2"	9"	2.00"	2.00"	1 3/4" x 28"	55	2" x 4"	4" x 12"	3 3/8"
	3 & 4	ALL	FIX	5	128	6 7/8"	3 13/16"	13"	10"	2	1/2"	1/4"	3 @ 12 ga.	1 13/16"	11"	24"	2 5/8"	2 5/8"	1/2"	9"	2.00"	2.00"	1 3/4" x 28"	55	2" x 4 1/8"	4" x 12"	3 3/8"
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DATE REVISED	DATE REVISED	FED. RO. DIST. NO.	STATE	JOB NO.	SHEET HO.	TOTAL SHEETS
		6	ARK.	080614	49	82
			07565	- 200'-0" UNIT	64438	



TYPICAL ROADWAY SECTION Looking Ahead
½" = 1'-0"

1 Working Point to Gutterline

(2) Tolerance: Minus = ¼";

Plus equal to the amount of slab thickening used to meet slab thickness tolerance. See "ADJUSTMENT FOR SLAB THICKNESS TOLERANCE" on Std. Dwg.

③ See "ADJUSTMENT FOR SLAB THICKNESS TOLERANCE" on Std. Dwg. No. 55007.

(4) If permanent steel bridge deck forms are used, the Fabricator shall clip plates as necessary to accommodate the deck form supports.

Slab Reinforcing:

Longitudinal: S401E (Bottom) and S502E (Top) placed as shown S505E placed as shown over end supports and S501E placed as shown over intermediate supports, see "HALF-REINFORCING PLAN AND SLAB POURING SEQUENCE", Dwg. No. 64441.

Transverse: S503E @ 6" o.c. in top, Alternate S402E & S503E @ 6" o.c. in bottom S504E @ 6" in top of overhang (bundled with S503E)

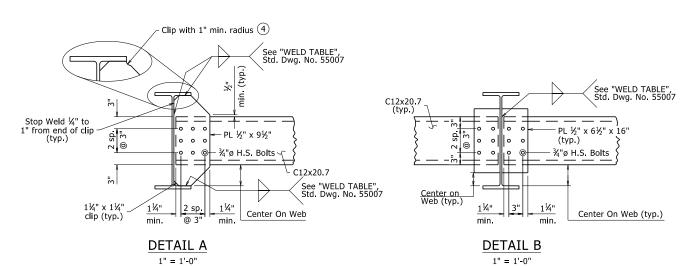
Bar positions or clearances from the forms shall be maintained by means of stays, ties, hangers, or other approved devices per Subsection 804.06. Placement of slab bolsters or high-chairs with full-length lower runners directly on removable deck forms will not be allowed.

Class 2 Protective Surface Treatment shall be applied to the Roadway Surface and the Roadway Face and Top of Concrete Bridge Rail in accordance with Section 803.

BAR LIST

	MARK	NO. REQ'D.	LENGTH	P.D.	BENDING DIAGRAMS
	S401E	360	41'-9"	Str.	2'-2"
	S402E	211	36'-2"	Str.	8'-0"
	S403E	84	7'-0"	2"	
					50 S403E S505E
	S501E	152	24'-0"	Str	N
	S502E	160	52'-0"	Str.	11"
	S503E	597	36'-2"	Str.	
	S504E	786	7'-9"	Str.	
	S505E	76	8'-9"	Str.	1 <u>S601E</u>
	S506E	66	5'-0"	3¾"	3001E
	S601E	12	8'-0"	4½"	<u>*</u>
					3'-5" W402E
(5)	R400E	80	5'-3"	2½"	 33
(5)	R401E	724	5'-11"	2½"	\\
	R402E	96	5'-6"	Str.	
(5)	R403E	804	3'-6"	3", 3¾"	W501E
(5)	R404E	160	9'-8"	Str.	
	R405E	32	10'-2"	Str.	
	R406E	80	19'-8"	Str.	
	R407E	32	4'-0"	Str.	
(5)	W401E	80	3'-11"	3¾"	
	W402E	60	6'-1"	2"	
	W501E	32	7'-1"	3¾"	
	W701E	40	12'-2"	Str.	(5) See Std. Dwg. No. 55070 for additional details.

All bars designated with an "E" suffix are to be epoxy coated. Dimensions are out to out of bars.



Bolts in diaphragm connections shall be properly installed and tightened in accordance with Subsection 807.71.



SHEET 1 OF 5 **DETAILS OF 200'-0"** INTEGRAL CONTINUOUS W-BEAM UNIT

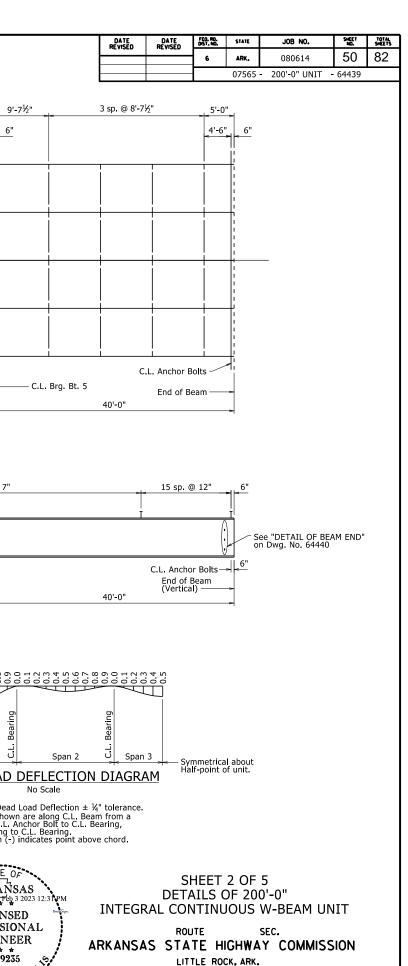
ROUTE ARKANSAS STATE HIGHWAY COMMISSION

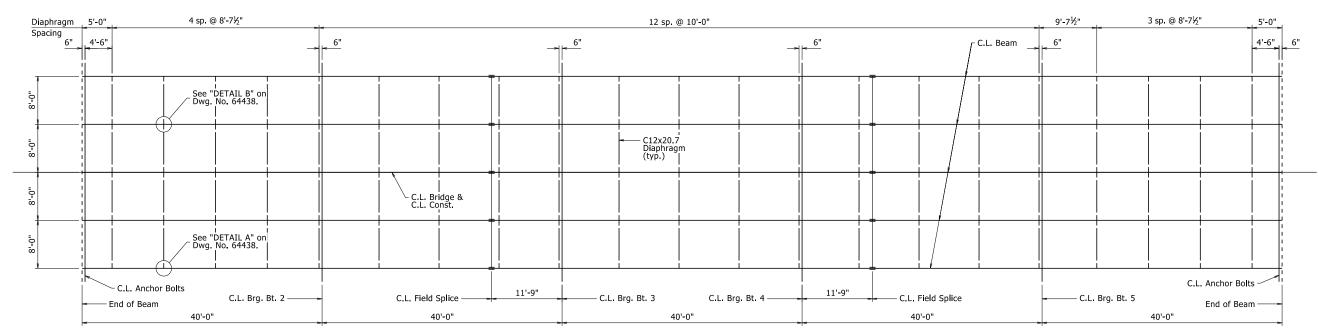
> LITTLE ROCK, ARK. JCG DATE: 6/17/2021 FILENAME: b080614_s1.dgn

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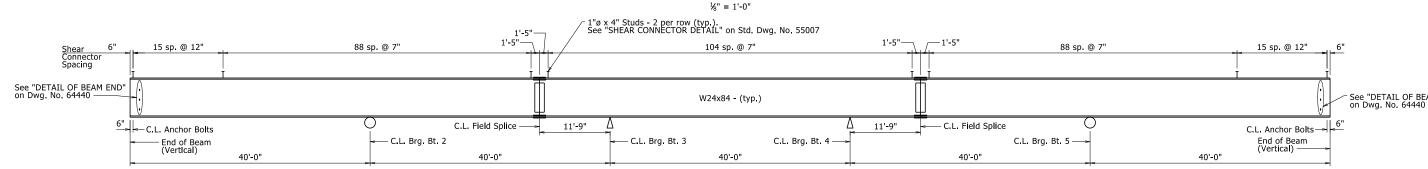
BRIDGE NO. 07565 DRAWING NO. 64438

SCALE: As Shown





FRAMING PLAN



All structural steel shall be ASTM A709, Grade 50W unless otherwise noted and shall be paid for as "Structural Steel in Beam Spans (A709, Gr. 50W)". See Std. Dwg. Nos. 55006 and 55007 for additional notes and details.

2'-1"

C.L. Beam -

3 sp. @ 3" 2" 2" 3 sp. @ 3"

-600|6:6006|

-0 0 0 0 1 0 \ 0 0 0

FLANGE SPLICE

0 0/0 0

- PLs ¾6" x 3¼" x 2'-1"

0 0 0

PL 16" x 9" x 2'-1"

-0 0 0

C.L. Field Splice -

1½"

0 0 0

TYPICAL BEAM ELEVATION No Scale

TABLE OF DEAD LOAD DEFLECTIONS - INCHES l + Slab

Span	Point of	Structu	ral Steel	Structural S	Steel + Slab	Structural S + F	Steel + Sla Rail
Sp	Deflection	Exterior Beam	Interior Beam	Exterior Beam	Interior Beam	Exterior Beam	Interior Beam
	0	0	0	0	0	0	0
	0.1	0.014	0.015	0.116	0.147	0.128	0.155
	0.2	0.025	0.028	0.214	0.273	0.236	0.287
	0.3	0.034	0.037	0.285	0.362	0.315	0,381
11	0.4	0.038	0.042	0.319	0.407	0.353	0.428
Span	0.5	0.037	0.041	0.315	0.402	0.349	0.423
S	0.6	0.033	0.036	0.277	0.353	0.307	0.372
	0.7	0.025	0.028	0.211	0.270	0.234	0.284
	0.8	0.015	0.017	0.130	0.166	0.144	0.175
	0.9	0.006	0.007	0.052	0.067	0.058	0.071
	0	0	0	0	0	0	0
	0.1	-0.001	-0.001	-0.009	-0.012	-0.009	-0.012
	0.2	0.001	0.001	0.011	0.014	0.014	0.016
	0.3	0.005	0.005	0.041	0.052	0.048	0.057
7	0.4	0.008	0.009	0.068	0.086	0.079	0.093
Span	0.5	0.010	0.011	0.081	0.104	0.093	0.112
ζ	0.6	0.009	0.010	0.078	0.099	0.090	0.106
	0.7	0.007	0.008	0.059	0.076	0.068	0.082
	0.8	0.004	0.004	0.031	0.040	0.036	0.043
	0.9	0.001	0.001	0.006	0.008	0.007	0.009
	0	0	0	0	0	0	0
	0.1	0.003	0.003	0.027	0.034	0.030	0.036
5 ر	0.2	0.008	0.009	0.071	0.091	0.079	0.096
Span	0.3	0.014	0.015	0.117	0.148	0.130	0.156
S	0.4	0.018	0.019	0.149	0.189	0.166	0.200
	0.5	0.019	0.021	0,160	0,204	0.178	0.215

LICENSED * * * No. 9235 BRIDGE ENGINEER

DEAD LOAD DEFLECTION DIAGRAM

> Camber for Dead Load Deflection $\pm \frac{1}{4}$ " tolerance. Deflections shown are along C.L. Beam from a chord from C.L. Anchor Bolt to C.L. Bearing, or C.L. Bearing to C.L. Bearing, Negative sign (-) indicates point above chord.

ARKANSAS Feb 3 2023 12:31 PM PROFESSIONAL **ENGINEER**

JCG DATE: 6/17/2021 FILENAME: b080614_s1.dgn CHECKED BY: DKS DATE: 8/23/2021 SCALE: As Shown

DESIGNED BY: JCG DATE: 6/2021 BRIDGE NO. 07565

DRAWING NO. 64439

Bolted field splices may be eliminated or shop welded splices may be substituted with the approval of the Engineer. Payment will be made on the basis of plan quantities. All holes for splice bolts shall be $^{15}\!\!/_{16}$ "ø.

2'-1"

0 0 0 1 0 0 0

0 0 0 0 0 0

0001000

2 sp. 2" 2" 2 sp.

WEB SPLICE

1'-0½"

C.L. Field Splice

-0 0 0

0 0 0

ο φ

@ 3"

172

17,

1'-0½"

0 0 0

0 0 0

QΟ

@ 3"

_ 1½"

FIELD SPLICE DETAIL 1" = 1'-0"

- PL ¾6" x 9" x 2'-1"

- 2 - PLs 1/₁₆" x 31/₄" x 2'-1"

2 - PLs 16" x 314" x 2'-1"

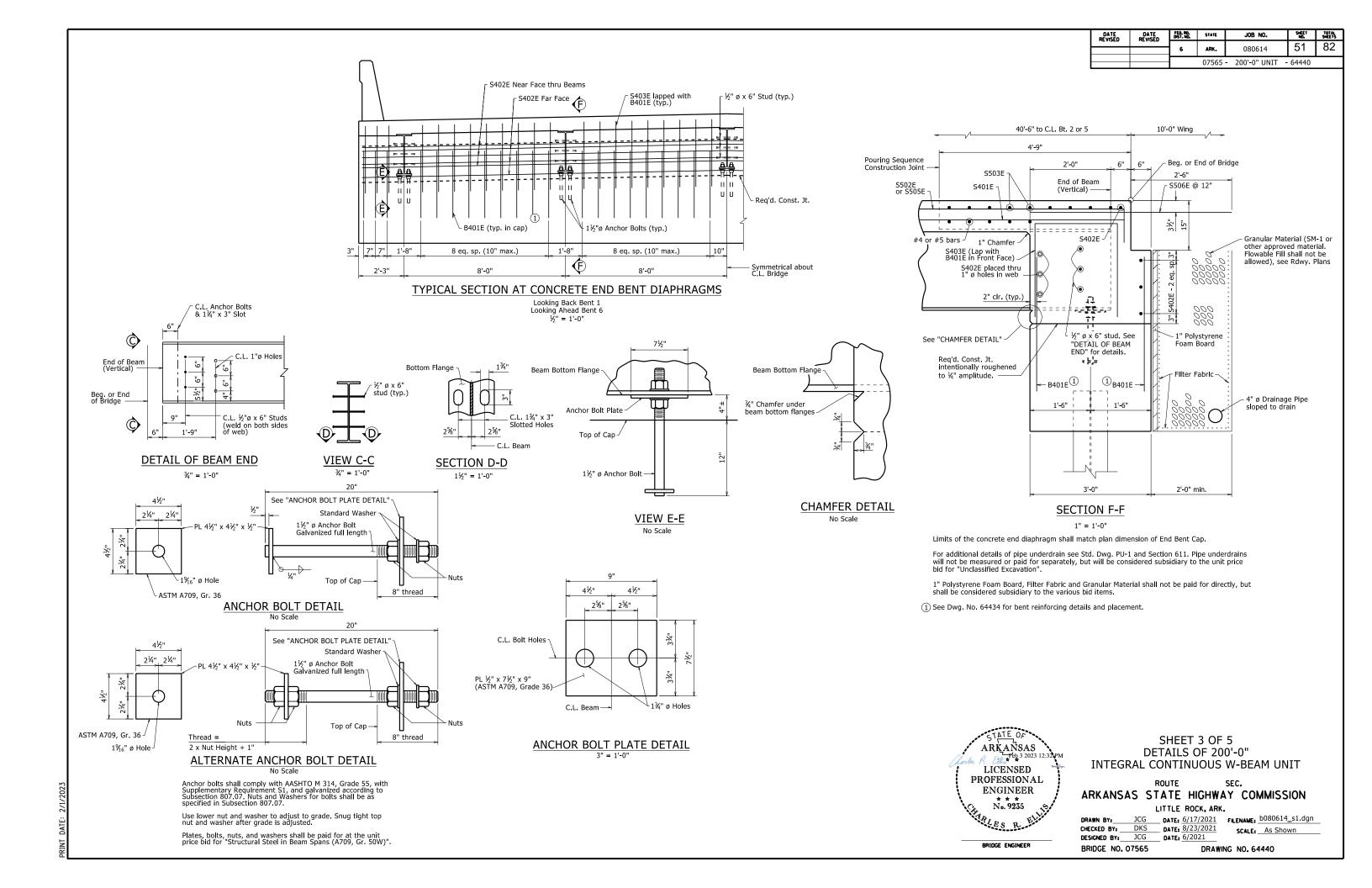
- PL ¾6" x 9" x 2'-1"

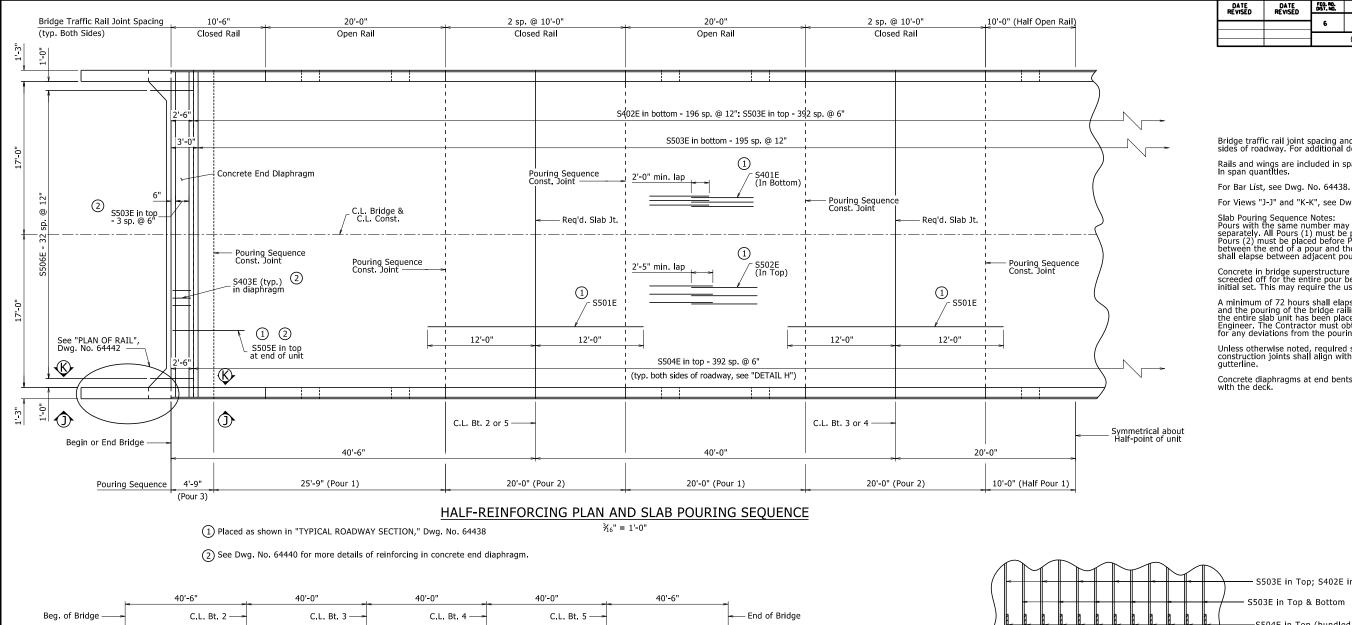
2-PL's ½6" x 1'-6" x 1'-7"

%"ø H.S. bolts with

 $^{15}/_{16}$ "ø holes (typ.)

All field splice bolts shall be \%"\otin H.S. bolts.





SHEET TOTAL NO. SHEETS FEO. RO. STATE JOB NO. 82 080614 52 ARK. 07565 - 200'-0" UNIT - 64441

Bridge traffic rail joint spacing and depth shown is typical for both sides of roadway. For additional details, see Std. Dwg. No. 55070.

Rails and wings are included in span construction and are included In span quantitles.

For Views "J-J" and "K-K", see Dwg. No. 64442.

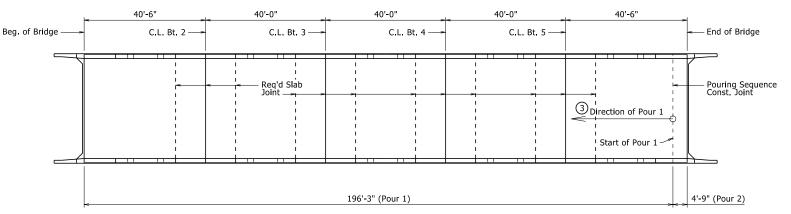
Slab Pouring Sequence Notes:
Pours with the same number may be placed simultaneously or separately. All Pours (1) must be placed before Pours (2), & all Pours (2) must be placed before Pours (3), 48 hours shall elapse between the end of a pour and the start of the next pour. 72 hours shall elapse between adjacent pours.

Concrete in bridge superstructure shall be placed, consolidated and screeded off for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent.

A minimum of 72 hours shall elapse between completion of the slab and the pouring of the bridge railing. Any railing pours made before the entire slab unit has been placed must be approved by the Engineer. The Contractor must obtain approval from the Engineer for any deviations from the pouring sequence(s) shown.

Unless otherwise noted, required slab joints and pouring sequence construction joints shall align with bridge traffic rail joints at the gutterline.

Concrete diaphragms at end bents shall be poured monolithically with the deck.



S503E in Top; S402E in Bottom S503E in Top & Bottom -S504E in Top (bundled with S503E bar) Gutterline (typ.) **DETAIL H** No Scale

ALTERNATE POURING SEQUENCE

½₁₆" = 1'-0"

3 Direction of pour shall be from near Bent 6 progressing to Bent 1. If stay-in-place forms are used and installed in a manner that requires pouring of the slab in the opposite direction, this Alternate Pouring Sequence shall be modified accordingly to where Closure Pour (2) is at Bent 1 and Pour (1) progresses from near Bent 1 to Bent 6.

TABLE OF VARIABLES BRIDGE TRAFFIC RAILING (TYPE SSTR36)

Close	d Rail Pa	inels		Open Rail Panels					
Panel Length	"A"	R4XXE Bar	Panel Length	"B"	"C"	"D"	"E"	"F"	R4XXE Bar
10'-0"	19	R404E	20'-0"	7	11	4'-0"	15	8'-0"	R406E
10'-6"	20	R405E							



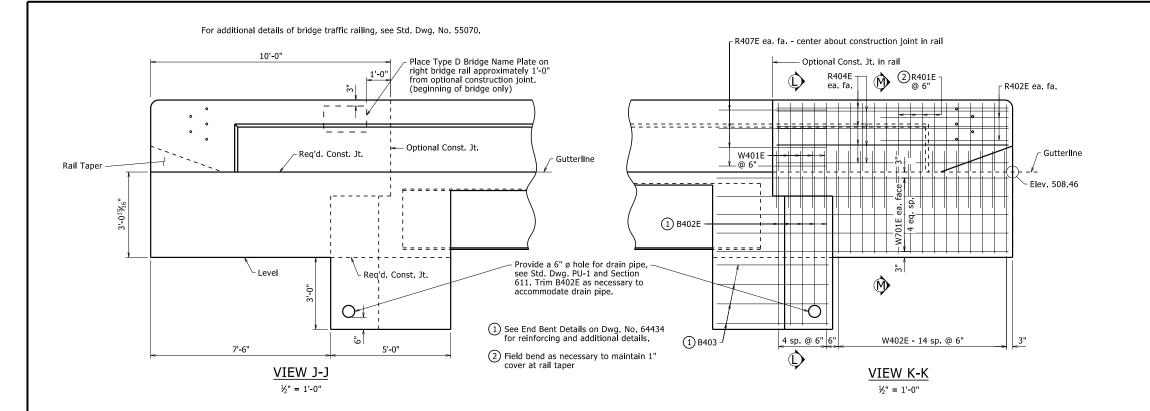
SHEET 4 OF 5 **DETAILS OF 200'-0"** INTEGRAL CONTINUOUS W-BEAM UNIT

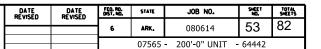
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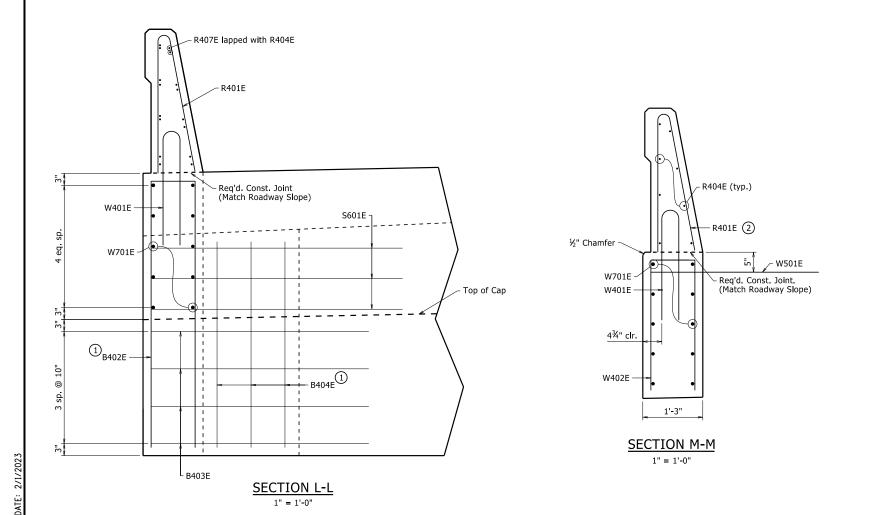
LITTLE ROCK, ARK.

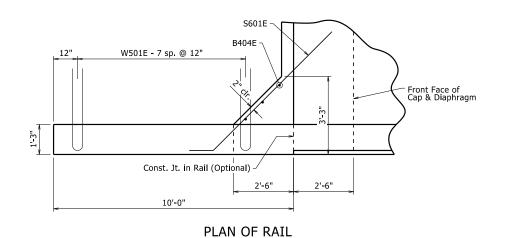
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DESIGNED BY: JCG DATE: 6/2021 BRIDGE NO. 07565 DRAWING NO. 64441









½" = 1'-0"



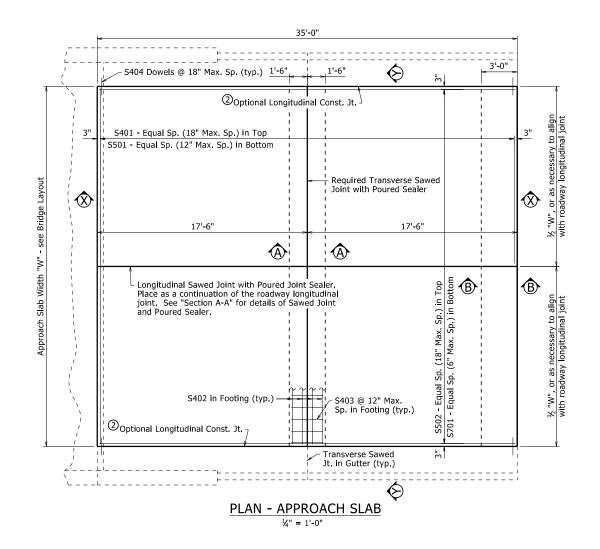
SHEET 5 OF 5 **DETAILS OF 200'-0"** INTEGRAL CONTINUOUS W-BEAM UNIT

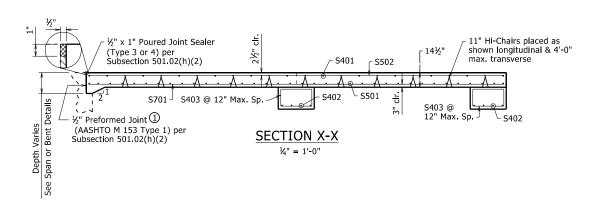
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LITTLE ROCK, ARK. JCG DATE: 6/17/2021 FILENAME: b080614_s1.dgn CHECKED BY: DKS
DESIGNED BY: JCG
DATE: 6/2021
DATE: 6/2021 SCALE: As Shown BRIDGE NO. 07565

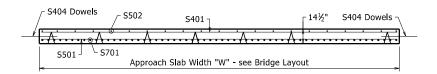
DRAWING NO. 64442

DATE REVISED	DATE REVISED	FEO. RO. 061. NO.	STATE	JOB NO.	94EE1 NO.	TOTAL SHEETS	
		6	ARK.	080614	54	82	
			07565	APPR, SLAB	- 64442A		





- ① Eliminate Type 1 Preformed Joint when bridge details show reinforcing dowels across these joints. Poured joint sealer is required, however, backer rod shall be eliminated.
- ② When construction joint is eliminated, place 1" Sawed Joint with ½" x 1" Poured Joint Sealer (Type 3 or 4) per Subsection 501.02(h)(2). Backer rod is not required.



SECTION Y-Y 1/4" = 1'-0"

QUANTITIES FOR ONE TYPE SPECIAL APPROACH SLAB

(FOR INFORMATION ONLY)

"W"	Reinforcing Steel (Lbs.)	Concrete (Cu. Yds.)
30'-0"	7,140	59.30

TYPE SPECIAL APPROACH SLAB

Mark	No. Req'd.	Length	Bending Diagram
S401	24	29'-8"	2'-7"
S402	8	29'-8"	E G .
S403	60	8'-4"	(typ. 7
S404	48	1'-6"	\$403 \$403 \$403
S501	36	29'-8"	<u> </u>
S502	21	34'-8"	
S701	60	34'-8"	†

BAR LIST FOR ONE

Dimensions are out to out of bar

½" x 1" Poured Joint Sealer (Type 3 or 4) per Subsection 501.02(h)(2). Backer rod is not required. 1½"

LONGITUDINAL CONSTRUCTION JOINT

½" = 1'-0"

GENERAL NOTES

 $\overline{\text{All concrete shall be Class S(AE)}}$ with a minimum 28 day compressive strength f'c = 4,000 psl and shall be poured in the dry.

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322, Type A, with mill test reports.

The surface finish for Approach Slabs shall match that used on the bridge deck.

Approach Slabs will be measured and paid for in accordance with Section 504.



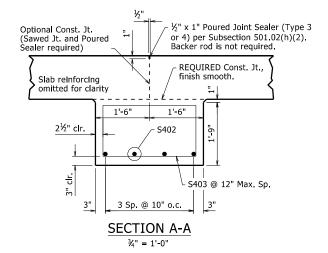
DETAILS OF TYPE SPECIAL APPROACH SLAB

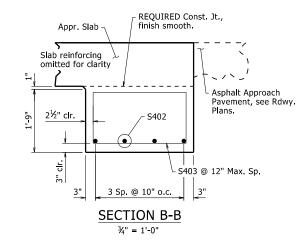
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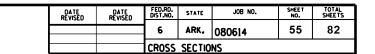
LITTLE ROCK, ARK.

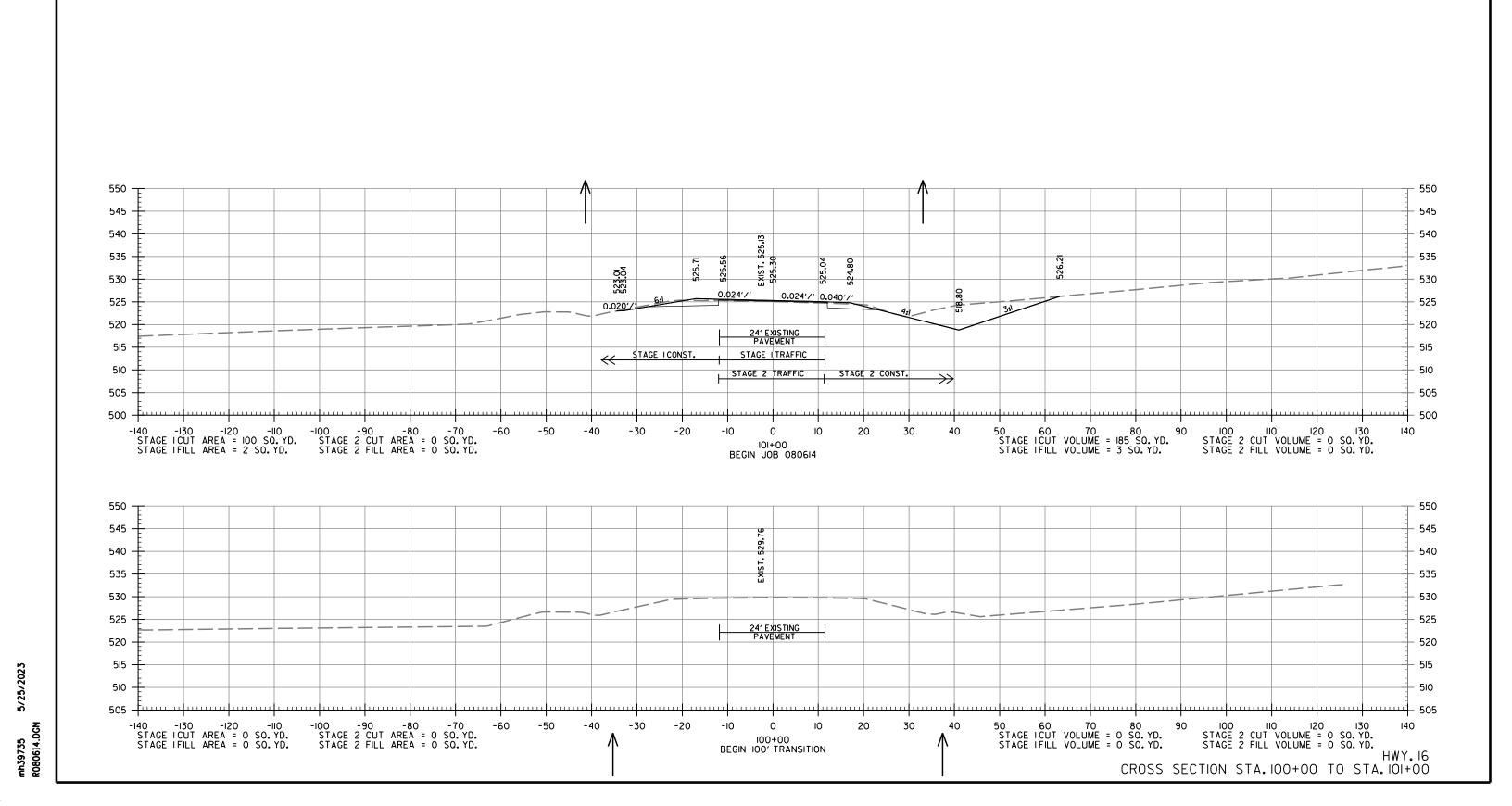
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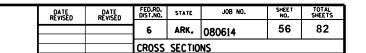
BRIDGE NO. 07565 DRAWING NO. 64442A

