VICINITY MAP

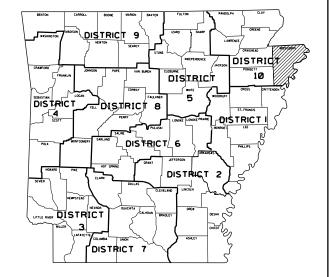
ARKANSAS DEPARTMENT OF TRANSPORTATION CONSTRUCTION PLANS

DITCH NO. 30 STR. & APPRS.

(BLYTHEVILLE) (S) MISSISSIPPI COUNTY HWY.61 SECTION FED. AID PROJ. BFP-9051(12)

DATE EVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101009		52

2 DITCH NO. 30 STR. & APPRS. (BLYTHEVILLE) (S)



ARKANSAS HIGHWAY DISTRICT 10

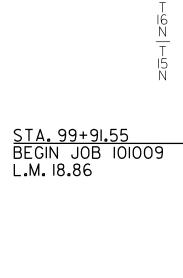
• DESIGN TRAFFIC DATA • DESIGN YEAR-----2043 2023 ADT------800 2043 ADT-----960 2043 DHV-----32 DIRECTIONAL DISTRIBUTION-----0.60

TRUCKS------4% DESIGN SPEED-----55 MPH

NOT TO SCALE

BRIDGE CONSTRUCTION DATA O STA. 109+97.50 BRIDGE END BRIDGE NO. 07497 OVER DITCH 30 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT (30'-40'-30')

30'-0" CLEAR ROADWAY 101'-0" BRIDGE LENGTH STA. 110+98.50 BRIDGE END



. 99+91.55 IN JOB 101009	61	3.	4.0 0 0/1/C/	18 I.O X	
	, 0, 0	1 10 7 1 1 1 1 1 1 1 1 1		2397 St. V SPURY + Hwy. 1 + 2	₽ <u></u>

NET LENGTH OF ROADWAY 1780.39 FEET 0.337 MILES NET LENGTH OF BRIDGES 101.00 FEET 0.019 MILES NET LENGTH OF PROJECT 1881.39 FEET 0.356 MILES

	BEGIN PROJECT	MID POINT OF PROJECT	END PROJECT
LATITUDE	N 35° 57′ 21″	N 35° 57′ 30″	N 35° 57′ 39″
LONGITUDE	W 89°54′37″	W 89°54′37″	W 89°54′36″

				11 11 12 12 12		
ARKANSAS INTERNATIONAL AIRPORT Ditch Color of the color	PROJECT CALCULATED A PROJECT 1881.39 FEE	LONG C.L. T 0.356 MILES	30 0° 18 18 239 18	Ditch	DES 202: 204: 204: DIRE TRUCTURE DES	IGN 3 AI 3 DI ECT CKS IGN
NET LENGTH OF B	20ADWAY 1780 39 FFF	T 0 337 MILES				

CERTIFICATION OF THE CONTRACT	MICHAEL BAKER INTERNATIONAL INC No. 1126	12 NOI 141
	INC	_ <
	No. 1126	,Q-
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				JOB	NO.	101009	2	52
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2 INDEX OF SHEETS AND STANDARD DRAWINGS

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INDEX OF SHEETS

SHEET NO.	TITLE	BRIDGE NO.	DRWG.NO.
1	TITLE SHEET		
2	INDEX OF SHEETS AND STANDARD DRAWINGS		
3	GOVERNING SPECIFICATIONS AND GENERAL NOTES		
4 - 5	TYPICAL SECTIONS OF IMPROVEMENT		
	SPECIAL DETAILS		
	TEMPORARY EROSION CONTROL DETAILS		
10 - 14	MAINTENANCE OF TRAFFIC DETAILS		
15	PERMANENT PAVEMENT MARKING DETAILS		
	QUANTITIES		
20	SCHEDULE OF BRIDGE QUANTITIES	07497	61804
	SUMMARY OF QUANTITIES AND REVISIONS		
22 - 23	SURVEY CONTROL DETAILS		
	PLAN AND PROFILE SHEETS	07497	04005
28	LAYOUT OF BRIDGE - HIGHWAY 610VER DITCH NO. 30 (SHEET 10 F 2)	07497 07497	
29 <u> </u>	LAYOUT OF BRIDGE - HIGHWAY 61 OVER DITCH NO. 30 (SHEET 2 OF 2)	07497	
30 <u> </u>	DETAILS OF INTERMEDIATE BENTS	07.107	
32 —	DETAILS FOR CONCRETE FILLED STEEL SHELL PILES AND PILE ENCASEMENTS		
32 —	DETAILS OF 100-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 1 OF 6)	07497	
	DETAILS OF 100-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 2 OF 6)	07497	
35	DETAILS OF 100-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 3 OF 6)		
36	DETAILS OF 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 4 OF 6)		
37	DETAILS OF 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 5 OF 6)	07497	61814
38	DETAILS OF 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT (SHEET 6 OF 6)	07497	61815
39	DETAILS OF TYPE SPECIAL APPROACH SLAB	07497	61816
40	DETAILS OF TYPE SPECIAL APPROACH GUTTERS	07497	61817
41 - 52	CROSS SECTIONS		

BRIDGE STANDARD DRAWINGS

DRWG.NO.	TITLE	DATE
55000STANDARD DETAILS FOR	R EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS	02-27-14
55001STANDARD DETAILS FOR	R DUMPED RIPRAP AND FILTER BLANKET AND COMPUTING EXCAVATION FOR STRUCTURE	S02-27-14
55005STANDARD DETAILS FOR	R PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS	03-24-16
55006STANDARD GENERAL NO	DTES FOR STEEL BRIDGE STRUCTURES	09-02-15
55007STANDARD DETAILS FOR	R STEEL BRIDGE STRUCTURES	02-11-16
55010 STANDARD DETAILS FOR	R TYPE D BRIDGE NAME PLATE	01-11-23

ROADWAY STANDARD DRAWINGS

DRWG.NO.	TITLE	DATE
DR-2DETAILS OF DRIVEWAYS & STREET TURNO	DUTS	05-19-22
GR-6GUARDRAIL DETAILS		05-19-22
GR-7GUARDRAIL DETAILS		11-07-19
GR-8GUARDRAIL DETAILS		11-07-19
GR-9GUARDRAIL DETAILS		11-07-19
GR-10GUARDRAIL DETAILS		11-07-19
GR-11GUARDRAIL DETAILS		11-07-19
GR-12GUARDRAIL DETAILS		05-14-20
MB-1MAILBOX DETAILS		11-18-04
PCC-1CONCRETE PIPE CULVERT FILL HEIGHTS &	BEDDING	02-27-14
PCM-1METAL PIPE CULVERT FILL HEIGHTS & BED	DDING	02-27-14
PCP-1PLASTIC PIPE CULVERT (HIGH DENSITY PO	OLYETHYLENE)	02-27-14
PCP-2PLASTIC PIPE CULVERT (PVC F949)	·	02-27-14
PCP-3PLASTIC PIPE CULVERT (POLYPROPYLENE	≣)	02-27-20
PM-1PAVEMENT MARKING DETAILS	<u> </u>	02-27-20
PU-1DETAILS OF PIPE UNDERDRAIN		12-08-16
TC-1STANDARD TRAFFIC CONTROLS FOR HIGH	HWAY CONSTRUCTION	11-07-19
TC-2STANDARD TRAFFIC CONTROLS FOR HIGH	HWAY CONSTRUCTION	05-20-21
TC-3STANDARD TRAFFIC CONTROLS FOR HIGH	IWAY CONSTRUCTION	08-12-21
TEC-1TEMPORARY EROSION CONTROL DEVICES	3	11-16-17
TEC-2TEMPORARY EROSION CONTROL DEVICES	3	06-02-94
TEC-3TEMPORARY EROSION CONTROL DEVICES	3	11-03-94

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101009	3	52
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GOVERNING SPECIFICATIONS

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS:

NUMBER	TITLE
ERRATA	LERRATA FOR THE BOOK OF STANDARD SPECIFICATIONS
	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTICE TO CONTRACTORS
	SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 140)
	_SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
	_SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
	_SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
	_SUPPLEMENT - WAGE RATE DETERMINATION
	_CONTRACTOR'S LICENSE
	_DEPARTMENT NAME CHANGE
	LISSUANCE OF PROPOSALS
	MAINTENANCE DURING CONSTRUCTION
	_RESTRAINING CONDITIONS
	LIQUIDATED DAMAGES
108-2	_WORK ALLOWED PRIOR TO ISSUANCE OF WORK ORDER
110-1	PROTECTION OF WATER QUALITY AND WETLANDS
210-1	_UNCLASSIFIED EXCAVATION
303-1	_AGGREGATE BASE COURSE
	_QUALITY CONTROL AND ACCEPTANCE
307-1	
308-1	
	_TACK COATS
400-5	_PERCENT AIR VOIDS FOR ACHM MIX DESIGNS
	LIQUID ANTI-STRIP ADDITIVE
	_TRACKLESS TACK
	_DESIGN OF ASPHALT MIXTURES
	_CONSTRUCTION REQUIREMENTS AND ACCEPTANCE OF ASPHALT CONCRETE PLANT MIX COURSES
	_DEVICES FOR MEASURING DENSITY FOR ROLLING PATTERNS
	_EVALUTATION OF ACHM SUBLOT REPLACEMEMENT MATERIAL
501-2	
	_PORTLAND CEMENT CONCRETE DRIVEWAY
600-2	LINCIDENTAL CONSTRUCTION
	_LANE CLOSURE NOTIFICATION
	RETROREFLECTIVE SHEETING FOR TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES
604-3	_TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES (MASH)
606-1	_PIPE CULVERTS FOR SIDE DRAINS
	_GUARDRAIL TERMINAL (TYPE 2)
620-1	_MULCH COVER
	_FILTER SOCKS
734-1	_BRIDGE END TERMINAL
	_STRUCTURES
802-3	_CONCRETE FOR STRUCTURES
802-4	
	_REINFORCING STEEL FOR STRUCTURES
807-2	_STEEL STRUCTURES
JOB 101009_	_AIRPORT CLEARANCE REQUIREMENTS
	_BIDDING REQUIREMENTS AND CONDITIONS
	_BROADBAND INTERNET SERVICE FOR ASPHALT CONCRETE PLANT
	_BROADBAND INTERNET SERVICE FOR FIELD OFFICE
	_BUY AMERICA - CONSTRUCTION MATERIALS
	_CARGO PREFERENCE ACT REQUIREMENTS
	_CLASS C FLY ASH IN PORTLAND CEMENT CONCRETE PAVEMENT AND CLASS S(AE) CONCRETE
	_COLD MILLING - COUNTY PROPERTY
	_CONCRETE BRIDGE DECK CURING AND SURFACE TREATMENT RESTRICTIONS
	_CONSTRUCTION IN SPECIAL FLOOD HAZARD AREAS
	_CULVERT CLEAN OUT
	_DESIGN AND QUALITY CONTROL OF ASPHALT MIXTURES
	_DIRECT TENSION INDICATORS FOR HIGH STRENGTH BOLT ASSEMBLIES
	_DISADVANTAGED BUSINESS ENTERPRISE BIDDER'S RESPONSIBILITIES
	_ESTABLISHING CONTRACT TIME - WORKING DAY CONTRACT
	_FLEXIBLE BEGINNING OF WORK
	LIQUIDATIED DAMAGES PROCEDURE FOR BID LETTINGS
	LONGITUDINAL JOINT DENSITIES FOR ACHM SURFACE COURSE
	_GOALS FOR DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION
	_MANDATORY ELECTRONIC CONTRACT
	_MANDATORY ELECTRONIC DOCUMENT SUBMITTAL
JOB 101009_	_NESTING SITES OF MIGRATORY BIRDS

JOB 101009_PROHIBITION OF CERTAIN TELECOMMUNICATIONS AND VIDEO SURVEILLANCE SERVICES OR EQUIPMENT JOB 101009_SECTION 404 NATIONWIDE 14 PERMIT REQUIREMENTS

JOB 101009_STORM WATER POLLUTION PREVENTION PLAN
JOB 101009_SUBMISSION OF ASPHALT CONCRETE HOT MIX ACCEPTANCE TEST RESULTS

JOB 101009_PLASTIC PIPE

JOB 101009_SHORING FOR CULVERTS
JOB 101009_SOIL STABILIZATION

JOB 101009_UTILITY ADJUSTMENTS JOB 101009_WARM MIX ASPHALT JOB 101009_WELLHEAD PROTECTION

JOB 101009_PRICE ADJUSTMENT FOR ASPHALT BINDER JOB 101009_PRICE ADJUSTMENT FOR FUEL

GENERAL NOTES

- 1. GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- 2. ALL PIPE LINES, POWER, TELEPHONE, AND TELEGRAPH LINES TO BE MOVED OR LOWERED BY THE RESPECTIVE OWNERS AS PER AGREEMENT WITH SUCH OWNERS.
- 3. ANY EQUIPMENT OR APPURTENANCE THAT INTERFERES WITH THE PROPOSED CONSTRUCTION AND WHICH MAY BE THE PROPERTY OF UTILITY SERVICE ORGANIZATIONS SHALL BE MOVED BY THE OWNERS UNLESS OTHERWISE PROVIDED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING U. S. MAILBOXES WITHIN THE PROJECT LIMITS IN SUCH A MANNER THAT THE PUBLIC MAY RECEIVE CONTINUED MAIL SERVICE. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS BID ITEMS.
- 5. ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- 6. ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED IF AND WHERE DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO ENSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A FENCE TO CONTROL LIVESTOCK IN AREAS WHERE PASTURES ARE SEVERED. WIRE FENCE MAY BE CONSTRUCTED INITIALLY, OR IN LIEU THEREOF, THE CONTRACTOR AT HIS OWN EXPENSE, MAY ELECT TO PROVIDE TEMPORARY FENCING SUITABLE TO CONTAIN LIVESTOCK.
- 8. THE SEQUENCE AS SHOWN ON THE MAINTENANCE OF TRAFFIC PLANS IS A GENERAL OUTLINE FOR THE CONSTRUCTION OF THIS PROJECT, AND IN NO WAY IS IT INTENDED TO COVER EVERY ITEM IN THE PROJECT. ITEMS NOT CRITICAL TO THE CONSTRUCTION SEQUENCE MAY BE CONSTRUCTED IN ANY STAGE AS APPROVED BY THE RESIDENT ENGINEER.
- 9. ALL FLEXIBLE BASE AND ASPHALTIC PAVEMENTS REMOVED SHALL BE PAID FOR UNDER THE ITEM NO. 210 UNCLASSIFIED EXCAVATION.
- 10. THE EXISTING ASPHALT PAVEMENT TO BE REMOVED FROM THE REMAINING PAVEMENT SHALL BE SEPARATED BY SAWING ALONG A NEAT LINE. AFTER SAWING, THE PAVEMENT TO BE REMOVED SHALL BE CAREFULLY REMOVED IN A MANNER THAT WILL NOT DAMAGE THE PAVEMENT THAT IS TO REMAIN. ANY DAMAGE OF THE ASPHALT PAVEMENT THAT IS TO REMAIN IN PLACE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.