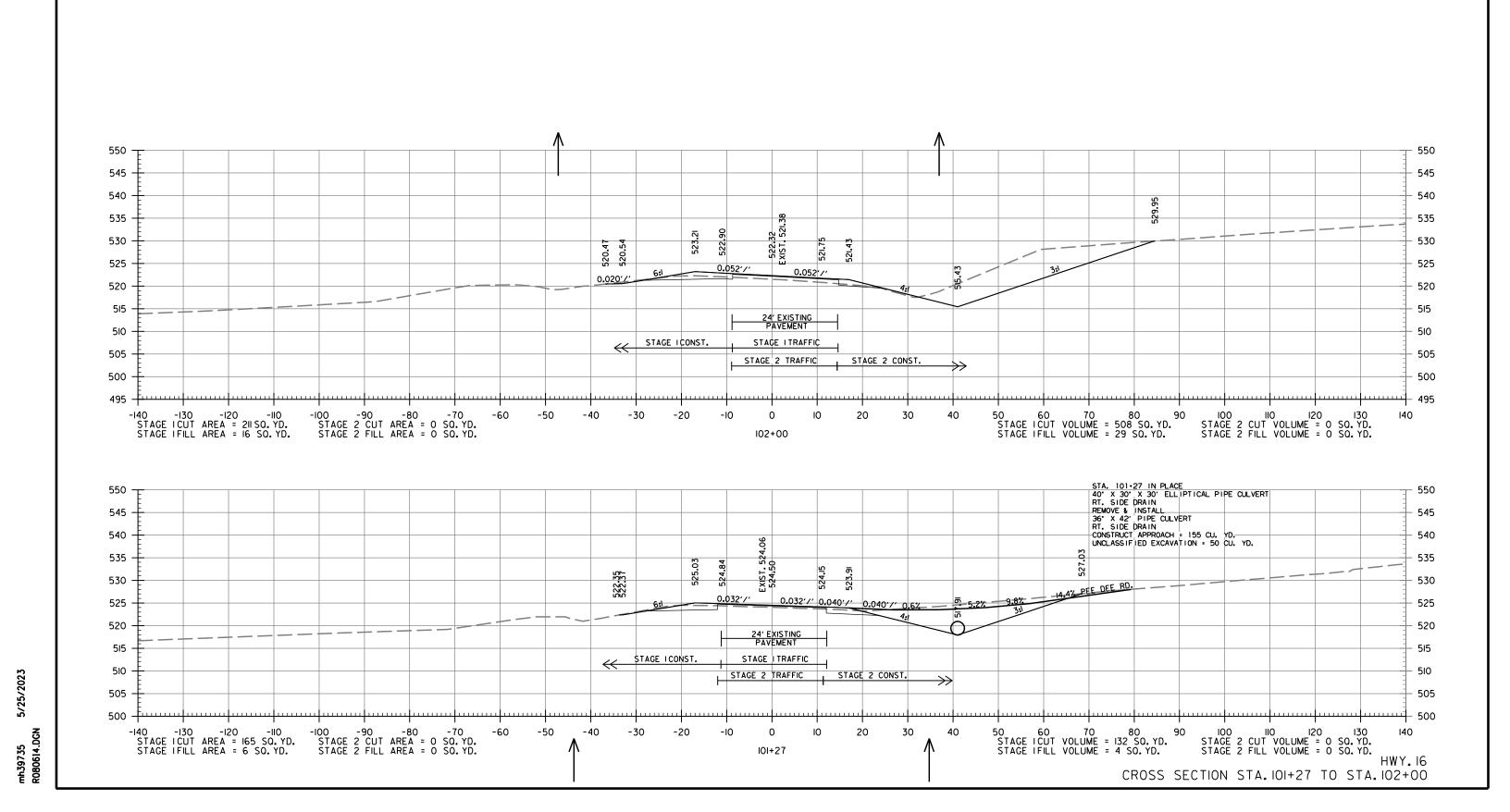


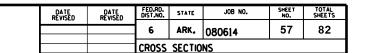


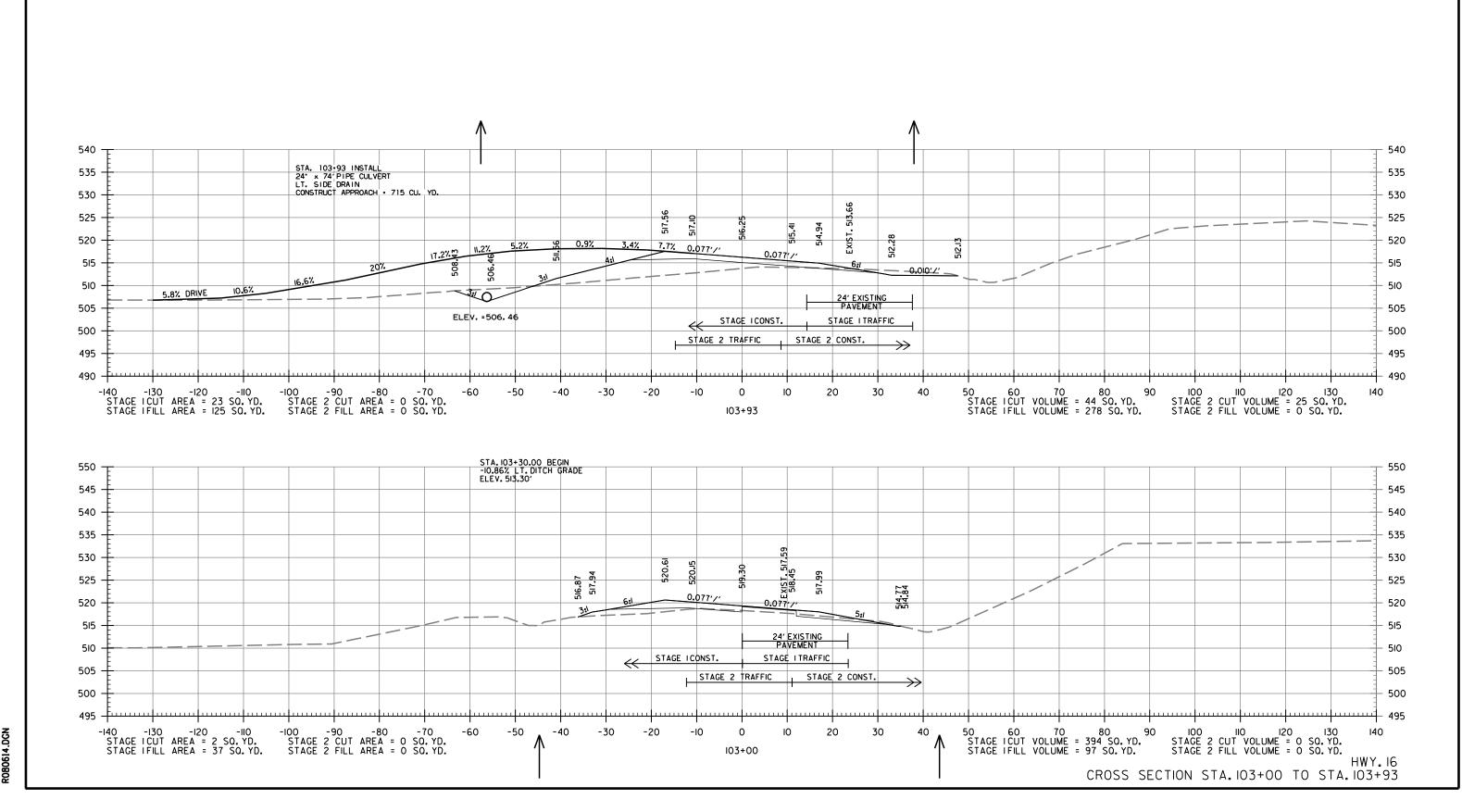


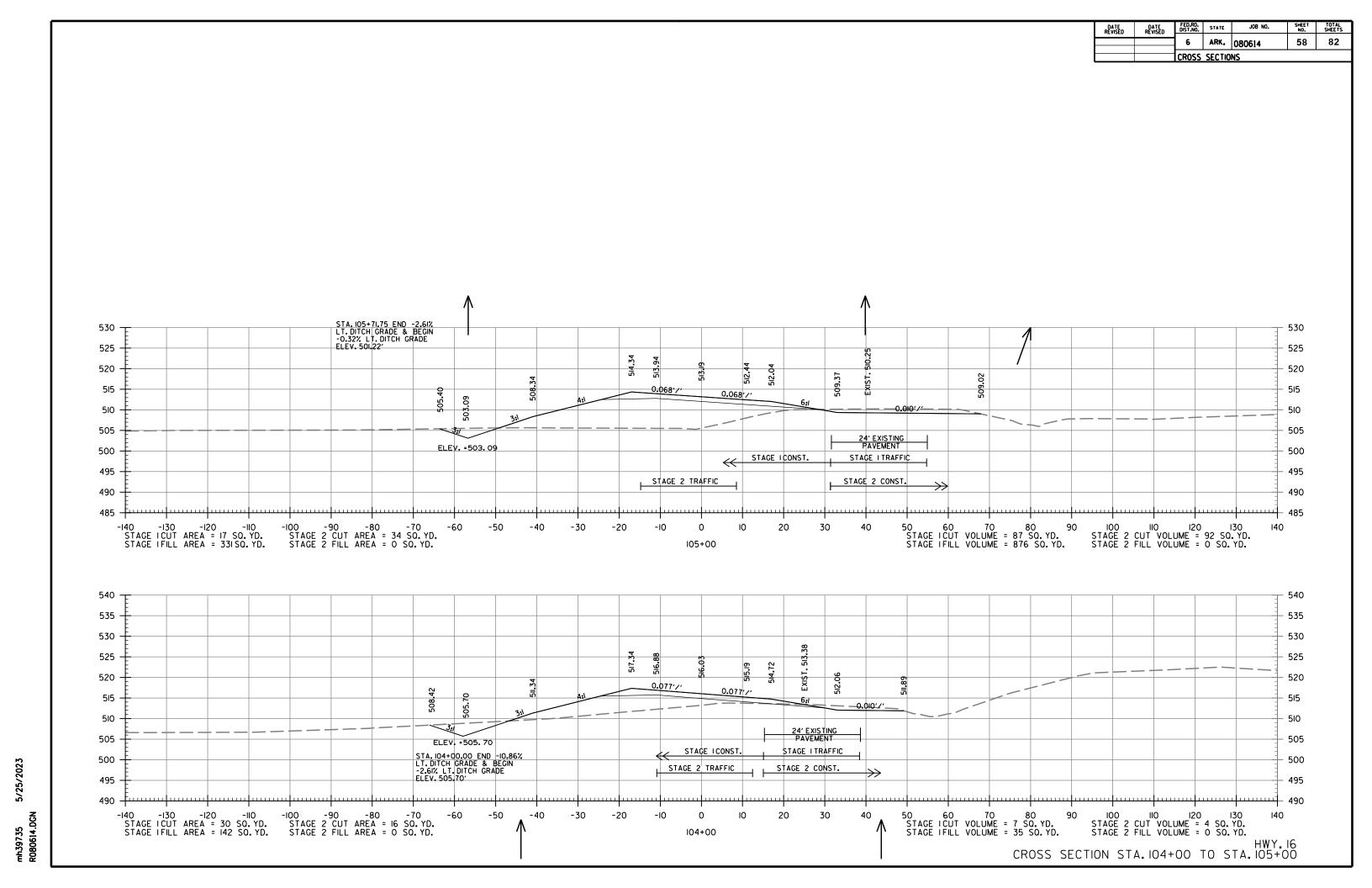


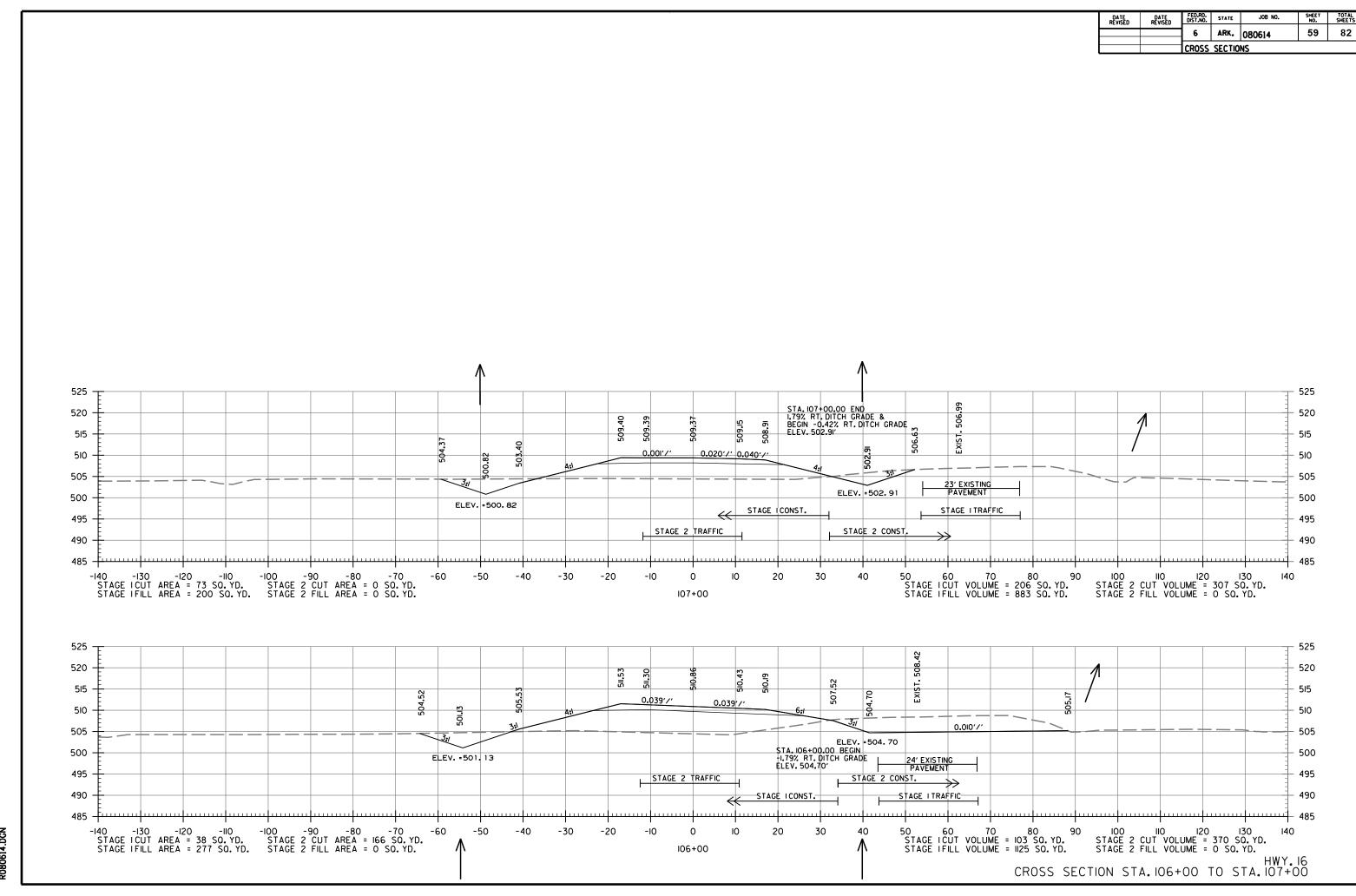






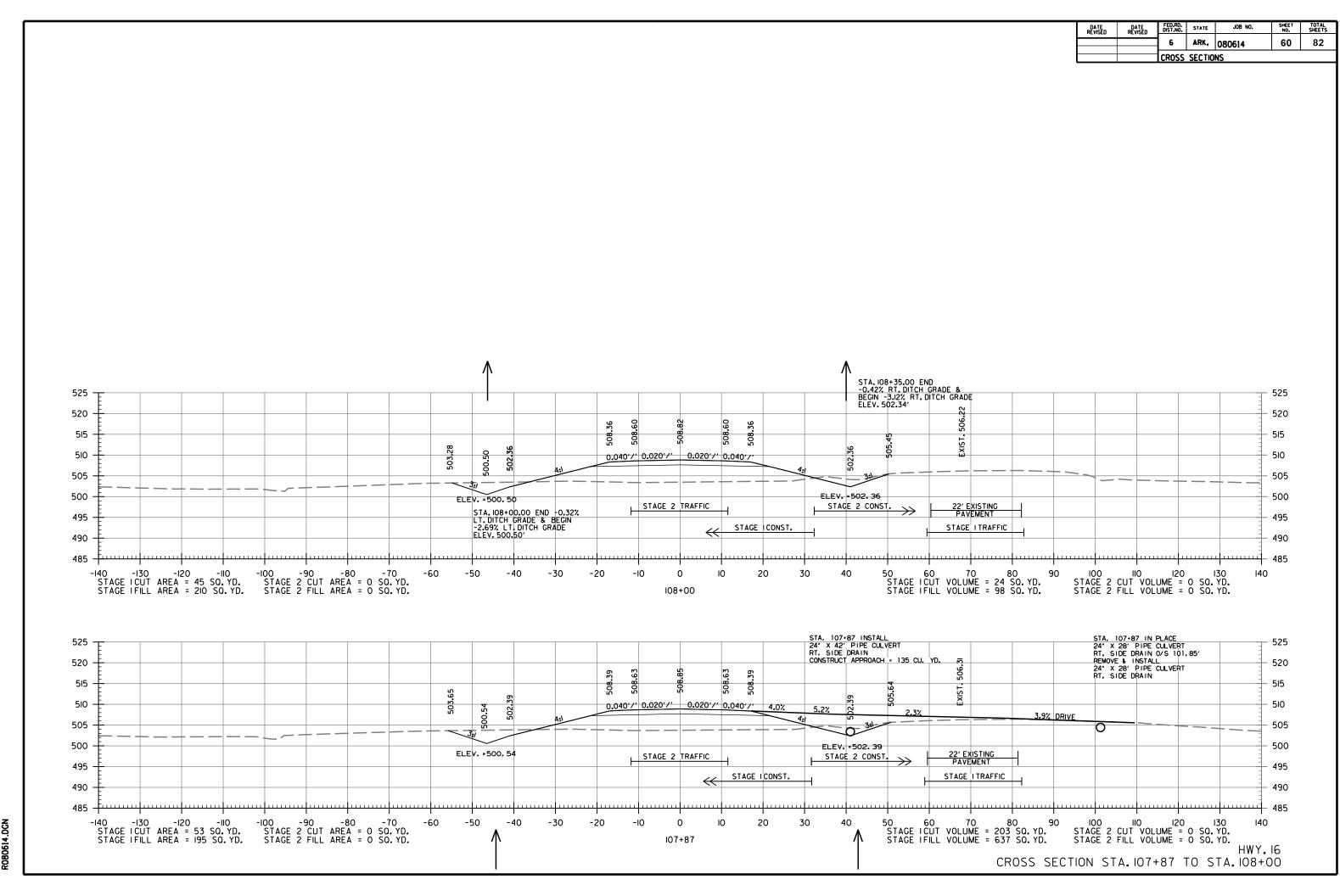






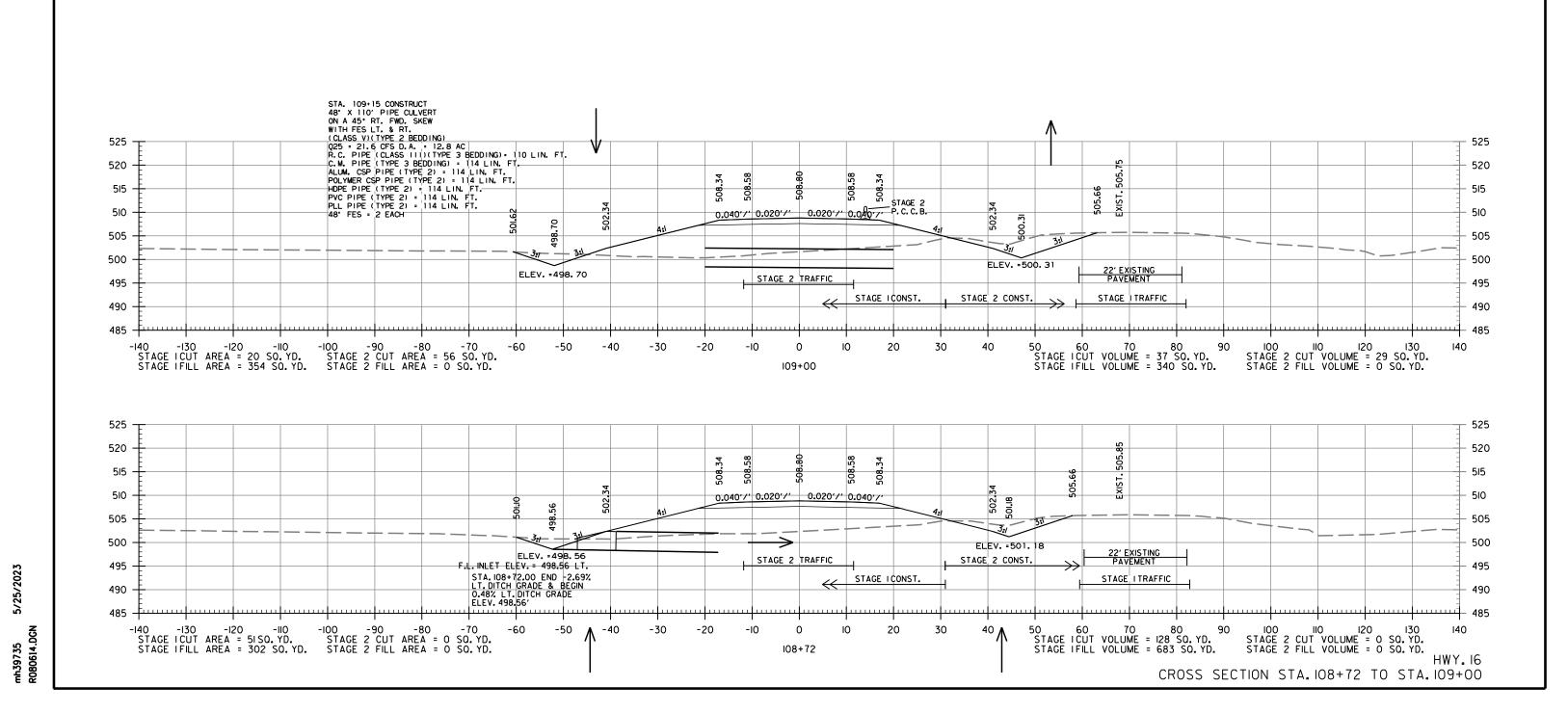
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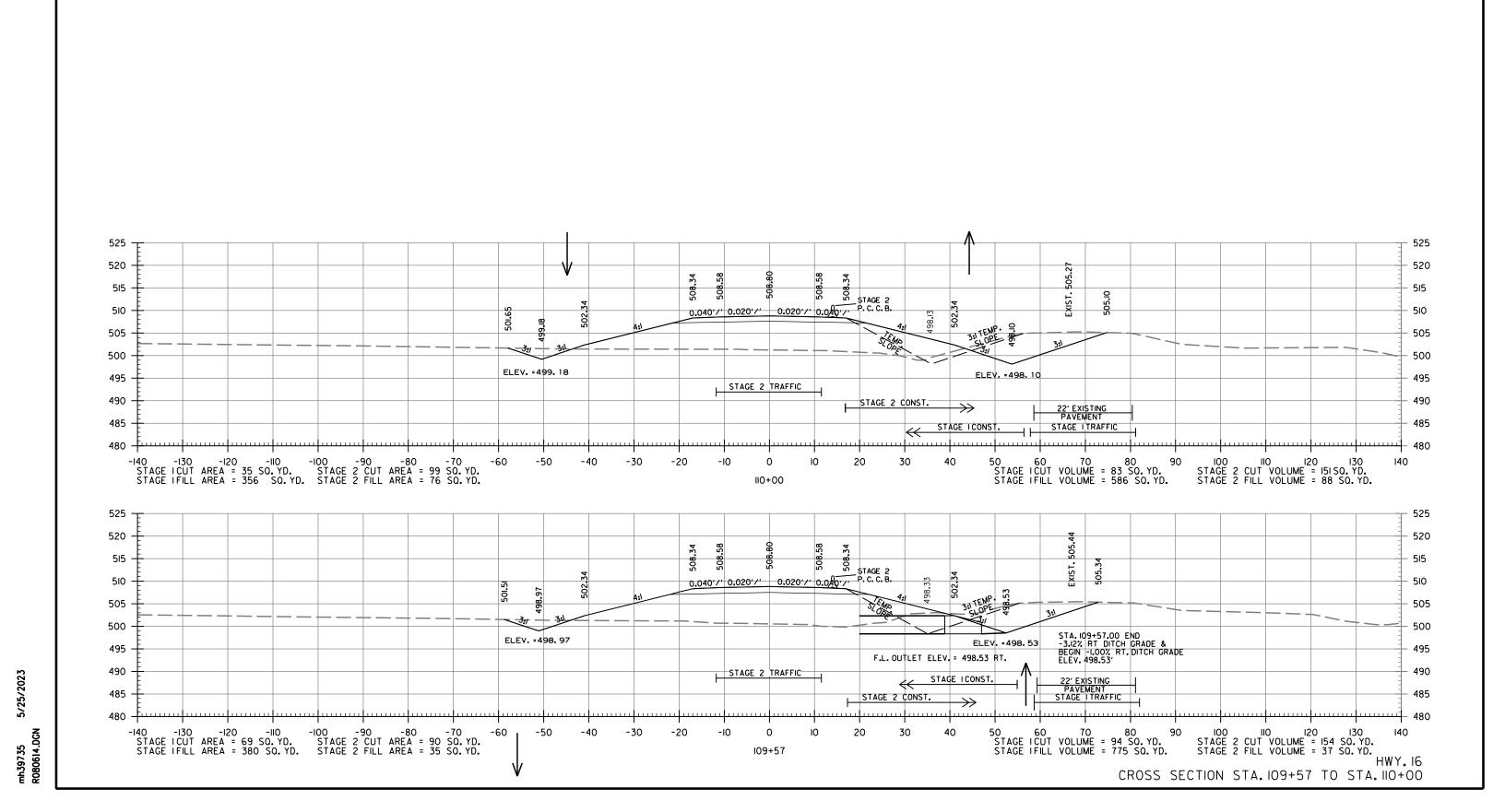


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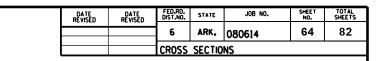
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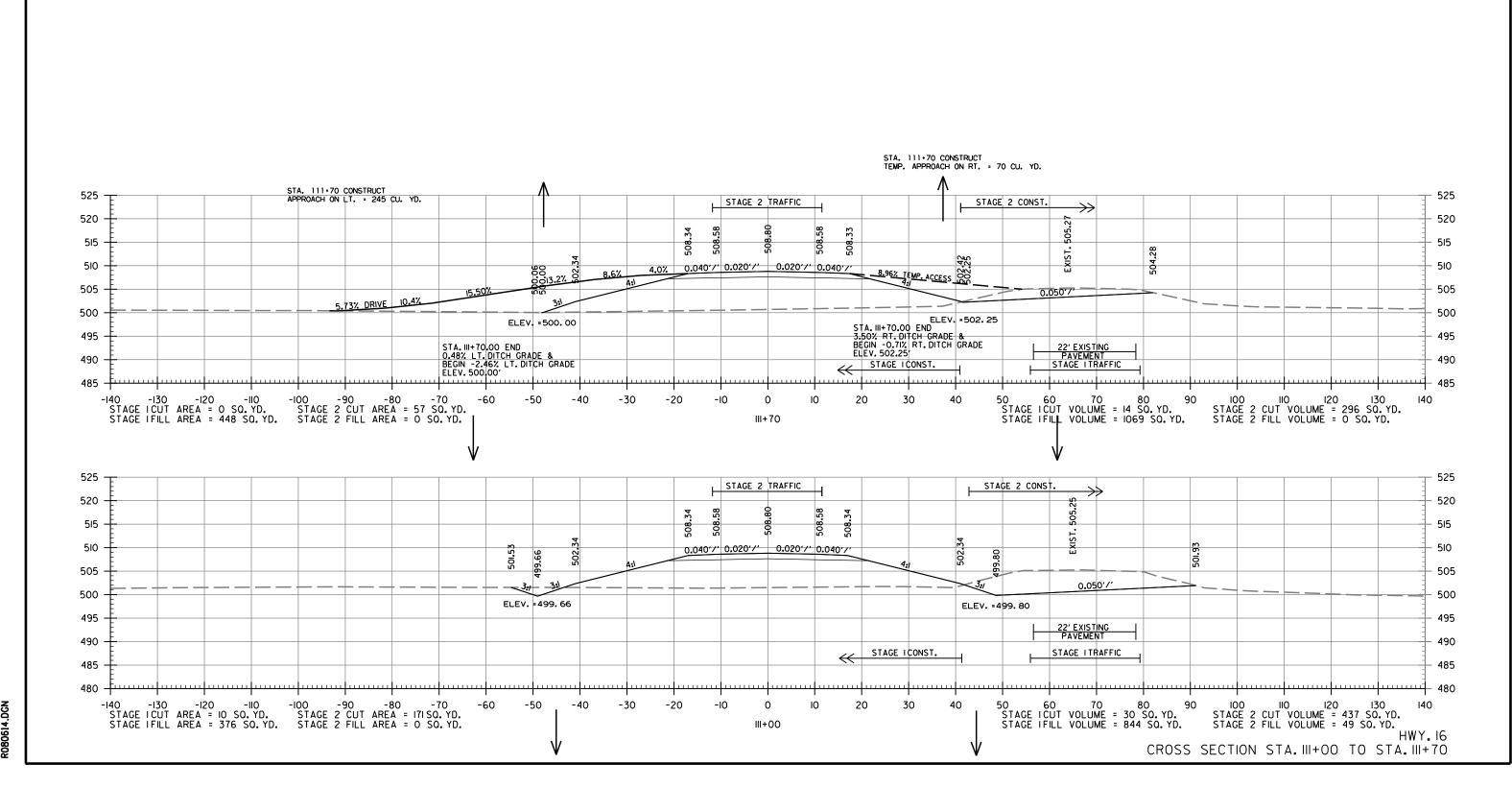


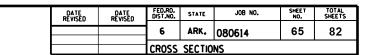
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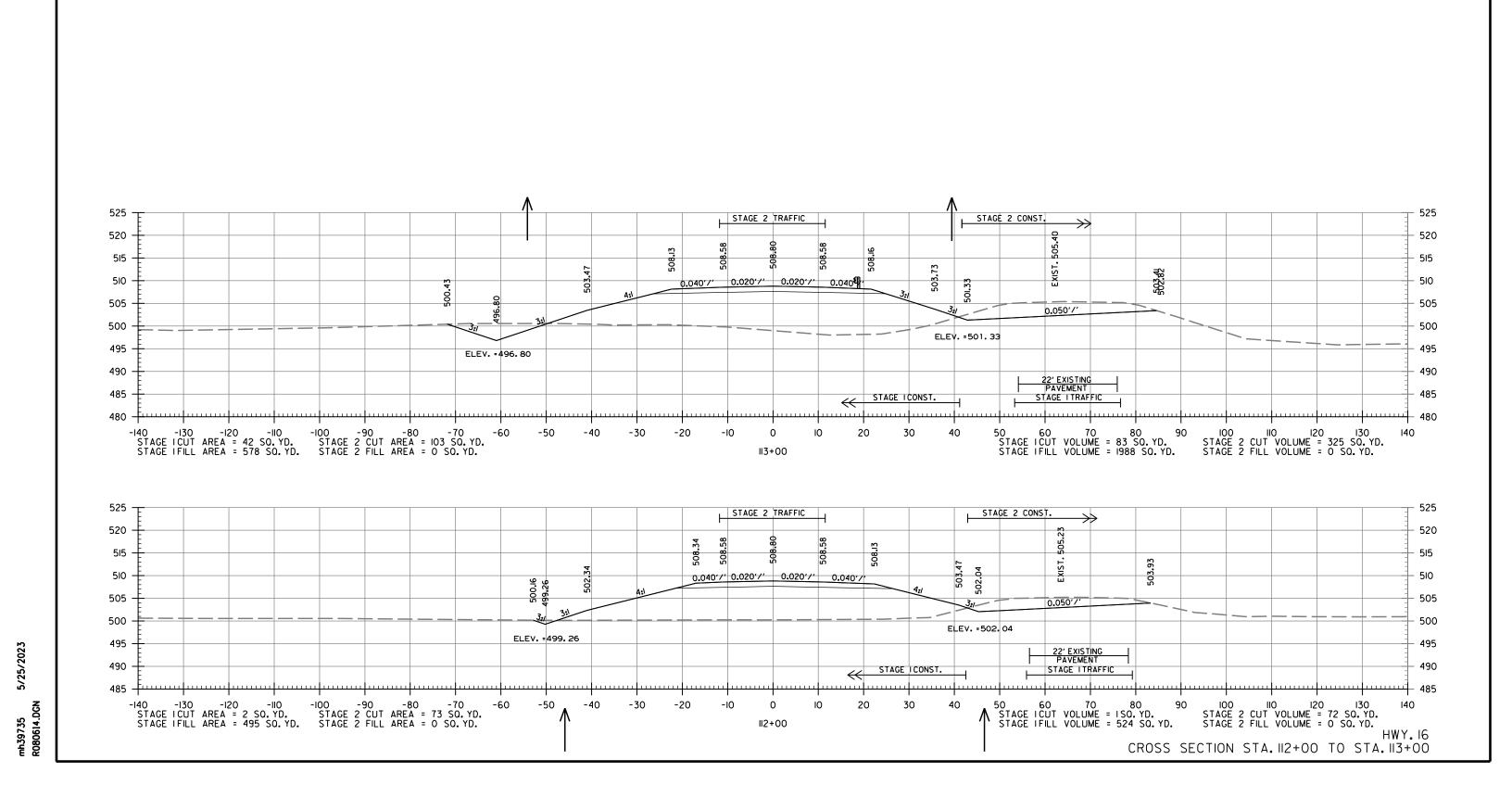


DATE REVISED DATE REVISED FED.RD. DIST.NO. STATE 6 ARK. 080614 63 82 CROSS SECTIONS 525 525 STAGE 2 TRAFFIC STA. 110+40.00 IN PLACE 4'X 5'X 32' R.C. BOX CULVERT WITH HEADWALLS LT.& RT. REMOVE 520 520 0.040'/' 0.020'/' 0.020'/' 0.040'/' 0.020'/' 515 515 510 510 2 505 505 500 500 STA. IIO+40.00 END -LOO% RT DITCH GRADE & BEGIN 3.50% RT. DITCH GRADE ELEV. 497.70 ELEV. = 499.37 ELEV. = 497.70 495 495 EXIST. F.L. INLET ELEV. = 497.87 LT. EXIST. F.L. OUTLET ELEV. = 497.68 RT. 490 STAGE 2 CONST. 490 22' EXISTING PAVEMENT STAGE I TRAFFIC 485 485 \leftarrow 480 -140 -130 -120 -110 -100 -90 -80 -70 STAGE I CUT AREA = 17 SO. YD. STAGE 2 CUT AREA = 222 SO. YD. STAGE 1 FILL AREA = 384 SO. YD. STAGE 2 FILL AREA = 44 SO. YD. 50 60 70 80 STAGE ICUT VOLUME = 39 SO. YD. STAGE IFILL VOLUME = 548 SO. YD. 100 110 120 130 STAGE 2 CUT VOLUME = 238 SQ. YD. STAGE 2 FILL VOLUME = 89 SQ. YD. 30 -60 -50 -40 -30 -20 -10 0 10 20 40 90 110+40 HWY.16 CROSS SECTION STA. IIO+40 TO STA. IIO+40

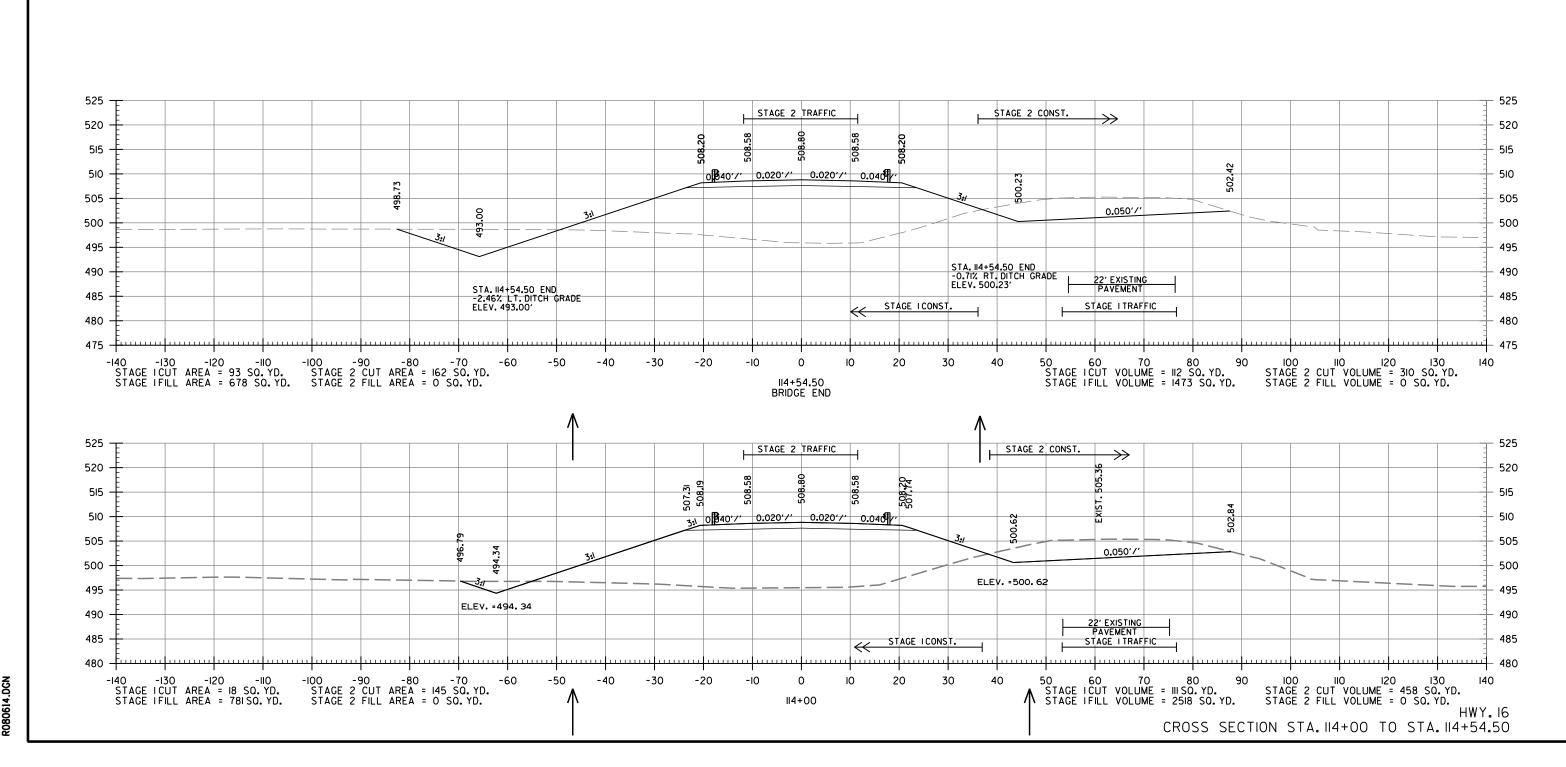




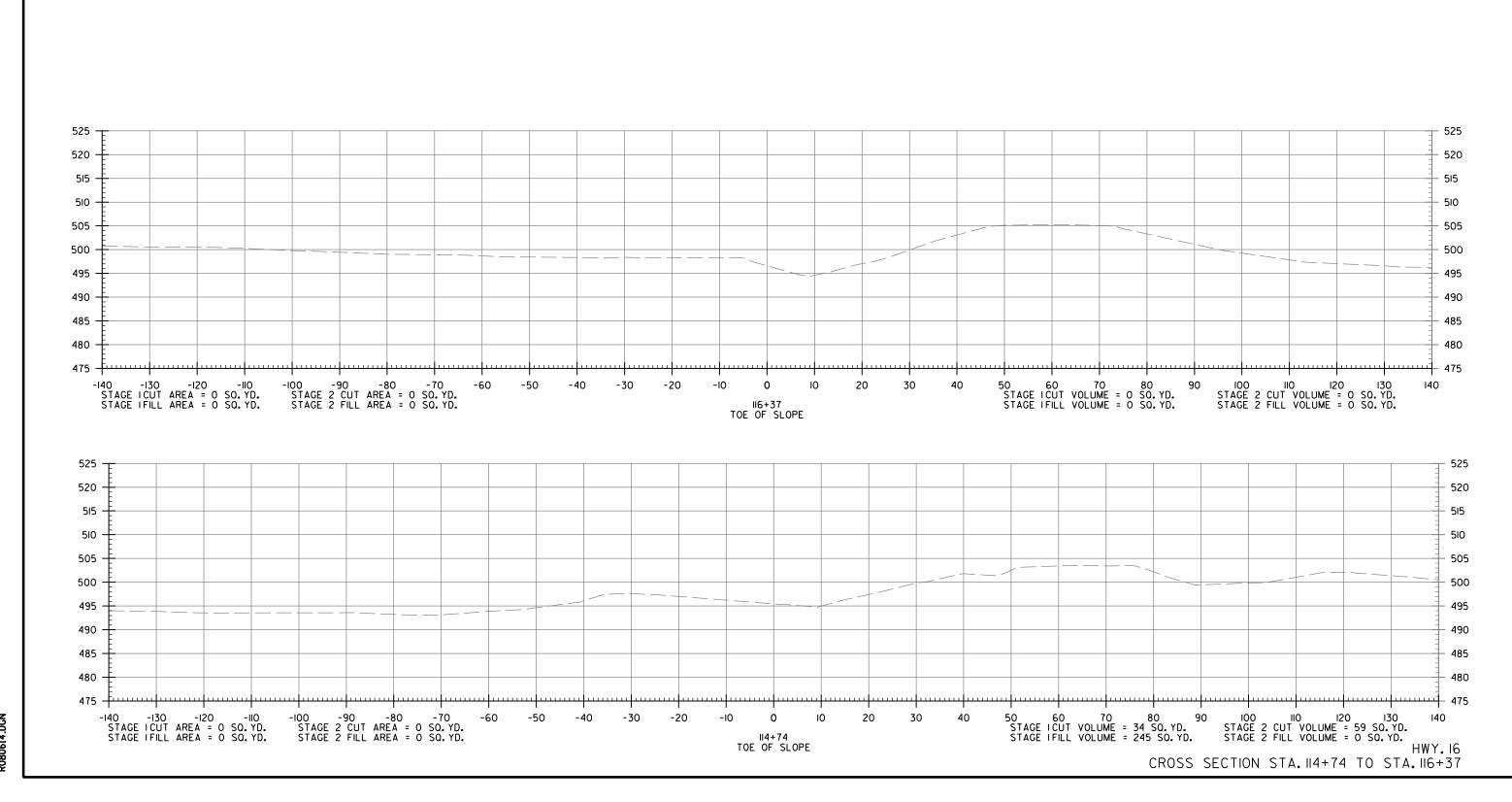




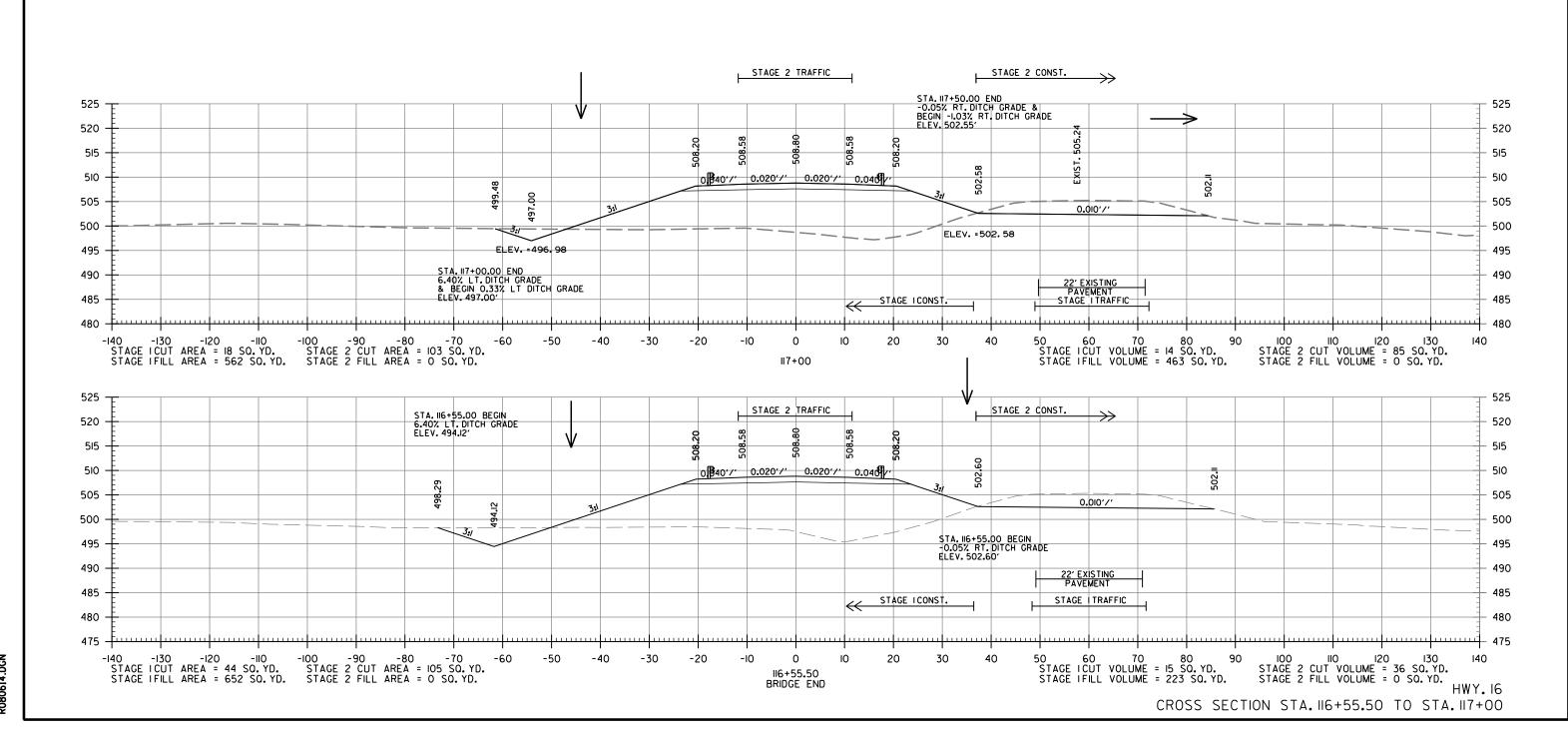
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		CROSS SECTIONS				

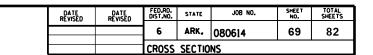


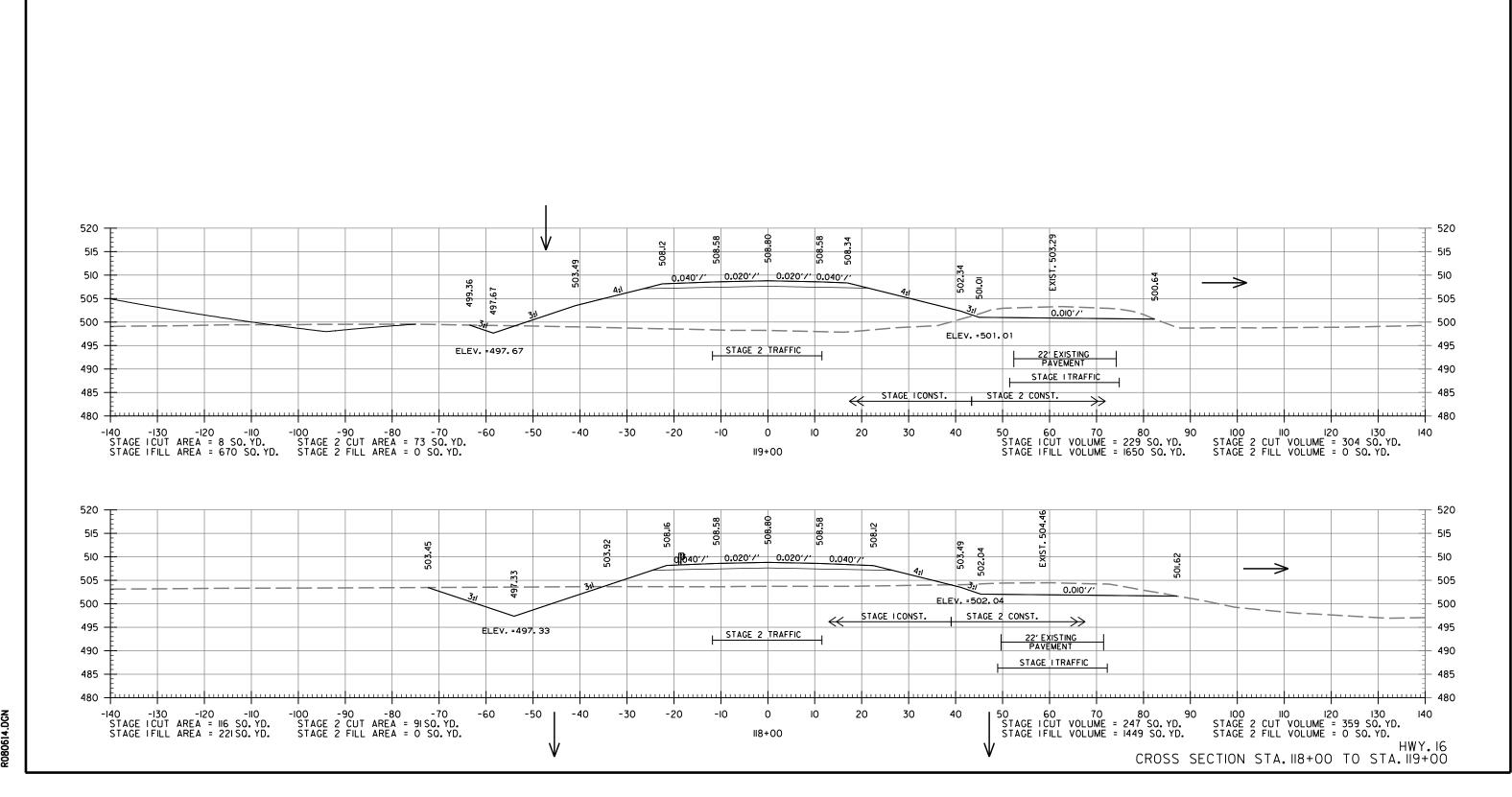
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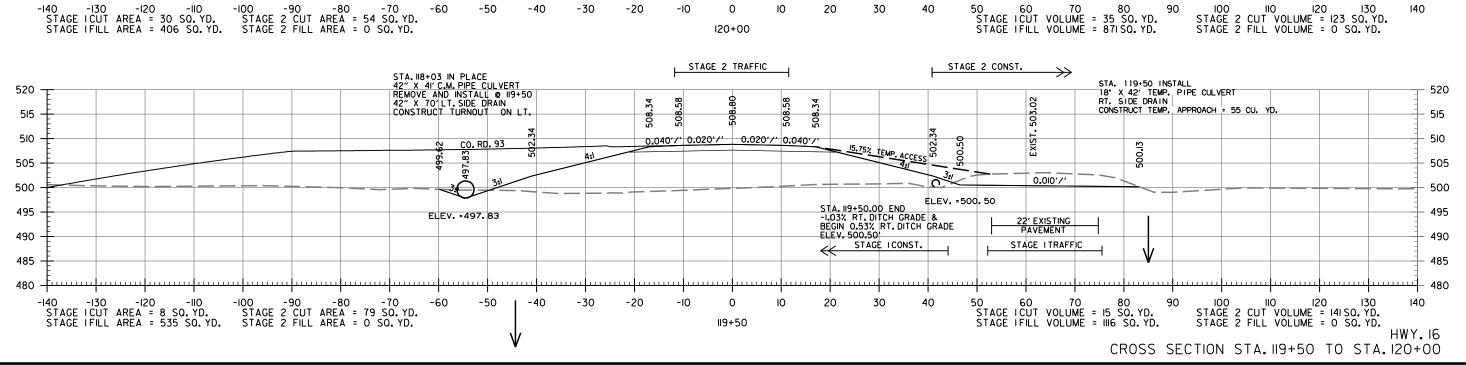
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		CROSS SECTIONS				







FED.RD. DIST.NO. STATE DATE REVISED DATE REVISED 6 ARK. 080614 70 82 CROSS SECTIONS 520 515 510 505 500 495 22' EXISTING PAVEMENT 490 STAGE ITRAFFIC 485 IOO IIO I2O I3O STAGE 2 CUT VOLUME = I23 SO. YD. STAGE 2 FILL VOLUME = O SO. YD. 90 STA. 119+50 INSTALL 18' X 42' TEMP. PIPE CULVERT RT. SIDE DRAIN 520 CONSTRUCT TEMP. APPROACH = 55 CU. YD 515 510 505 500 495



0.040'/' 0.020'/' 0.020'/' 0.040'/

STAGE 2 TRAFFIC

ELEV. =498, 01

-60

STA. 120+00.00 END 0.33% LT. DITCH GRADE & BEGIN 1.93% LT. DITCH GRADE ELEV. 498.00'

-50

-40

-30

-20

ELEV. =500, 76

STAGE 2 CONST.

STAGE I CONST.

30

20

520

515

510

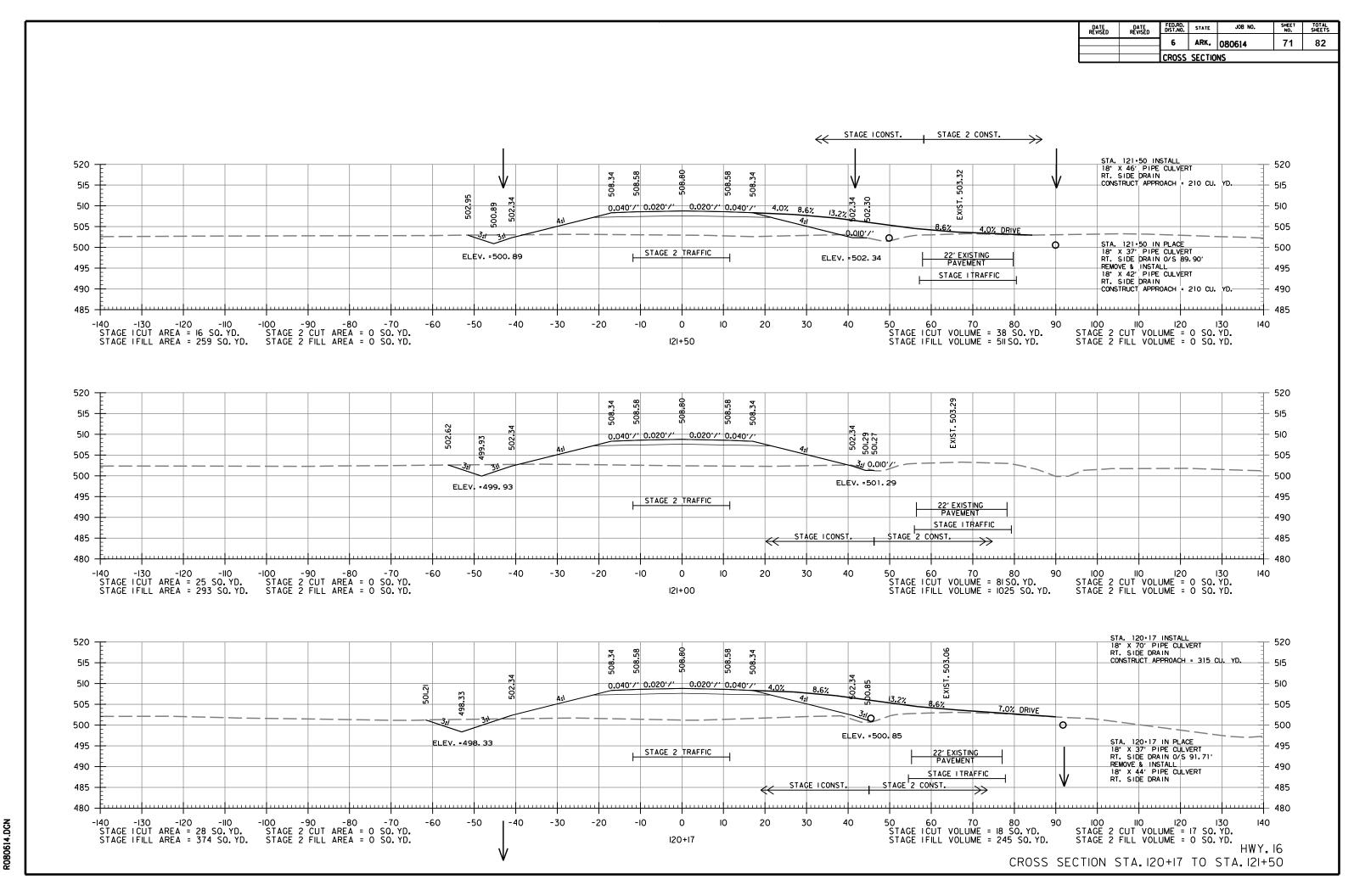
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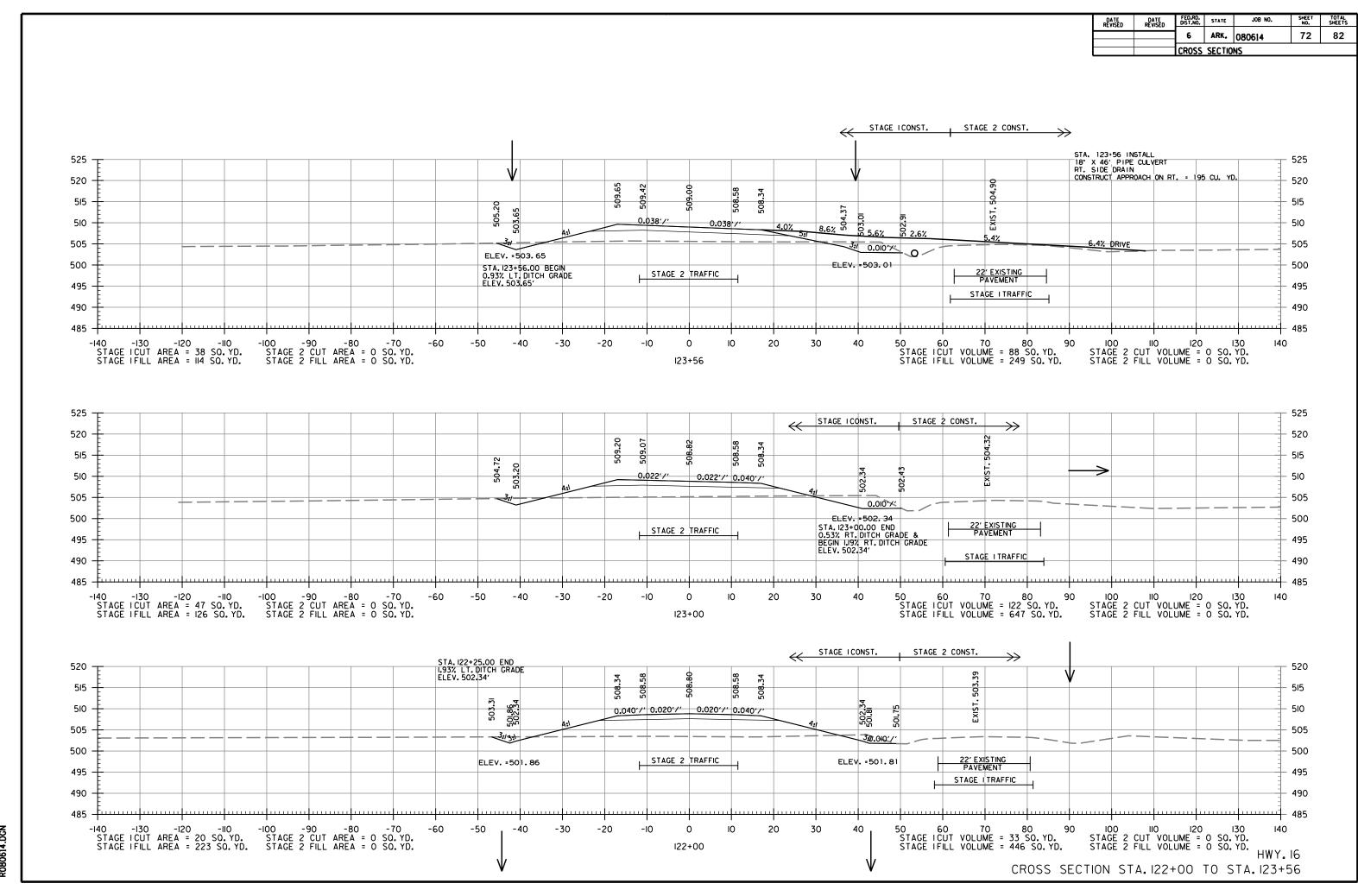
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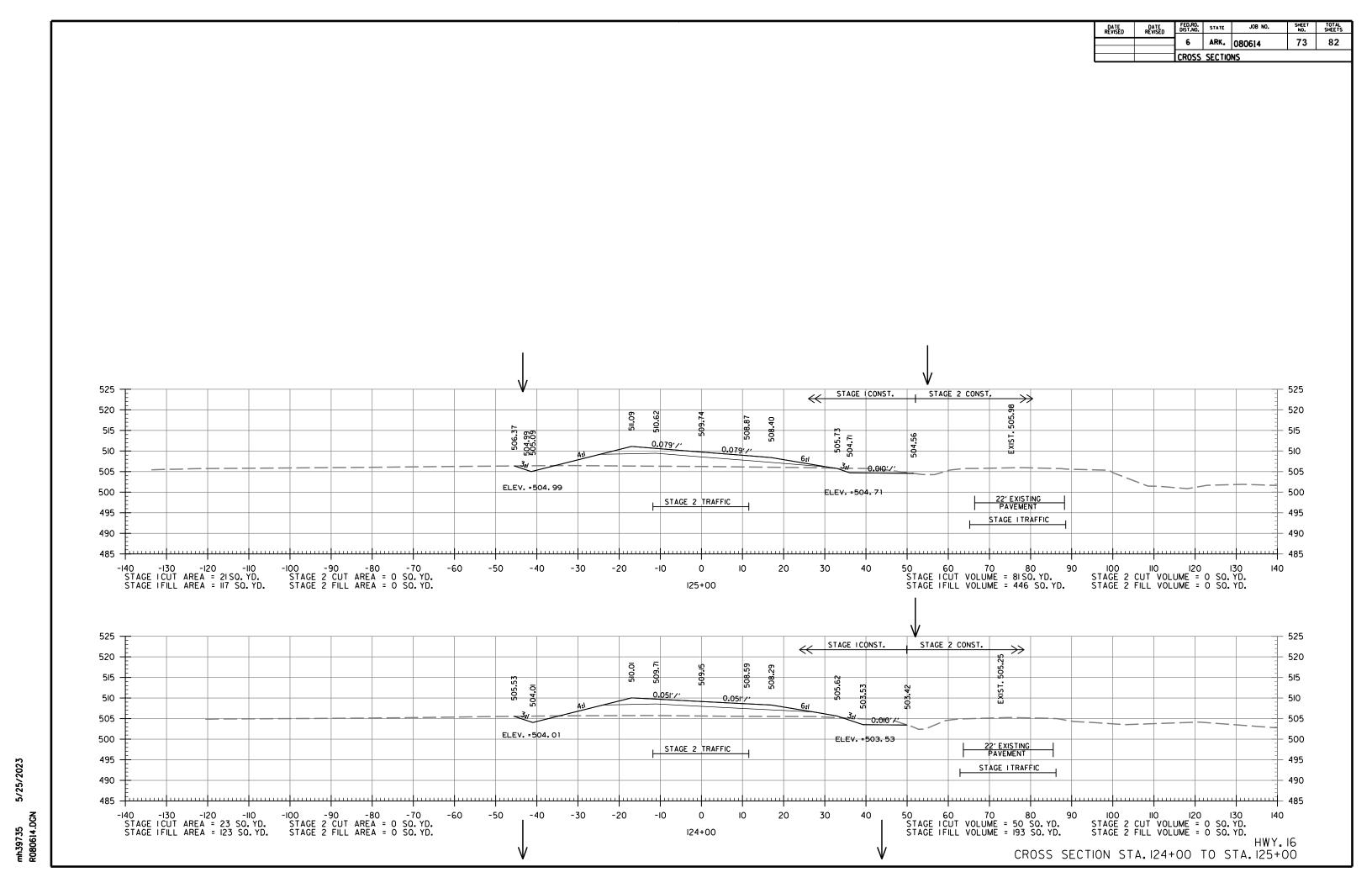
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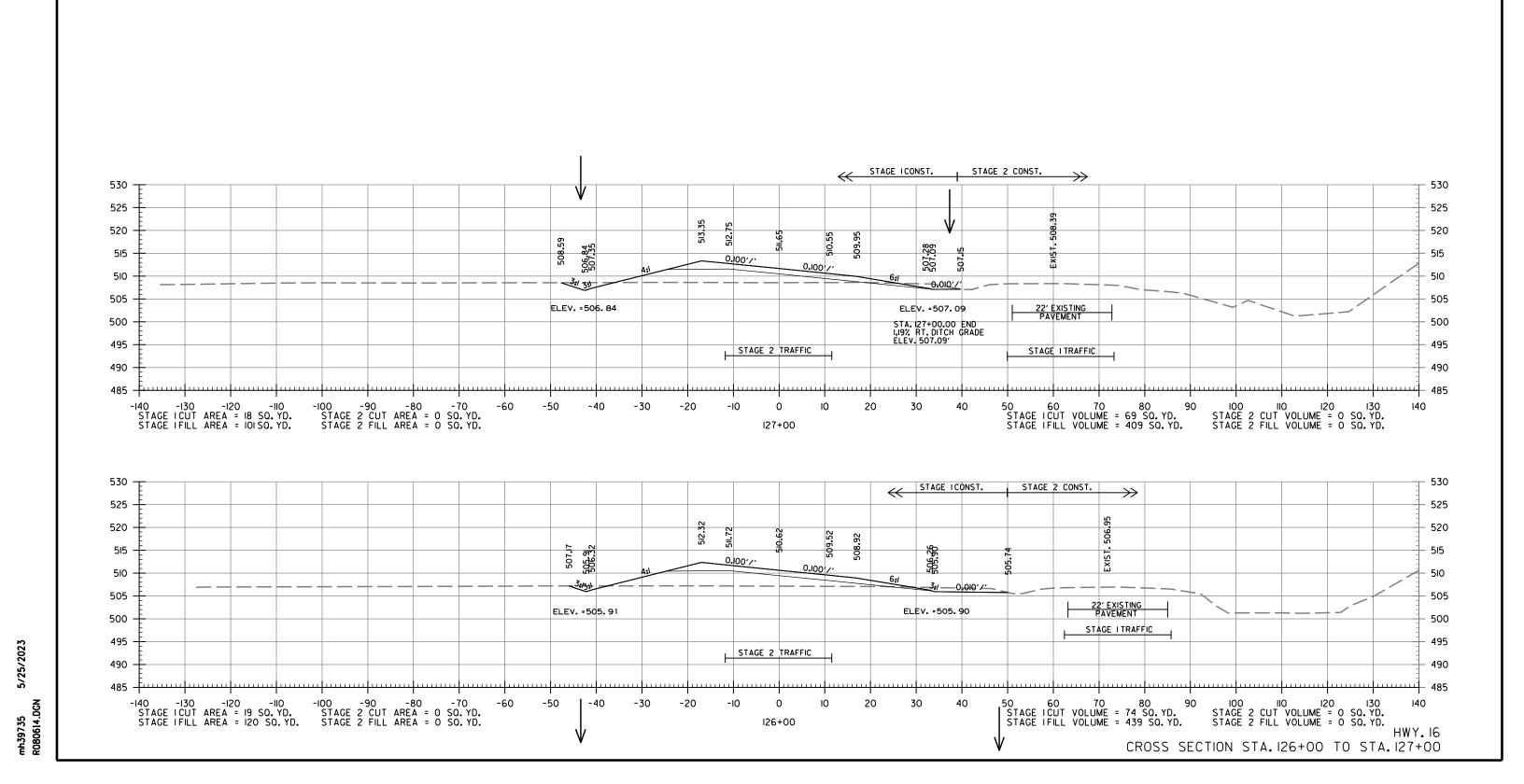
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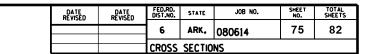


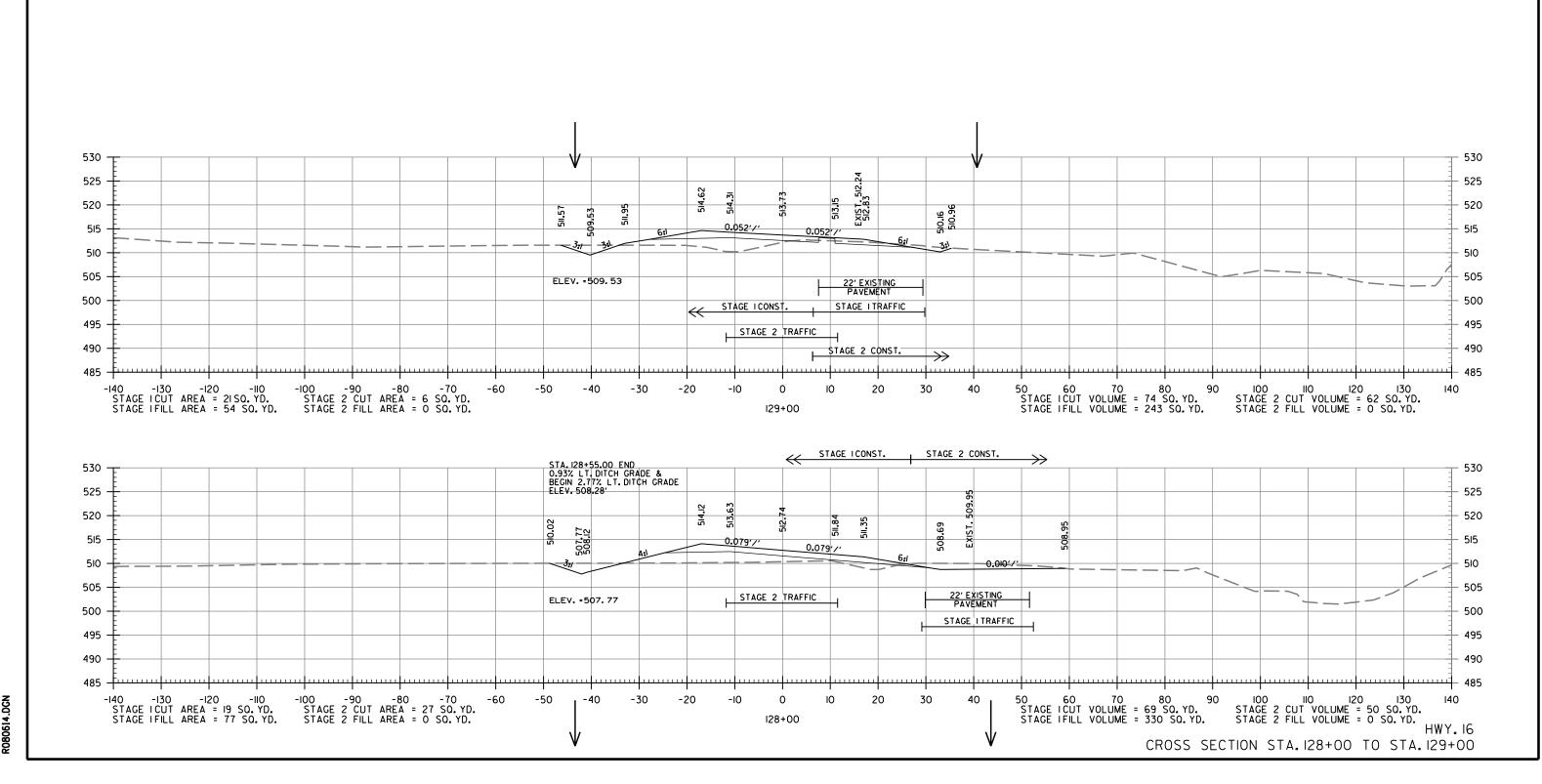


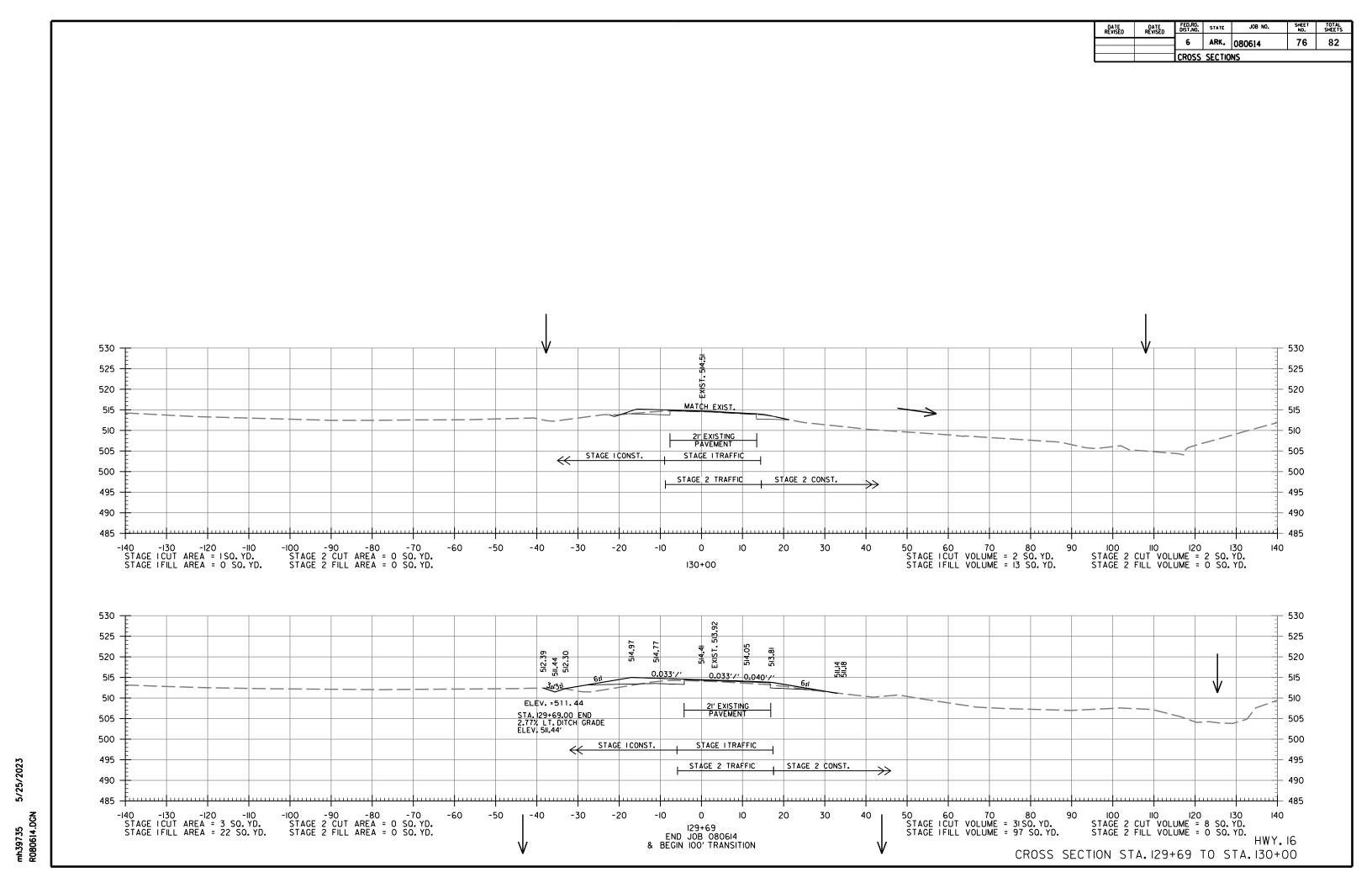


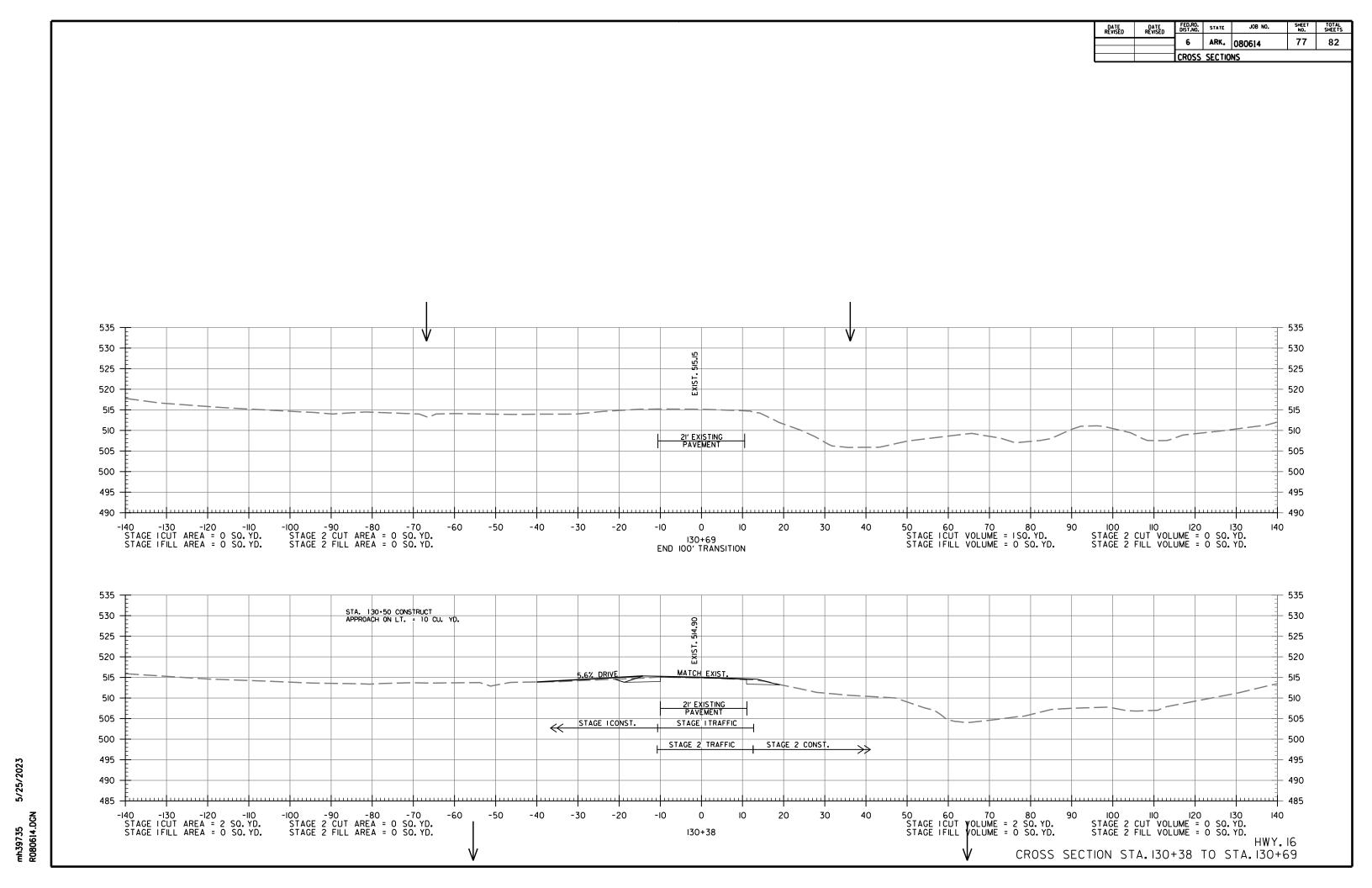
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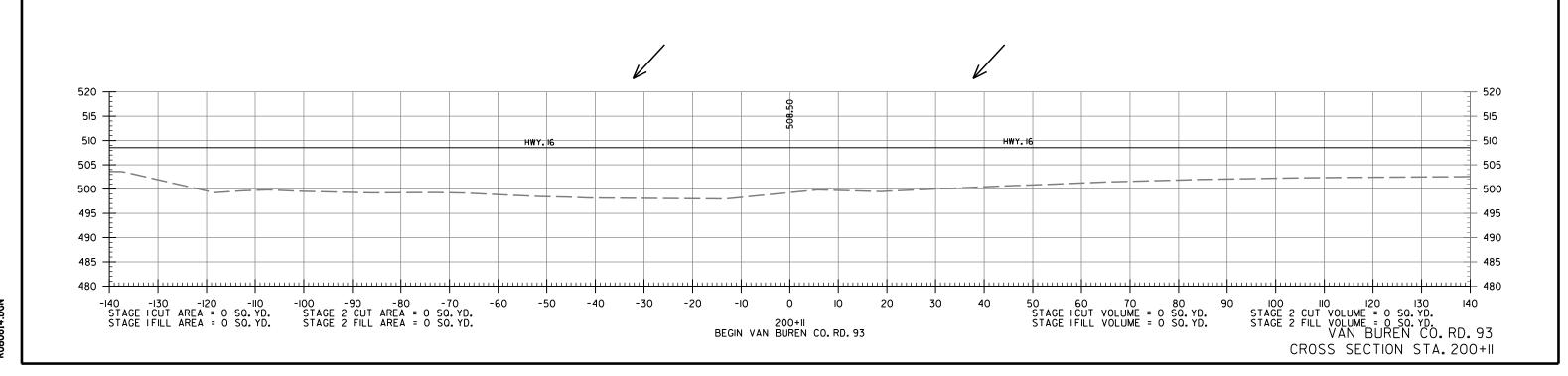