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TYPICAL SECTIONS OF IMPROVEMENT

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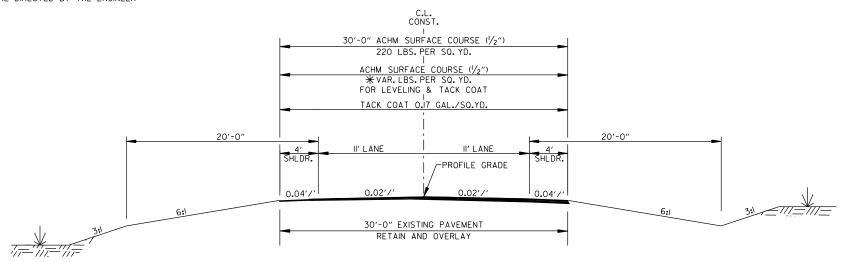
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ENGINEER

No.13640

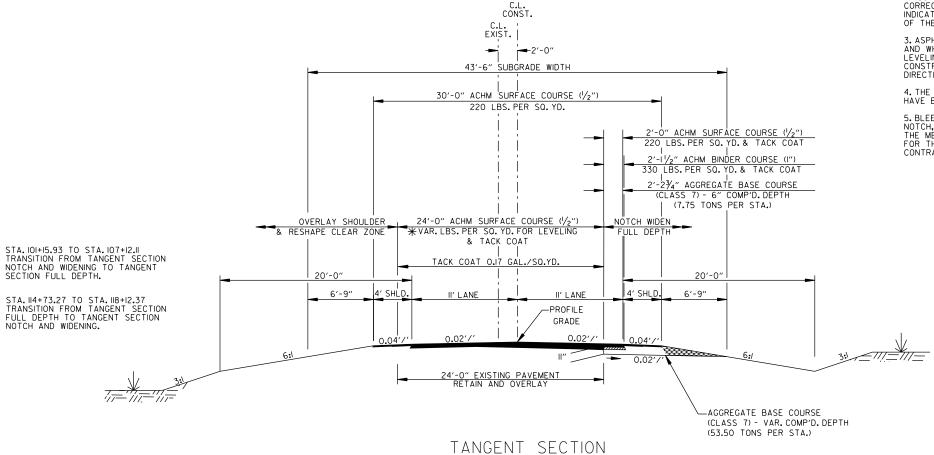
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* TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER



TANGENT SECTION OVERLAY

STA. 99+9I.55 TO STA. IOI+I5.93 STA. II8+I2.37 TO STA. II8+72.94



NOTCH AND WIDENING

STA. IOI+I5.93 TO STA. IO7+I2.II STA. II4+73.27 TO STA. II8+I2.37 NOTES:

I. REFER TO CROSS SECTIONS FOR DEVIATIONS FROM NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.

2. THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN, THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET THE TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.

3. ASPHALT FOR LEVELING OF EXISTING PAVEMENT SHALL BE PLACED ONLY IF AND WHERE DIRECTED BY THE ENGINEER. CALCULATIONS FOR THE AMOUNT OF LEVELING AND LEVELING OPERATIONS SHALL BE PERFORMED BEFORE CONSTRUCTING NOTCH AND WIDENING. CALCULATIONS WILL NOT BE PAID FOR DIRECTLY, BUT PAYMENT WILL BE CONSIDERED INCLUDED IN THE VARIOUS PAY ITEMS.

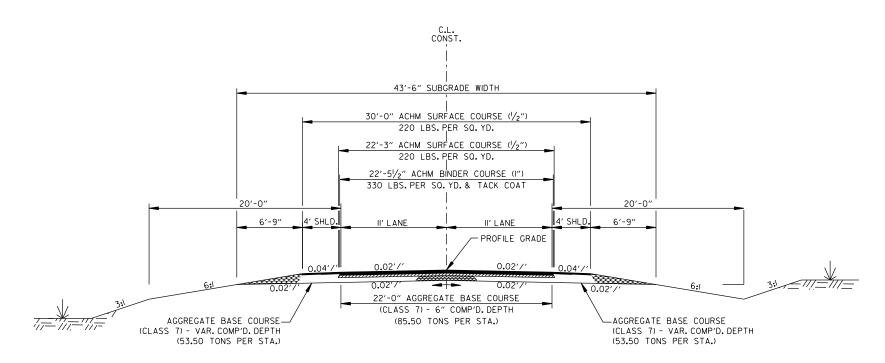
4. THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT THE LANE LINES.

5. BLEEDER DITCHES - PRIOR TO AND DURING PLACEMENT OF PAVEMENT AT THE NOTCH, THE CONTRACTOR SHALL PROVIDE POSITIVE DRAINAGE AT ALL TIMES. THE METHODIS) AND SPACING USED SHALL BE APPROVED BY THE ENGINEER, PAYMENT FOR THIS WORK SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

	DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
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(2) TYPICAL SECTIONS OF IMPROVEMEN

ARKANSAS LIČENSED PROFESSIONAL ENGINEER No.13640



TANGENT SECTION STA. 107+12.11 TO STA. 109+64.50 STA. 111+31.50 TO STA. 114+73.27

I. REFER TO CROSS SECTIONS FOR DEVIATIONS FROM NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.

2. THE THICKNESS OF AGGREGATE BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF THE PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET THE TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.

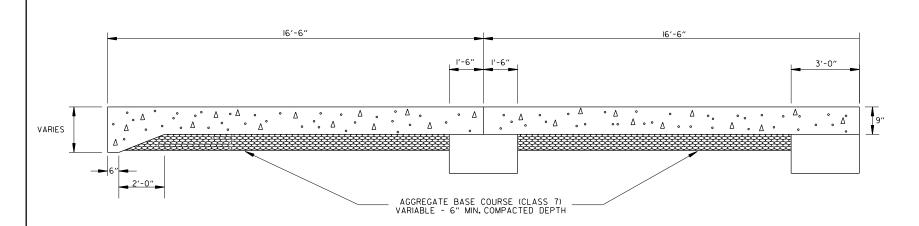
3. THE FINAL 2" OF SURFACE COURSE IS TO BE PLACED AFTER ALL OTHER COURSES HAVE BEEN LAID. LONGITUDINAL JOINTS SHALL BE AT THE LANE LINES.

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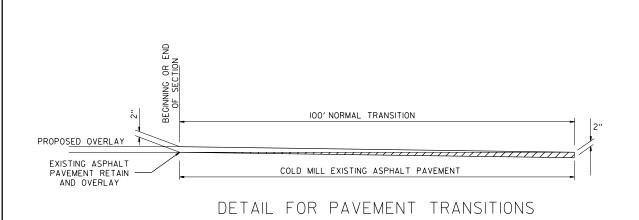
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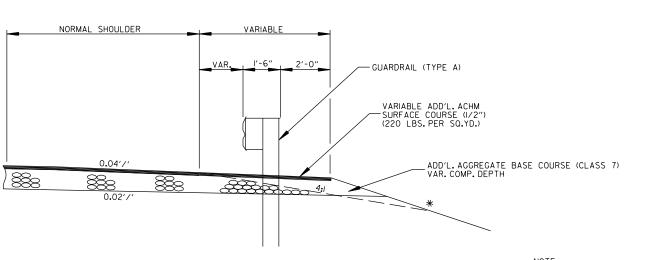
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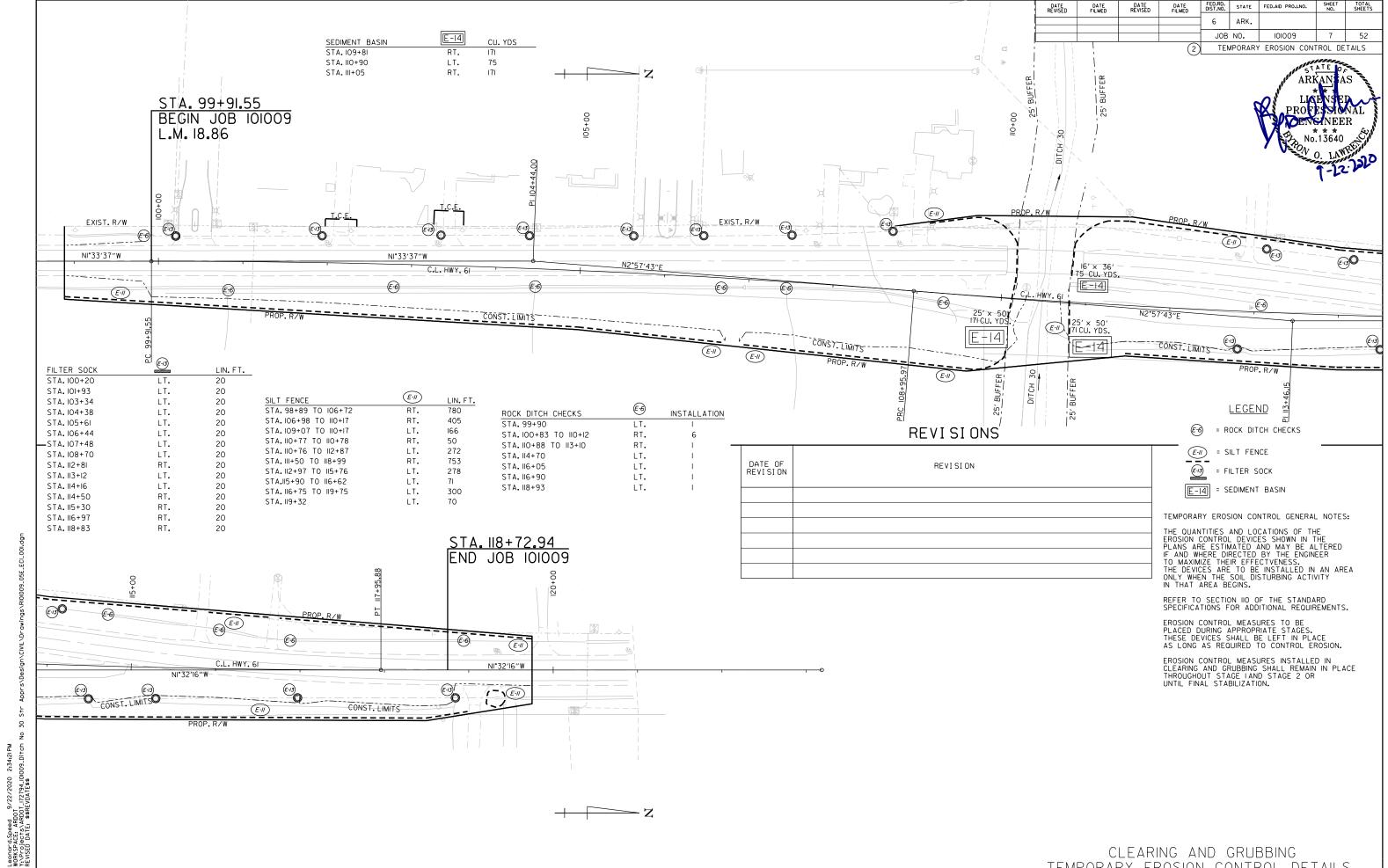


SECTION OF APPROACH SLAB

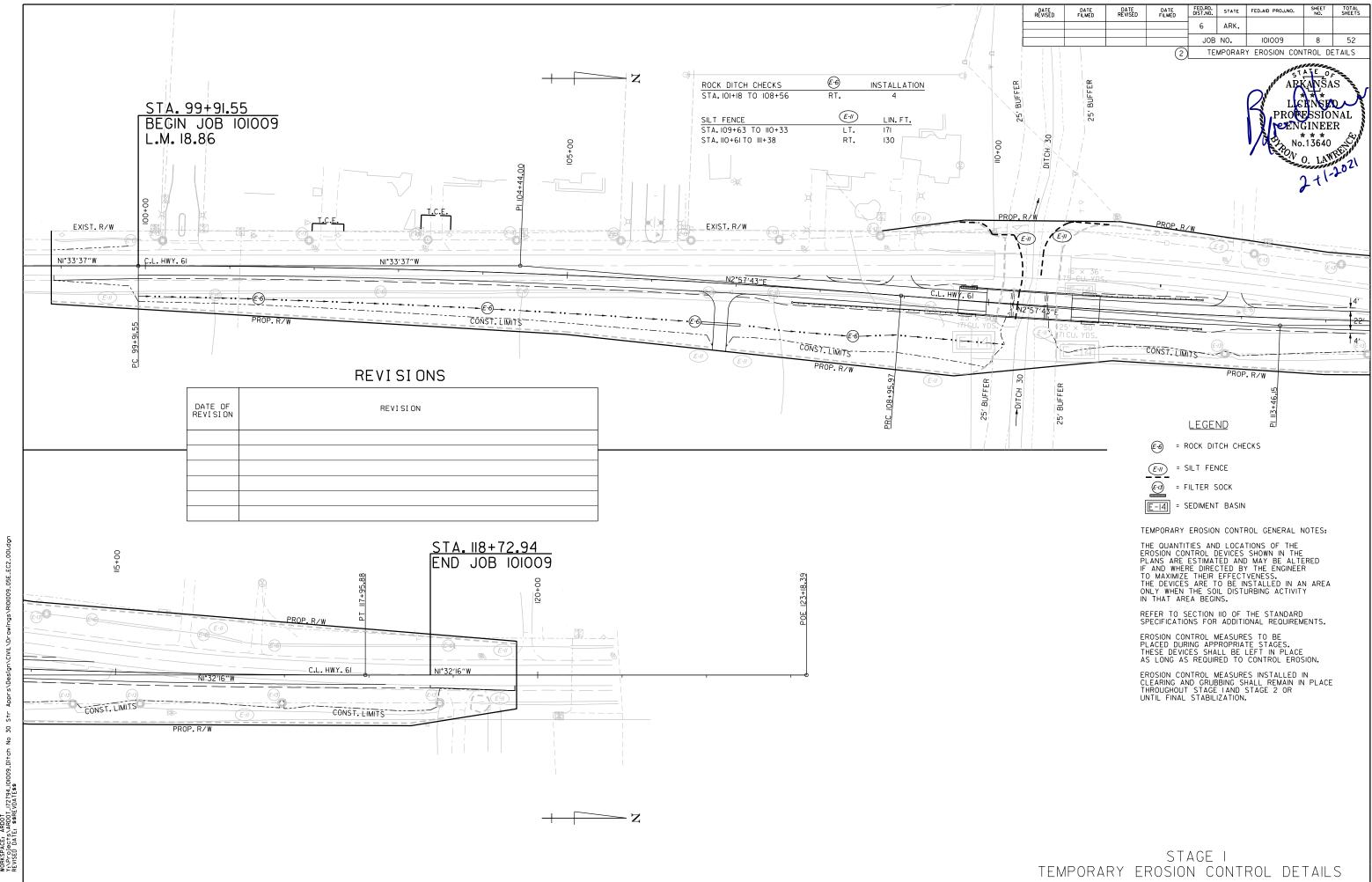


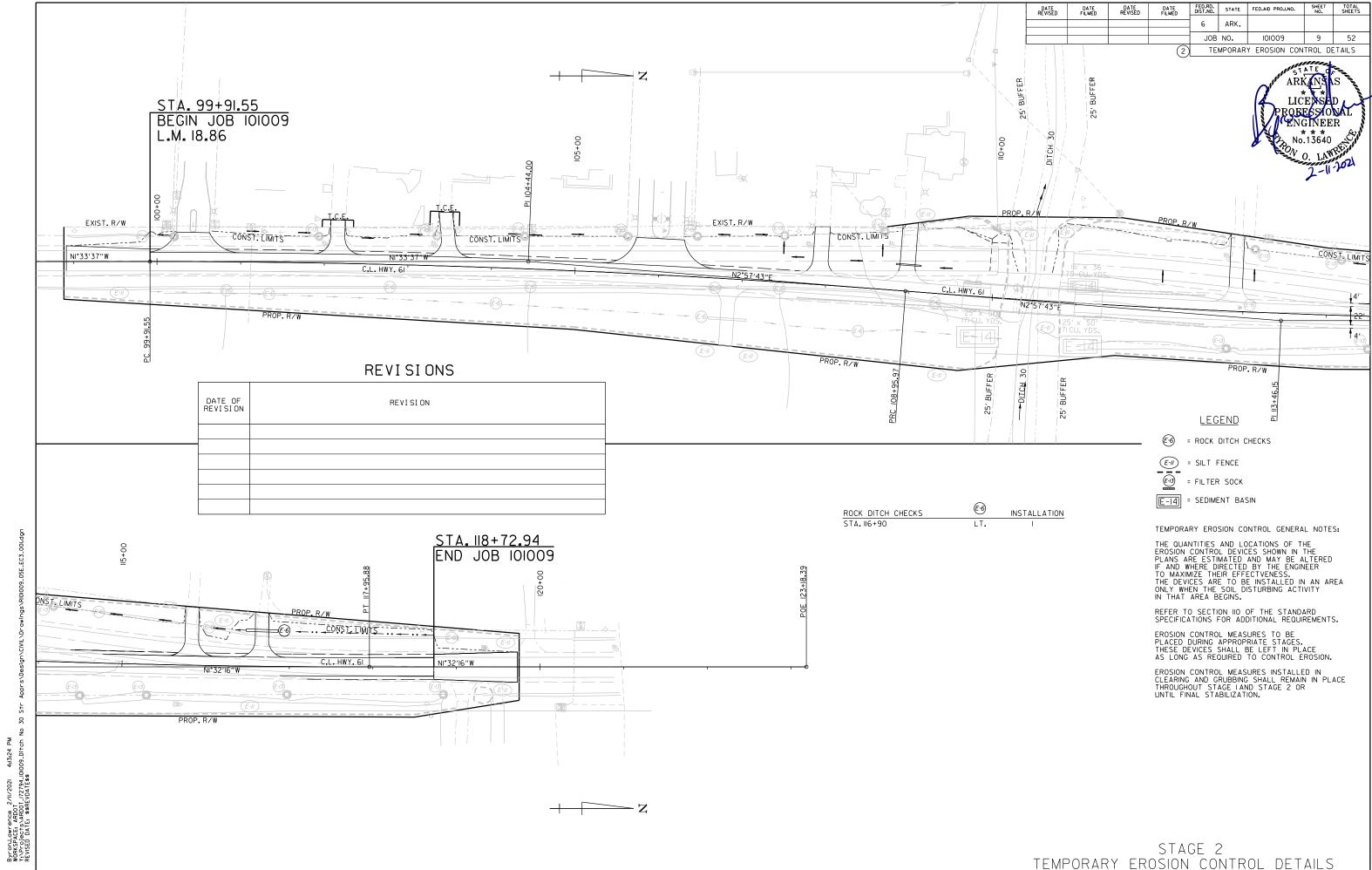


* NOTE: REFER TO STD.DWG.GR-9 AND CROSS SECTIONS FOR SLOPE REQUIREMENTS BEHIND GUARDRAIL.



TEMPORARY EROSION CONTROL DETAILS





ALL STAGES

CONSTRUCTION SEQUENCE

INSTALL ADVANCE WARNING SIGNS AND END ROAD WORK SIGNS AT THE LOCATIONS SHOWN ON THE ADVANCE WARNING DETAILS.

CLEARING AND GRUBBING OPERATIONS MAY BEGIN IF AND WHERE DIRECTED BY THE ENGINEER.

INSTALL MAINTENANCE OF TRAFFIC DEVICES AND CONSTRUCT EMBANKMENT, ROADWAY, BRIDGE, AND DRAINAGE FOR PROJECT AS SHOWN IN STAGE IMAINTENANCE OF TRAFFIC DETAILS.

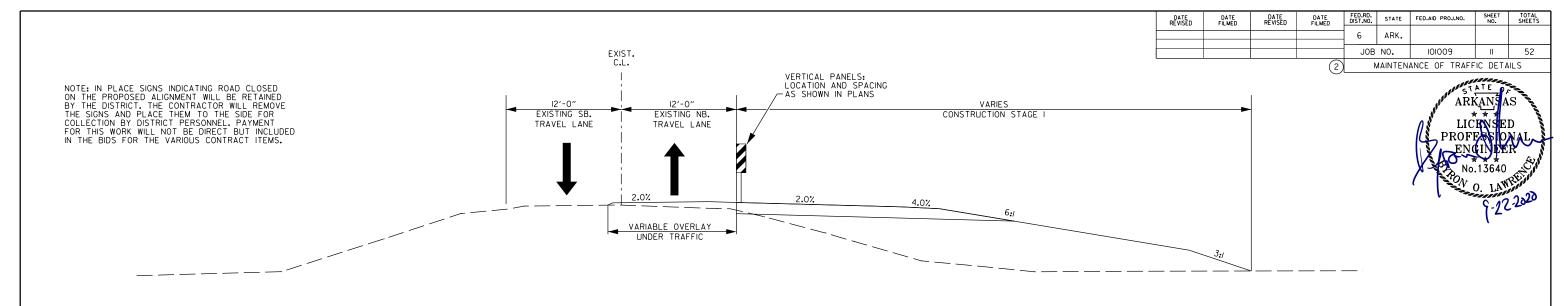
INSTALL STAGE 2 MAINTENANCE OF TRAFFIC DEVICES AND CONSTRUCTION PAVEMENT MARKINGS. SHIFT TRAFFIC ONTO NEWLY CONSTRUCTED ROADWAY AND BRIDGE.

REMOVE EXISTING BRIDGE AND ROADWAY AS SHOWN IN CROSS SECTIONS.

CONSTRUCT REMAINING MILL AND INLAY AREAS, ROADWAY TIES, DRAINAGE, FINAL ACHM SURF. COURSE, FINAL GRADING AND EMBANKMENT AS SHOWN ON THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

NOTE: IN PLACE SIGNS INDICATING ROAD CLOSED ON THE PROPOSED ALIGNMENT WILL BE RETAINED BY THE DISTRICT. THE CONTRACTOR WILL REMOVE THE SIGNS AND PLACE THEM TO THE SIDE FOR COLLECTION BY DISTRICT PERSONNEL. PAYMENT FOR THIS WORK WILL NOT BE DIRECT BUT INCLUDED IN THE BIDS FOR THE VARIOUS CONTRACT ITEMS.

FED.RD. STATE FED.AID PROJ.NO.



CONSTRUCTION SEQUENCE STAGE I:

INSTALL ADVANCE WARNING SIGNS AND END ROAD WORK SIGNS AT THE LOCATIONS SHOWN ON THE ADVANCE WARNING DETAILS.

CLEARING AND GRUBBING OPERATIONS MAY BEGIN IF AND WHERE DIRECTED BY THE ENGINEER.

INSTALL MAINTENANCE OF TRAFFIC DEVICES AND CONSTRUCT EMBANKMENT, ROADWAY, BRIDGE, AND DRAINAGE FOR PROJECT AS SHOWN IN STAGE I MAINTENANCE OF TRAFFIC DETAILS.

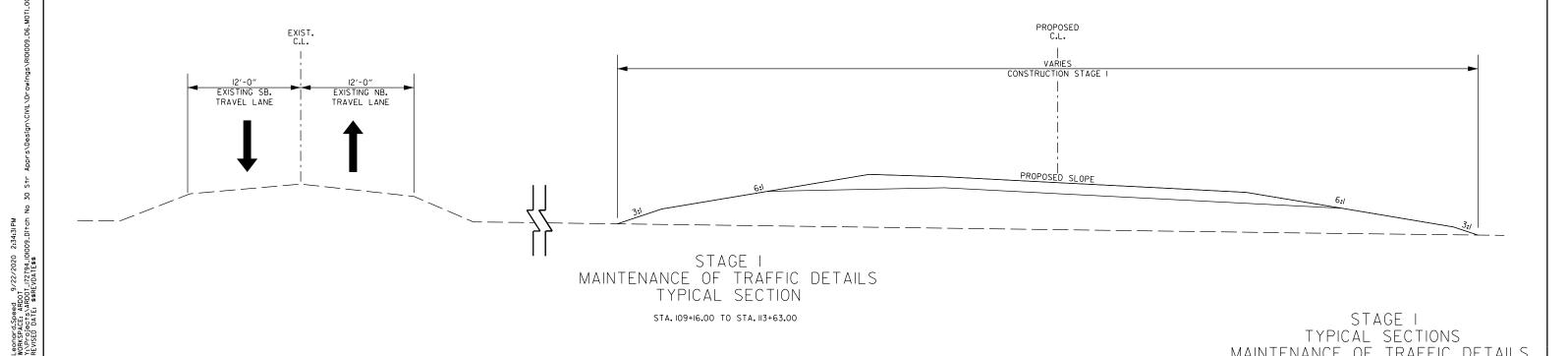
INSTALL STAGE 2 MAINTENANCE OF TRAFFIC DEVICES AND CONSTRUCTION PAVEMENT MARKINGS. SHIFT TRAFFIC ONTO NEWLY CONSTRUCTED ROADWAY AND BRIDGE.

REMOVE EXISTING BRIDGE AND ROADWAY AS SHOWN IN CROSS SECTIONS.

CONSTRUCT REMAINING MILL AND INLAY AREAS, ROADWAY TIES, DRAINAGE, FINAL ACHM SURF. COURSE, FINAL GRADING AND EMBANKMENT AS SHOWN ON THE STAGE 2 MAINTENANCE OF TRAFFIC DETAILS.

STAGE I - NOTCH AND WIDEN MAINTENANCE OF TRAFFIC DETAILS TYPICAL SECTION

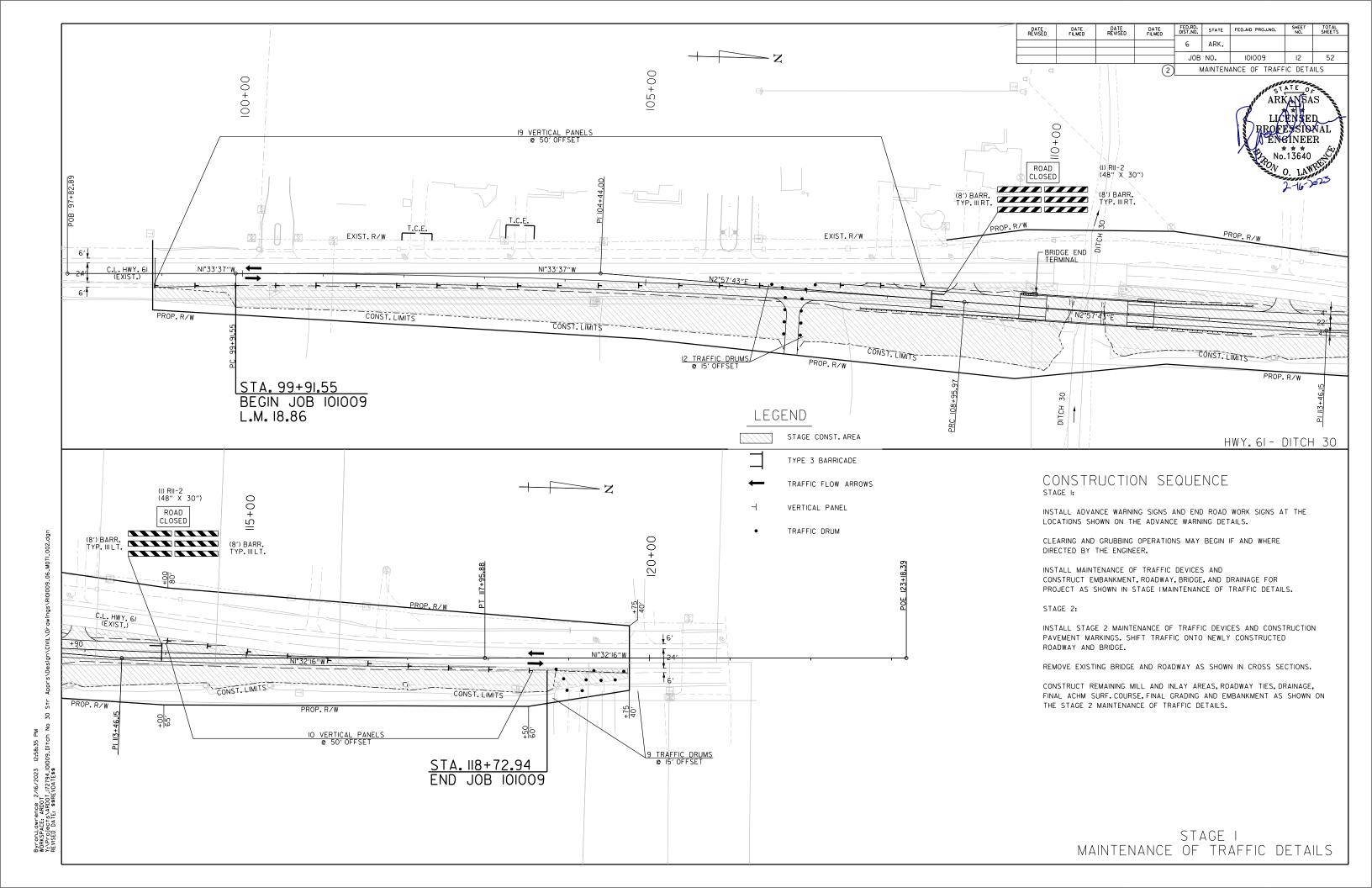
STA. 99+91.03 TO 109+16.00 STA. 113+63.00 TO 118+72.94