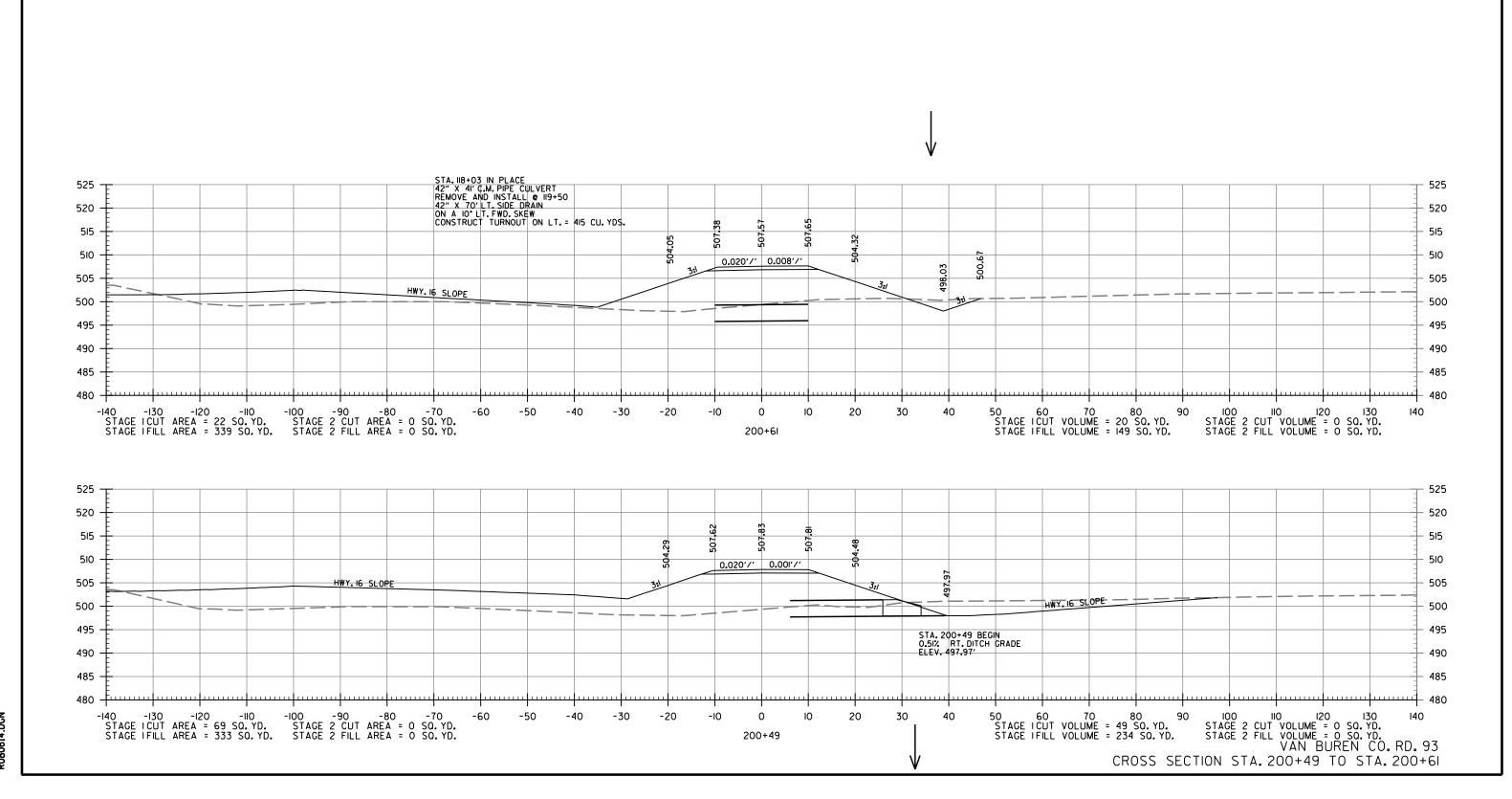




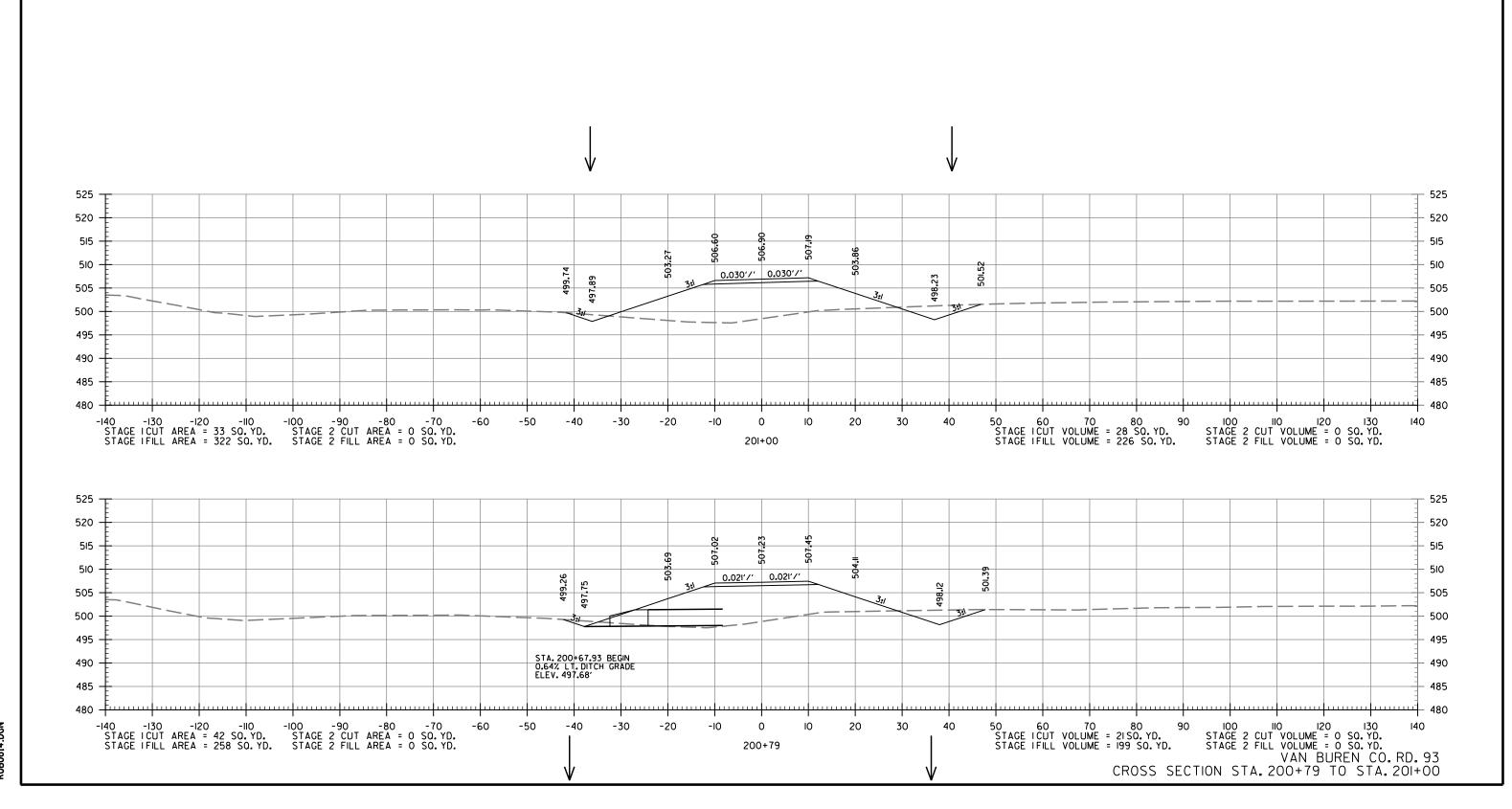
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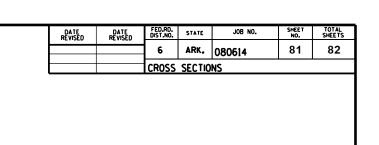


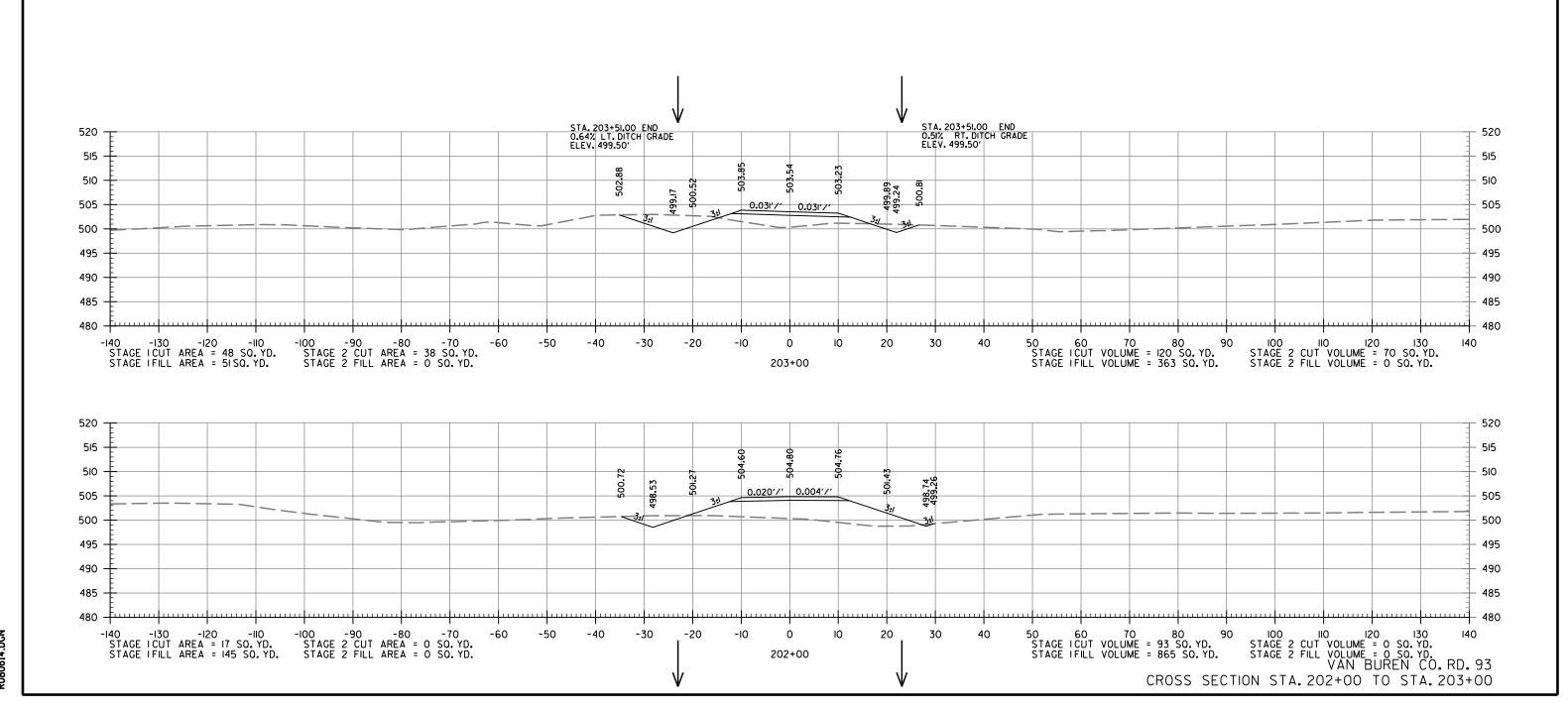
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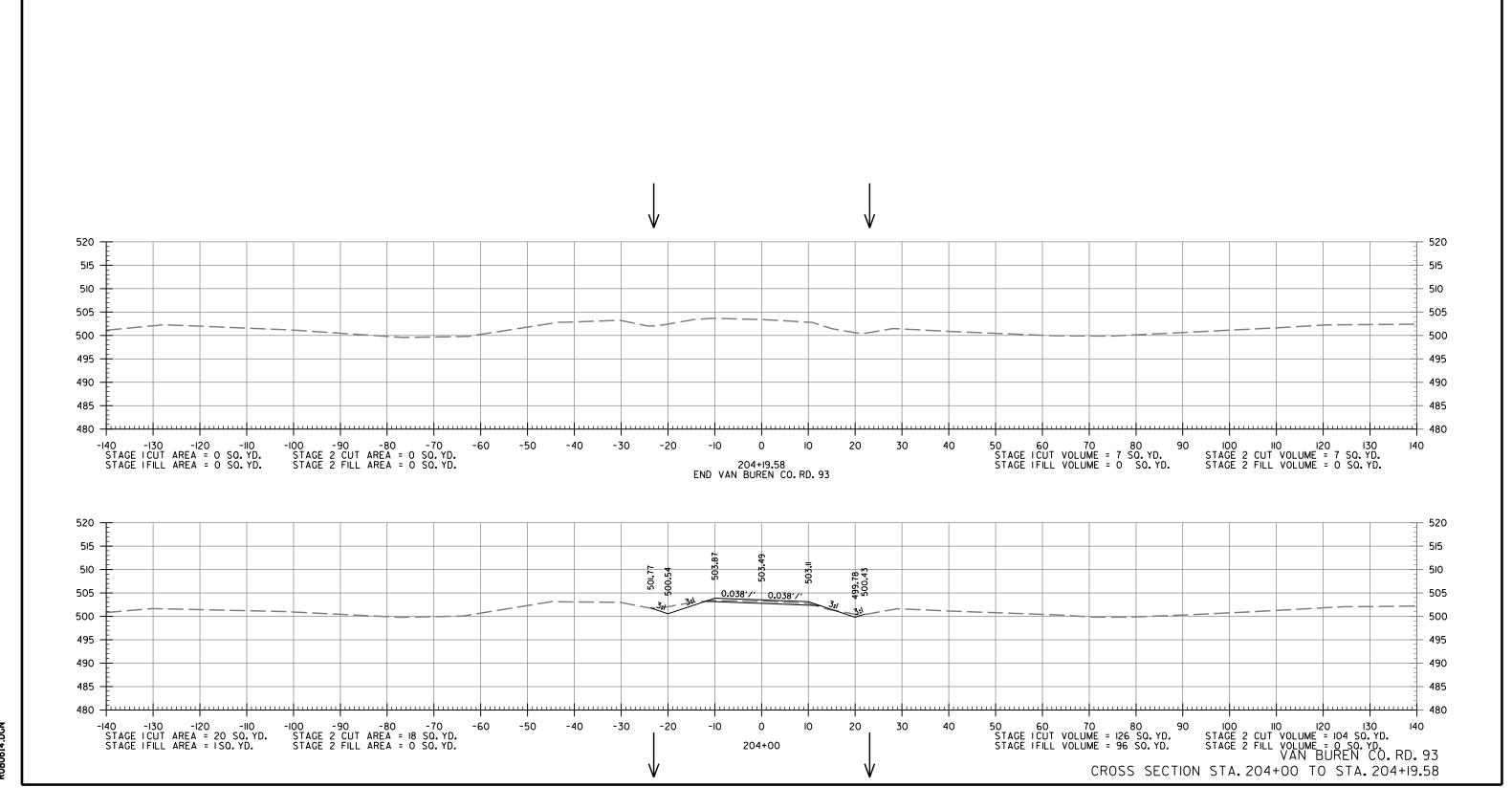
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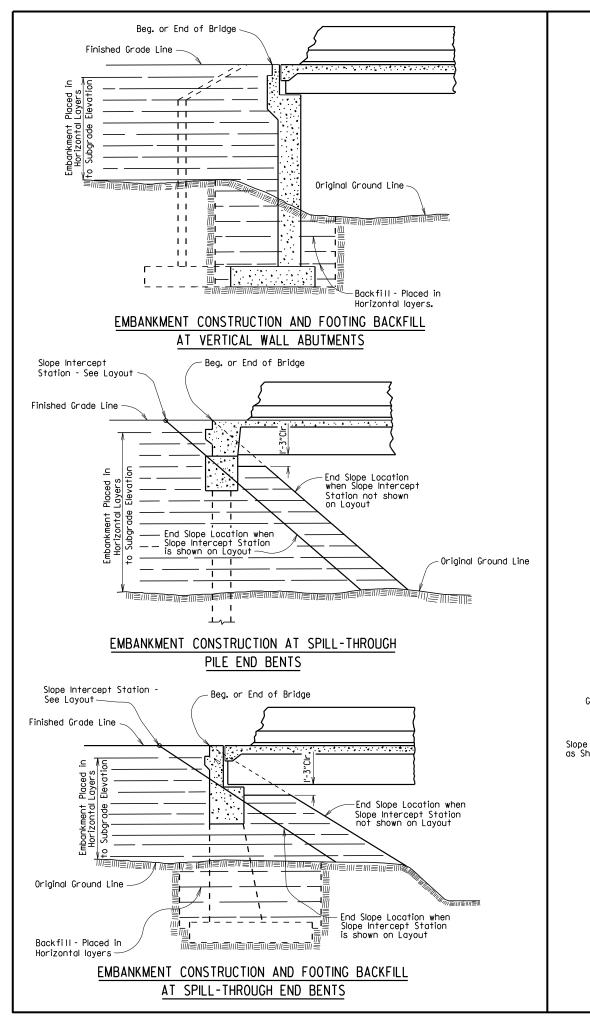


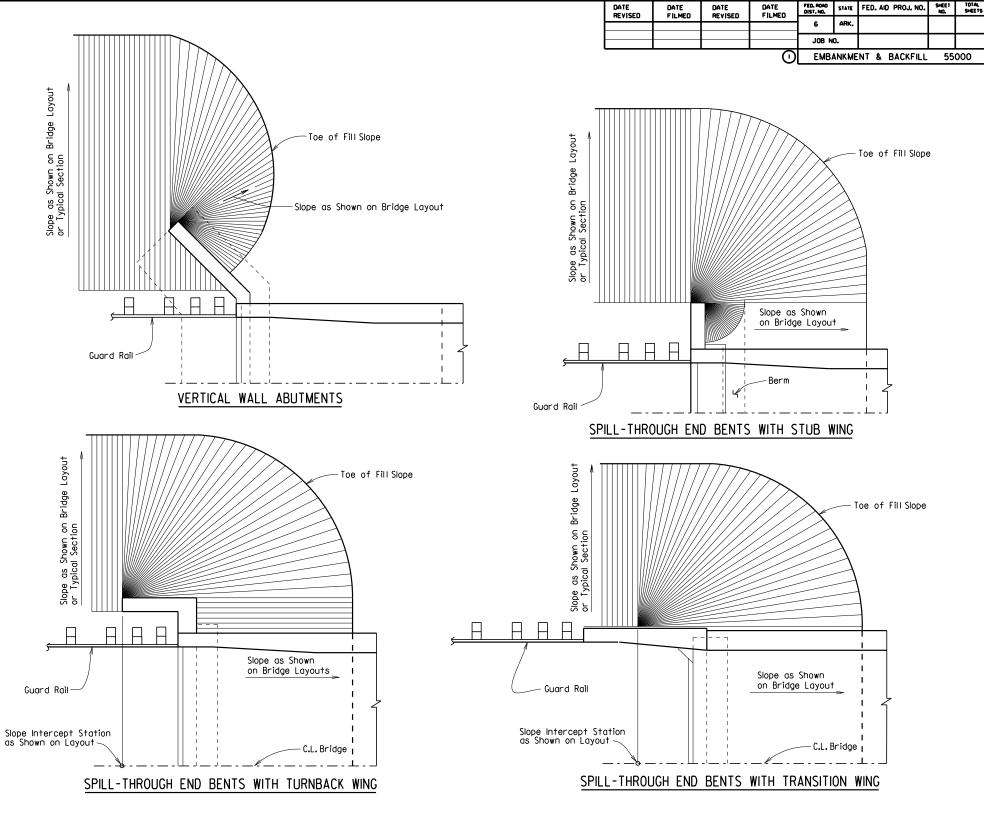




DATE REVISED	DATE REVISED	FED.RD. DIST.NO.	STATE	JOB NO.	SHEET NO.	TOTAL SHEETS					
		6 ARK.		080614	82	82					
		CROSS SECTIONS									







METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 6 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to Subsections 210.09, 210.10 and 801.08 for construction requirements.

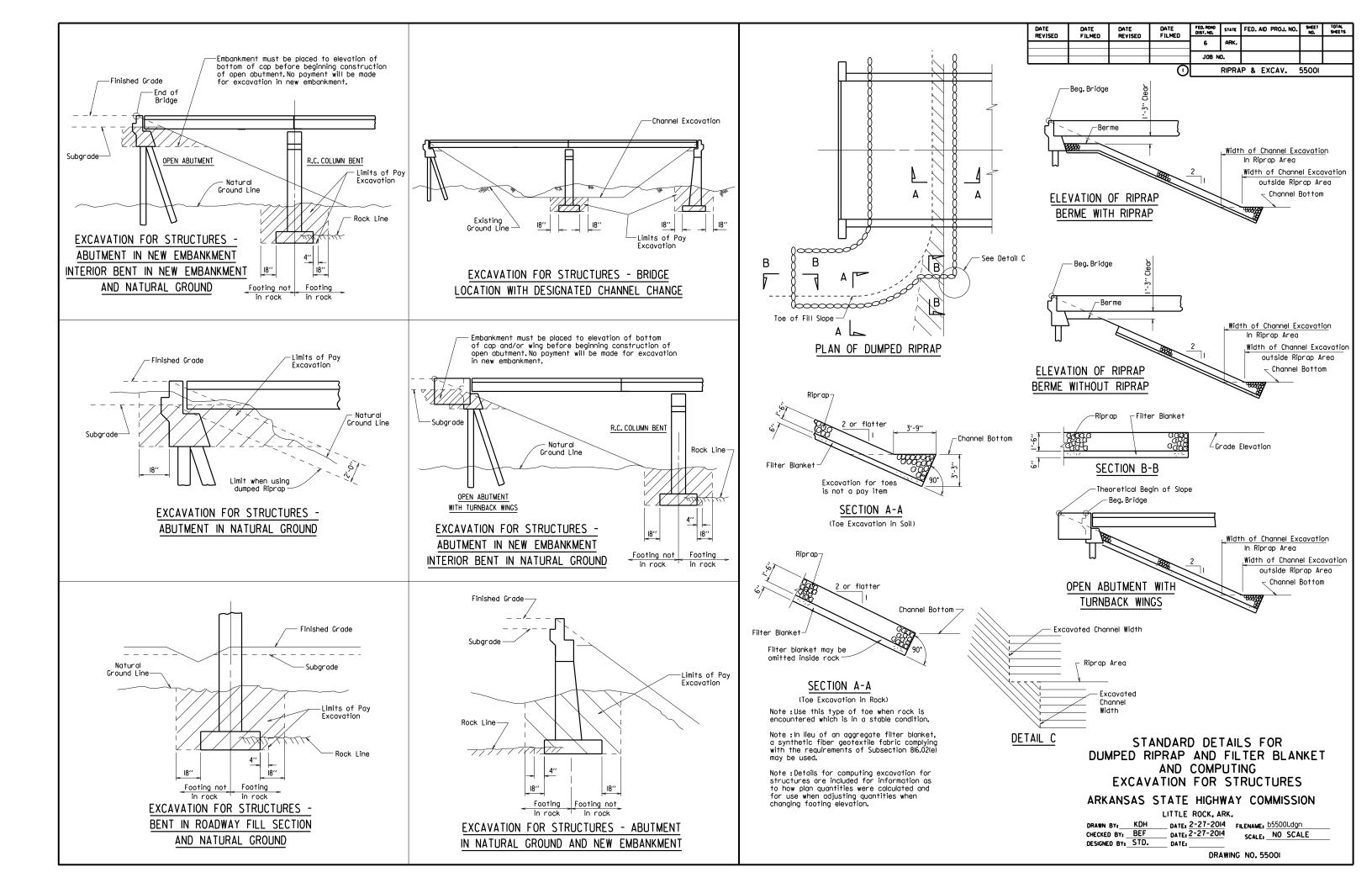
STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

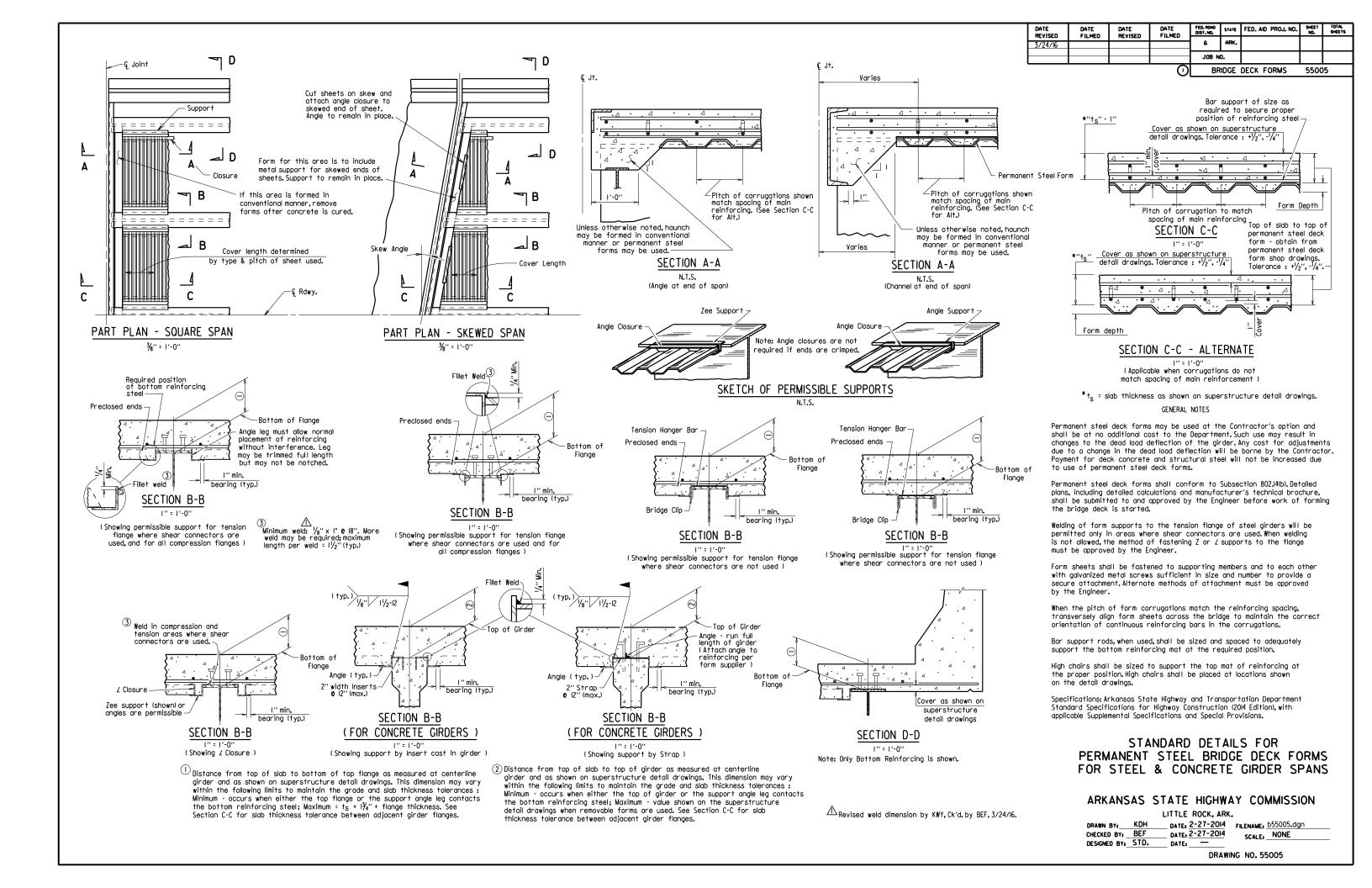
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 2-27-2014 FILENAME: b55000.dgn

| DATE: 2-27-2014 | FILENAME: 050000.0911 | CHECKED BY: | BEF | DATE: 2-27-2014 | SCALE: | NO SCALE | DESIGNED BY: | STD. | DATE: | -





GENERAL NOTES

These GENERAL NOTES are applicable unless otherwise shown in the Plan Details, Special Provisions, or Supplemental Specifications.

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 Edition) with applicable Supplemental Specifications and Special Provisions. Section and Subsection refer to the Standard Specifications.

DESIGN SPECIFICATIONS: See Bridge Layout(s).

SUPERSTRUCTURE NOTES:

MATERIALS AND STRENGTHS:

Class S(AE) Concrete	f'c	=	4,000 psi
Reinforcing Steel (Gr. 60, AASHTO M 31 or M 322, Type A)	fy	=	60,000 psi
Structural Steel (AASHTO M 270, Gr. 36)	Fy	=	36,000 psi
Structural Steel (AASHTO M 270, Gr. 50)	Fy	=	50,000 psi
Structural Steel (AASHTO M 270, Gr. 50W)	Fу	=	50,000 psi
Structural Steel (AASHTO M 270, Gr. HPS70W)	Fу	=	70,000 psi

See Plan Details for Grade(s) of Structural Steel required.

CONCRETE:

All concrete shall be Class S(AE) with a minimum 28 day compressive strength f'c = 4,000 psi. Concrete shall be poured in the dry and all exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

The superstructure details shown are for use when removable deck forming is used and are the basis for measurement of Class S(AE) Concrete. See Standard Drawing No.55005 for allowable modifications and for tolerances when Permanent Steel Bridge Deck Forms are used.

Use of a longitudinal screed is not permitted on any span of a bridge deck with horizontal curvature.

The concrete deck (roadway surface) shall be given a tine finish in accordance with Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Sidewalks shall receive a broomed finish as specified for final finishing in Subsection 802.19 for Class 6 Broomed Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour. Sufficient concrete must be placed ahead of the strike-off to fully load the beam or girder. When permitted, the use of a longitudinal strike-off will require that a vertical camber adjustment be made in the strike-off to account for the future dead load deflection due to any railings, median barrier, and sidewalks.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 conforming to AASHTO M 3lor M 322, Type A, with mill test reports and shall be epoxy coated. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Epoxy Coated Reinforcing Steel (Grade 60)".

STRUCTURAL STEEL (COMMON TO W-BEAMS AND PLATE GIRDERS):

Structural steel shall be AASHTO M 270 with grade and payment as specified in the plans. Grade 50W steel shall not be painted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84(e). Grade 36 and Grade 50 steel shall be painted unless otherwise noted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84. Structural steel completely embedded in concrete may be AASHTO M 270, Gr. 36, Gr. 50 or Gr. 50W unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with the specifications, submitted and approval secured before fabrication is begun.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval; however, additional welds used for attaching falsework support devices or screed rail supports to the structural steel that do not exceed the limitations of Subsection 802.13 will not require approval prior to construction. All welding shall conform to Subsection 807.26.

Unless otherwise noted, field connections shall be bolted with 3/4" ø high-strength bolts using 13/6" ø open holes. Holes for $\frac{3}{4}$ " ø high-strength bolts may be $\frac{1}{6}$ " ø if a washer is supplied for use under both the nut and head of the bolt. The use of oversized holes will not be allowed on main members unless otherwise noted. Bolts shall be placed with heads on the outside face of the exterior beam or girder webs and on the bottom of the beam or girder flanges.

All stud shear connectors shall be granular flux filled, solid fluxed, or equal and shall be automatically end welded in accordance with recommendations of the Manufacturer.

When painting is required, all structural steel except galvanized steel and steel completely encased in concrete shall be painted in accordance with Subsection 807.75. The color of paint shall be as specified in the plans.

FILMED FILMED 6 JOB NO. \odot GENERAL NOTES 55006

FED. AID PROJ. NO. SHEET

STRUCTURAL STEEL (W-BEAMS):

All beams and field splice plates, and all diaphragms and connection plates attached to horizontally curved beams are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Beam Spans (M 270, Gr. ...)".

All beams in continuous units and simple spans with field splices shall be blocked in their true position in the shop in groups as specified in Subsection 807.54(b)(2) with the webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All beams in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Flange field splice plates shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

All beam dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for comber.

Bent plate diaphragms for horizontally curved beams shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight beams may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved beams.

Unless otherwise noted, diaphragms shall be installed as beams are erected. All bolts in diaphragms and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

STRUCTURAL STEEL (PLATE GIRDERS):

All references to cross-frames shall include "X" or "K" types.