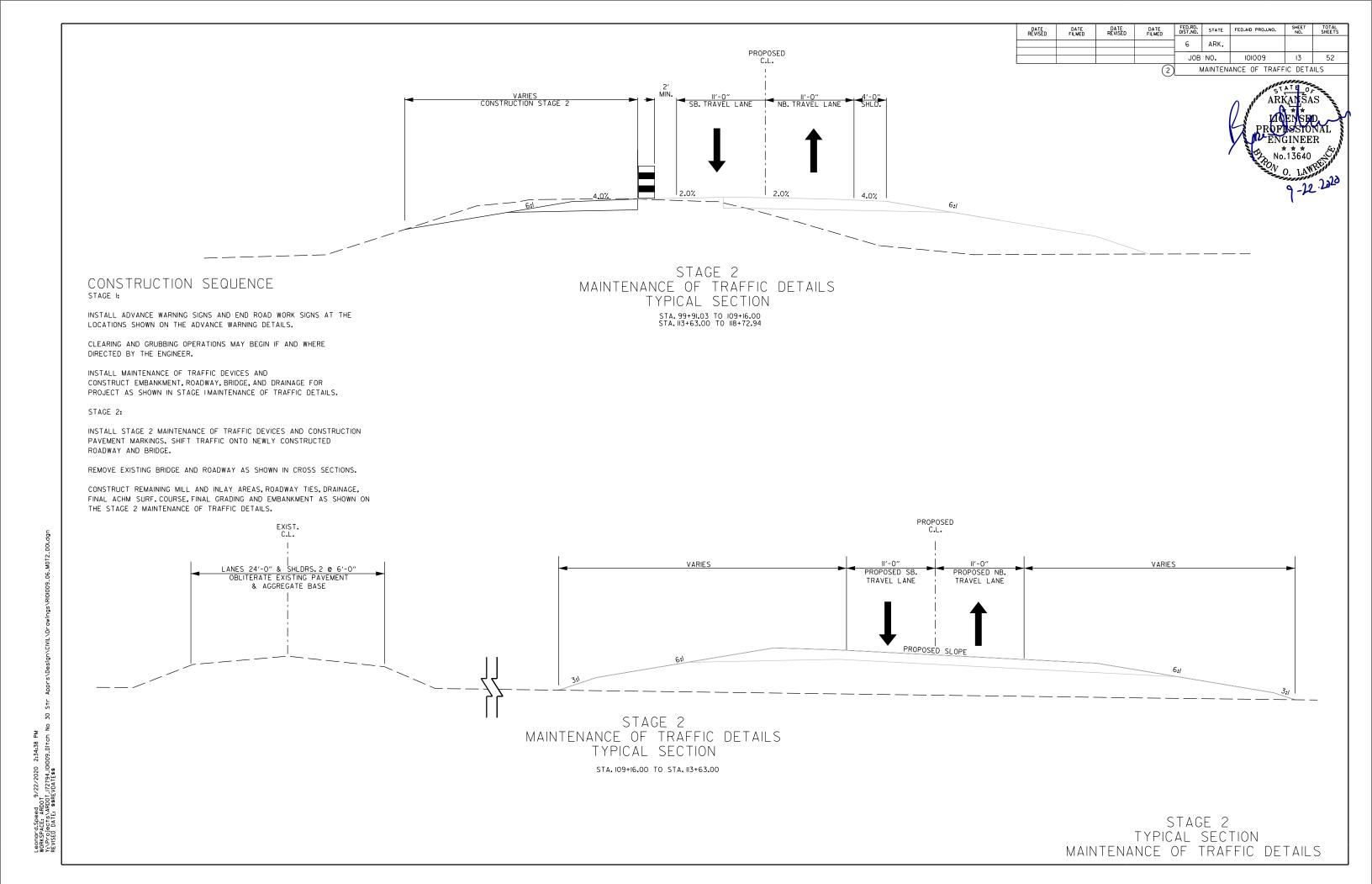


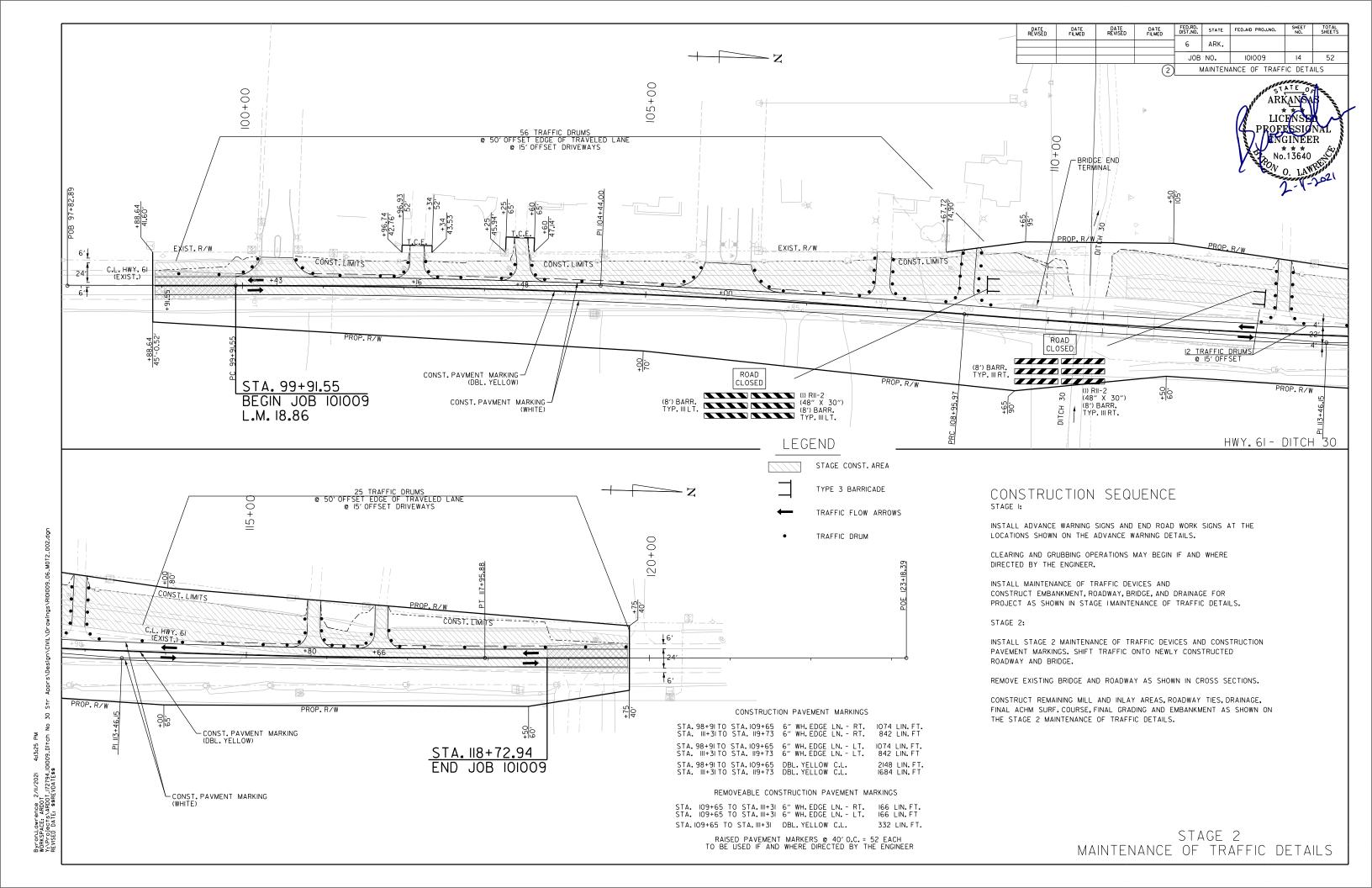
STAGE I TYPICAL SECTIONS MAINTENANCE OF TRAFFIC DETAILS

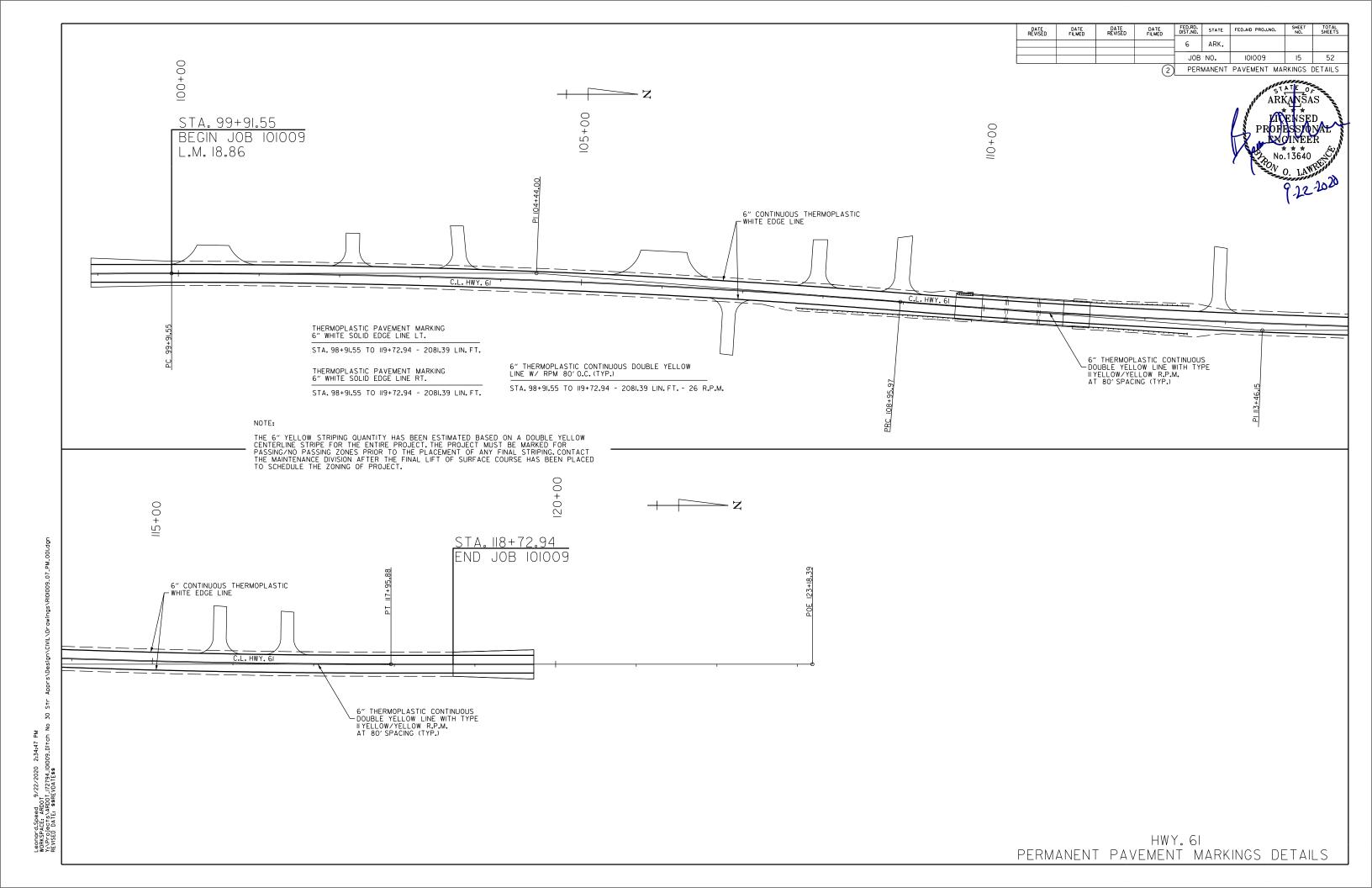
TYPICAL SECTION

STA. 109+16.00 TO STA. 113+63.00









ADVANCE WARNING SIGNS AND DEVICES

SIGN NUMBER	DESCRIPTION	SIGN SIZE	STAGE 1	STAGE 2	END OF JOB	MAXIMUM NUMBER REQUIRED		. SIGNS JIRED	VERTICAL PANELS	TRAFFIC DRUMS	BARRICADI	ES (TYPE III
						REQUIRED					RIGHT	LEFT
			L	IN FT. EAC	Н		NO.	SQ FT.	EA	CH	LIN.	FT.
W20-1	ROAD WORK 1500 FT.	36"x36"	2	2		2	2	18.0				
W20-1	ROAD WORK 1000 FT.	36"x36"	2	2		2	2	18.0				
W20-1	ROAD WORK 500 FT.	36"x36"	2	2		2	2	18.0				
G20-2	END ROAD WORK	36"x18"	2	2		2	2	9.0				
R11-2	ROAD CLOSED	48"x30"	2	2		2	2	20.0				
R4-1	DO NOT PASS	24"x30"	2	2		2	2	10.0				
W21-5a	RIGHT SHOULDER CLOSED	36"x36"	2	2		2	2	18.0				
	VERTICAL PANELS		29			29			29			
	TRAFFIC DRUMS		21	93		93				93		
	TYPE III BARRICADE-RT. (8')		2	2		2					16	
	TYPE III BARRICADE-LT. (8')		2	2		2						16
OTALS:	1							111.0	29	93	16	16

* QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

CONSTRUCTION PAVEMENT MARKINGS AND PERMANENT PAVEMENT MARKINGS

CONSTRUCTION FAVEMENT MARKINGS AND FERMANENT FAVEMENT MARKINGS											
STAGE 1	STAGE 2	STAGE 3	END OF JOB	CONSTRUCTION PAVEMENT	REMOVABLE CONSTRUCTION PAVEMENT	RAISED PAVEMENT MARKERS	THERMOPLASTIC PAVEMENT MARKING				
				MARKINGS MARKINGS		TYPE II	6	,"			
						(YELLOW/YELLOW)	WHITE	YELLOW			
	LIN. FT.	- EACH		LIN. FT.	LIN. FT.	EACH	LIN. FT.				
	7664			7664		52					
	664				664						
			26			26					
			4163				4163				
			4163					4163			
TOTALS: 7664 664 78 4163 4163											
		STAGE 1 STAGE 2 LIN. FT. 7664	STAGE 1 STAGE 2 STAGE 3 LIN. FT EACH 7664	STAGE 1 STAGE 2 STAGE 3 END OF JOB LIN. FT EACH 7664 664 26 4163	STAGE 1 STAGE 2 STAGE 3 END OF JOB CONSTRUCTION PAVEMENT MARKINGS	STAGE 2 STAGE 3 END OF	STAGE 2 STAGE 3 END OF JOB CONSTRUCTION PAVEMENT MARKINGS CONSTRUCTION PAVEMENT MARKINGS TYPE II (YELLOW) EACH	STAGE 2 STAGE 3 END OF JOB CONSTRUCTION PAVEMENT MARKINGS RAISED PAVEMENT MARKINGS TYPE II (YELLOW/YELLOW) WHITE			

NOTES: THIS IS A LOW TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT.
THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING.
CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

CULVERT CLEAN OUT

COLVENT	COLVERT CLEAR COT								
LOCATION	CULVERT CLEAN OUT	DROP INLET CLEAN OUT							
	Е	EACH							
ENTIRE PROJECT TO BE USED IF AND WHERE									
DIRECTED BY THE ENGINEER	20	17							
TOTAL S.	20	17							

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.3 OF THE STANDARD SPECIFICATIONS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

GUARDRAIL

STATION	STATION	LOCATION	GUARDRAIL (TYPE A)	THRIE BEAM GUARDRAIL TERMINAL		BRIDGE END TERMINAL		
			LIN. FT.		EACH 1			
107+67.35	109+86.10	RT. OF HWY. 61	150	1	1			
109+87.50	109+87.50	LT. OF HWY. 61				1		
111+09.90	112+53.65	LT. OF HWY. 61	75	1	1			
111+09.90	112+53.65	RT. OF HWY. 61	75	1	1			
TOTALS:			300	3	3	1		

STATE DATE REVISED DATE FILMED 6 ARK. JOB NO. 52 101009 16 QUANTITIES

DENCHMARKS

BENCH WARKS								
STATION	LOCATION	BENCH MARKS						
		EACH						
109+98	C.L. HWY. 61- BRIDGE OVER DITCH 30	1						
		, and the second						

NOTE: SHOWN FOR INFORMATION ONLY. BENCH MARKS SHALL BE FURNISHED AND PLACED BY STATE FORCES.

LICENSED PROFESSIONAL ENGINEER No.13640

MAILBOXES

	MAILBOXES	MAILBOX SUPPORTS								
LOCATION	WAILBUXES	(SINGLE)								
		EACH								
ENTIRE PROJECT	9	9								
TOTALS:	9	9								

SELECTED PIPE BEDDING

GEEEGIEDI II E BEBBING						
LOCATION	SELECTED PIPE BEDDING					
	CU.YD.					
ENTIRE PROJECT TO BE USED IF						
AND WHERE DIRECTED BY THE	20					
ENGINEER						
TOTAL:	20					
NOTE: OUANITITY ESTIMATED						

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

				DRIVEW	AYS & TUR	NOUTS					
STATION	SIDE	LOCATION	WIDTH CONCRETE (1/2") 220 LBS. PER SQ. YD. BASE (AGGREGATE BASE COURSE (CLASS 7)	BASE COURSE (CLASS 7) 18" 30" 36"			STANDARD DRAWINGS		
			FEET	SQ, YD.	SQ. YD.	TON	TON	L	IN. F		
100+43	LT.	C.L. OF HWY. 61	40		211.19	23.23	86.24				
102+16	LT.	C.L. OF HWY, 61	17	39.67	38.79	4.27	15.84				
103+48	LT.	C.L. OF HWY, 61	16	59.27	37.01	4.07	15.11				
106+00	LT.	C.L. OF HWY, 61	56		275.19	30.27	112.37				
106+85	RT.	C.L. OF HWY. 61	16		124.09	13.65	50.67		46		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
107+93	LT.	C.L. OF HWY, 61	18	78.00	40.56	4.46	16.56				
109+00	LT.	C.L. OF HWY, 61	18	100.00	40.56	4.46	16.56	30			PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
112+90	LT.	C.L. OF HWY. 61	16		143.68	15.80	58.67				
115+80	LT.	C.L. OF HWY. 61	16		106.34	11.70	43.42				
116+66	LT.	C.L. OF HWY, 61	16		97.45	10.72	39.79			38	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
ENTIRE PRO	JECT TEMP	ORARY DRIVES					150.00				
TOTALS:				276.94	1114.86	122,63	605.23	30	46	38	
BASIS OF ES	TIMATE							1 00			

ACHM SURFACE COURSE (1/2").......95.0% MIN. AGGR..........5.0% ASPHALT BINDER MAXIMUM NUMBER OF GYRATIONS = 115 FOR PG 64-22

* QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED.

NOTE: FOR C.M. PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.

REMOVAL AND DISPOSAL OF ITEMS

1121110 1712 71110 5101 00712 01 1121110											
STATION	STATION	LOCATION	GUARDRAIL	IMPACT ATTENUATION BARRIER							
			LIN. FT.	EACH							
109+25	110+29	LT. OF EXIST. BRIDGE (LT. OF C.L.)	104								
110+95	111+97	RT. OF EXIST. BRIDGE (LT. OF C.L.)	102								
		BEGIN & END OF EXIST. BRIDGE		2							
TOTALS:			206	2							

NOTE: THE QUANTITY SHOWN ABOVE FOR THE REMOVAL AND DISPOSAL OF GUARDRAIL SHALL INCLUDE THE REMOVAL AND DISPOSAL OF GUARDRAIL SHALL INCLUDE THE REMOVAL AND DISPOSAL OF ALL GUARDRAIL TERMINALS AND TERMINAL ANCHOR POSTS.

4" PIPE UNDERDRAIN

STATION	STATION	LOCATIONS	4" PIPE UNDERDRAINS	UNDERDRAIN OUTLET PROTECTORS		
			LIN. FT.	EACH		
*ENTIRE PF	ROJECT TO	BE USED IF AND	4000	16		
WHERE DI	RECTED B'	Y THE ENGINEER				
TOTALS:			4000	16		

*NOTE: QUANTITY ESTIMATED.
SEE SECTION 104.03 OF THE STD. SPECS.

JOB NO. 101009 17 52 QUANTITIES

STATE

ARK.

6

ACHM PATCHING OF EXISTING ROADWAY

DATE REVISED

DATE FILMED

DESCRIPTION	TON
ENTIRE PROJECT - TO BE USED IF AND WHERE	10
DIRECTED BY THE ENGINEER	
TOTAL:	10
NOTE: OLIANTITY ECTIMATED	

NOTE: QUANTITY ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

CLEARING AND GRUBBING

STATION	STATION	LOCATION	CLEARING	GRUBBING
			STA	TION
99+91	118+73	RT. OF HWY. 61	19	19
TOTALS:			19	19

REMOVAL AND DISPOSAL OF CULVERTS

	TEMOTIAL MID BIOL COME	O. OOL	
STATION	DESCRIPTION	PIPE CULVERTS	BOX CULVERTS
		EACH	EACH
106+85	RT. OF HWY. 61		1
107+46	RT. OF HWY. 61	1	
109+54	RT. OF HWY. 61	1	
116+66	LT. OF HWY. 61	1	
TOTALS:		3	1
NOTE: OLIA	NITITIES SUOWN ABOVE SUALL INCLUIS	E DEMOVAL &	DISBOSVI

OF ALL HEADWALLS AND FLARED END SECTIONS IF APPLICABLE.

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC

LOCATION	TON	TACK COAT
		GALLON
ENTIRE PROJECT - TO BE USED IF AND WHERE	9	18
DIRECTED BY THE ENGINEER		
TOTALS:	9	18

BASIS OF ESTIMATE:
ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC...25 TON/MILE TACK COAT FOR MAINTENANCE OF TRAFFIC.....

EROSION CONTROL

				PI	ERMANENT E	ROSION CO	NTROL						TEMPORARY I	EROSION CON	TROL		
STATION	STATION	LOCATION	SEEDING	LIME	MULCH COVER	WATER	SECOND SEEDING APPLICATION	SOLID SODDING	TEMPORARY SEEDING	MULCH COVER	WATER	ROCK DITCH CHECKS	SILT FENCE	FILTER SOCK (18")	SEDIMENT BASIN	OBLITERATION OF SEDIMENT BASIN	*SEDIMENT REMOVAL & DISPOSAL
							ALLEGATION					(E-6)	(E-11)	(E-13)	(E-14)	BAGIN	DIO! OUAL
			ACRE	TON	ACRE	M.GAL.	ACRE	SQ.YD.	ACRE	ACRE	M.GAL.	CU.YD.	LIN FT	LIN FT.	CU.YD.	CU.YD.	CU. YD.
ENTIRE	PROJECT	CLEARING AND GRUBBING							6.37	6.37	129.9	36	3145	300	417	417	545
ENTIRE	PROJECT	STAGE 1	2.35	4.70	2.35	239.7	2.35		4.70	4.70	95.9	12	301				15
ENTIRE	PROJECT	STAGE 2	4.30	8.60	4.30	438.6	4.30		8.60	8.60	175.4	3					1
98+92	109+98	END OF PROJECT - LT. OF HWY. 61				37.2		2950									
*ENTIRE PR	OJECT TO BE	USED IF AND WHERE DIRECTED BY THE ENGINEER.	0.67	1.34	0.67	68.3	0.67		1.34	1.34	27.3	6	345				15
									•								
TOTALS:			7.32	14.64	7.32	783.8	7.32	2950	21.01	21.01	428.5	57	3791	300	417	417	576

BASIS OF ESTIMATE:

..2 TONS / ACRE OF SEEDING LIME WATER .102.0 M.G. / ACRE OF SEEDING

WATER. .20.4 M.G. / ACRE OF TEMPORARY SEEDING

....12.6 GAL. / SQ. YD. OF SOLID SODDING3 CU.YD./LOCATION WATER ROCK DITCH CHECKS....

NOTE: THE TEMPORARY EROSION CONTROL DEVICES SHOWN ABOVE AND ON THE PLANS SHALL BE INSTALLED IN SUCH A SEQUENCE AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM PERMIT.

*QUANTITIES ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

SOIL STABILIZATION

		SOIL STADILIZATION	
STATION	STATION	LOCATION / DESCRIPTION	SOIL STABILIZATION TON
ENTIRE	PROJECT	TO BE USED IF AND WHERE	200
		DIRECTED BY THE ENGINEER	
TOTAL			200

QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

APPROACH GUTTERS AND SLABS

STATION	STATION	LOCATION	APPROACH GUTTER (TYPE SPECIAL) CU.YD.	APPROACH SLABS (TYPE SPECIAL) CU.YD.	REINFORCING STEEL-RDWY. (GR. 60)	AGGREGATE BASE CRS. (CLASS 7) TON
109+64.50	100 : 07 50	C.L. HWY. 61	CO.ID.	29.50	2115	22.24
				29.50		
109+64.50	109+97.50	C.L. HWY. 61	4.20		358	4.90
109+64.50	109+97.50	C.L. HWY. 61	4.20		358	4.90
110+98.50	111+31.50	C.L. HWY. 61		29.50	2115	22.24
110+98.50	111+31.50	C.L. HWY. 61	4.20		358	4.90
110+98.50	111+31.50	C.L. HWY. 61	4.20		358	4.90
TOTALC			40.00	F0.00	FCCO	64.00
TOTALS:			16.80	59.00	5662	64.08
NOTE: USE 1	Γ = 9" FOR 4'	SHOULDER.				

COLD MILLING ASPHALT PAVEMENT

	U	OLD MILLING ASPHALT PA	AVENENI	
STATION	STATION	LOCATION	AVG. WIDTH	COLD MILLING ASPHALT PAVEMENT
			FEET	SQ. YD.
98+91.55	99+91.55	C.L.OF HWY. 61	33.00	366.67
118+72.94	119+72.94	C.L.OF HWY. 61	33.00	366.67
TOTAL:	•	•		733.34

NOTE: COORDINATE COLD MILLING STOCKPILE LOCATIONS WITH DISTRICT ENGINEER. STOCKPILE LOCATIONS SHALL BE NO FURTHER THAN FIVE MILES FROM EACH SITE.

FARTHWORK

		EARTHWORK		
STATION	STATION	LOCATION / DESCRIPTION	UNCLASSIFIED EXCAVATION	COMPACTED EMBANKMENT
			CU.	YD.
ENTIRE	PROJECT	STAGE 1-MAIN LANES	3058	2387
ENTIRE	PROJECT	STAGE 2-MAIN LANES	2274	628
ENTIRE	PROJECT	APPROACHES		570
109+64	114+00	EXISTING DIRT BERM	30	
ENTIRE	PROJECT	BRIDGE EXCAVATION	844	
TOTALS:			6206	3585

NOTE: EARTHWORK QUANTITIES SHALL BE PAID AS PLAN QUANTITY.

BASE AND SURFACING

			LENGTH	AGGREGA COURSE (TACK COAT				А	CHM BINDE	R COURSE (1	I")				ACHM SU	IRFACE COU	RSE (1/2")			
STATION	STATION	LOCATION	LENGTH	TON/			GAL. PER SO			GAL. PER SO	Q. YD.)	TOTAL	AVG. WID.		POUND/	PG 64-22	AVG. WID.		POUND/	PG 64-22	AVG. WID.		POUND /	PG 64-22	TOTAL
			FEET	STATION	TON	TOTAL WID	SQ.YD.	GALLON	TOTAL WID. FEET	SQ.YD.	GALLON	GALLONS		SQ.YD.	SQ.YD.	TON	FEET	SQ.YD.	SQ.YD.	TON	FEET	SQ.YD.	SQ.YD.	TON	PG 64-22 TON
MA	IN LANES											1		1		1011		1	1	1011		ı		1011	
		HWY.61TRANSITION	100.00						33.00	366.67	62.33	62.33									33.00	366.67	220.00	40.33	40.33
		HWY. 61- TANGENT SECTION OVERLAY	124.38			30.00	414.60	20.73				20.73									30.00	414.60	220.00	45.61	45.61
		HWY. 61- TANGENT SECTION NOTCH AND WIDENING	596.18	61.25	365.16	21.09	1397.05	69.85				69.85	11,17	739.93	330.00	122.09	9.92	657.12	220,00	72.28	30.00	1987,27	220.00	218.60	290.88
107+12.11	109+64.50	HWY. 61- TANGENT SECTION	252.39	192.50	485.85	44.71	1253.82	62.69				62.69	22.46	629.85	330.00	103.93	22.25	623.96	220.00	68.64	30.00	841.30	220.00	92.54	161.18
111+31.50	114+73.27	HWY. 61 - TANGENT SECTION	341.77	192.50	657.91	44.71	1697.84	84.89				84.89	22.46	852.91	330.00	140.73	22.25	844.93	220.00	92.94	30.00	1139.23	220.00	125.32	218.26
114+73.27	118+12.37	HWY. 61 - TANGENT SECTION NOTCH AND WIDENING	339.10	61.25	207.70	21.63	814.97	40.75				40.75	11,44	431.03	330.00	71.12	10.19	383.94	220,00	42.23	30.00	1130.33	220,00	124.34	166,57
		HWY. 61 - TANGENT SECTION OVERLAY	60.57			30.00	201.90	10.10				10.10									30.00	201.90	220.00	22.21	22.21
		C.L. HWY. 61- TRANSITION	100.00						33.00	366.67	62.33	62.33									33.00	366.67	220.00	40.33	40.33
AD	DITIONAL F	OR LEVELING	•			•							•	•		•			•	-			•		
99+91.55	101+15.93	HWY. 61 - TANGENT SECTION OVERLAY	124.38						30.00	414.60	70.48	70.48					30.00	414.60	105.00	21.77					21.77
		HWY. 61 - TANGENT SECTION NOTCH AND WIDENING	596.18						17.58	1164.54	197.97	197.97					17.58	1164.54	460.00	267.84					267.84
114+73.27	118+12.37	HWY. 61 - TANGENT SECTION NOTCH AND WIDENING	339.10						17.31	652.20	110.87	110.87					17.31	652.20	660.00	215.23					215.23
118+12.37	118+72.94	HWY. 61 - TANGENT SECTION OVERLAY	60.57						30.00	201.90	34.32	34.32					30.00	201.90	350.00	35.33					35.33
AD	DITIONAL F	OR GUARDRAIL																							
107+24.35	107+57.35	HWY. 62 - GUARDRAIL WIDENING - RT.	33.00	19.50	6.44																2.75	10.08	220.00	1.11	1.11
107+57.35	107+67.35	HWY. 62 - GUARDRAIL WIDENING - RT.	10.00	39.25	3.93																5.50	6.11	220.00	0.67	0.67
		HWY. 62 - GUARDRAIL WIDENING - RT.	175.00	32.00	56.00																4.50	87.50	220.00	9.63	9.63
		HWY. 62 - GUARDRAIL WIDENING - RT.	43.75	25.00	10.94																3.50	17.01	220.00	1.87	1.87
109+34.50		HWY. 62 - BRIDGE END TERMINAL - LT.	21.00	19.50	4.10																2.75	6.42	220.00	0.71	0.71
109+55.50		HWY. 62 - BRIDGE END TERMINAL - LT.	30.00	25.00	7.50																3.50	11.67	220.00	1.28	1.28
111+09.90		HWY. 62 - GUARDRAIL WIDENING - RT.	43.75	25.00	10.94																3.50	17.01	220.00	1.87	1.87
111+53.65		HWY. 62 - GUARDRAIL WIDENING - RT.	100.00	32.00	32.00																4.50	50.00	220.00	5.50	5.50
112+53.65		HWY. 62 - GUARDRAIL WIDENING - RT.	10.00	39.25	3.93																5.50	6.11	220.00	0.67	0.67
112+63.65		HWY. 62 - GUARDRAIL WIDENING - RT.	33.00	19.50	6.44																2.75	10.08	220.00	1.11	1.11
111+09.90		HWY. 62 - GUARDRAIL WIDENING - LT.	43.75	25.00	10.94																3.50	17.01	220.00	1.87	1.87
111+53.65	112+53.65	HWY. 62 - GUARDRAIL WIDENING - LT.	100.00	32.00	32.00																4.50	50.00	220.00	5.50	5.50
	112+63.65	HWY. 62 - GUARDRAIL WIDENING - LT.	10.00	39.25	3.93																5.50	6.11	220.00	0.67	0.67
112+63.65	112+96.65	HWY. 62 - GUARDRAIL WIDENING - LT.	33.00	19.50	6.44																2.75	10.08	220.00	1.11	1.11
TOTALS:					1912.15		5780.18	289.01		3166.58	538.30	827.31		2653.72		437.87		4943.19		816.26		6753.16		742.85	1559.11