All girder web and flange plates, all field splice plates, and all diaphragms, cross-frames and connection plates attached to horizontally curved girders are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr. ...)".

All girders in continuous units and simple spans with field splices shall be assembled in the shop as specified in Subsection 807.54(b)(2) and blocked in their true position with webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All girders in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Web and flange plates for main members and flange splice plates for main members shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

Girder webs may be made by shop splicing with minimum lengths of 25 feet for sections. Flange plates longer than 50 feet may be made by shop splicing with minimum lengths of 25 feet for sections. No additional payment will be made for shop welded splices.

All girder dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for camber.

Groove welds in web and flange plates shall be Quality Control (Q.C.) tested by nondestructive testing, as required in Subsection 807.23(b). Fillet welds at flange to web plate connections shall be O.C. tested by the magnetic particle method. All O.C. testing shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr. ...)".

Bent plate diaphragms for horizontally curved girders shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight girders may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved girders.

Unless otherwise noted, cross-frames and diaphragms shall be installed as girders are erected. All bolts in cross-frames, diaphragms, and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

SUBSTRUCTURE NOTES:

CONCRETE:

Unless otherwise noted concrete in caps, columns and footings (except seal footings) shall be Class "S" with a minimum 28 day compressive strength f'c = 3,500 psi and shall be poured in the dry. Seal Concrete for footings shall have a minimum 28 day compressive strength f'c = 2,100 psi.

Concrete in drilled shafts shall be Class "S" as modified by Job SP "Drilled Shaft Foundations".

All exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322. Type A. with mill test reports.

Top reinforcing bars in cap shall be properly placed to avoid interference with anchor bolts or sheet metal sleeves.

STRUCTURAL STEEL:

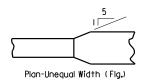
Structural steel in end bents shall be AASHTO M 270 with grade and payment as specified in the

FOR ADDITIONAL INFORMATION AND NOTES, SEE LAYOUT(S) AND PLAN DETAILS.

STANDARD GENERAL NOTES FOR STEEL BRIDGE STRUCTURES

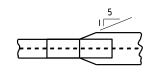
ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARK.

DRAWN BY: A.M.S. DATE: 9-2-2015 FILENAME: b55006.dgn CHECKED BY: B.E.F. DATE: 9-2-2015 SCALE: NO SCALE DESIGNED BY: STD. DATE:

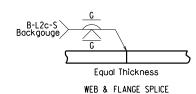


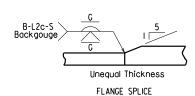
FLANGE SPLICE

Plate Girder Spans (____)".

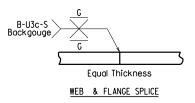


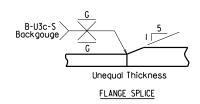
FLANGE SPLICE AT UNEQUAL BOTTOM FLANGE WIDTHS





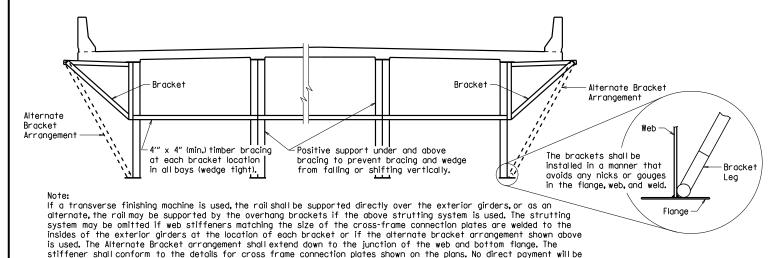
(Use when Base Metal Thickness is Equal to or Less than 2")





(Use when Base Metal Thickness is Greater than 2")

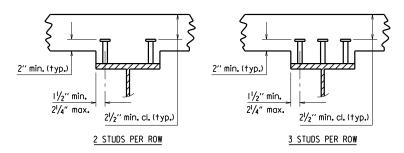
DETAILS OF WELDED SPLICES FOR PLATE GIRDERS



SCREED RAIL SUPPORT FOR PLATE GIRDERS

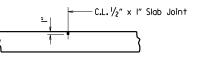
(USE WHEN WEB DEPTHS ARE 48" OR GREATER)

made for brackets, timber bracing, supports, or welded stiffeners. Payment shall be subsidiary to "Structural Steel in



Stud Shear Connectors shall be automatically end welded to the beam or girder flange in accordance with the recommendations of the Manufacturer. See plan details for number and size.

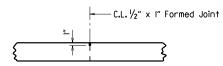
SHEAR CONNECTOR DETAIL



Use Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint Sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. Slab Joints shall extend to the outside edge of the deck slab and shall align with open joints at the front face of the parapet. Slab joints shall be installed before the parapet railing is poured. If slab joints are to be sawed, they shall be sawed as soon as the concrete has sufficiently set to allow sawing of the joint without damage to the slab. Slab joints shall be placed at all pouring sequence construction joints and required slab joint locations. The joint sealer shall extend across the deck from gutterline to gutterline.

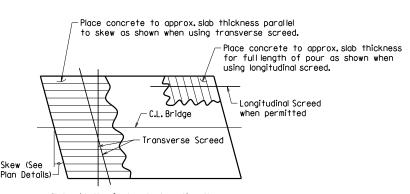
ADDITIONAL NOTES IF SIDEWALKS OR RAISED MEDIANS ARE REQUIRED: Slab Joints shall be installed before the sidewalk or raised median is poured. After installation of the joint in the sidewalk or raised median and prior to pouring the parapet rail, the joint sealer shall be placed extending across the deck slab from gutterline to gutterline and acrosss the top of the sidwalk or raised median to the edge of the slab. No joint sealer shall be placed on the deck slab under the sidewalk or raised median.

TRANSVERSE SLAB JOINT DETAIL



Use $\frac{1}{2}$ " x I" Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. This joint shall be formed. Seal color shall be gray or other color similar to concrete.

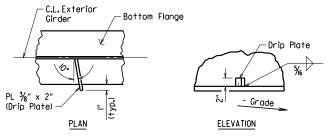
LONGITUDINAL CONSTRUCTION JOINT



Note: At the Contractor's option, the transverse screed may be placed parallel to the skew or perpendicular to C.L. Bridge.

CONCRETE PLACEMENT PROCEDURE

FOR BRIDGES WITH SKEW



Drip Plate to be welded to the outer side of the bottom flange of the exterior girders.

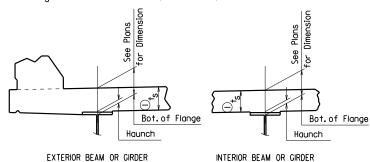
Locate drip plate 5'-0" from C.L. Bearing on high side of each Bent, unless otherwise noted in the plans.

BOTTOM FLANGE DRIP PLATE

(USE WHEN WEB DEPTHS ARE 54" OR GREATER AND UNIT OR SPAN IS NOT IN LEVEL GRADE)

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS		
REVISED	FILMED	REVISED	TIENED	6	ARK.					
				JOB NO.						
STEEL BRIDGE STRUCTURES 55007										

 $t_{\rm S}$ = slab thickness. See "Typical Roadway Section" in the plans.



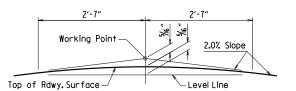
Tolerance when removable deck forming is used is + ½",- ¼". Haunch forming is required and shall be adjusted to maintain slab thickness tolerance.

NOTES:

Haunch dimension may vary within the following limits to maintain the grade and slab thickness tolerance: Minimum occurs when top flange contacts bottom reinforcing steel; Maximum = top flange thickness plus 1½" unless otherwise noted in the plans. No increase in concrete and structural steel quantities will be made to maintain tolerances.

Tolerances shown are applicable only when removable deck forming is used. See Std. Dwg. No. 55005 for tolerances when permanent steel deck forms are used. Payment for concrete shall be based on removable deck forming.

ADJUSTMENT FOR SLAB THICKNESS TOLERANCE



NOTE: Working Point matches Theoretical Roadway Grade.

ROUNDING DETAIL BRIDGES IN NORMAL CROWN

WELD TABLE

Material Thickness of Thicker Part Joined (Inches)	Minimum Size of Fillet Weld (Inches)	Single Pass Weld Must
To ¾" Inclusive	1/4"	Be
0ver ¾"	5/6 ′′	Used

NOTE: When a fillet weld size, as shown on the plans, is larger than the minimum, the first pass shall be that specified for minimum size of fillet weld.

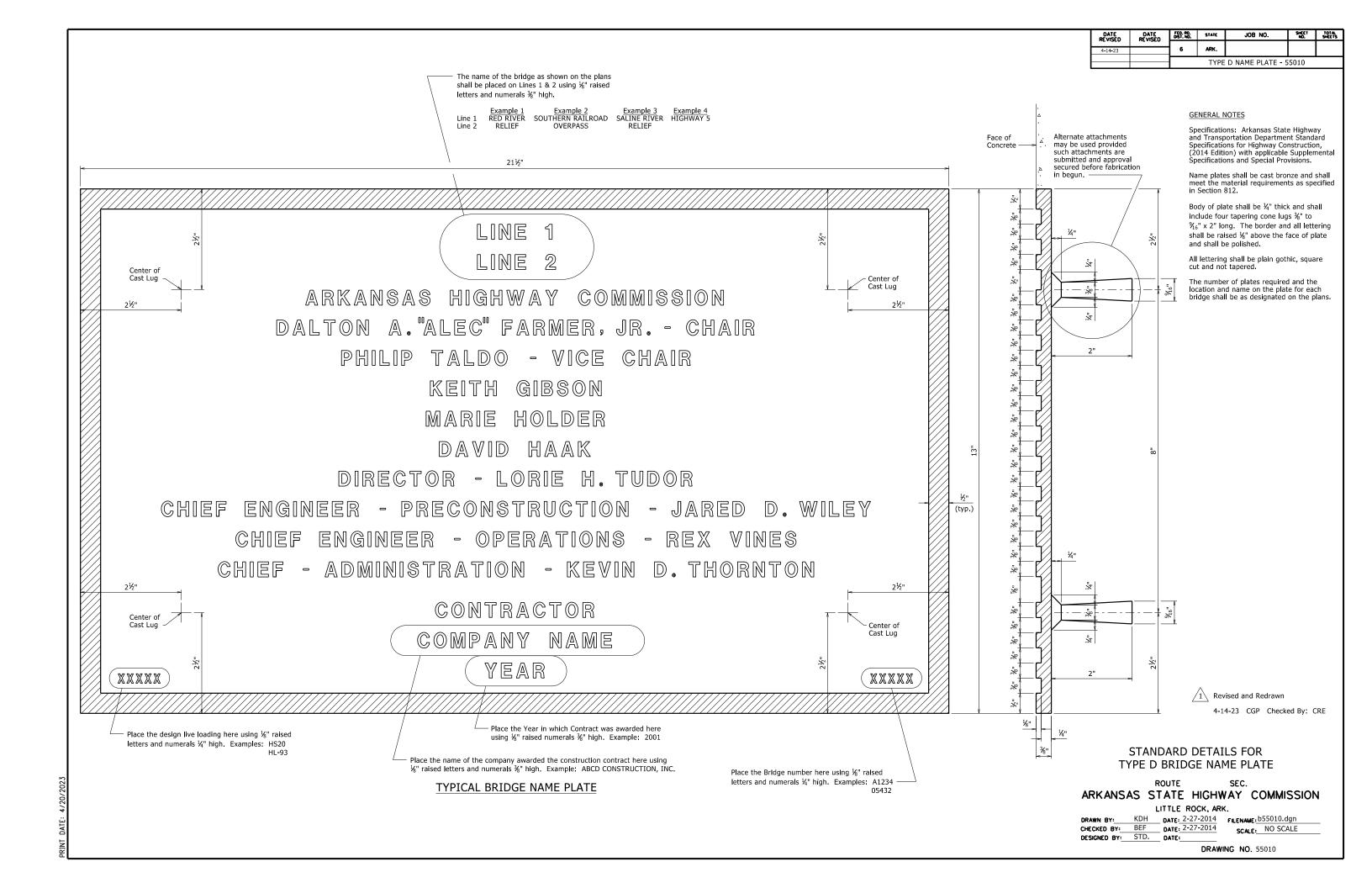
SECTION AND SUBSECTION REFER TO THE ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2014 EDITION).

THESE DETAILS ARE APPLICABLE UNLESS OTHERWISE SHOWN IN THE PLAN DETAILS, SPECIAL PROVISIONS, OR SUPPLEMENTAL SPECIFICATIONS.

STANDARD DETAILS FOR STEEL BRIDGE STRUCTURES

ARKANSAS STATE HIGHWAY COMMISSION

		LITTLE ROCK, AR	N _e
DRAWN BY:	JYP	DATE: 2/11/2016	FILENAME: b55007.dgn
CHECKED BY:	AMS	DATE: 2/11/2016	SCALE: No Scale
DESIGNED BY.	STD.	DATE: -	50422



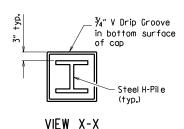
GENERAL NOTES FOR STEEL H-PILES:

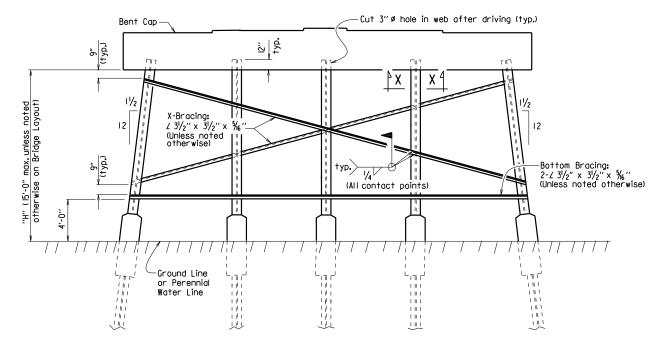
Steel H-Piles shall conform to AASHTO M 270, Grade 36 or greater.

See Bridge Layout and Bent Details for pile size, estimated length, spacing, pile anchorage (if required) and for driving information.

Steel H-Piles that extend above the ground and are not protected by pile encasement shall be painted in accordance with Subsection 805.02.

Brackets, lugs, cap plates, pile tips, driving points, pile painting, splicing and welding shall not be paid for directly, but shall be considered subsidiary to the item "Steel Piling".





Notes:

All bracing shall be cut and welded in the field. Each brace shall be furnished in one piece, Payment shall be made under Item 807.

Unless noted otherwise, omit X-Bracing when "H" is less than 8 feet.

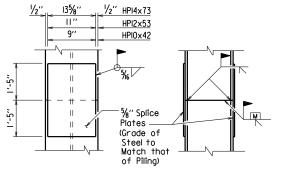
Omit X-Bracing and Bottom Bracing when "H" is 5 feet or less.

When required on the Bridge Layout sheet, pile encasements shall be constructed. See Notes and Details for H-Pile Encasements.

Omit all bracing (and V-groove in cap) when pile encasement is extended to bottom of bent cap.

TYPICAL DETAILS OF H-PILE TRESTLE INTERMEDIATE BENT

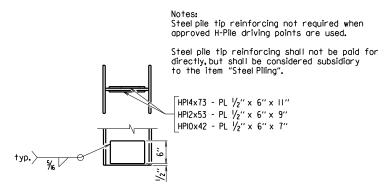
(Shown with Partial Height Encasement)



The Contractor may for his own convenience and at his own expense provide as many as three splices per pile. Minimum spacing between splices shall be 5 feet.

TYPICAL SPLICE DETAILS

H-pile splicers manufactured by Associated Pile and Fitting Corporation, LB Foster Piling, Skyline Steel or equivalent may be used in lieu of the "Typical Splice Details" shown. H-pile splicers shall match the same grade of steel specified for the piling and shall be welded to the pile with a %" fillet weld around the entire perimeter of the splice. Flanges shall be welded with a complete penetration groove weld complying with AASHTO/AWS Joint Designation B-U4a or B-U4b. All welding shall conform to Subsection 807.26 of the AHTD Standard Specifications for Highway Construction (2014 Edition).



REINFORCING DETAIL FOR STEEL H-PILE TIP

GENERAL NOTES FOR H-PILE ENCASEMENTS:

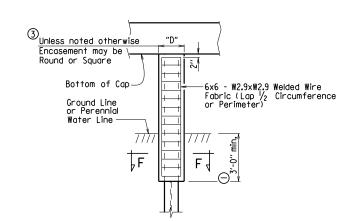
See Bridge Layout for additional notes, any pile encasement restrictions and required location of pile encasements.

All concrete shall be Class S with a minimum 28-day compressive strength, f'c = 3,500 psi. If concrete cannot be placed in the dry, Seal Concrete may be used from top to bottom of encasement.

Reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A.

Welded Wire Fabric shall conform to AASHTO M 55 or M 221. Galvanized Corrugated Steel Pipe shall conform to AASHTO M 36 and M 218.

Concrete, welded wire fabric or reinforcing steel and galvanized pipe shall not be paid for directly, but shall be considered subsidiary to the item "Pile Encasement".



PILE ENCASEMENT DETAIL FOR STEEL H-PILES (4) (Shown with Encasement to Bottom of Cap)

Round

Encasement

DATE FILMED

 \odot

6

JOB NO.

REVISED

3/24/16

FILMED

SECTION F-F

*Measured out-to-out of bar.

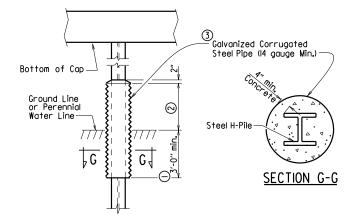
FED. AID PROJ. NO.

STEEL H-PILES

55020

TABLE OF VARIABLES FOR PILE ENCASEMENT

	"[
Pile Size	Square Encsmt.	Round Encsmt.	"L"*	
HPI0×42	l'-7"	2'-0"	l'-4"	
HPI2x53	l'-8"	2'-2"	l'-5"	
HPI4x73	l'-l1"	2′-6″	l'-8"	



- () Unless otherwise noted on Bridge Layout.
- $^{\circ}$ 3'-0" minimum or as shown on Bridge Layout.
- The state of the pile. Encasement dimensions shall be sized to maintain a minimum concrete cover of 4" from the H-Pile. Reinforcement shall be sized to provide a minimum concrete cover of $1\frac{1}{2}$ " and a minimum clearance of $1\frac{1}{4}$ " from the pile.
- Alternate pile encasement, when not extended to bottom of cap, shall have 2" concrete taper for water runoff as shown in the Partial Height Encasement detail.

ALTERNATE PILE ENCASEMENT DETAIL FOR STEEL H-PILES

(Shown with Partial Height Encasement)

Added alternate method of splicing H-piles and revised pile encasement note. 3/24/2016 AMS

This document was originally issued and sealed by Charles R. Ellis, PE No. 9235, on March 24, 2016.

This copy is not a signed and sealed document.

REGISTERED PROFESSIONAL ENGINEER
No. 9235

BRIDGE ENGINEER

STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS

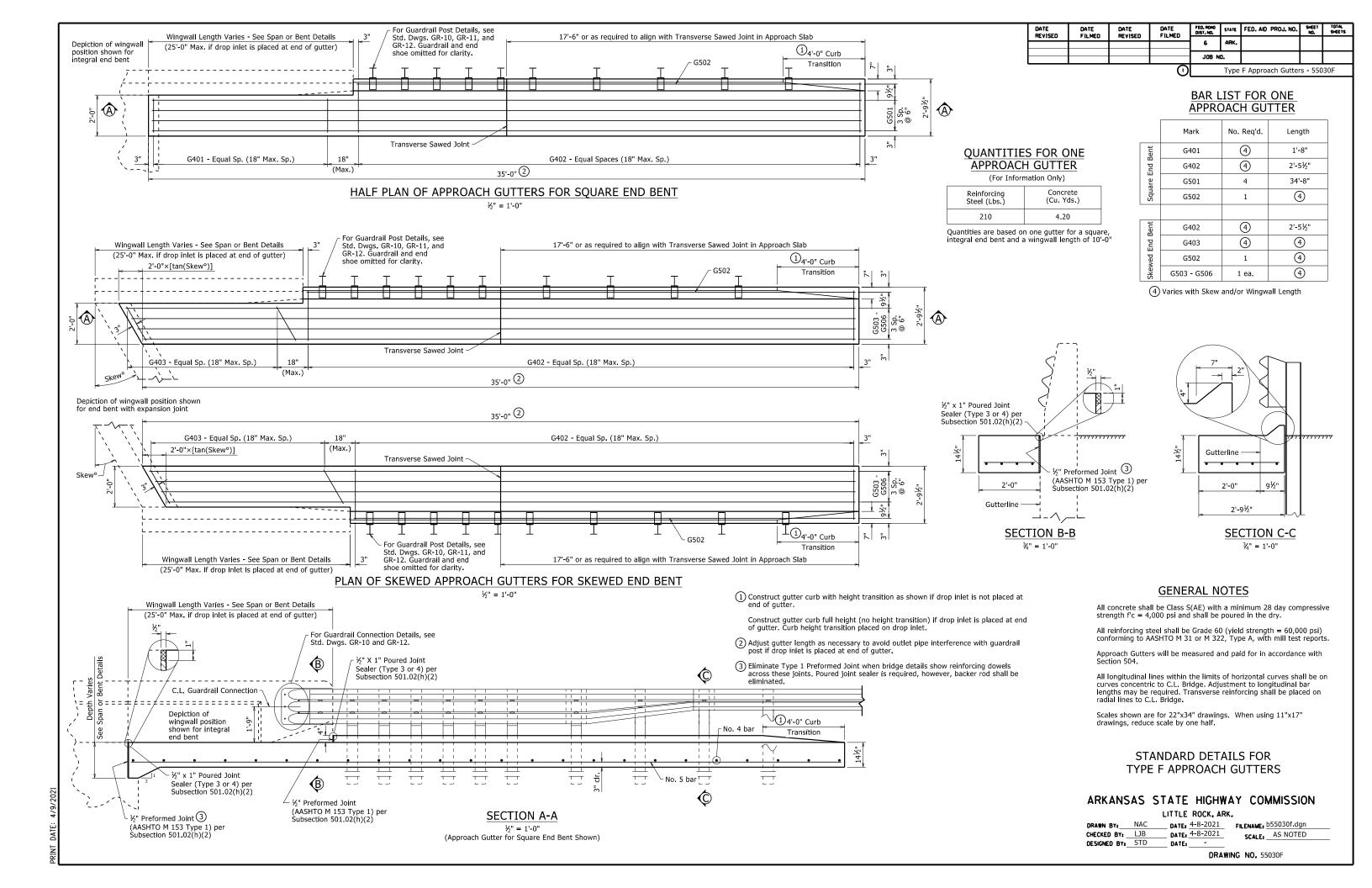
ARKANSAS STATE HIGHWAY COMMISSION

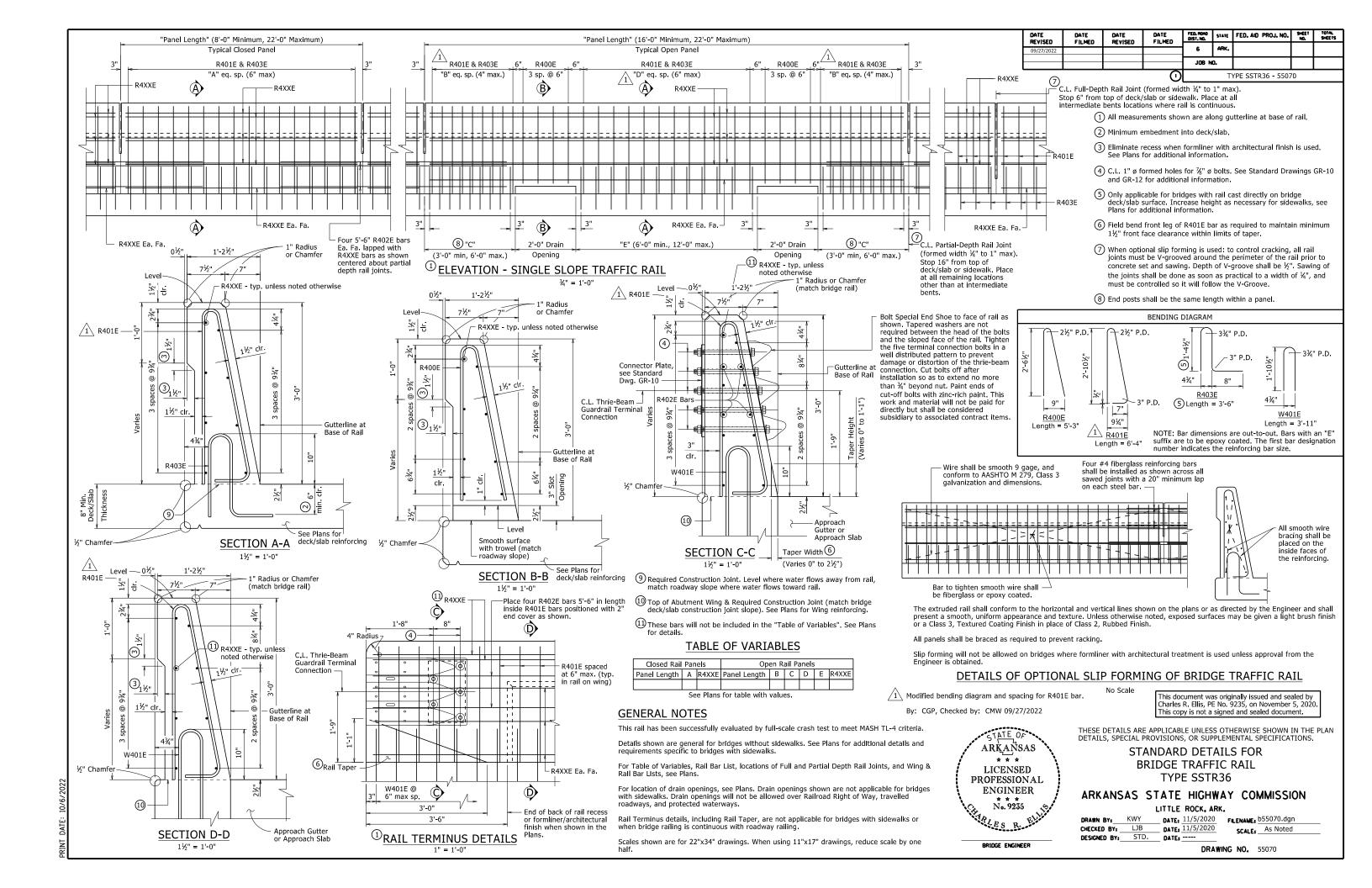
LITTLE ROCK, ARK.

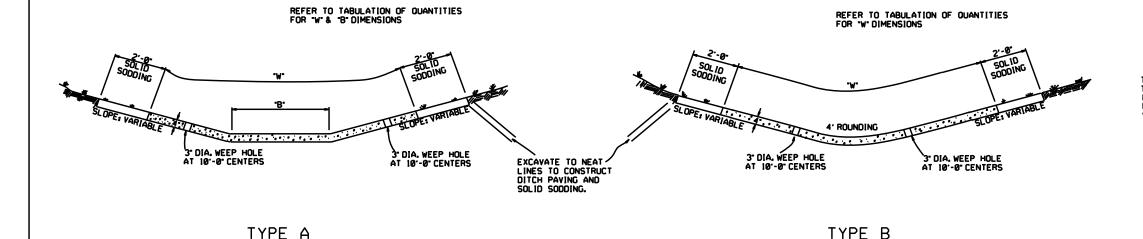
 DRAWN BY:
 A.M.S.
 DATE:
 2/27/2014
 FILENAME:
 b55020.dgn

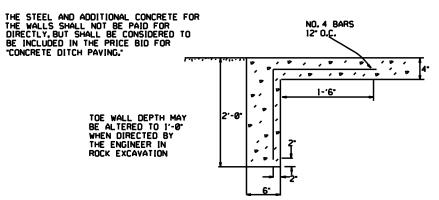
 CHECKED BY:
 B.E.F.
 DATE:
 2/27/2014
 SCALE:
 NO SCALE

 DESIGNED BY:
 STD.
 DATE:
 —
 NO SCALE

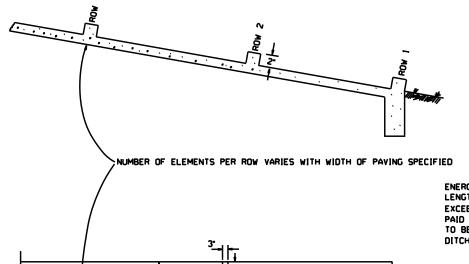








TOE WALL DETAIL FOR CONCRETE DITCH PAVING



ENERGY DISSIPATORS

6'-6"

ENERGY DISSIPATORS TO BE USED FOR THE ENTIRE LENGTH OF DITCH WHEN SLOPE OF DITCH PAVING EXCEEDS 7%. THE DISSIPATORS WILL NOT BE PAID FOR DIRECTLY, BUT SHALL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID FOR CONCRETE DITCH PAVING.

GENERAL NOTES:

THE FULL WIDTH OF EACH SECTION SHALL BE POURED MONOLITHICALLY.

TOE WALLS TO BE CONSTRUCTED FULL WIDTH AT EACH END OF DITCH PAVING, AND POURED MONOLITHICALLY.

SOLID SOD ALONG DITCH PAVING TO BE PLACED WITHIN 14 DAYS OF DITCH PAVING CONSTRUCTION.

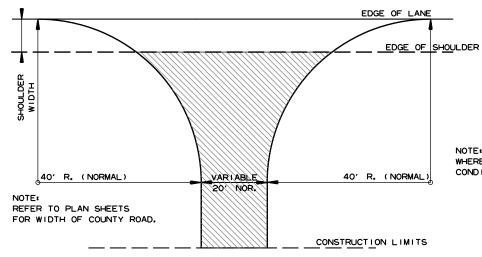
I' WIDE TRANSVERSE EXPANSION JOINTS SHALL BE PLACED IN CONCRETE DITCH PAVING AT 45' INTERVALS. THE SPACE SHALL BE FILLED WITH APPROVED JOINT FILLER COMPLYING WITH AASHTO M213.

		ı
		
12-8-16	CORRECTED ENERGY DISSIPATOR DRAWING AND NOTE	
11-17-10	ADDED GENERAL NOTE	
	ADDED GENERAL NOTE ABOUT SOLID SODDING	
11-30-8	ELIMINATED MIN. ROWS OF ELEMENTS	1111-30-89
7-15-88	REVISED DISSIPATOR NOTE	1653-7-15-88
4-3-87	REVISED ENERGY DISSIPATOR	671 - 4 - 3 - 87
1-9-87		1532-1-9-87
11-3-86	ADDED NOTE TO ENERGY DISS.	599-12-1-86
11-1-84	ENERGY DISSIPATOR DETAILS	1508-11-1-84
	ADDED	
11-1-84	EXCAVATION DETAILS ADDED	
	TYPED A & B	
10-2-72	REVISED AND REDRAWN	1508-10-2-72
	DATE REVISION	DATE FILM D

ARKANSAS STATE HIGHWAY COMMISSION

CONCRETE DITCH PAVING

STANDARD DRAWING CDP-1

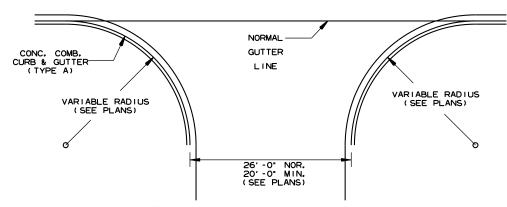


DETAIL FOR COUNTY ROAD TURNOUTS
OPEN SHOULDER SECTION

NOTE: TURNOUTS SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

> ACH (22 AGO

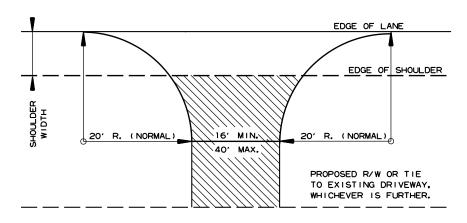
ACHM SURFACE COURSE (1/2")
(220 LBS, PER SQ. YD.) AND
AGGREGATE BASE COURSE (CLASS 7)
7' COMP, DEPTH, UNLESS OTHERWISE
SPECIFIED IN PLANS.



NOTE: PAVEMENT STRUCTURE FOR STATE HIGHWAYS, CITY STREETS, & COUNTY ROADS TO BE SAME AS MAIN LANES.

DETAIL OF TURNOUTS, ASPHALT STREETS, COUNTY ROADS & STATE HIGHWAYS

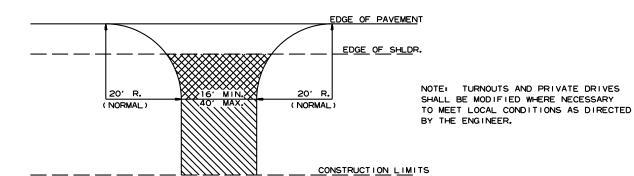
CURB & GUTTER SECTION



DETAIL FOR DRIVEWAY TURNOUTS OPEN SHOULDER SECTION (ARTERIALS) NOTE: TURNOUTS AND PRIVATE DRIVES
SHALL BE MODIFIED WHERE NECESSARY
TO MEET LOCAL CONDITIONS AS DIRECTED
BY THE ENGINEER.



ACHM SURFACE COURSE (1/2°)
(220 LBS. PER SQ. YD.) AND
AGGREGATE BASE COURSE (CLASS 7)
7' COMP. DEPTH IF ASPHALT OR
GRAVEL DRIVE EXISTING; OR 6'
CONCRETE IF CONCRETE DRIVE
EXISTING.

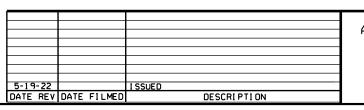


ASPHALT CONCRETE HOT MIX SURFACE COURSE (220 LBS, PER SQ, YD.) AGGREGATE BASE COURSE (CLASS 7) 7' COMP, DEPTH IF ASPHALT DRIVE EXIST OR 6' CONCRETE IF CONCRETE DRIVE EXIST.