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101009	19	52
						OLIANITITIES		

ARKANJAS

LICENSED

PROCESSIONAL

ENGINEER

No.13640

O. LAWREN

12-10-20

SUMMARY OF CLASSIFICATION TEST RESULTS U.S. Highway 61 Bridge #01257 Mississippi County, Arkansas ARDOT 101009

		्र		≥		Sieve Analysis								
Boring No.	Depth	Liquid Limit (%)	Plastic Limit (%)	Plasticity Index (%)	2 in.	1 in.	3/4 in.	3/8 in.	#4	#10	#40	#200	AASHTO CLASS.	USCS CLASS.
D-1	0	56	19	37		-		_		-		_	A-7-6	CH
D-1	5	52	22	30	-			_		_		90.0	A-7-6 (29)	CH
D-1	8	-	_	1	100.0	100.0	100.0	100.0	100.0	100.0	72.4	6.3	A-3	SP-SM
D-1	18		_	-	100.0	100.0	100.0	100.0	100.0	100.0	99.9	5.7	A-3	SP-SM
D-1	33.5		_		100.0	100.0	100.0	100.0	100.0	99.9	63.3	3.9	A-3	SP
D-1	43.5				100.0	100.0	100.0	100.0	100.0	99.9	67.2	3.4	A-3	SP
D-2	3	0	0	0	100.0	100.0	100.0	96.2	95.0	93.6	62.6	24.8	A-2-4 (0)	SM
D-2	6	39	23	16	100.0	100.0	100.0	100.0	100.0	99.9	98.4	90.9	A-6 (15)	CL
D-2	18.5		-		100.0	100.0	100.0	100.0	100.0	100.0	99.8	5.5	A-3	SP-SM
D-2	33.5		_		100.0	100.0	100.0	100.0	100.0	99.3	47.2	4.3	A-1-b	SP
D-2	48.5		-		100.0	100.0	100.0	100.0	100.0	100.0	65.9	4.5	A-3	SP
D-2	108.5	74	28	46	-			_		_		-	A-7-6	CH
D-3	18.5		-		100.0	100.0	100.0	100.0	99.9	99.3	98.6	13.9	A-2-4	SM
D-3	28.5		_		100.0	100.0	100.0	100.0	100.0	99.7	97.8	7.6	A-3	SP-SM
D-3	43.5		_	-	100.0	100.0	100.0	100.0	100.0	100.0	67.6	3.4	A-3	SP
D-3	78.5		_		100.0	100.0	100.0	100.0	100.0	99.2	18.1	3.6	A-1-b	SP
D-3	113.5		_		-	-	-	_		-	-	_	A-7-6	CH
D-4	0		_		1	-		_		-	-	_	A-7-6	CH
D-4	10	0	0	0	-	-	-	-		-	-	-	A-4	ML
D-4	18.5		_		100.0	100.0	100.0	100.0	100.0	100.0	99.9	32.6	A-2-4	SM
D-4	28.5		-		100.0	100.0	100.0	100.0	99.8	99.6	51.8	3.7	A-3	SP

NOTE: SOIL CHARACTERISTICS TABULATED ABOVE ARE REPRESENTATIVE AT THE LOCATION OF THE SAMPLE, AND FROM SURFACE INDICATIONS ARE TYPICAL FOR THE LIMITS SHOWN. THESE DATA ARE SHOWN FOR INFORMATION ONLY. THE STATE WILL NOT BE RESPONSIBLE FOR VARIATIONS IN THE SOIL CHARACTERISTICS AND/OR EXTENT OF THE SAME DIFFERING FROM THE ABOVE TABULATIONS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101009	20	52
		-	$\overline{}$		07497	- OLIANTITIES -	61804	

07497 - QUANTITIES - 61804

SCHEDULE OF BRIDGE QUANTITIES - JOB 101009

Æ	IAME PLATE TITLE	UNIT OF STRUCTURE	ITEM NUMBER	205	801	SP, SS, & 802	SP, SS, & 802	SP & 803	SS & 804	SS & 804	SS & 805	SS & 805	SS & 805	SS & 805	SP, SS, & 807	812	SS & 816	SS & 816
RIDGE NUME			ITEM	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1)	UNCLASSIFIED EXCAVATION FOR STRUCTURES- BRIDGE	CLASS S CONCRETE-	CLASS S(AE) CONCRETE- BRIDGE	CLASS 2 PROTECTIVE SURFACE TREATMENT	REINFORCING STEEL-BRIDGE (GRADE 60)	EPOXY COATED REINFORCING STEEL (GRADE 60)	① STEEL SHELL PILING (18" DIAMETER)	STEEL SHELL PILING (24" DIAMETER)	PILE ENCASEMENT	PREBORING	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50W)	BRIDGE NAME PLATE (TYPE D)	DUMPED RIPRAP	FILTER BLANKET
	ž		UNIT	LUMP SUM	CU. YD.	CU. YD.	CU. YD.	SQ. YD.	POUND	POUND	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	POUND	EACH	CU. YD.	SQ. YD.
	OVER 30	END BENT NO. 1 INTERMEDIATE BENT NO. 2	!		22	14.40 20.10			4,474 6,191	1,085 1,950	375	375	25	50			61	110
161	9 S	INTERMEDIATE BENT NO. 3				20.10			6,191	1,950		375	35					
7/0	₩AY	END BENT NO. 4			26	14.40			4,474	1,085	415			50			69	124
	훒吉	100'-0" CONT. INTEGRAL W	-BEAM UNIT				155.00	439.0		39,700					36,430	1		
	HIG	EXIST. BRIDGE NO. 01257	(SITE NO. 1)	1														
		TOTALS FOR JOB NO.	101009		48	69.00	155.00	439.0	21,330	45,770	790	750	60	100	36,430	1	130	234

① Steel Shell Piles shall conform to ASTM A252, Grade 3, Fy = 45 ksi.

ARKANSAS

LICENSED

PROFESSIONAL

ENGINEER

No. 18919

BRIDGE ENGINEER

SCHEDULE OF BRIDGE QUANTITIES
HIGHWAY 61 OVER DITCH NO. 30
DITCH NO. 30 STR. & APPRS. (BLYTHEVILLE) (S)
MISSISSIPPI COUNTY

ROUTE 61 SECTION 3

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

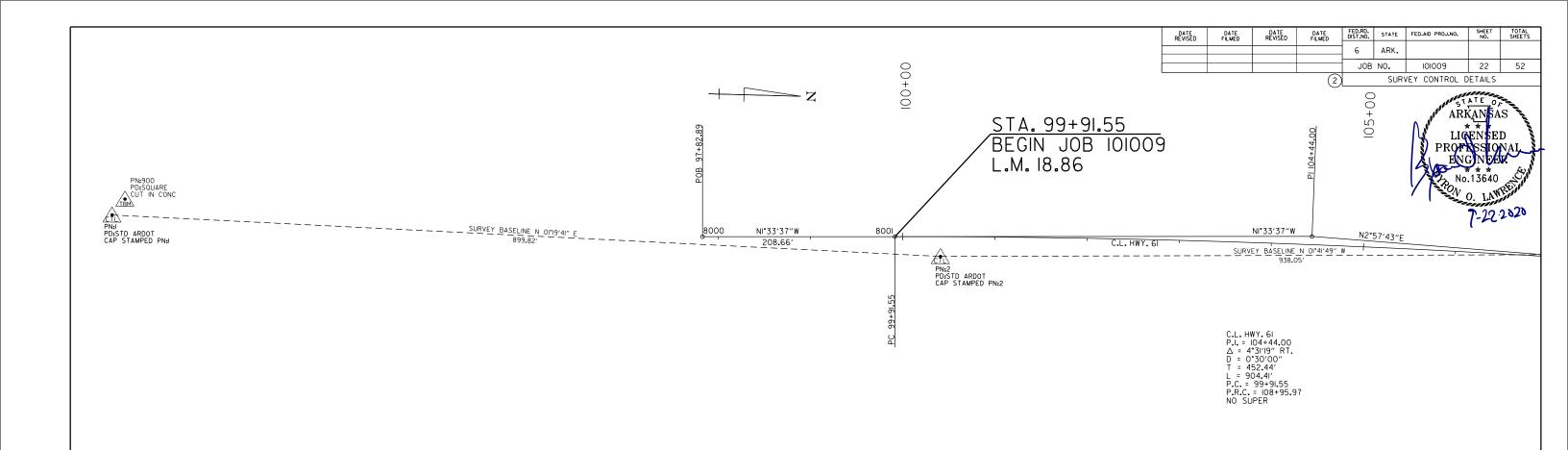
ITEM NUMBER

		DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
						6	ARK.			
						JOB	NO.	101009	21	52
				I	(2	+		F QUANTITIES	AND REV	ISIONS
						/		-46	11111000	
								11115	ATELO	See.
SUMMARY OF QUANTITIES								AR.	KANSA	S &
ITEM	QUANTITY UNIT							/ 🏂 Lic	ENSE	D 🔰
LW	QUANTITI UNIT								KKK10	

201	CLEARING	19	STATION
201	GRUBBING	19	STATION
202	REMOVAL AND DISPOSAL OF PIPE CULVERTS	3	EACH
202	REMOVAL AND DISPOSAL OF BOX CULVERTS	1	EACH
202	REMOVAL AND DISPOSAL OF GUARDRAIL	206	LIN. FT.
202	REMOVAL AND DISPOSAL OF IMPACT ATTENUATION BARRIER	2	EACH
	INCLASSIFIED EXCAVATION	6206	CU.YD.
SP & 210	COMPACTED EMBARKMENT	3585	CU.YD.
SP & 210	COMPACTED EMBANRIMENT SOIL STABILIZATION		TON
		200	
	AGGREGATE BASE COURSE (CLASS 7)	2581	TON
SS & 401	TACK COAT	845	GAL.
	MINERAL AGGREGATE IN ACHM BINDER COURSE (1")	420	TON
	ASPHALT BINDER (PG 64-22) IN ACHM BINDER COURSE (1")	18	TON
	MINERAL AGGREGATE IN ACHM SURFACE COURSE (1/2")	1598	TON
SP, SS, & 407	ASPHALT BINDER (PG 64-22) IN ACHM SURFACE COURSE (1/2")	84	TON
SP & 412	COLD MILLING ASPHALT PAVEMENT	733	SQ. YD.
SP, SS, & 414	ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC	9	TON
	ACHM PATCHING OF EXISTING ROADWAY	10	TON
	APPROACH SLABS	59.00	CU.YD.
	APPROACH GUTTERS	16.80	CU YD.
	PORTLAND CEMENT CONCRETE DRIVEWAY	276.94	SQ YD
601	MOBILIZATION	1.00	LUMP SUM
SP & 602	FURNISHING FIELD OFFICE	1 100	EACH
	MAINTENANCE OF TRAFFIC	1.00	LUMP SUM
SS & 604	SIGNS	111	SQ.FT.
SS & 604	BARRICADES	32	LIN. FT.
SS & 604	TRAFFIC DRUMS	93	EACH
604	CONSTRUCTION PAVEMENT MARKINGS	7664	LIN. FT.
604	REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	664	LIN. FT.
SS & 604	VERTICAL PANELS	29	EACH
SP	CULVERT CLEAN OUT	20	EACH
SP, SS, & 606	18" SIDE DRAIN	30	LIN. FT.
SP, SS, & 606	30" SIDE DRAIN	46	LIN. FT.
	36° SIDE DRAIN	38	LIN. FT.
SS & 606	SELECTED PIPE BEDDING	20	CU YD
SP SP	DEROP INLET CLEAN OUT	17	EACH
SS & 611	HELT TIME TO THE TOTAL THE TOTAL TO THE TOTA	4000	LIN FT
SS & 611	UNDERDRAIN OUTLET PROTECTORS	16	EACH
SS & 617	GUARDRAIL (TYPE A)	300	LIN. FT.
SS & 617	GUARDRAIL TERMINAL (TYPE 2)	3	EACH
SS & 617	THRIE BEAM GUARDRAIL TERMINAL	3	EACH
620	LIME	15	TON
620	SEEDING	7.32	ACRE
SS & 620	MULCH COVER	28.33	ACRE
620	WATER	1212.3	M. GAL.
621	TEMPORARY SEEDING	21.01	ACRE
621	SILT FENCE	3791	LIN. FT.
621	SEDIMENT BASIN	417	CU.YD.
621	OBLITERATION OF SEDIMENT BASIN	417	CU.YD.
621	SEDIMENT REMOVAL AND DISPOSAL	576	CU. YD.
621	ROCK DITCH CHECKS	57	CU. YD.
SS & 621	FILTER SOCK (18")	300	LIN. FT.
623	SECOND SEEDING APPLICATION	7.32	ACRE
624	SOLID SODDING	2950	SQ.YD.
635	ROADWAY CONSTRUCTION CONTROL	1.00	LUMP SUM
637	MAILBOXES	9	EACH
637	MAILBOX SUPPORTS (SINGLE)	9	EACH
719	IMALEDA SUFTANTS (SINGLE) THERMOPLASTIC PAVEMENT MARKING WHITE (6")	4163	LIN.FT.
719	THERMOPLASTIC PAVEMENT MARKING WHITE (6.) THERMOPLASTIC PAVEMENT MARKING YELLOW (6")	4163	LIN.FT.
	RAISED PAVEMENT MARKING YELLOW (6.)		
721		78	EACH
SS & 734	BRIDGE END TERMINAL	1 5000	EACH
SS & 804	REINFORCING STEEL-ROADWAY (GRADE 60)	5662	POUND
	STRUCTURES OVER 20' SPAN		
205	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO.1)	1.00	LUMP SUM
636	BRIDGE CONSTRUCTION CONTROL	1.00	LUMP SUM
801	UNCLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE	48	CU. YD.
	CLASS S CONCRETE-BRIDGE	69.00	CU.YD.
SP, SS, & 802	CLASS S(AE) CONCRETE-BRIDGE	155.00	CU. YD.
SP & 803	CLASS 2 PROTECTIVE SURFACE TREATMENT	439.0	SQ. YD.
SS & 804	REINFORCING STEEL-BRIDGE (GRADE 60)	21330	POUND
SS & 804	EPOXY COATED REINFORCING STEEL (GRADE 60)	45770	POUND
SS & 805	STEEL SHELL PILING (18" DIAMETER)	790	LIN. FT.
SS & 805	STEEL SHELL PILING (24" DIAMETER) STEEL SHELL PILING (24" DIAMETER)	750	LIN FT
SS & 805	PREBORING	100	LIN.FT.
		60	LIN.FT.
SS & 805	PILE ENCASEMENT		
SP, SS, & 807	STRUCTURAL STEEL IN BEAM SPANS (A709, GR. 50W)	36430	POUND
812	BRIDGE NAME PLATE (TYPE D)	1	EACH
SS & 816	FILTER BLANKET	234	SQ.YD.
SS & 816	DUMPED RIPRAP	130	CU.YD.
		1	

REVISIONS

DATE	REVISION	SHEET NUMBER



SURVEY CONTROL COORDINATES

Project Name: s101009 Date: 5/1/2019

ARKANSAS STATE PLANE - NORTH ZONE BASED ON GPS CONTROL. 470018A - 470019A Coordinat

Units: U.

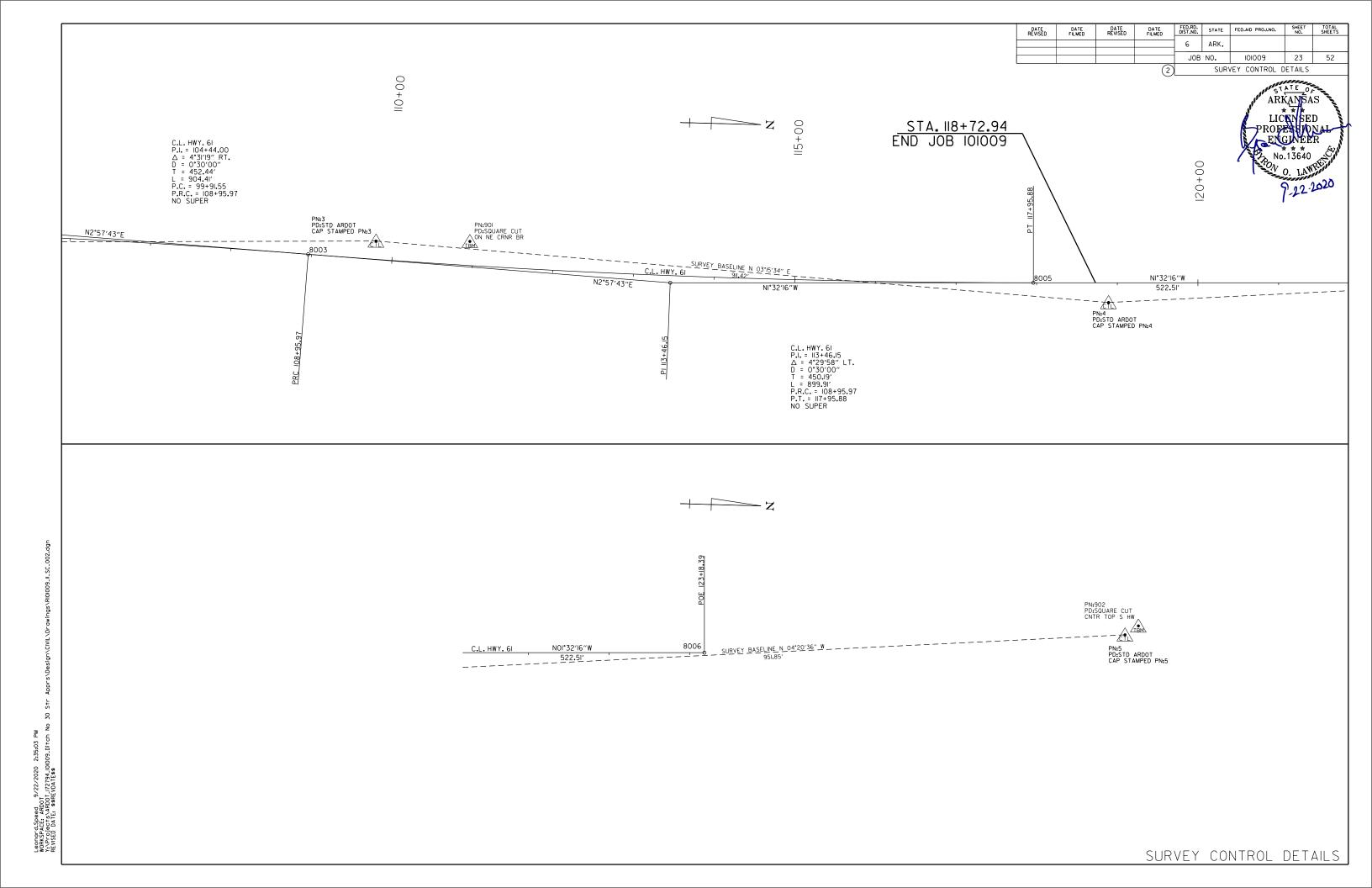
Point.					
Name	Northing	Easting	Elev	Feature	Description
1	596273, 2816	1930901.1890	253.74	CTL	STD ARDOT CAP STAMPED PN: 1
2	597172.8558	1930922.0460	253.22	CTL	STD ARDOT CAP STAMPED PN: 2
3	598110.4942	1930894.2666	254.41	CTL	STD ARDOT CAP STAMPED PN: 3
4	599020.4373	1930946.0862	253.79	CTL	STD ARDOT CAP STAMPED PN: 4
5	599969.5495	1930873, 9996	253.48	CTL	STD ARDOT CAP STAMPED PN: 5
100	610645.4083	1940985.1283	253.87	GPS	ARDOT GPS MON 470018A
101	611466.0124	1930665.9461	254.26	GPS	ARDOT GPS MON 470019A
900	596286.7757	1930883,6001	254.13	TBM	SQUARE CUT IN CONC
901	598226.8860	1930891.6615	254.75	TBM	SQUARE CUT ON NE CRNR BR
902	599985.8761	1930862.4940	252.61	TBM	SQUARE CUT CNTR TOP S HW
903	592847.1017	1930983.9391	250.74	TBM	SQUARE CUT IN CONC E DI
904	589585.3477	1931140, 9831	254.38	TBM	SÕUARE CUT IN CONC N DI
905	588460.8221	1932812.5145	256.83	TBM	X CUT IN BOLT OF FH
999	588409, 4088	1935203, 8373	252, 78	BM	NGS BM BLYTHVILLE 1955

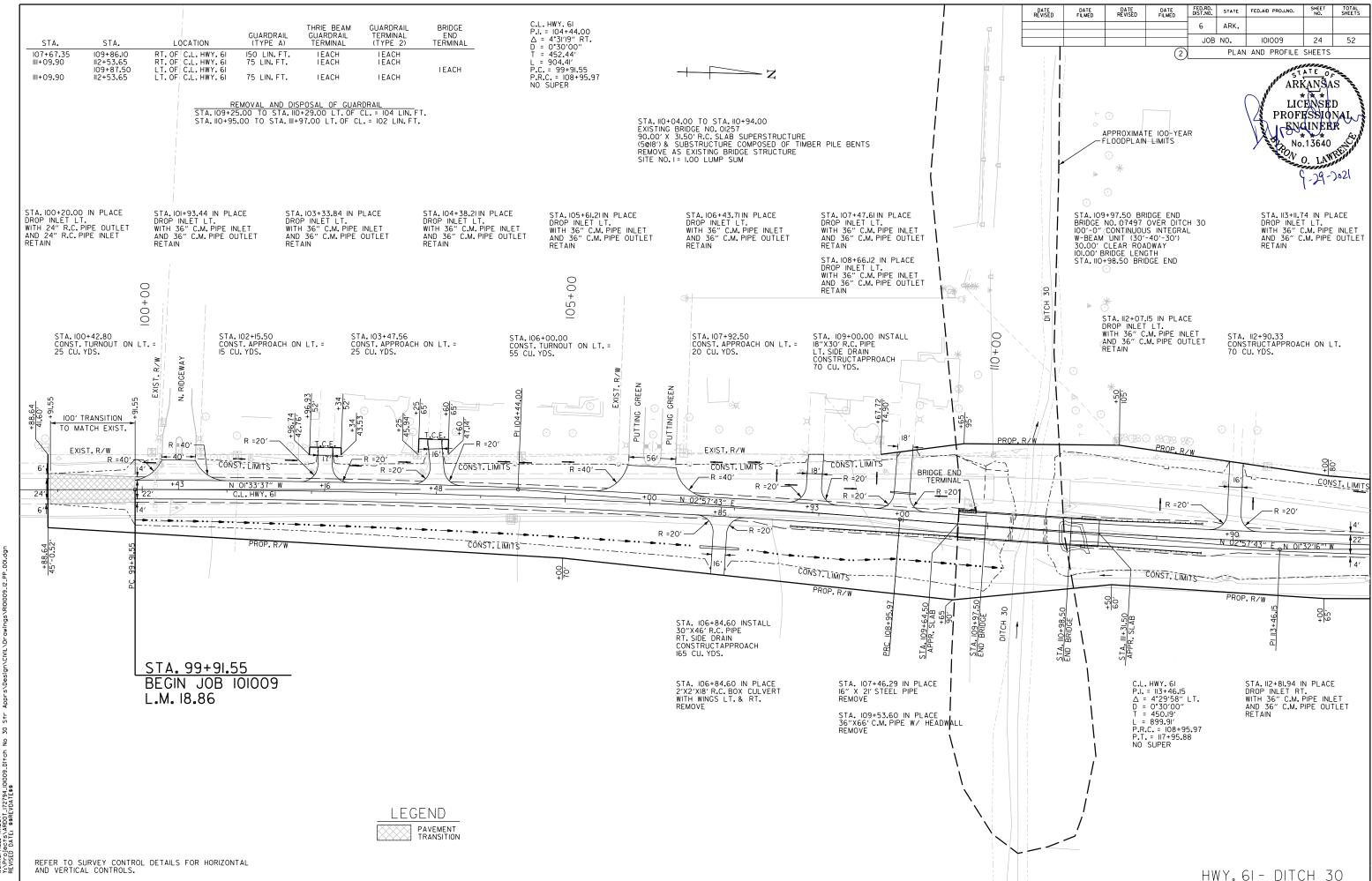
*Note - Rebar and Cap - Standard - 5/8' Rebar with 2' Aluminum Cap stamped
*(standard markings common to all caps), or as indicated
(other markings indicated in the point description of the individual point).
ALL DISTANCES ARE GROUND.
USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT.
A PROJECT CAF OF 0.999949492 HAS BEEN USED TO COMPUTE THE ABOVE GROUND COORDINATES.
THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS.
GRID DISTANCE = GROUND DISTANCE X CAF.
GRID COORDINATES ARE STORED UNDER FILE NAME \$101009gi.CTL
HORIZONTAL DATUM: NAVD 88 POSITIONAL ACCURACY THIRD ORDER, UNLESS SPECIFIED OTHERWISE
AT A SPECIFIC POINT.

REFERENCE POINTS (1500 SERIES) ARE TO BE USED TO ESTABLISH CONTROL IF THE PRIMARY CONTROL POINTS LISTED ABOVE HAVE BEEN DESTROYED. REFERENCE POINTS ARE NOT TO BE USED FOR VERTICAL CONTROL

BASIS OF BEARING: ARKANSAS STATE PLANE GRID BEARINGS - 0301-NORTH ZONE DETERMINED FROM GPS CONTROL POINTS: 470018A - 470019A CONVERGENCE ANGLE: 01-12-58 RIGHT AT PN:3 LT:N35-57-30 LG:W089-54-36 GRID AZIMUTH = ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

at.e				I ZONE BAS	SED ON GPS CONTROL, 470018A - 470019A	•	AL I GN	MENT N	AME: HWY. 61	
		CTED TO GROUND.				POINT	STATION	TYPE	NORTHING	EASTING
U . 5	. SURVEY FOOT					NUMBER				
						8000	97+82.89	POB	596914.2666	1930907.5666
	Northing	Easting	Elev	Feature	Description	8001	99+91.55	PC	597122.8524	1930901.8853
							104+44.00	PΙ	597575.1266	1930889.5665
	596273, 2816	1930901.1890	253.74	CTL	STD ARDOT CAP STAMPED PN: 1	8002		CC	597434.8528	1942356.7970
	597172.8558	1930922.0460	253.22	CTL	STD ARDOT CAP STAMPED PN: 2	8003	108+95.97	PRC	598026.9641	1930912.9449
	598110.4942	1930894.2666		CTL	STD ARDOT CAP STAMPED PN: 3		113+46.15	PΙ	598476.5501	1930936.2067
	599020.4373	1930946.0862		CTL	STD ARDOT CAP STAMPED PN: 4	8004		CC	598619.0752	1919469.0968
	599969.5495	1930873.9996		CTL	STD ARDOT CAP STAMPED PN: 5	8005	117+95.88	PT	598926,5754	1930924, 1262
	610645.4083	1940985. 1283		GPS	ARDOT GPS MON 470018A	8006	123+18.39	POE	599448, 8932	1930910.1050
	611466.0124	1930665.9461	254.26	GPS	ARDOT GPS MON 470019A					





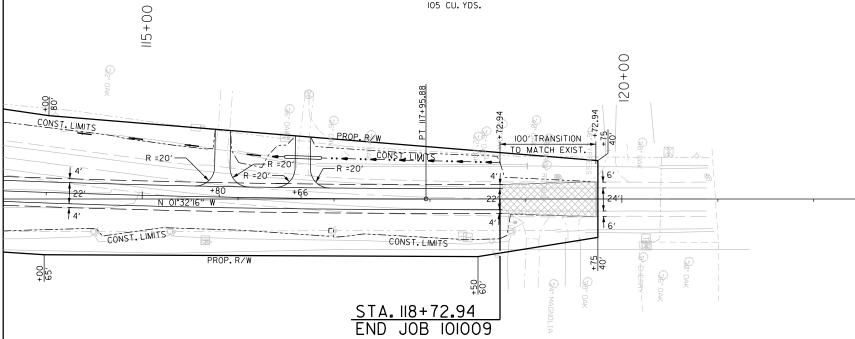
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101009	26	52
PLAN AND PROFILE SHEETS								

ARKANSAS
LICENSED
PROFESSIONAL
ENCINEER
No.13640
OV O. LAWREN
7-22-1020

STA.114+15.92 IN PLACE DROP INLET LT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN STA, 115+54.27 IN PLACE DROP INLET LT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN

STA. 115+80.30 IN PLACE 36" X 30' PIPE CULVERT LT. SIDE DRAIN RETAIN CONSTRUCT APPROACH 20 CU. YDS. STA. II6+65.60 IN PLACE 36" X 30' PIPE CULVERT LT. SIDE DRAIN REMOVE AND INSTALL 36"X38" PIPE CULVERT CONSTRUCT APPROACH IO5 CU. YDS.