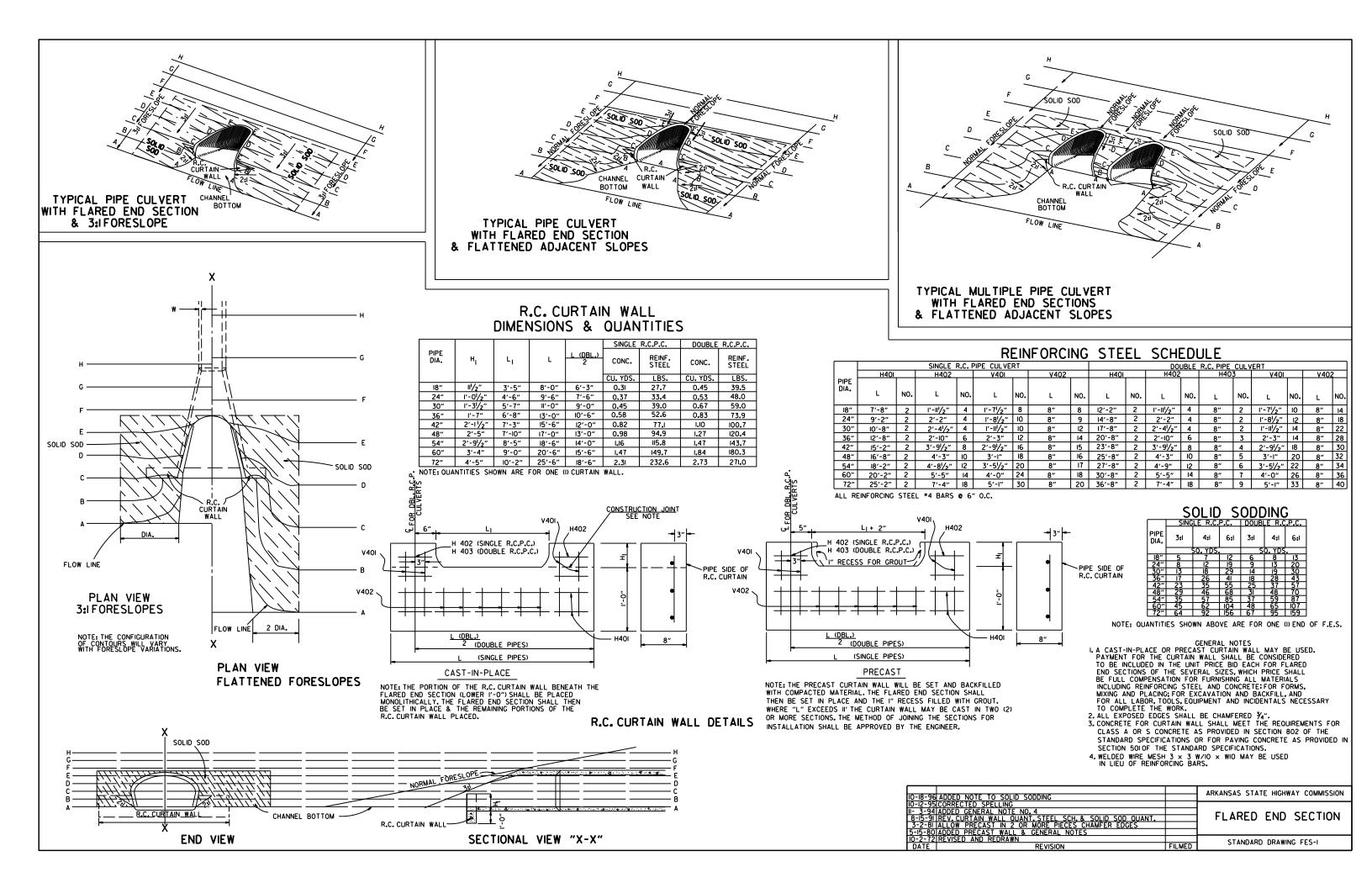


AGGREGATE BASE COURSE (CLASS 7) 9° COMP. DEPTH OR CONFORM TO EXISTING DRIVEWAY

DETAIL FOR DRIVEWAY TURNOUTS (COLLECTORS)



ARKANSAS STATE HIGHWAY COMMISSION
DETAILS OF DRIVEWAYS & STREET
TURNOUTS
STANDARD DRAWING DR-2



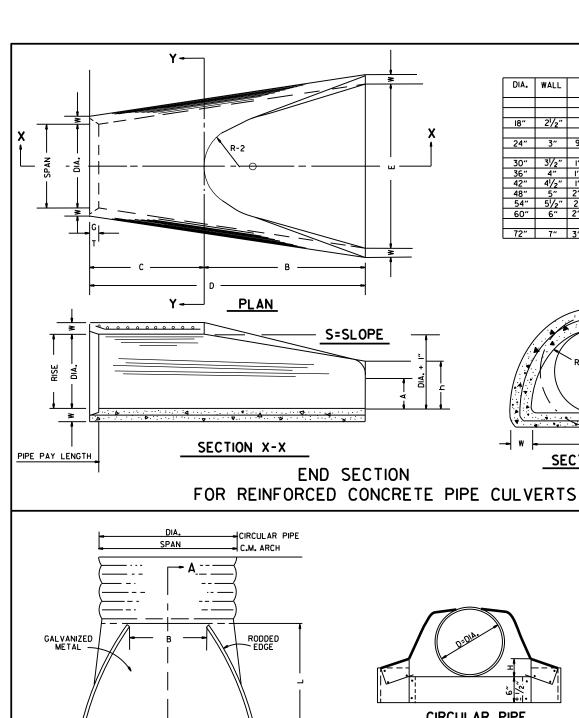
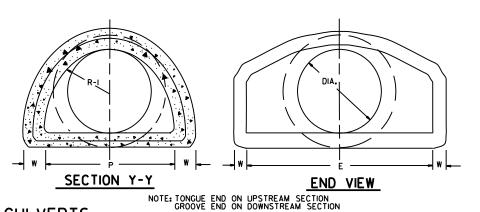


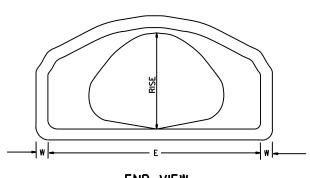
TABLE OF DIMENSIONS 6" 2'-10" 6'-6" 1'-10" 8'-4" 8'-0" 3:1 61" 72 2" 36 6" 24" 4" 9270 3'-5"



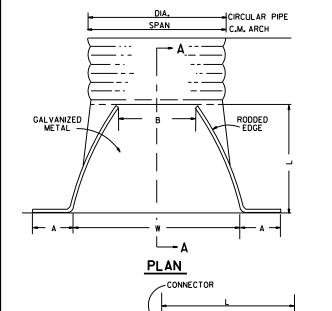
ARCH PIPE

EQUIV.	• SPAN		• RISE											
	AASHTO M 206	AHD NOMINAL	AASHTO M 206	AHD NOMINAL	w	A	В	С	D	E	Р	R2	G-T	s
	INCHES													
15	18	18	II	II	2"	4"	2'-0"	4'-0"	6′-0″	3′-0"	29"	12"	11/2"	21/2:1
18	22	22	131/2	14	21/2"	5"	2'-0"	4'-1"	6'-1"	3'-6"	321/8"	13"	21/2"	21/2:1
21	26	26	151/2	16	23/4"	7"	2'-3"	3'-10"	6'-1"	4'-0"	341/8"	14"	21/2"	21/2:1
24	281/2	29	18	18	3"	9"	2'-3"	3′-10″	6'-1"	5′-0″	36⅓ "	15"	21/2"	21/2:1
30	36 ¹ / ₄	36	221/2	23	31/2"	10"	3′-1"	3'-01/2"	6'-11/2"	6′-0″	47 ¹ 3/6 "	20"	3"	21/2:1
36	43¾	44	26%	27	4"	101/2"	4'-0"	2'-1/2"	6'-11/2"	6'-6"	54¾"	22"	31/2"	21/2:1
42	51/8	51	315/16	31	41/2"	II ¹ /2"	4'-7"	1-101/4"	6'-51/4"	7′-2″	591/2"	23"	3¾"	21/2:1
48	581/2	59	36	36	5"	1'-3"	5′-3″	2′-10¾"	8'-13/4"	7'-10"	70%"	24"	41/4"	21/2:1
54	65	65	40	40	51/2"	l'-7"	5'-3"	2'-11"	8'-2"	8′-6″	721/16"	24"	43/4"	21/4:1
60	73	73	45	45	6"	1'-10"	5′-6″	2′-8″	8'-2"	9′-0″	7713/16 "	24"	5"	21/4:1

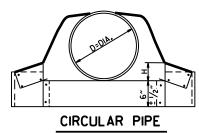
THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PER CENT FROM THE VALUES SPECIFIED BY AASHTO M 206.

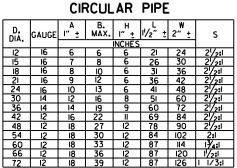


END VIEW CONCRETE ARCH PIPE



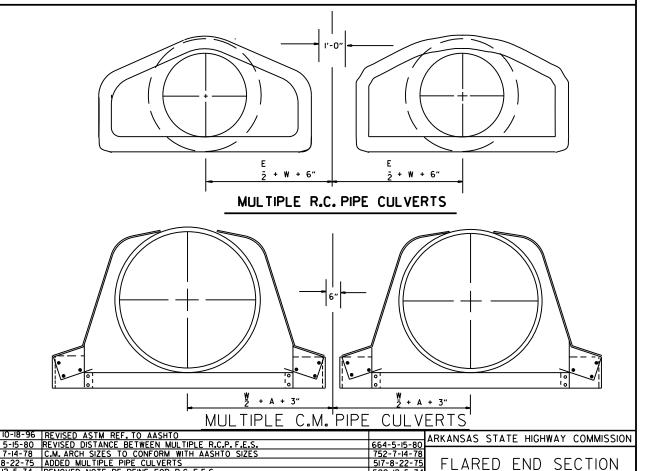
PIPE PAY LENGTH



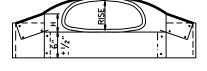


	15	16	工	7	8	6	26		30	21/		
	18	16		8	10	6	31		36	21/2	ادد	
	21	16		9	12	6	36	Ŀ	42	21/2	ادد	
	24	16		10	13	6	41	Ŀ	48	21/2	2:1	
	30	14	Ш	12	16	8	51		60	21/2	2:1	
	36	14	L	14	19	9	60		72	21/2		
	42	12	I	16	22	II	69		84	21/2	ادر	
	48	12		18	27	12	78		90	21/2	ائح	
	54	12		18	30	12	84	Π	02	2:1		
	60	12	\mathbf{I}	18	33	12	87		114	13/4	:	
	66	12		18	36	12	87	D	120	1/2:	:I	
	72	12	ı	18	39	12	87	\Box	26	1 1/3	3:1	
			C.	м.	ARC	н Р	IPE_					
s	SPAN	RISE	и И" <u>+</u>			l½″	± 2"	ţ	s	,	GA	UGE
				INICHE								

EQUIV.	SPAN	RISE	_			l½″ ±	w 2″ <u>+</u>	s	GAUGE
				INCHE:	5				
15"	17	13	7	9	6	19	30	21/2:1	16
.81	21	15	7	ō	6	23	36	21/2:1	16
21"	24	18	8	12	6	28	42	21/2:1	16
24"	28	20	9	14	6	32	48	21/2:1	16
30"	35	24	10	16	6	39	60	21/2:1	14
36"	42	29	12	18	8	46	75	21/2:1	14
42"	49	33	13	21	9	53	85	21/2:1	12
48"	57	38	18	26	12	63	90	21/2:1	12
54"	64	43	18	30	12	70	102	21/4:1	12
60"	71	47	18	33	12	77	114	21/4:1	12



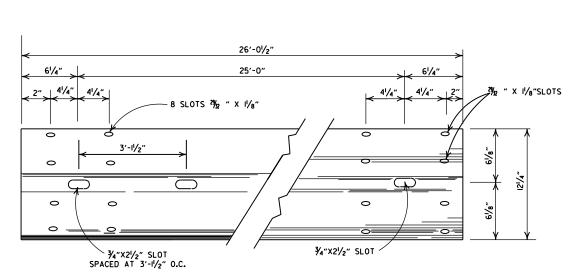
STANDARD DRAWING FES-2

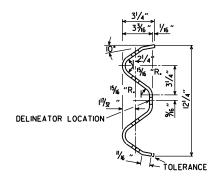


C.M. ARCH PIPE

SECTION A-A NOTE: ALTERNATE CONNECTIONS TO THE PIPE CULVERTS, IN ACCORDANCE WITH MANUFACTURER'S STANDARD PRACTICES, MAY BE MADE SUBJECT TO THE APPROVAL OF THE ENGINEER.

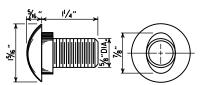
END SECTIONS FOR CORRUGATED METAL PIPE CULVERTS



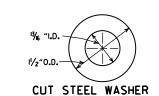


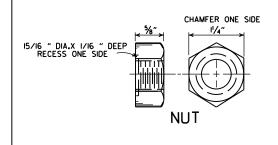
DETAILS OF W-BEAM GUARDRAIL

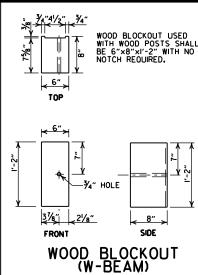
RAIL SECTION OF CLOSELY SIMILAR DIMENSIONS AND COMPARABLE STRENGTH MAY BE SUBSTITUTED IF APPROVED BY THE ENGINEER.

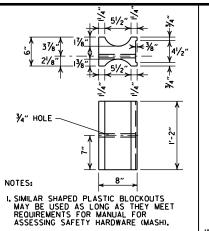


SPLICE BOLT POST BOLT - SAME EXCEPT LENGTH



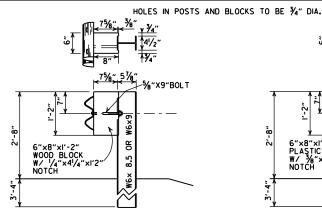




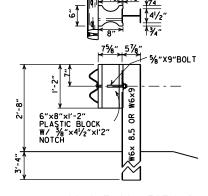


2.DIMENSIONS ARE SUBJECT TO MANUFACTURERS TOLERANCES.

PLASTIC BLOCKOUT (W-BEAM)

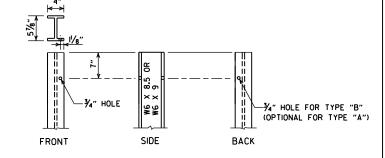


WOOD BLOCKOUT CONNECTIONS

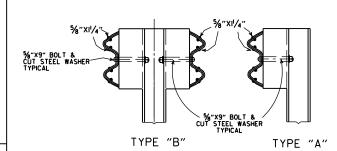


PLASTIC BLOCKOUT CONNECTIONS

DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)



STEEL POST



DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)

-GENERAL NOTES-

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN %" BEYOND IT.

WHERE W-BEAM GUARDRAIL CONTINUES, THE INTERMEDIATE SECTIONS SHALL HAVE A POST SPACING OF 6'-3" UNLESS OTHERWISE NOTED.

W-BEAM GUARDRAIL REPRESENTING INTERMEDIATE SECTIONS WILL BE MEASURED ALONG THE ROADWAY FACE FROM CENTERLINE OF POST TO CENTERLINE OF POST.

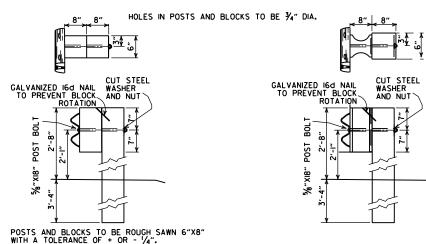
USE W-BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. FOR EXTENSIONS OR MODIFICATION OF EXISTING GUARDRAIL, W-BEAM GUARDRAIL COMPONENTS OF THE SAME TYPE AS THOSE EXISTING SHALL BE USED.

ANY BACKFILLING UNDER OR AROUND POST SHALL BE DAMP SAND THOROUGHLY TAMPED IN PLACE.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO. I 1350 f SOUTHERN PINE.

CONTRACTOR SHALL HAVE THE OPTION OF USING WOOD BLOCKOUTS FOR W-BEAM GUARDRAIL OR PLASTIC BLOCKOUTS, AS LONG AS BLOCKOUT USED MEETS REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) FOR W-BEAM GUARDRAIL.

DELINEATORS SHALL BE MOUNTED AT 37.5° SPACING ON THE FRONT FACE OF THE GUARDRAIL. SPACING MAY BE REDUCED IN CURVES, AS DIRECTED BY THE ENGINEER. COLOR SHALL BE IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR DELINEATORS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID PER LIN.FT.FOR GUARDRAIL.

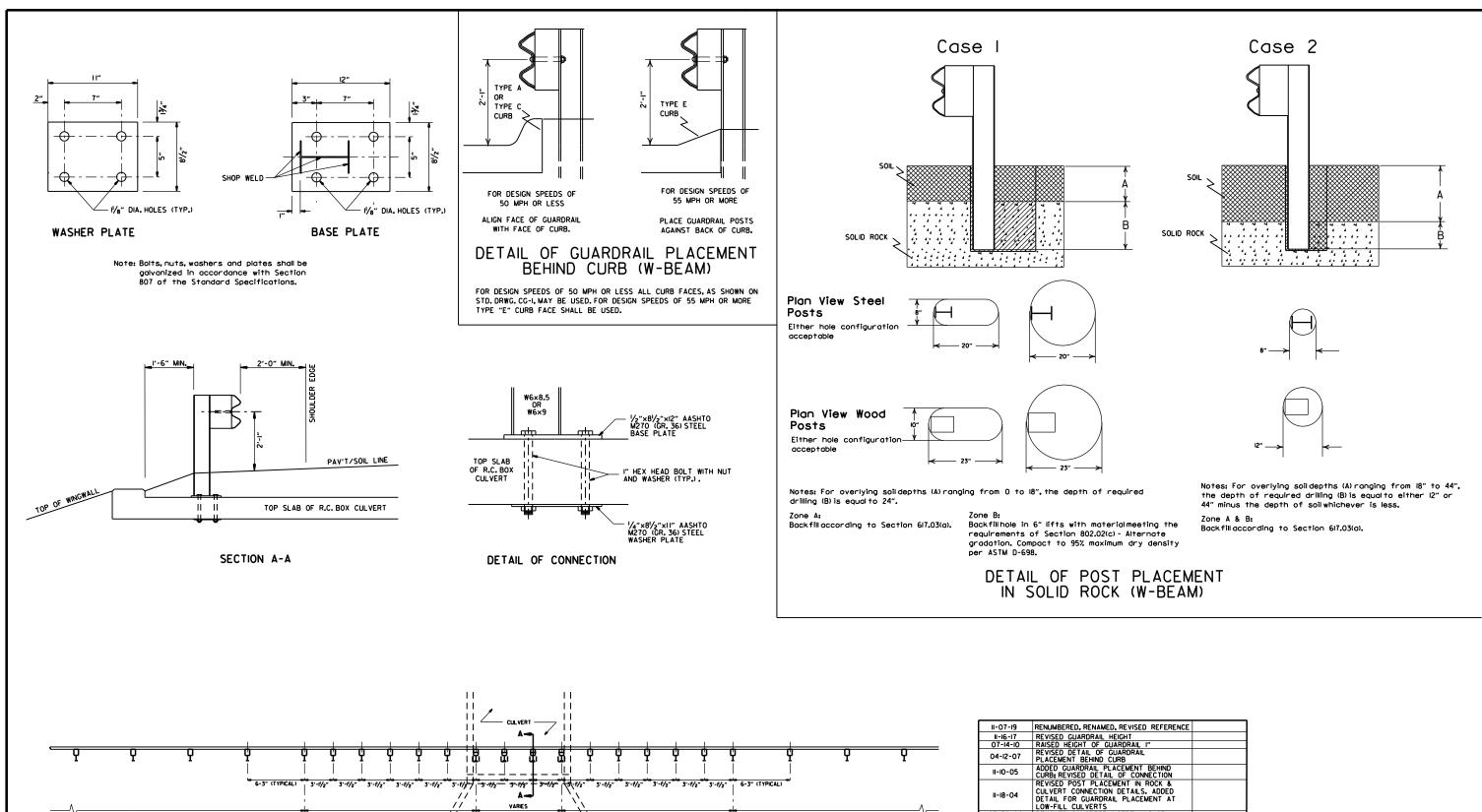


WOOD BLOCKOUT CONNECTIONS

PLASTIC BLOCKOUT CONNECTIONS

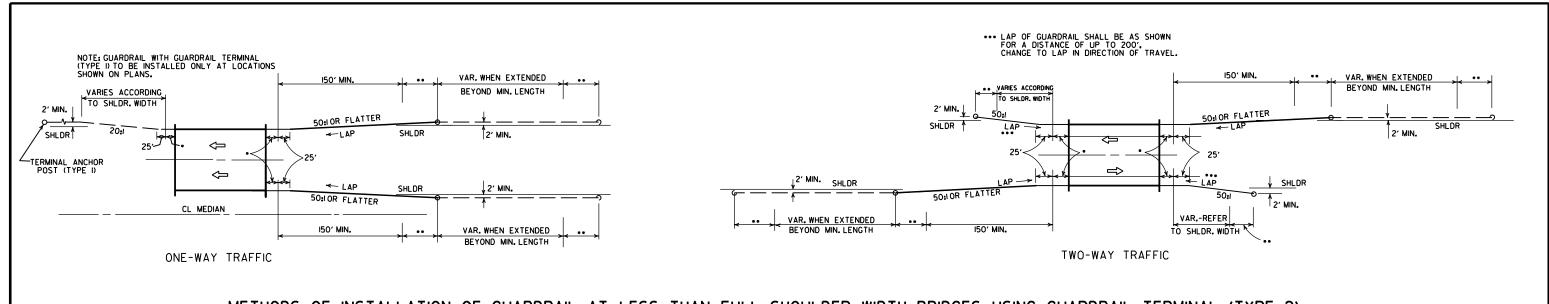
DETAILS OF WOOD LINE POST CONNECTIONS (W-BEAM)

05-19-22	REVISED GENERAL NOTES.		
11-07-19	RENUMBERED AND RENAMED		
11-16-17	REVISED GENERAL NOTES AND RAISED GUARDRAIL HEIGHT 3"		
07-14-10	RAISED HEIGHT OF GUARDRAIL I"		
10-15-09	ADDED REFERENCE TO MASH		
04-10-03	REVISED GENERAL NOTES		
08-22-02	REVISED DIMENSION ON WOOD & PLASTIC BLOCKOUT CONNECTIONS & STEEL POST		
11-16-01	REVISED WOOD BLOCKOUT & DETAILS OF WOOD LINE POST CONNECTIONS		
03-30-00	REMOVED GUARDRAIL AT BRIDGE ENDS		
01-12-00	ADDED PLASTIC BLOCKOUT		
	REV. BLOCKOUTS TO WOOD, DELETED CONC. POST & REV. GENERAL NOTE, DELETED DET. OF GUARDRAIL REPLACE. BEHIND CURB &		
08-12-98	DET. OF POST PLACE. IN SOLID ROCK,& ADDED DETAILS OF STEEL LINE POST CONN. REMOVED BACK-UP PLATE, REVISED HOLES IN STEEL POLES		
04-03-97	REMOVED "LAP IN DIRECTION OF TRAFFIC" NOTE & PLACED ARROWS ON WASHERS		
10-18-96	REVISED WOOD POST NOTE		
06-02-94	ADDED ALT. STEEL POST SIZE		
08-05-93	REVISED STEEL POST SIZE	8-5-93	ADVANCAC CTATE HICHWAY COMMICCION
10-01-92	REDRAWN & REVISED	10-1-92	ARKANSAS STATE HIGHWAY COMMISSION
08-15-91	REVISED WASHER NOTE	8-15-91	
08-02-90	REV. GEN. NOTE & DEPTH OF ANC. POST IN ROCK	8-2-90	0114555411 057411 6
07-15-88	REVISED SECTION 3 & GENERAL NOTES		GUARDRAIL DETAILS
03-04-88	REV. ANCHOR POST ,ELEV. NOTES & POST IN ROCK	780-3-4-88	
10-30-87	REVISED WOOD LINE POST DETAIL	546-10-30-87	
10-09-87	REDRAWN & REVISED	802-10-9-87	STANDARD DRAWING GR-6
DATE	REVISION	FILMED	STARBARD DRAWING OR G
			•

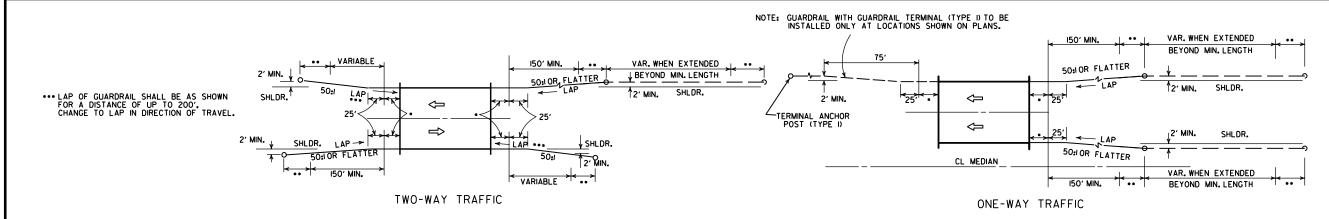


PLAN LA	AYOUT OF	TYPE A	GUARDRAIL	AT LOW-	FILL CULVERTS
NOTE: THIS PERM	DETAIL IS TO	BE USED ONL	Y WHEN THE CO	VER OVER THE AS SHOWN ON	CULVERT DOES NOT STD. DWG. GR-6.

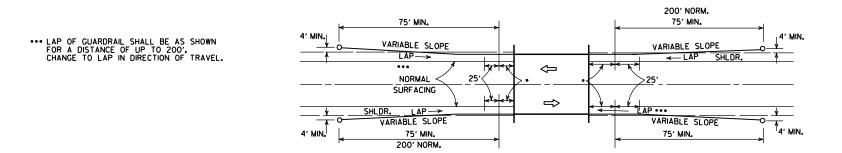
11-16-17	REVISED GUARDRAIL HEIGHT		
07-14-10	RAISED HEIGHT OF GUARDRAIL I"		
04-12-07	REVISED DETAIL OF GUARDRAIL PLACEMENT BEHIND CURB		
11-10-05	ADDED GUARDRAIL PLACEMENT BEHIND CURB; REVISED DETAIL OF CONNECTION		
11-18-04	REVISED POST PLACEMENT IN ROCK & CULVERT CONNECTION DETAILS, ADDED DETAIL FOR GUARDRAIL PLACEMENT AT LOW-FILL CULVERTS		
03-30-00	REMOVED CONCRETE INSERT ANCHOR		
08-12-98	CHANGED STEEL SPACER BLOCK TO WOOD BLOCKOUT, ADDED DET. OF GUARDRAIL CONNECTION TO R.C. BOX CULV'I., DELETED DET. OF STEEL LINE POST CONN. & ADDED DET. OF GUARDRAIL PLACE, BEHIND CURB & DET. OF POSTPLACE, IN SOLID ROCK		
04-03-96	PLACED ARROWS AT CUT STEEL WASHERS	4-3-96	
10-18-96	REV. ASTM REF. TO AASHTO		
II-22-95	ADDED OPTIONAL HOLES		ADVANCAS STATE HICHWAY COMMISSION
06-02-94	REVISED ALTERNATE POST SIZE		ARKANSAS STATE HIGHWAY COMMISSION
08-05-93	REVISED STEEL POST SIZE		
10-01-92	REDRAWN & REVISED	10-1-92	
08-02-90	DEL. WASHER ON ANCHOR ASSEMBLY	8-2-90	0114BBBA# BETA# 6
07-15-88	CONFORMED TO 1988 SPECS		GUARDRAIL DETAILS
03-04-88	REVISED ANCHOR NOTE		
10-30-87	REVISED ANCHOR ASSEMBLY	712-10-30-87	
10-30-87	REVISED PLACEMENT BEHIND CURB	547-10-30-87	
10-09-87	REDRAWN & REVISED	803-10-9-87	STANDARD DRAWING GR-7
DATE	REVISION	FILMED	STANDAND DIVAMINO ON T



METHODS OF INSTALLATION OF GUARDRAIL AT LESS THAN FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



METHOD OF INSTALLATION OF GUARDRAIL AT FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



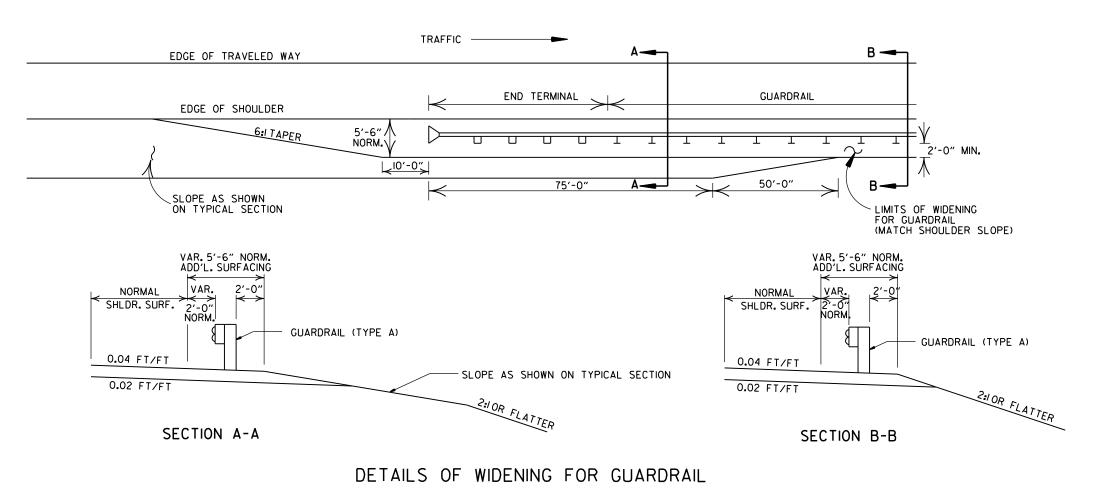
METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERMINAL (TYPE I) (FULL SHOULDER WIDTH OR LESS BRIDGES)

		_		
			ARKANSAS STATE HIGHWAY COMMISSION	
11-07-19	RENUMBERED AND RENAMED			
4-17-08	REVISED LAYOUTS			
11-10-05	REMOVED GUARDRAIL NOTES AND DETAILS			
11-16-01	DELETED NOTE-METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERM. (TY. I)		GUARDRAIL DETAILS	
1-12-00	ADDED CONSTRUCTION NOTE	1-12-00		
6-26-97	REVISED LAYOUT			
10-1-92	REDRAWN & REVISED	10-1-92		
	ADDED NOTE			
10-9-87	REDRAWN & REVISED		STANDARD DRAWING GR-8	
DATE	REVISION	DATE FILM		

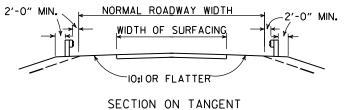
LEGEND

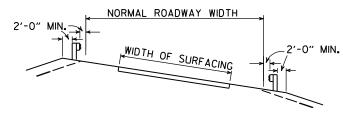
. THRIE BEAM GUARDRAIL TERMINAL

.. GUARDRAIL TERMINAL (TYPE 2)



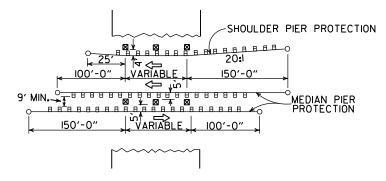
NOTE: NORMAL SECTION TO BE WIDENED APPROX.5'-6" EACH SIDE TO SUPPORT GUARDRAIL.





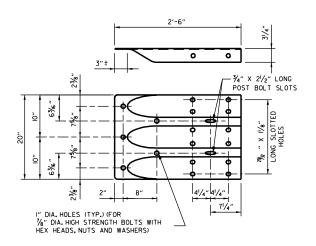
SECTION ON CURVE

DETAILS SHOWING POSITION OF GUARDRAIL ON HIGHWAY



METHOD OF INSTALLATION OF GUARDRAIL AT FIXED OBSTACLE

			ARKANSAS STATE HIGHWAY COMMISSION
			GUARDRAIL DETAILS
11-07-19	RENUMBERED AND RENAMED		
4-17-08	MINOR REVISION		
11-10-05	DRAWN		STANDARD DRAWING GR-9
DATE	REVISION	DATE FILM	



SECTION THRU
THRIE BEAM RAIL

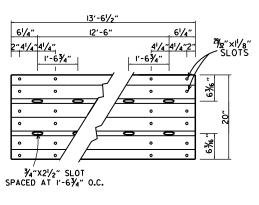
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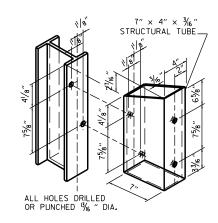


7/-3/2" 5/4" x21/2" SLOT 6/4" 71/4" 6/4" 6/4" 6/4" 6/4" 6/8" 6/8" 6/8" 6/8" 6/8" 6/8" 6/8" 6/8"

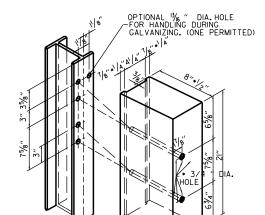
SPECIAL END SHOE

THRIE BEAM RAIL

TRANSITION SECTION



ATTACH BLOCKOUT TO POST USING 5%" DIA. HEX HEAD BOLTS WITH 11/2" O.D. CUT STEEL WASHERS AND NUT.

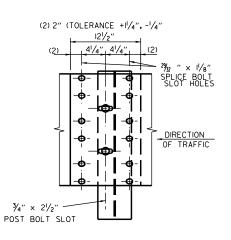


ALL HOLES 13/16 " DIAMETER EXCEPT AS NOTED

HOLE PUNCHING DETAIL FOR STEEL POST & WOOD OR PLASTIC BLOCKOUTS

NOTE: BLOCKS SHALL BE THE SAME TYPE THROUGHOUT THE PROJECT LIMITS.

STRUCTURAL STEEL TUBING BLOCKOUT DETAIL



THRIE BEAM RAIL SPLICE AT POST

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 CAGE. ZINC COATING SHALL BE TYPE I.

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

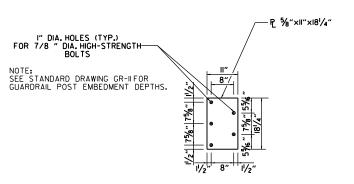
ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-13.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.

THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.

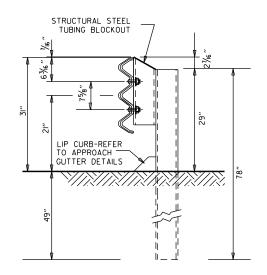
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR
BETTER 9.7f (1400 f) OR NO. 1350 f SOUTHERN PINE.



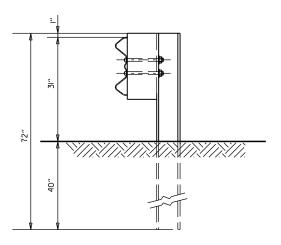
CONNECTOR PLATE

CONNECTOR PLATE SHALL BE AASHTO M270, GR. 36 AND SHALL BE GALVANIZED AFTER FABRICATION, GALVANIZING SHALL CONFORM TO SUBSECTION 807.19 OF THE STANDARD SPECIFICATIONS. CONNECTOR PLATE TO BE BOLTED TO SPECIAL END SHOE USING % DIA, HIGH STRENGTH BOLTS, WITH THE HEADS PLACED ON THE TRAFFIC FACE. WASHERS SHALL BE USED UNDER THE HEAD AND NUT. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND SHALL CONFORM TO SUBSECTION 807.06.

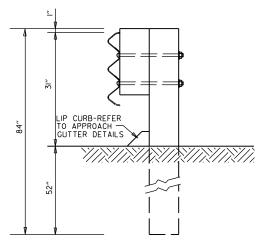
11-07-19	RENAMED AND REVISED REFERENCES		
11-16-17	REVISED TRANSITION SECTION, GUARD RAIL HEIGHT, AND GENERAL NOTES; MOYED THRIE BEAM GUARD RAIL CONNECTIONS AT BRIDGES ENDS TO STD. DRWG. GR-12		
07-14-10	RAISED HEIGHT OF W-BEAM I"		
II-29-07	ADDED PLASTIC BLOCKOUTS		ADVANCAC CTATE HICHWAY COMMICCION
11-10-05	ADDED NOTE FOR ATTACHING STEEL		ARKANSAS STATE HIGHWAY COMMISSION
	BLOCKOUT		
II-I8-04	REVISED GENERAL NOTES		
10-9-03	REVISED GENERAL NOTES		0
04-10-03	REVISED GENERAL NOTES		TOTAL TOTAL STAILS
08-22-02	REVISED NOTE (2)		
06-29-00	MOVED DIMENSION LINES		
05-18-00	ADDED NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-10
DATE	REVISION	FILMED	JI STANDAND DINAMINO DIN 10



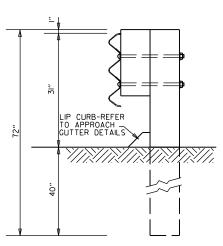
THRIE BEAM RAIL WITH STEEL TUBING BLOCKOUT AND STEEL POST POSTS 1-7



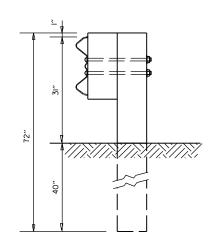
W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT AND STEEL POST POST 8



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUTS & WOOD POSTS
POSTS I-6



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUT & WOOD POST
POST 7



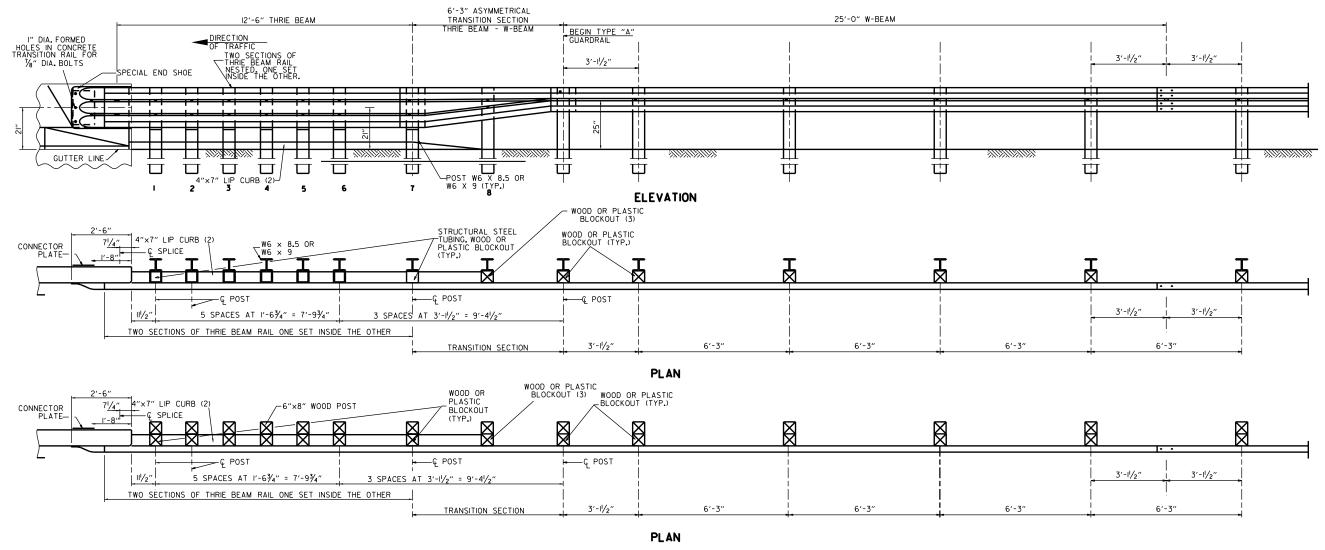
W-BEAM TO THRIE BEAM
TRANSITION RAIL WITH WOOD OR
PLASTIC BLOCKOUT & WOOD POST
POST 8

GENERAL NOTES:

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (4000 f) OR NO.11350 f SOUTHERN PINE.

			ARKANSAS STATE HIGHWAY COMMISSION
11-07-19	RENAMED		
11-16-17	REVISED GUARDRAIL HEIGHT, CHANGED STD. DWG. NUMBER FROM GR-IOA TO GR-II		GUARDRAIL DETAILS
07-14-10	REVISED POST 8 DIMENSIONS]
II-29-07	ADDED PLASTIC BLOCKOUTS		
08-22-02	REVISED LIP CURB NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-II
DATE	REVISION	FILMED	JIANDAND DINAWING ON II



- (I) VERIFY BOLT SPACING FROM RAIL TRANSITION PRODUCER.
 (2) REFER TO APPROACH GUTTER DETAILS.
- (3) LENGTH OF BLOCKOUT ON POST 8 TO BE MODIFIED TO FIT RAIL WIDTH.

THRIE BEAM GUARDRAIL CONNECTION AT BRIDGE ENDS

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I.

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $3\prime4^{\prime\prime}$ BEYOND IT.

ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-13.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

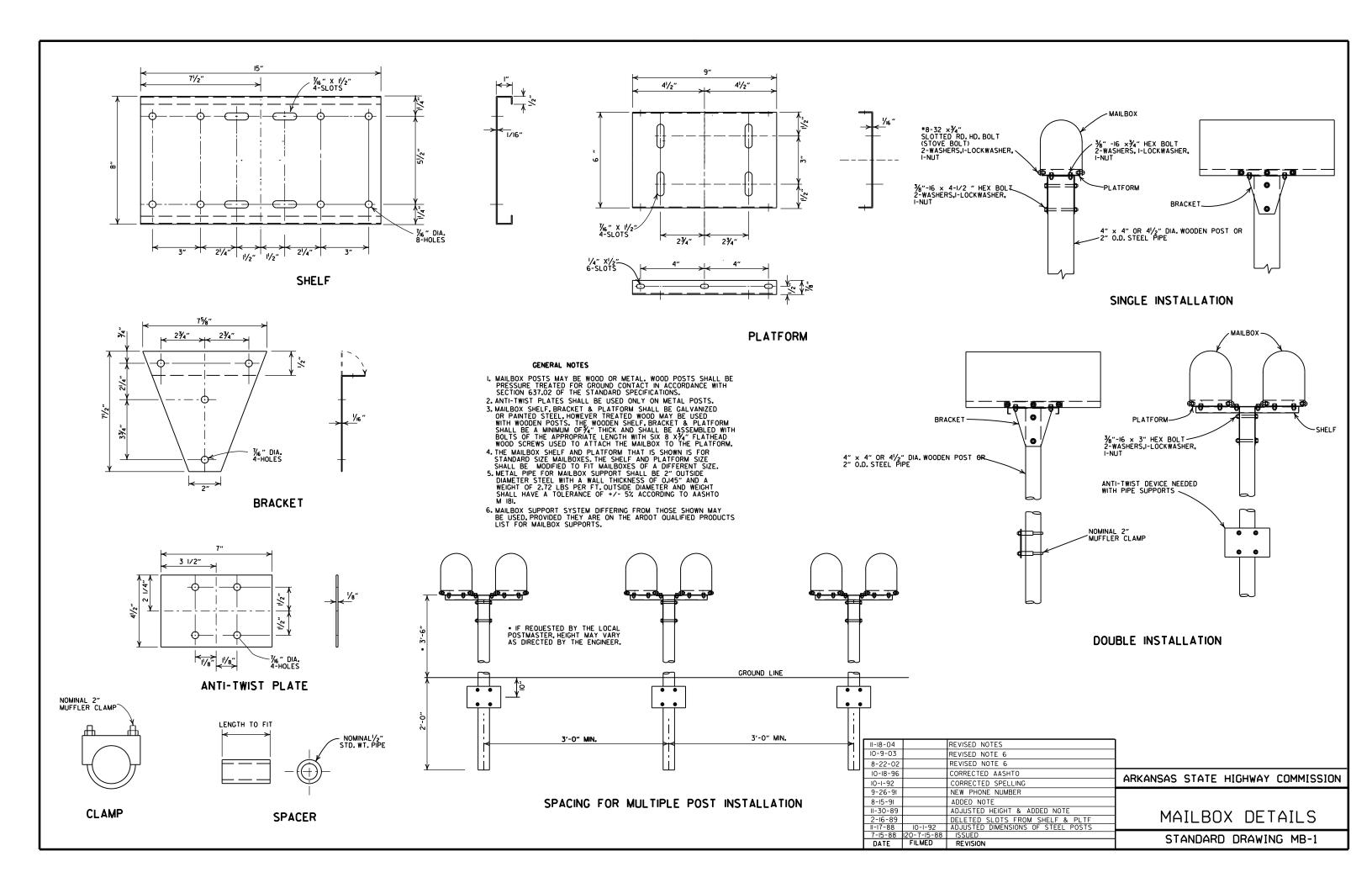
USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.

THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.

POSTS SHALL NOT BE PLACED AT SPLICE LOCATIONS ALONG W-BEAM RAILS.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO.1 1350 f SOUTHERN PINE.

		ARKANSAS STATE HIGHWAY COMMISSION
		CHARDRAIL DETAILS
PO REVISED NOTES		GUARDRAIL DETAILS
	FILMED	STANDARD DRAWING GR-12
Ç	9 RENAMED & REVISED REFERENCES 7 RE-DRAWN FROM STD.DWG.GR-IO & ISSUED	9 RENAMED & REVISED REFERENCES 7 RE-DRAWN FROM STD. DWG. GR-IO & ISSUED



REINFORCED CONCRETE ARCH PIPE DIMENSIONS

EQUIV.	SP	AN	RISE		
DIA.	AASHTO M 206	ARDOT NOMINAL	AASHTO M 206	ARDOT NOMINAL	
INCHES		INC	HES		
15 18 21 24 30 36 42 48 54 60 72 84 90 96 108 120 132	18 22 26 28½ 36¼ 43¾ 51½ 65 73 88 102 115 122 138 154 168¾	18 22 26 29 36 44 51 59 65 73 88 102 115 122 138 154 169	11 13½/2 15½/2 18 22½/2 26%/3 31%/6 36 40 45 54 62 77½/2 87½/6 106½/2	11 14 16 18 23 27 31 36 40 45 54 62 72 77 87 97	

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE DIMENSIONS

Γ	-11-	DIME	11210112			
	EQUIV.	AASHTO) М 207			
	DIA.	SPAN	RISE			
	INCHES	INC	HES			
	18	23	14			
	24	30	19			
	27	34	22			
	30	38	24			
	33	42	27			
	36	45	29			
	39	49	32			
	42	53	34			
	48	60	38			
	54	68	43			
	60	76	48			
	66	83	53			
	72	91	58			
	78	98	63			
	84	106	68			
	THE MEASURED SHAN AND					

HE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN
± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE. 5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(I).

NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE

- LEGEND -

D₁ = NORMAL INSIDE DIAMETER OF PIPE D₀ = OUTSIDE DIAMETER OF PIPE H = FILL COVER HEIGHT OVER PIPE (FEET) MIN. = MINIMUM

= UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

- *SM-3 WILL NOT BE ALLOWED.
- ** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.

MINIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

	CLASS OF PIPE				
	CLASS	III	CLASS IV	CLASS V	
INSTALLATION TYPE	TYPE 1 OR 2	TYPE 3	ALL	ALL	
PIPE ID (IN.)		FEE	Т		
12-15	2	2 . 5	2	1	
18-24	2.5	3	2	1	
27-33	3	4	2	1	
36-42	3.5	5	2	1	
48	4.5	5.5	2	1	
54-60	5	7	2	1	
66-78	6	8	2	1	
84-108	7.5	8	2	1	

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MINIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS OF PIPE				
INSTALLATION TYPE	CLASS III	CLASS IV			
	FE	EΤ			
TYPE 2 OR TYPE 3	2.5	1.5			

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

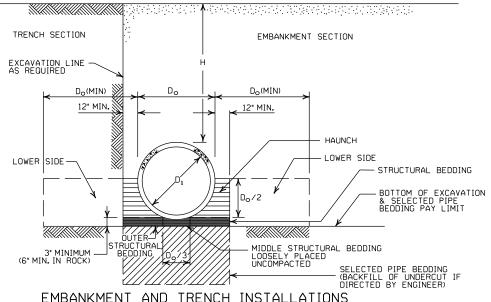
	С	LASS OF PIP	Ē			
INSTALLATION TYPE	CLASS III	CLASS IV	CLASS V			
1176	FEET					
TYPE 1	21	32	50			
TYPE 2	16	25	39			
TYPE 3	12	20	30			

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MAXIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS OF PIPE				
INSTALLATION TYPE	CLASS III	CLASS IV			
1111	FEET				
TYPE 2	13	21			
TYPE 3	10	16			

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH. IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND RECOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
- 3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

- I. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. ALL PIPE SHALL CONFORM TO SECTION 606.CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO MI70, R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING. HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SOUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
- 9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- IO. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

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ı				
	2-27-14	REVISED GENERAL NOTE I.		
ı	12-I5-II	REVISED FOR LRFD DESIGN SPECIFICATIONS		
ı	5-18-00	REVISED TYPE 3 BEDDING & ADDED NOTE		
ı	3-30-00	REVISED INSTALLATIONS		
ı	II-06-97	ISSUED		
	DATE	REVISION	DATE	FILMED

ARKANSAS STATE HIGHWAY COMMISSION CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING

STANDARD DRAWING PCC-1



CORRUGATED STEEL PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX.FILL	HEIGHT "	H" ABOVE	TOP OF PI	PE (FEET)
DIAMETER	PIPE TO TOP		METAL	THICKNESS	(INCHES)	
(INCHES)	OF GROUND ''H'' (FEET)	0.064	0.079	0.109	0.138	0.168
	2⅓ RIVET	INCH BY ED, WELDE	½ INCH D, OR HEL	CORRUGATI		
12 15 18 24 30 36 42 48	 	84 67 56 42 34	9I 73 6I 46 36 30 43	59 47 39 67 58	41 70 61	73 64
40	② 3 INCH BY		OR 5 INCH	I BY 1 INCI OR HELICA	- CORRUGA	TION
36 42 48 54 60 66 72 78 84 90 96 102 108 114	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	48 41 36 32 29 26 24	60 51 45 40 36 33 30 28 26 24 22	88 72 64 59 53 47 44 41 38 35 33 31 28 27	III 90 77 71 64 58 53 49 45 43 40 38 38 35 34	II8 IO2 85 79 71 64 59 54 51 45 44 42 39 37

CORRUGATED ALUMINUM PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX. FILL	_ HEIGHT '	'H'' ABOVE	TOP OF F	PIPE (FEET)
DIAMETER	PIPE TO TOP		METAL TH	HICKNESS I	N INCHES	
(INCHES)	OF GROUND "H" (FEET)	0.060	0.075	0.105	0.135	0.164
		2 ² / ₃	INCH B		CORRUGA	
12 18 24 30 36 42 48 54 60 66	1 2 2 2,5 2 2 2 2 2 2	45 30 22	45 30 22 I8 I5	52 39 31 26 43 40 35	4I 32 27 43 4I 37 33	34 28 44 43 38 34 31 29

CONSTRUCTION SEQUENCE

- 1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. COMPLETE STRUCTURAL BACKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE, WHITCHEVER IS 155
- NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

3 SM-3 WILL NOT BE ALLOWED.

EQUIVALENT METAL THICKNESSES AND GAUGES

	METAL			
	ST	EEL		GAUGE NUMBER
	ZINC COATED	UNCOATED	ALUMINUM	
1	0.064	0.0598	0.060	16
	0.079	0.0747	0.075	14
	0.109	0.1046	0.105	12
	0.138	0.1345	0.135	10
l	0.168	0.1644	0.164	8

ALUMINUM

2 3 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM

MAX. HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

TYPE 1

(1) MIN. HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

TYPE 1

2.25 2.5

CORRUGATED METAL PIPE ARCHES

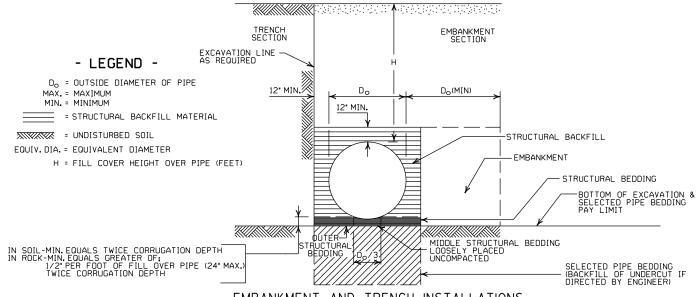
MINUMUM MIN. (1) MIN. HEIGHT OF

MAX. HEIGHT OF

MIN.

EQUIV.	DIMENSION	CORNER	THICKNESS	FILL,"		FILL,"	H'' (FT.)	THICKNESS
DIA.	SPAN X RISE		REQUIRED	INSTAL	LATION	INSTAL	LATION	REQUIRED
(INCHES)	(INCHES)	(INCHES)	INCHES	TYPI	Ε 1	TYPI	Ξ 1	INCHES
			RIV	2 ² / ₃ INCH E	BY ½ INCH (ED. OR HELIC	ORRUGATION	м	
15	17×13	3	0.064	2		I IS		0.060
18	21×15	3	0.064	2		İ		0.060
21	24×18	3	0.064	2.2		l is		0.060
24	28×20	3	0.064	2.		15	j	0.075
30	35×24	3	0.079	3		12		0.075
36	42×29	31/2	0.079	3		12		0.105
42	49×33	4	0.079	3		12		0.105
48	57×38	5	0.109	3		13		0.135
54	64×43	6 7	0.109	3		14		0.135
60 66	71×47 77×52	8	0.138 0.168	3		15		0.164
72	83×57	9	0.168] 3		15		
12	03831		3 INCH	RY 1 INCH	OR 5 INCH E	RY 1 INCH CO	ORRUGATION	1
			RIVE	2 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM				
				INSTAL	LATION	INSTAL	LATION	1
				TYPE 2	TYPE 1	TYPE 2	TYPE 1	2
36	40×3I	5	0.079	3	2	12	15	
42	46×36	6	0.079	3	2	13	15	
48	53×4I	[[0.079	3	2	13	15	
54	60×46	8 9	0.079	3 3	2	13 13	15	
60 66	66×5I 73×55	12	0.079 0.079	3	2	15	15 15	
72	81x59	14	0.079	3	2	15	15	
78	87×63	14	0.079	3	2	15	15	
84	95×67	i6	0.109	3	2	15	15	
90	103×71	16	0.109	ž	l 2	15	1 15	
96	112×75	18	0.109	3	2	15	i5	
102	117×79	18	0.109	3	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	15	15	
108	128×83	18	0.138	3	2	15	15]

- ① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.
- ② WHERE THE STANDARD 2 2/3'x ½ CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3*x 1*OR 5*x 1*CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS GAUGED FOR A FILL HEIGHT CONDITION EQUAL TO OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. INSTALLATION TYPE FOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
- 3. INSTALALTION TYPE ISHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 23" X 1/2"
- 4. INSTALLATION TYPE IOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X I" OR 5" X I" CORRUGATION.

GENERAL NOTES

- I. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE OUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL),
 BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE.

 IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

2-27-14 REVISED GENERAL NOTE I. 12-15-11 REVISED FOR LRFD DESIGN SPECT 3-30-00 REVISED INSTALLATIONS II-06-97 ISSUED REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

METAL PIPE CULVERT FILL HEIGHTS & BEDDING

STANDARD DRAWING PCM-1



INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	*SELECTED MATERIALS (CLASS SM-I, SM-2 OR SM-4)

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF HOPE PIPE.

MULTIPLE INSTALLATION OF HIGH DENSITY POLYETHYLENE PIPES

BETWEEN PIPES
1'-6"
2'-0"
2'-6"
3'-0"
3'-6"
4'-0"

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"	
18"	4'-6"	4'-6"	
24"	5′-0″	6′-0″	
30"	5′-6″	7′-6″	
36"	6′-0″	9'-0"	
42"	7′-0″	10'-6"	
48"	8'-0" 12'-0"		

(DNOTE:
18" MIN. (18" - 30" DIAMETERS)
24" MIN. (36" - 48" DIAMETERS)
MINIMUM COVER VALUES, "H"
SHALL INCLUDE A MINIMUM 12"
OF PAVEMENT AND/OR BASE.

MINIMUM COVER FOR CONSTRUCTION LOADS

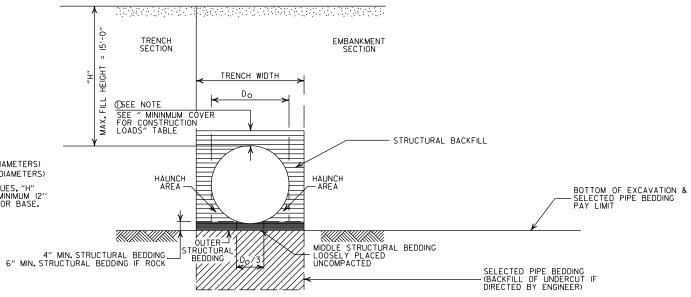
	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	110.0-175.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3'-0"	3'-0"
42" OR GREATER	3'-0"	3′-0″	3′-6″	4'-0"

☑ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE

MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M294, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE OUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE, IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. HIGH DENSITY POLYETHYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR HDPE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

H = FILL HEIGHT (FT.)
B = OUTSIDE DIAMETER OF PIPE
MAX. = MAXIMUM
MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-I4 REVISED GENERAL NOTE I. 12-I5-II REVISED GENERAL NOTES & MINIMUM COVER NOTE II-17-I0 ISSUED DATE REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT
(HIGH DENSITY POLYETHYLENE)

STANDARD DRAWING PCP-1

INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	SELECTED MATERIALS(CLASS SM-I, SM-2, OR SM-4)

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INNCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF PVC PIPE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)			
PIPE DIAMETER				
18"	4'-6"	4′-6″		
24"	5′-0″	6′-0″		
30"	5′-6″	7′-6″		
36"	6'-0"	9'-0"		

MULTIPLE INSTALLATION OF PVC PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2′-6″
36"	3'-0"
	J -0

MAXIMUM FILL HEIGHT BASED ON STRUCTURAL BACKFILL

PIPE DIAMETER	"H"
18"	45'-0"
24"	45'-0"
30"	40′-0″
36"	40'-0"

① NOTE: 12" MIN. (18" - 36" DIAMETERS) MINIMUM COVER VALUE, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

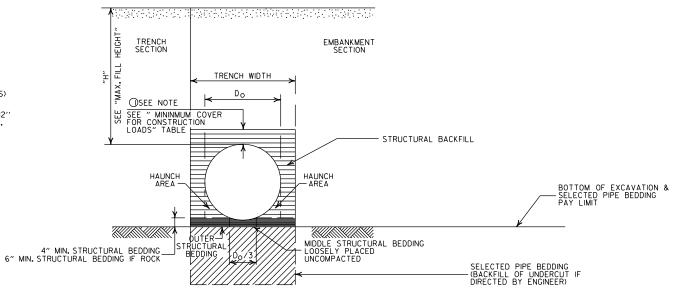
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	110.0-175.0 (KIPS)
18" THRU 36"	2'-0"	2'-6"	3'-0"	3'-0"

 ${}^{\textcircled{O}}$ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I.PIPE SHALL CONFORM TO ASTM F949, CELL CLASS 12454. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. PVC PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR PVC PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.)

 D_{O} = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM

MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I. 12-15-11 REV GENERAL NOTES & MINIMUM COVER NOTE; DELETED SM3 MATERIAL II-I7-IO ISSUED REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (PVC F949)

STANDARD DRAWING PCP-2



INSTALLATION TYPE	** MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE I	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	*SELECTED MATERIALS (CLASS SM-1, SM-2 OR SM-4) OR TYPE I INSTALLATION MATERIAL

*SM3 WILL NOT BE ALLOWED.

** STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF POLYPROPYLENE PIPE.

MULTIPLE INSTALLATION OF POLYPROPYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2′-6″
36"	3′-0″
42"	3′-6″
48"	4′-0″
60″	5′-0″

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)			
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"		
18"	4'-6"	4'-6"		
24"	5′-0″	6′-0″		
30"	5′-6″	7′-6″		
36"	6′-0″	9'-0"		
42"	7′-0″	10'-6"		
48"	8'-0"	12'-0"		
60"	10'-0" 15'-0			

12" MIN. (18" - 42" DIAMETERS) 24" MIN. (60" DIAMETER) MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12'' OF PAVEMENT AND/OR BASE.

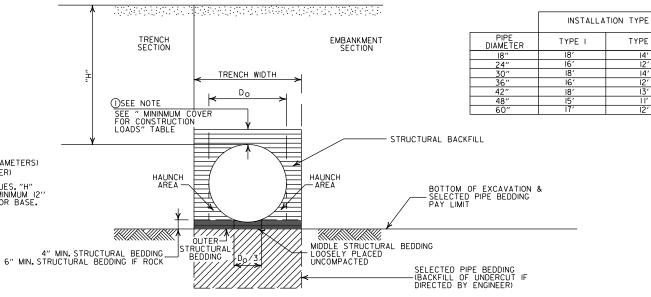
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	110.0-150.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3'-0"	3'-0"
42" OR GREATER	3'-0"	3'-0"	3′-6″	4'-0"

②MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M330, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SIXTH EDITION (2012) WITH 2013 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. POLYPROPYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR POLYPROPYLENE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN SECTION 26.4.2.4 AND 30.4.2 OF THE AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS 3RD EDITION (2010) WITH 2012 INTERIMS. JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.) Do = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM MIN. = MINIMUM

MAXIMUM HEIGHT OF FILL "H"

TYPE 2

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

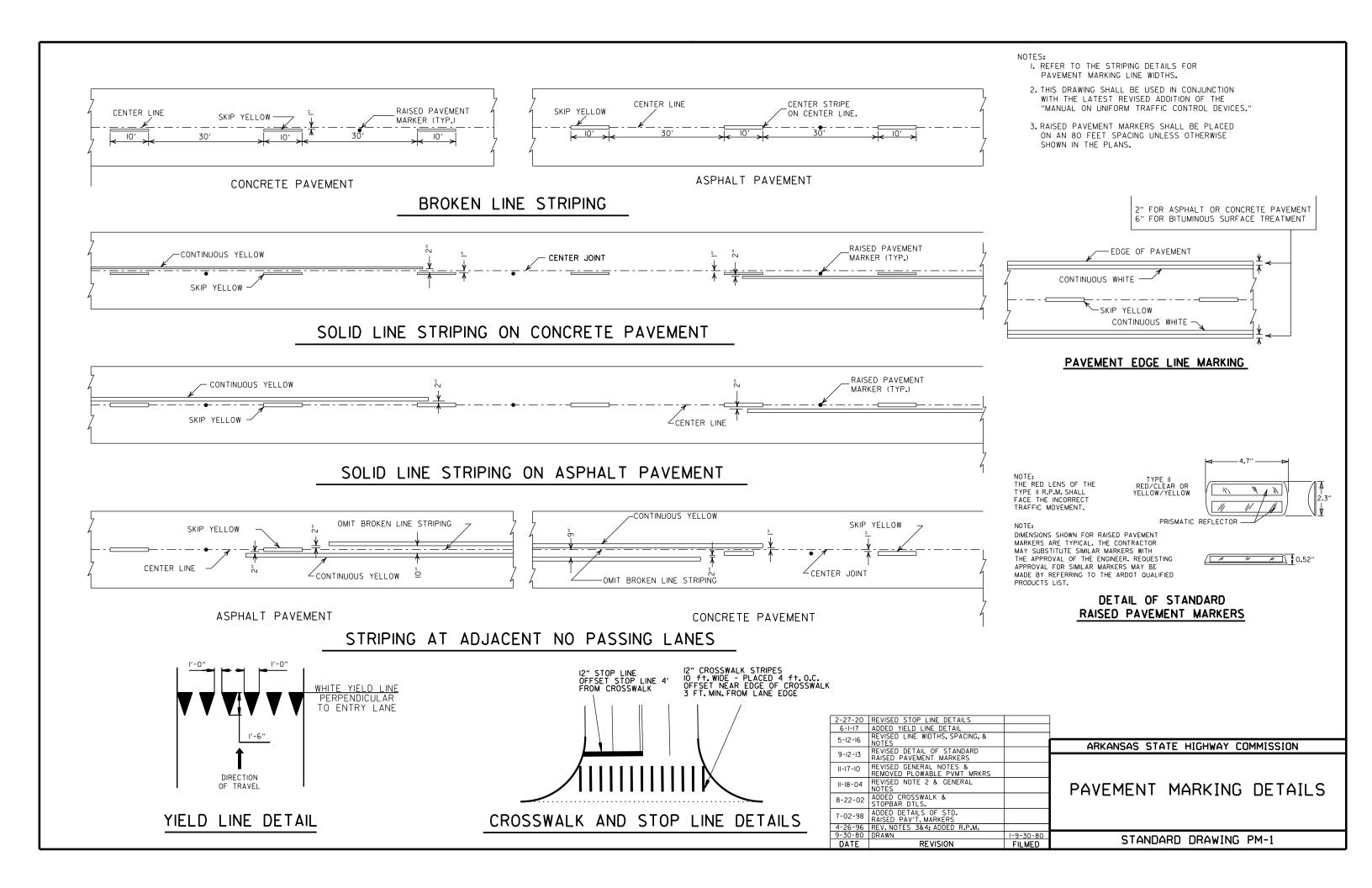
02-27-20	REVISED	
11-07-19	ISSUED	
DATE	REVISION	DATE FILMED

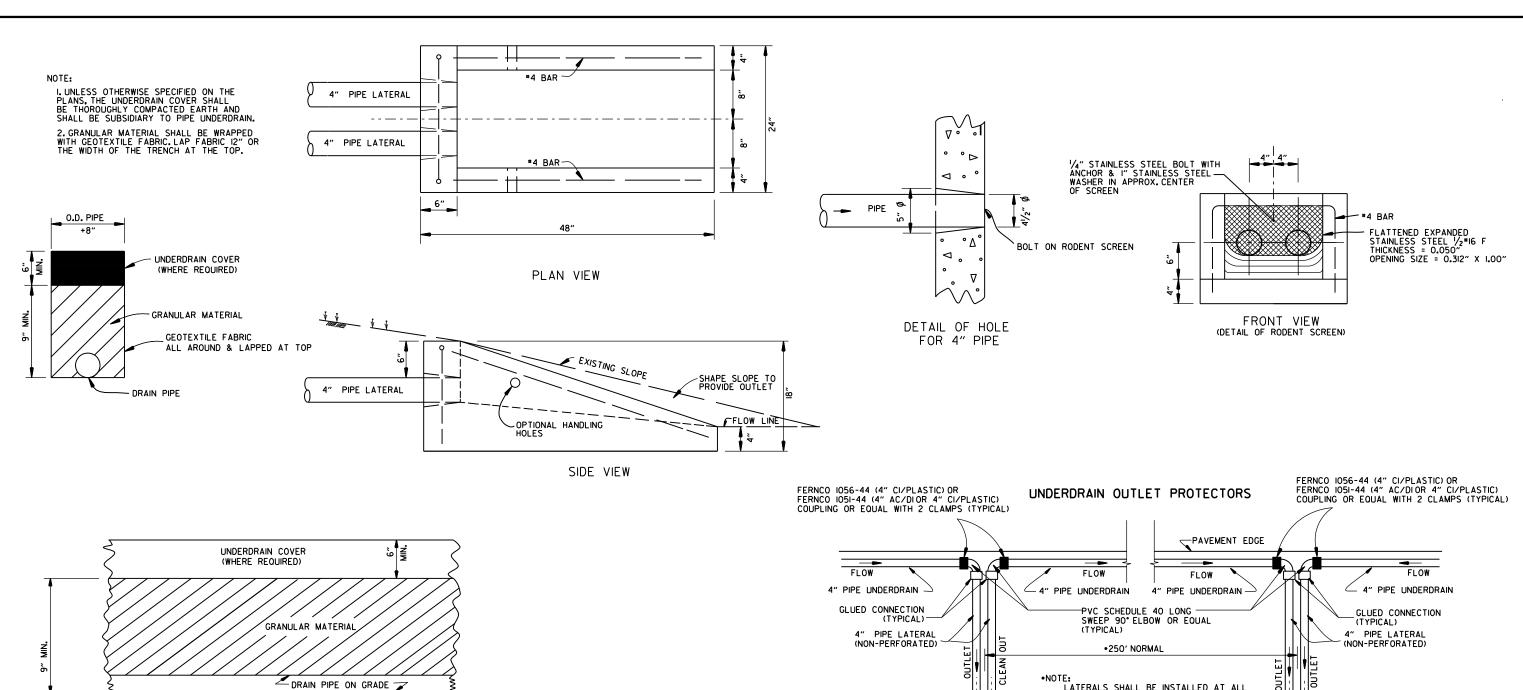
ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (POLYPROPYLENE)

STANDARD DRAWING PCP-3







DETAILS OF PIPE UNDERDRAIN

NOTES FOR PIPE UNDERDRAINS

I. GEOTEXTILE FABRIC SHALL MEET THE REQUIREMENTS OF SECTION 625 FOR TYPE I. PAYMENT FOR GEOTEXTILE FABRIC AND GRANULAR FILTER MATERIAL SHALL BE INCLUDED IN THE PRICE BID PER LIN. FT. FOR "4" PIPE UNDERDRAINS" IN ACCORDANCE WITH SECTION 611 OF THE STANDARD SPECIFICATIONS.

2.4" NON-PERFORATED SCHEDULE 40 PVC PIPE LATERALS WITH OUTLET PROTECTORS SHALL BE INSTALLED AS SHOWN HEREON, LATERALS WILL BE MEASURED AND PAID FOR BY THE UNIT IN ACCORDANCE WITH SECTION GII OF THE STANDARD SPECIFICATIONS.

3. EXISTING 4" PIPE UNDERDRAINS MAY BE CONNECTED TO PROPOSED DROP INLETS OR EXTENDED WHERE DIRECTED BY THE ENGINEER. PAYMENT FOR CONNECTING TO DROP INLETS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR "4" PIPE UNDERDRAINS."

4. THE LOCATION OF ALL LATERALS SHALL BE MARKED WITH 4" X 12" PERMANENT PAVEMENT MARKING TAPE (TYPE III WHITE) AT THE OUTSIDE EDGE OF THE SHOULDER, PLACED TRANSVERSE TO TRAFFIC. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

5. PAYMENT FOR THE RODENT SCREEN SHALL BE INCLUDED IN THE PRICE BID PER EACH FOR "UNDERDRAIN OUTLET PROTECTORS."

6. ANY EXISTING UNDERDRAINS THAT INTERFERE WITH INSTALLATION OF THE NEW UNDERDRAIN SYSTEM SHALL BE REMOVED AND DISPOSED OF AS DIRECTED BY THE ENGINEER. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS. EXISTING UNDERDRAIN OUTLET PROTECTORS SHALL BE REMOVED UNDER THE ITEM "REMOVAL AND DISPOSAL OF UNDERDRAIN OUTLET PROTECTORS."

7. AT LOCATIONS WHERE A SINGLE LATERAL IS USED THE CONTRACTOR SHALL HAVE THE FOLLOWING OPTIONS: I. INSTALL OUTLET PROTECTOR AS SHOWN ON STANDARD DRAWING PU-LAND GROUT THE UNUSED HOLE OR 2. INSTALL AN OUTLET PROTECTOR WITH A SINGLE HOLE.

FERNCO 1051-44 (4" AC/DIOR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)	UNDERDRAIN OUTLET FROTECTORS	COUPLING OR EQUAL WITH 2 CLAMPS (TYPIC
FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (TYPICAL) 4" PIPE LATERAL (NON-PERFORATED) ON GRADIENT	PAVEMENT EDGE FLOW FLO	FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (TYPICAL) 4" PIPE LATERAL (NON-PERFORATED) AT SAGS

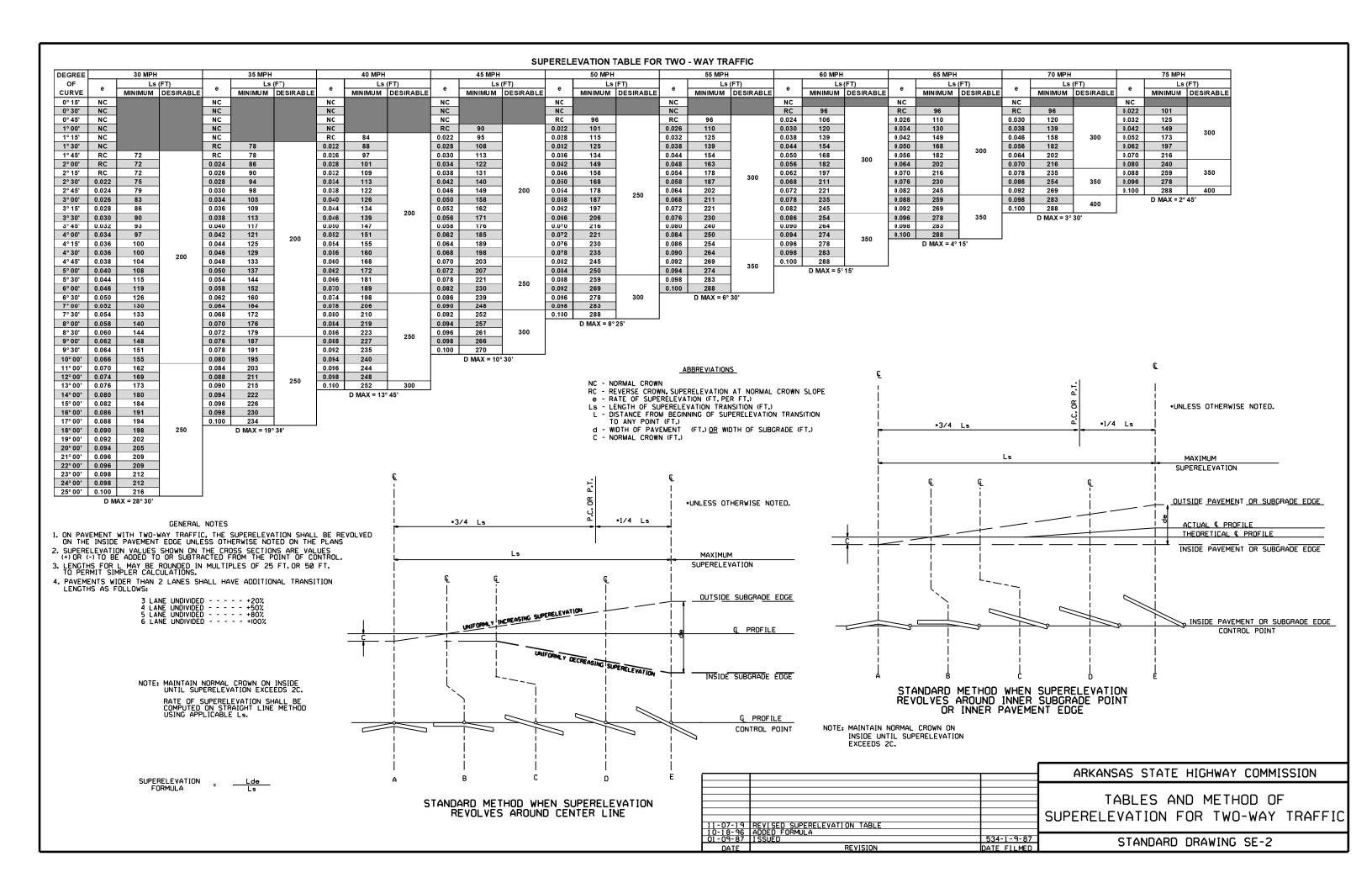
DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.

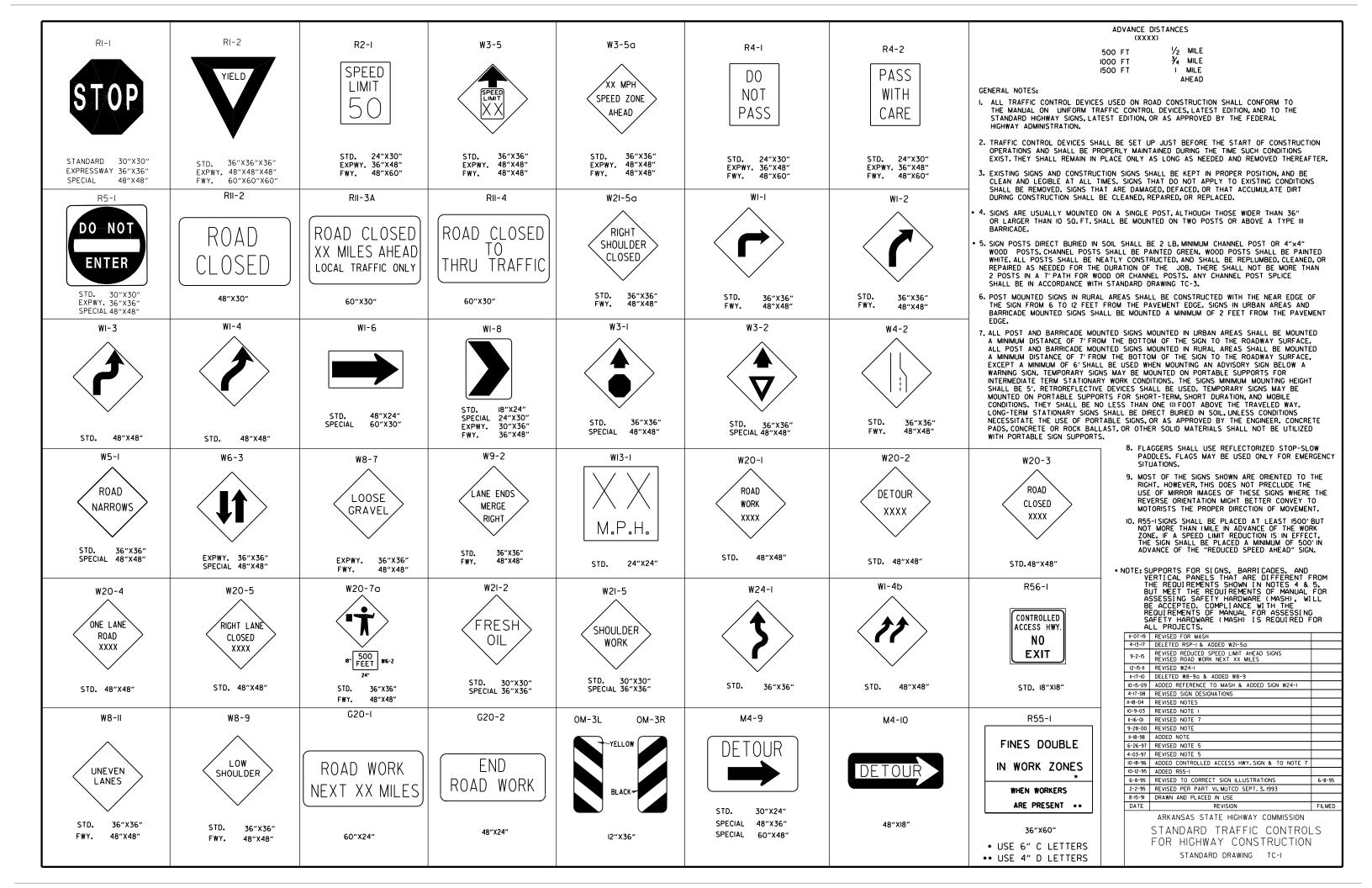
12-8-16	ADDED NOTES FOR PIPE UNDERDRAINS, REVISED RODENT SCREEN DETAIL AND NOTES, REMOVED NOTE IFOR GRANULAR MATERIAL, ADDED NOTE FOR GEOTEXTILE FABRIC		
4-10-03	REVISED NOTE 3		l
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS		l
11-18-98	REVISED NOTE		l
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC		l
4-26-96	ADDED LATERAL NOTE; 51/2" TO 5"		l
II-22-95	REVISED LATERALS		
7-20-95	REVISED LATERALS & ADDED NOTE		
II- 3-94	REVISED FOR DUAL LATERALS	II- 3-94	l
10- 1-92	SUBSTITUTED GEOTEXTILE	10- 1-92	
8-15-91	ADDED POLYEDTHYLENE PIPE	8-15-91	ł
II- 8-90	DELETED ALTERNATE NOTE	II- 8-90	ł
I-25-90	ADDED 4" SNAP ADAPTER	I-25-90	
II-30-89	DEL. (SUBGRADE); ADDED (WHERE REQUIRED)	II-30-89	
7-15-88	ISSUED P.L.M.	647-7-15-88	
DATE	REVISION	DATE FILMED	

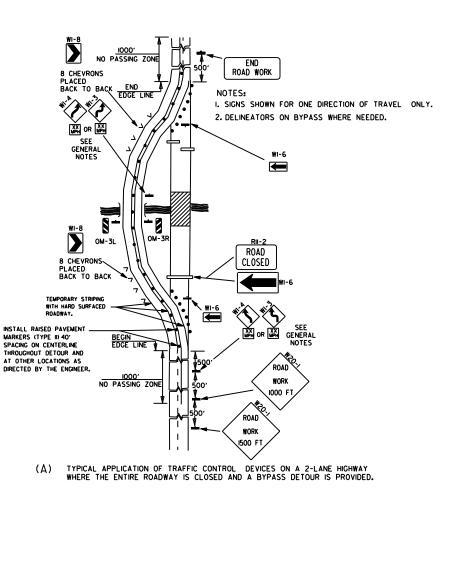
ARKANSAS STATE HIGHWAY COMMISSION

DETAILS OF PIPE UNDERDRAIN

STANDARD DRAWING PU-I







(DETOUR)

DETOUR

DETOUR

1

DETOUR

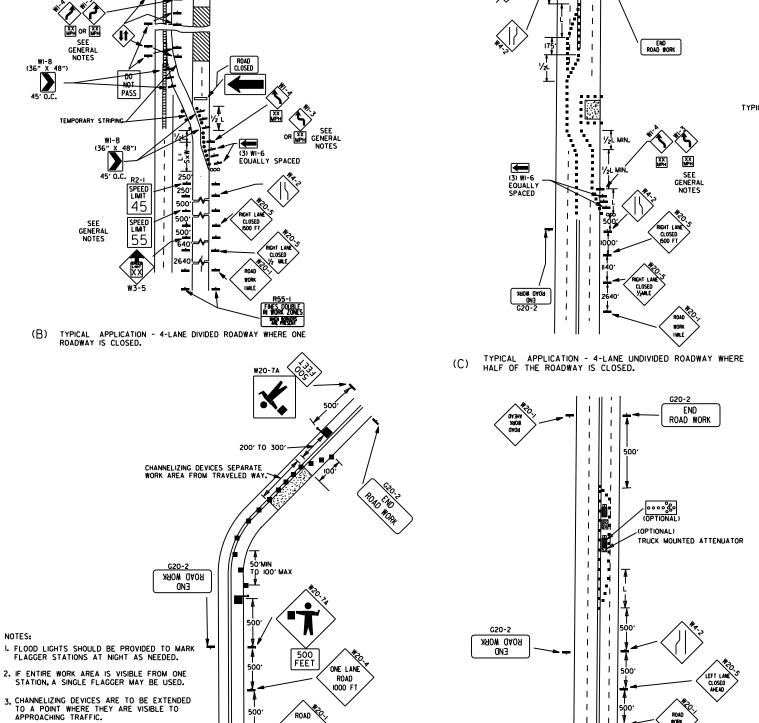
()) TYPICAL APPLICATION - ROADWAY CLOSED BEYOND DETOUR POINT.

₩EST 4

I. REGULATORY TRAFFIC CONTROL DEVICES TO BE MODIFIED AS NEEDED FOR THE DURATION OF

2.STREET NAMES MAY BE USED WHEN DESIRABLE FOR DIRECTING DETOURED TRAFFIC.

NOTES:



(F) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WITH INSIDE LANE CLOSED.

WORK

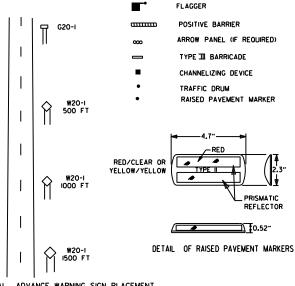
(E) TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES ON 2-LANE HIGHWAY WHERE ONE LANE IS CLOSED AND FLAGGING IS PROVIDED.

I, COMPLETE SIGNING SHOWN ONLY IN CROSSOVER DIRECTION. 2. TWO WAY TRAFFIC SEPARATED WITH POSITIVE BARRIER.

R4-7a RIGHT

4. AUTOMATED FLAGGER ASSISTANCE DEVICE

(AFAD) OPTIONAL. REFER TO MUTCD.



KEY:

TYPICAL ADVANCE WARNING SIGN PLACEMENT

TAPER FORMULAES

L=SXW FOR SPEEDS OF 45MPH OR MORE.

L= WS FOR SPEEDS OF 40MPH OR LESS.

WHERE:

G20-2

L= MINIMUM LENGTH OF TAPER.

S= NUMERICAL VALUE OF POSTED SPEED LIMIT PRIOR TO WORK OR 85TH PERCENTILE SPEED.

W= WIDTH OF OFFSET.

GENERAL NOTES:

I. THE MAINTENANCE DIVISION SHALL CONDUCT A BALL BANK STUDY TO DETERMINE THE ADVISORY SPEED LIMIT PRIOR TO OPENING TO TRAFFIC. THE ADVISORY SPEED WILL BE POSTED ON WI-3 OR WI-4 CURVE WARNING SIGNS. USE WI-4 WHEN SPEED IS GREATER THAN 30MPH AND WI-3 WHEN 30MPH OR LESS

2. WHEN THE EXISTING SPEED LIMIT IS 55MPH AND THE PLANS REQUIRE A SPEED LIMIT OF 45MPH, THE R2-K55) SHALL BE OMITTED AND THE W3-5 SHALL BE INSTALLED AT THAT LOCATION, ADDITIONAL R2-145MPH SPEED LIMIT SIGNS SHALL BE INSTALLED AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK AREA A R2-KXX)
SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

3. WHEN THE EXISTING SPEED LIMIT IS 65MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 55MPH, THE R2-I445 SHALL BE OMITTED.

ADDITIONAL R2-I55MPH SPEED LIMIT SIGNS SHALL BE INSTALLED
AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK
AREA A R2-I(XX) SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

4. THE MAXIMUM SPACING BETWEEN CHANNELIZING DEVICES IN A TAPER
SHOULD BE APPROXIMATELY EQUAL IN FEET TO THE SPEED LIMIT.

BEYOND THE TAPER, MAXIMUM SPACING SHALL BE TWO TIMES
THE SPEED LIMIT, OR AS DIRECTED BY THE ENGINEER.

5. WARNING LIGHTS AND/OR FLAGS MAY BE MOUNTED
TO SIGNS OR CHANNELIZING DEVICES AT NIGHT AS NEEDED.

CONFUSION IN THE MINDS OF VEHICLE OPERATORS SHALL BE REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

7. TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE DELINEATED BY AFFIXING CONSPICUITY MATERIAL IN A CONTINUOUS LINE ON THE FACE OF THE TRAILER, WHEN PLACED ON ON A DAJACENT TO THE SHOULDER AND NOT BEHIND A POSITIVE BARRIER, THESE DEVICES SHALL BE DELINEATED BY PLACING FIVE 15) TRAFFIC DRUMS, EQUALLY SPACED ALONG THE TRAFFIC SIDE OF THE DEVICE, PAYMENT FOR TRAFFIC DRUMS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR VARIOUS TRAILER MOUNTED DEVICES.

B. DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE ARDOT QUALIFIED PRODUCTS LIST.

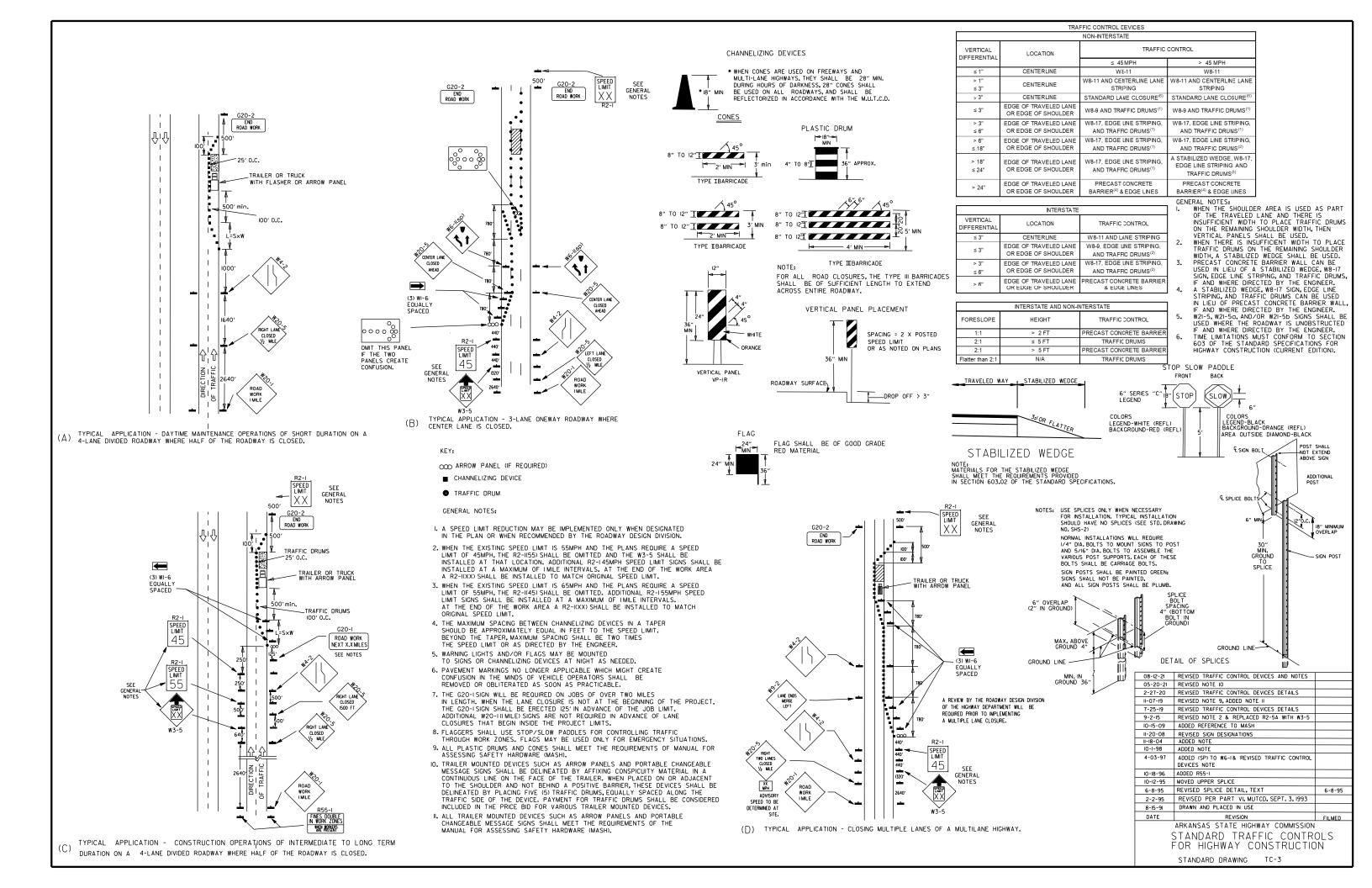
9. ALL TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL MEET THE REQUIREMENTS OF THE MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).

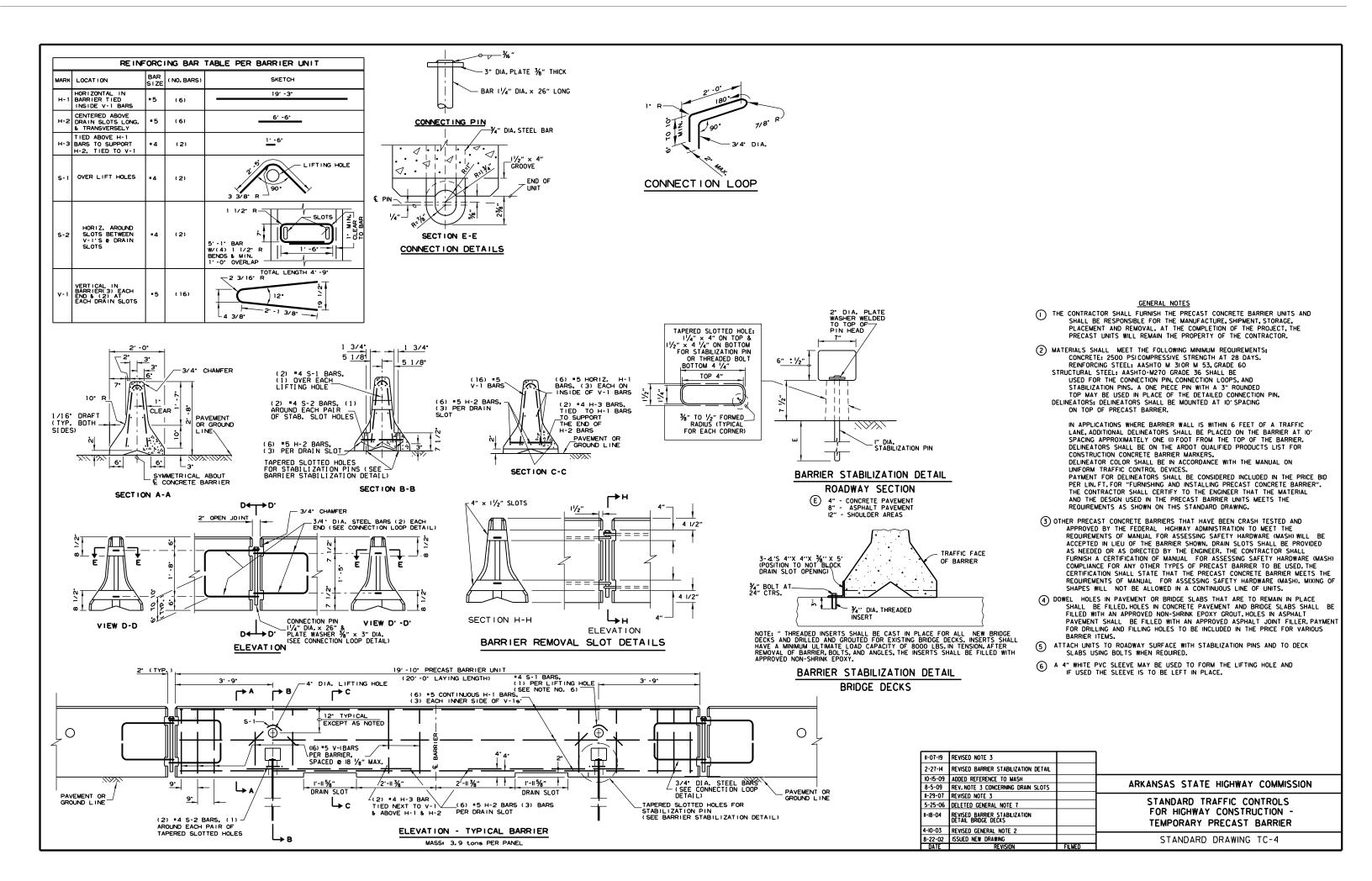
05-20-21	REVISED NOTE 7	
II-07-I9	REVISED NOTE I. ADDED NOTE 9	
9-2-15	REVISED NOTE 2, ADDED NOTE 8, REVISED DRAWING (A) & REPLACED R2-5A WITH W3-5	
9-12-13	REVISED DETAIL OF RAISED PAVEMENT MARKERS	
3-11-10	ADDED (AFAD)	
II-20-08	REVISED SIGN DESIGNATIONS	
II-I8-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-I	
4-26-96	CORRECTED (a) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON WI-4A	6-8-95
2-2-95	REVISED PER PART VI. MUTCO, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED

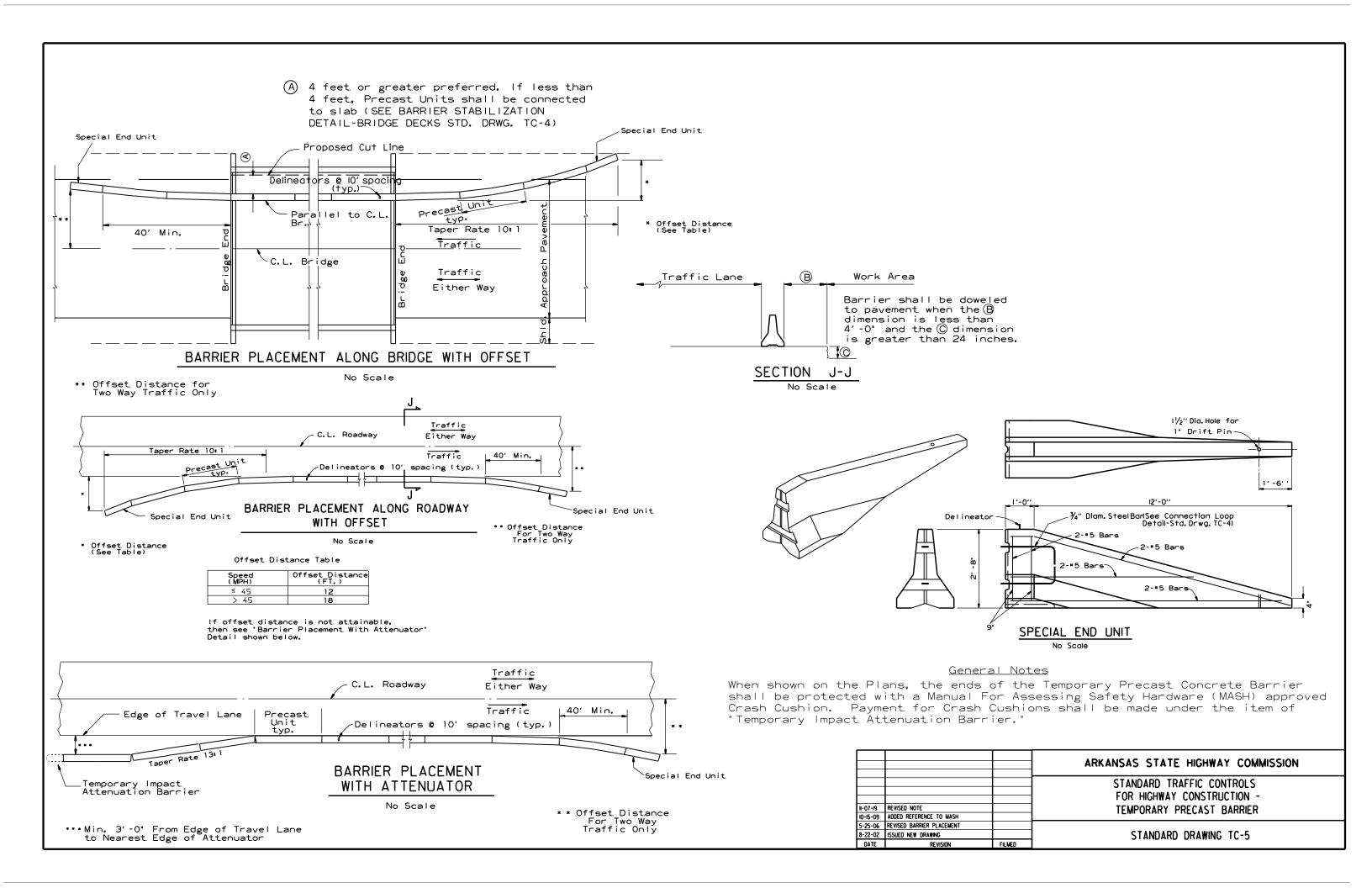
ARKANSAS STATE HIGHWAY COMMISSION

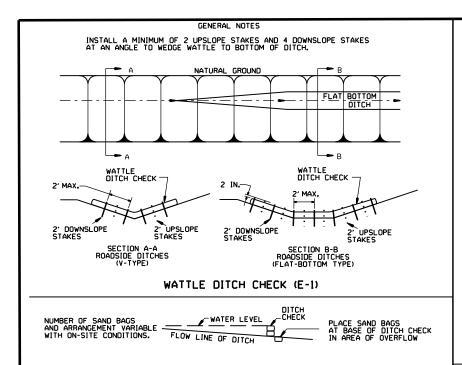
STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION

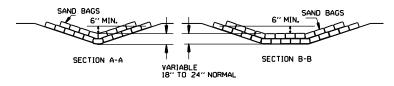
STANDARD DRAWING TC-2



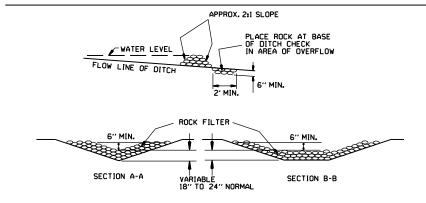








SAND BAG DITCH CHECK (E-5)

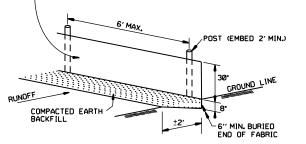


ROCK DITCH CHECK (E-6)

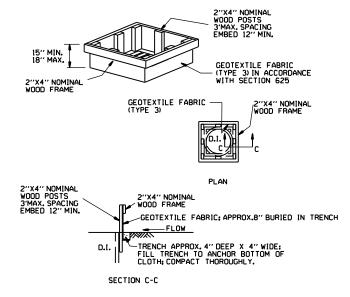
GENERAL NOTES

GEOTEXTILE FABRIC (TYPE 4) IN ACCORDANCE WITH SECTION 625

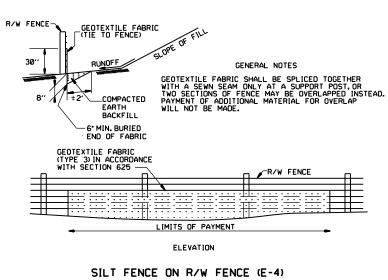
GEOTEXTILE FABRIC SHALL BE SPLICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.



SILT FENCE (E-11)



DROP INLET SILT FENCE (E-7)

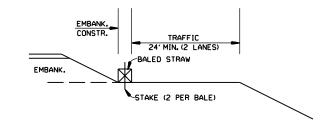


GENERAL NOTES

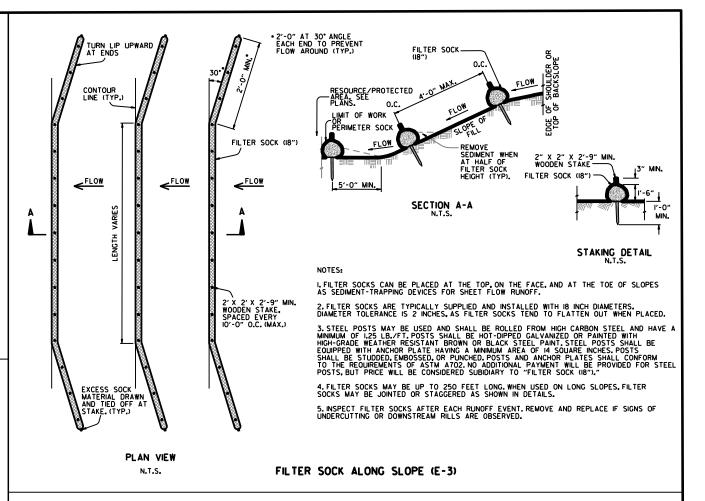
1. STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.

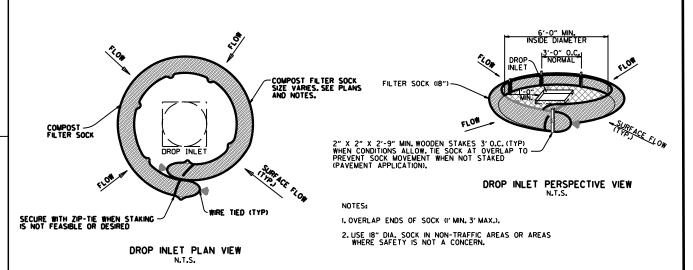
2. NO GAPS SHALL BE LEFT BETWEEN BALES.

3, BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.



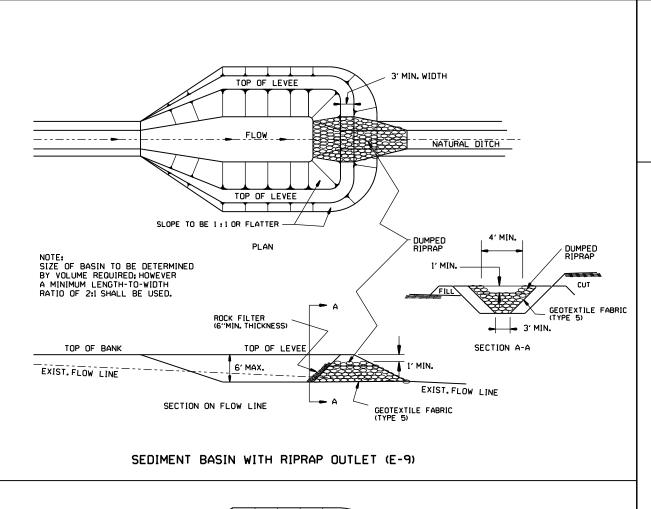
BALED STRAW FILTER BARRIER (E-2)

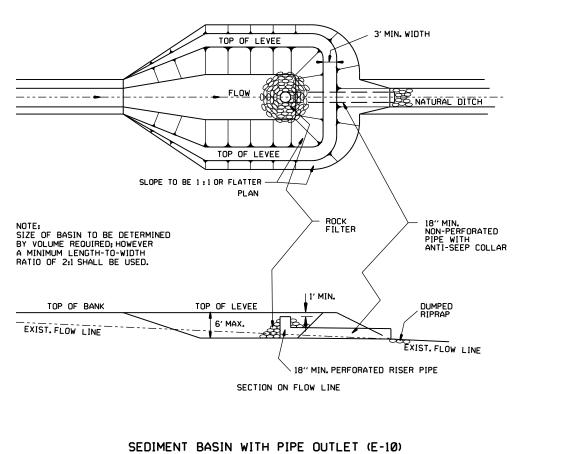


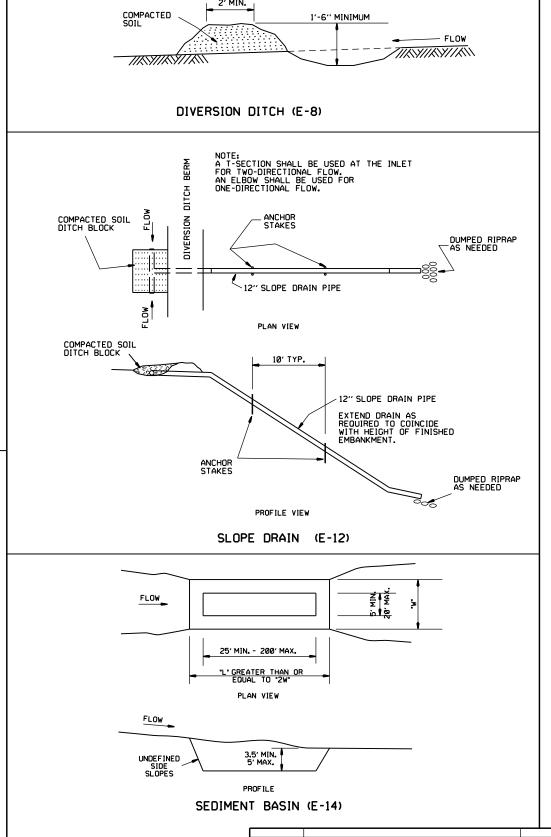


COMPOST FILTER SOCK DROP INLET PROTECTION (E-I3)

11-16-17	ADDED FILTER SOCK E-3 AND E-13		
12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ARKANSAS STATE HIGHWAY COMMISSION
11-18-98	ADDED NOTES		ARRANSAS STATE HIGHWAT COMMISSION
07-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)	7 00 05	TELIDOD LDV. EDOCION
07-20-95	REVISED SILT FENCE E-4 AND E-II	7-20-95	TEMPORARY EROSION
07-15-94	REV. E-4 & E-II MIN. 13" BURIED END OF FABRIC		
06-02-94	REVISED E-1,4.7 & II; DELETED E-2 & 3	6-2-94	CONTROL DEVICES
04-01-93	REDRAWN		CONTINUE DEVICES
10-01-92	REDRAWN		
08-02-76	ISSUED R.D.M.	298-7-28-76	STANDARD DRAWING TEC-I
DATE	REVISION	FILMED	STANDARD DRAWING TECT







6-2-94 Revised E-8 & E-12; Added E-14 & Deleted E-13
4-1-93 ISSUED
DATE REVISION

ARKANSAS STATE HIGHWAY COMMISSION

TEMPORARY EROSION CONTROL DEVICES

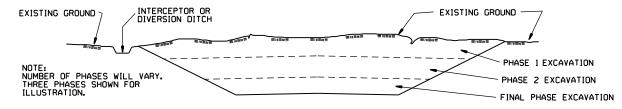
STANDARD DRAWING TEC-2

CLEARING AND GRUBBING

CONSTRUCTION SEQUENCE

2. PERFORM CLEARING AND GRUBBING OPERATION.

EXCAVATION



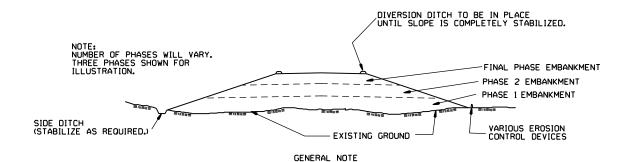
GENERAL NOTE

ALL CUT SLOPES SHALL BE ORESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

- 1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
- 2. PERFORM PHASE 1 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
- 3. PERFORM PHASE 2 EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING.
- 4. PERFORM FINAL PHASE OF EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING. STABILIZE DITCHES, CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

EMBANKMENT



ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

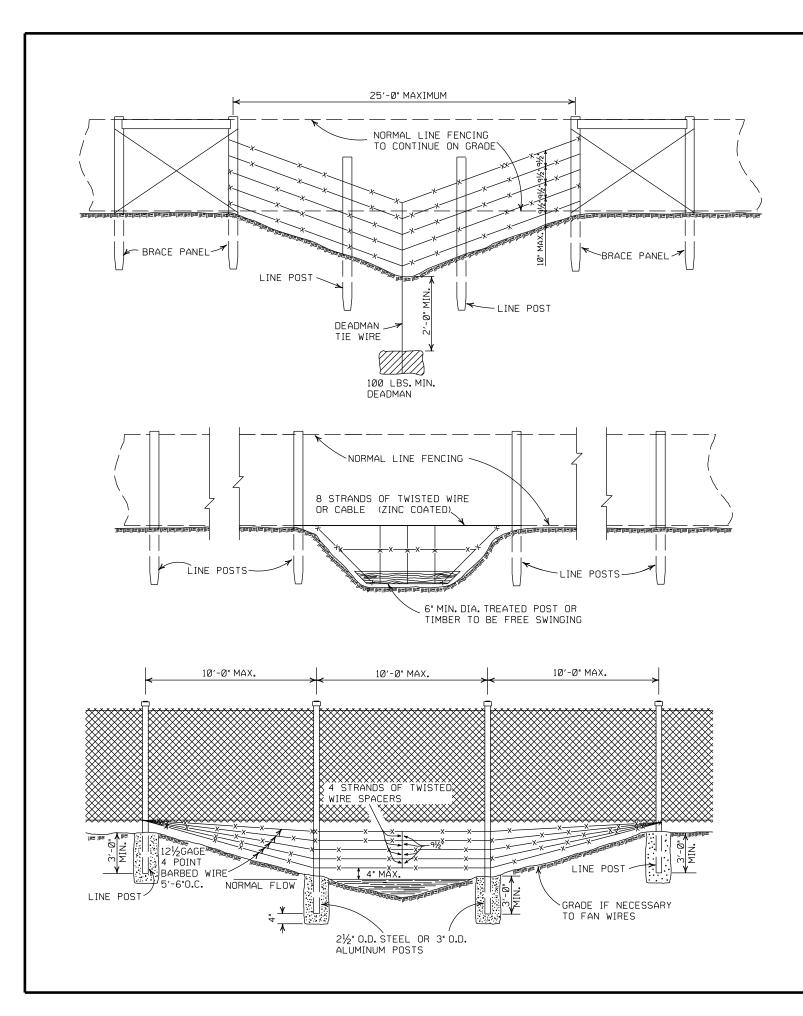
1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.

2. PLACE PHASE 1 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

			ARKANSAS STATE HIGHWAY COMMISSION	
			TEMPORARY EROSION CONTROL DEVICES	
11-03-94	CORRECTED SPELLING			
6-2-94	Drawn & Issued	6-2-94	STANDARD DRAWING TEC-3	
DATE	REVISION	FILMED	SIMPHID DIVENTING IFC 2	



GENERAL NOTES:

THESE INSTALLATIONS TO BE USED WHERE NORMAL FENCING INSTALLATION WOULD CAUSE THE COLLECTING OF DRIFT IN THE CHANNEL OR THE DEPRESSION WILL NOT PERMIT NORMAL INSTALLATION. INSTALLATIONS WILL BE MADE ONLY WHERE DIRECTED BY THE ENGINEER.

WHEN A FENCE LINE APPROACHES A DITCH, GULLY OR DEPRESSION, THE LAST POST ON LEVEL GROUND SHALL BE PLACED CLOSE ENOUGH TO THE EDGE OF THE DROP OFF THAT THE FENCE MAY BE STRUNG TO THE POST IN THE DEPRESSION WITHOUT TOUCHING THE GROUND.

IN TERRAIN OF SUCH EXTREME IRREGULARITY THAT MINOR GRADING WILL NOT BE FEASIBLE, THE NORMAL FENCE SHALL CONTINUE ON GRADE AND THE GULLIES OR DEPRESSIONS TREATED BY AUXILIARY FENCES AS SHOWN.

PAYMENT FOR THE TYPE INSTALLATION USED WILL NOT BE MADE DIRECTLY BUT WILL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR WIRE FENCE OR CHAIN LINK FENCE.

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4-20-79	REVISED TOP RAIL & TENSION WIRE	696-4-20-79	
10-2-72	REVISED AND REDRAWN	529-10-2-72	
DATE	REVISION	FILMED	

ARKANSAS STATE HIGHWAY COMMISSION

WIRE FENCE WATER GAPS

STANDARD DRAWING WF-2