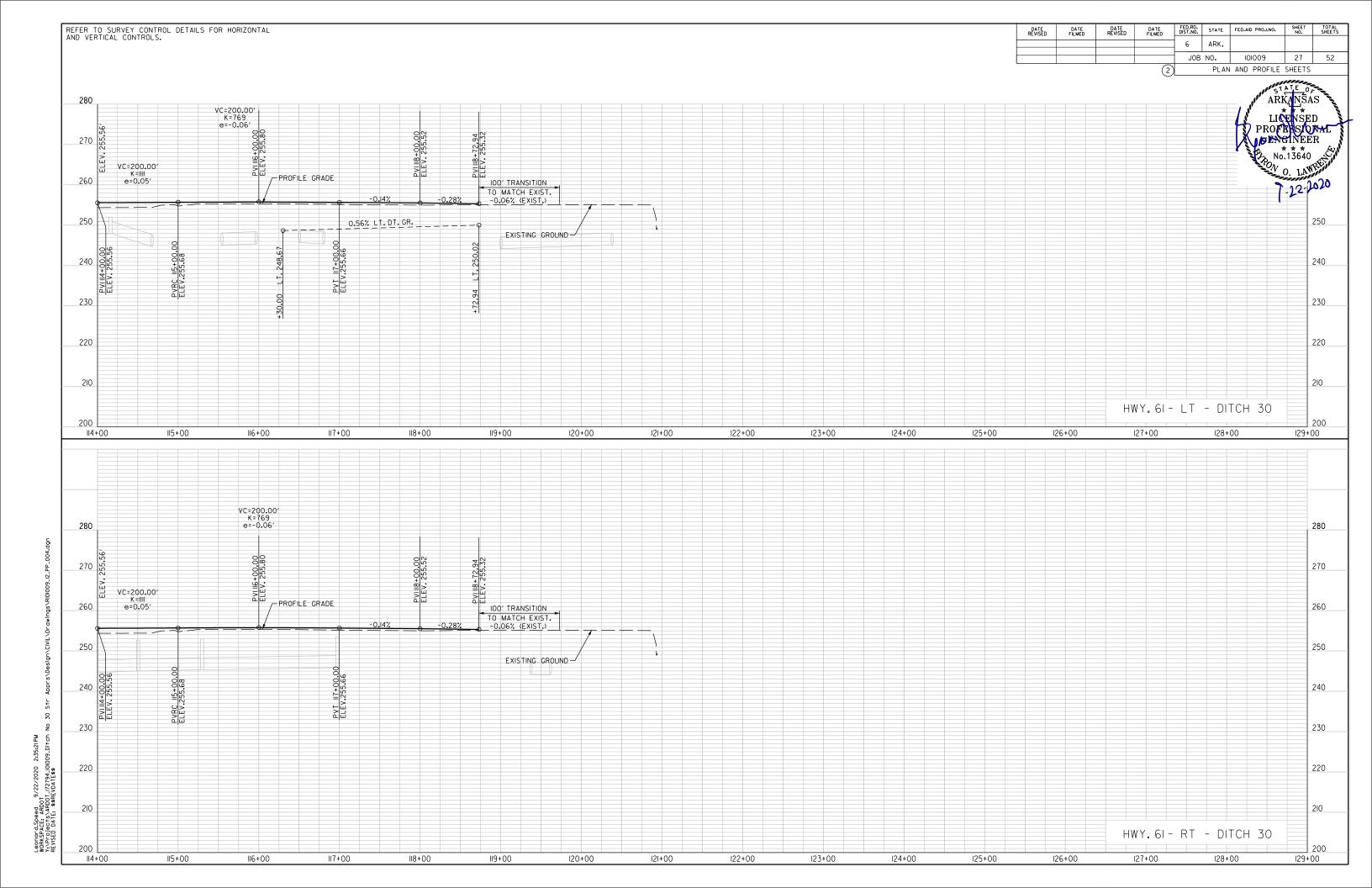


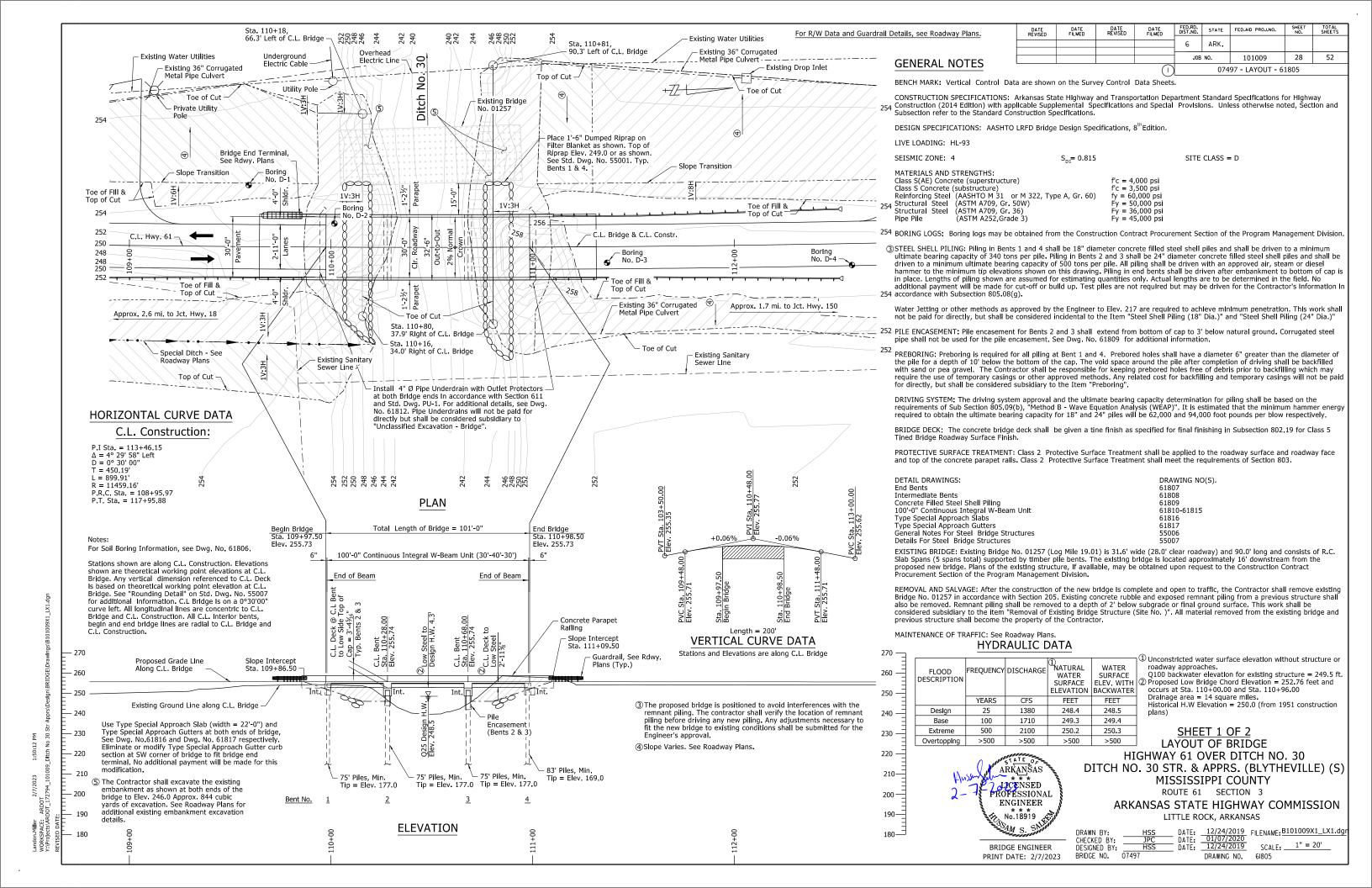
STA, 114+50.44 IN PLACE DROP INLET RT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN STA, 115+29,92 IN PLACE DROP INLET RT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN STA, 116+97.34 IN PLACE DROP INLET RT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN STA. 118+84.00 IN PLACE DROP INLET RT. WITH 36" C.M. PIPE INLET AND 36" C.M. PIPE OUTLET RETAIN

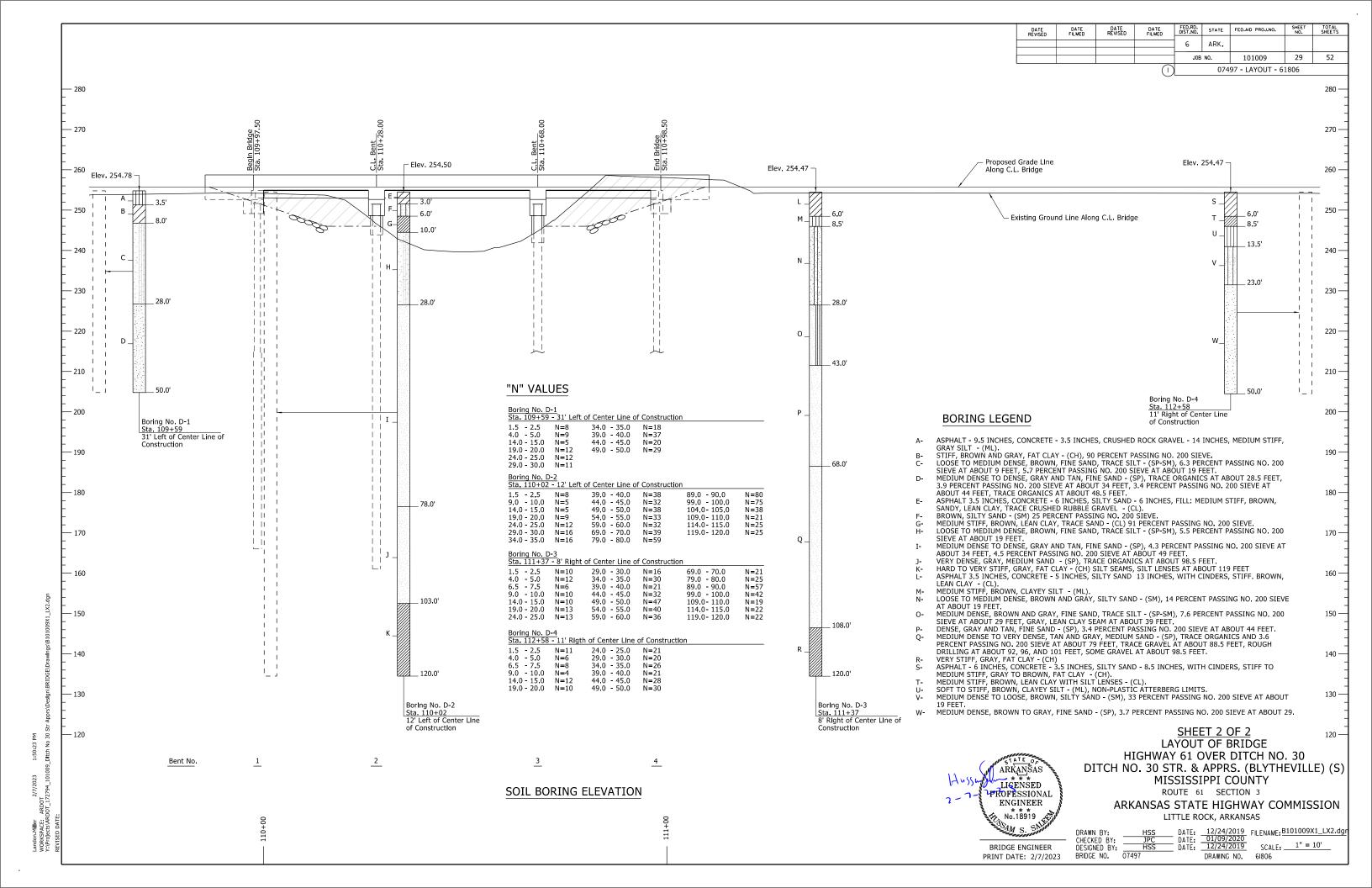
C.L. HWY. 6I P.I. = 113+46.15 \[\Delta = 4"29'58" \] LT. \[D = 0"30'00" \] \[T = 450.19' \] \[L = 899.91' \] P.R.C. = 108+95.97 P.T. = 117+95.88 NO SUPER

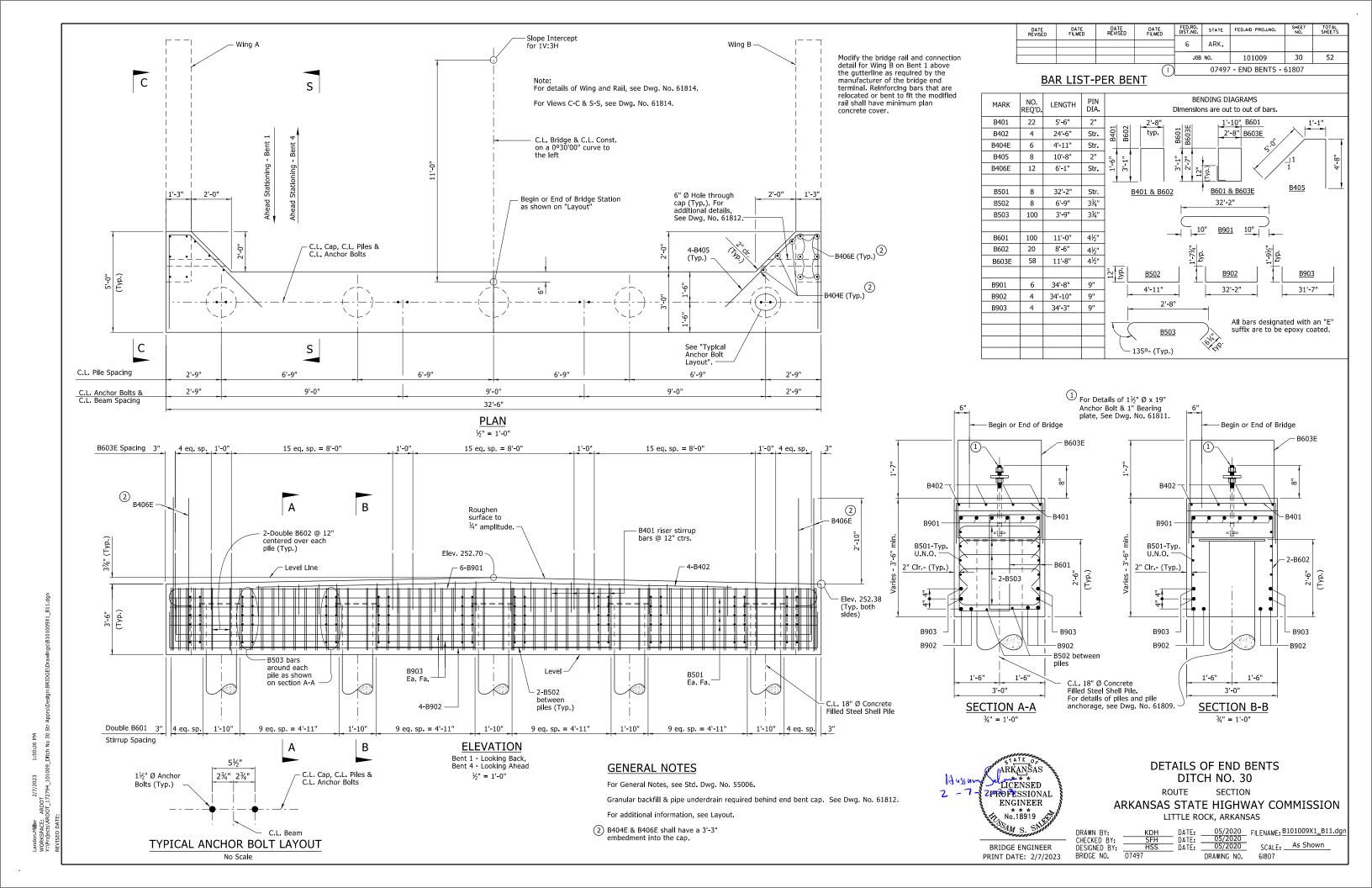
Leonard.Speed 9/22/2020 2:35:20 PM WORKSPACE, ARDO YA.Projects.NARDOI_I72794_L01009_Ditch No 30 Str REVISED DATE: \$\$REVDATE\$\$ THE STATE OF THE S

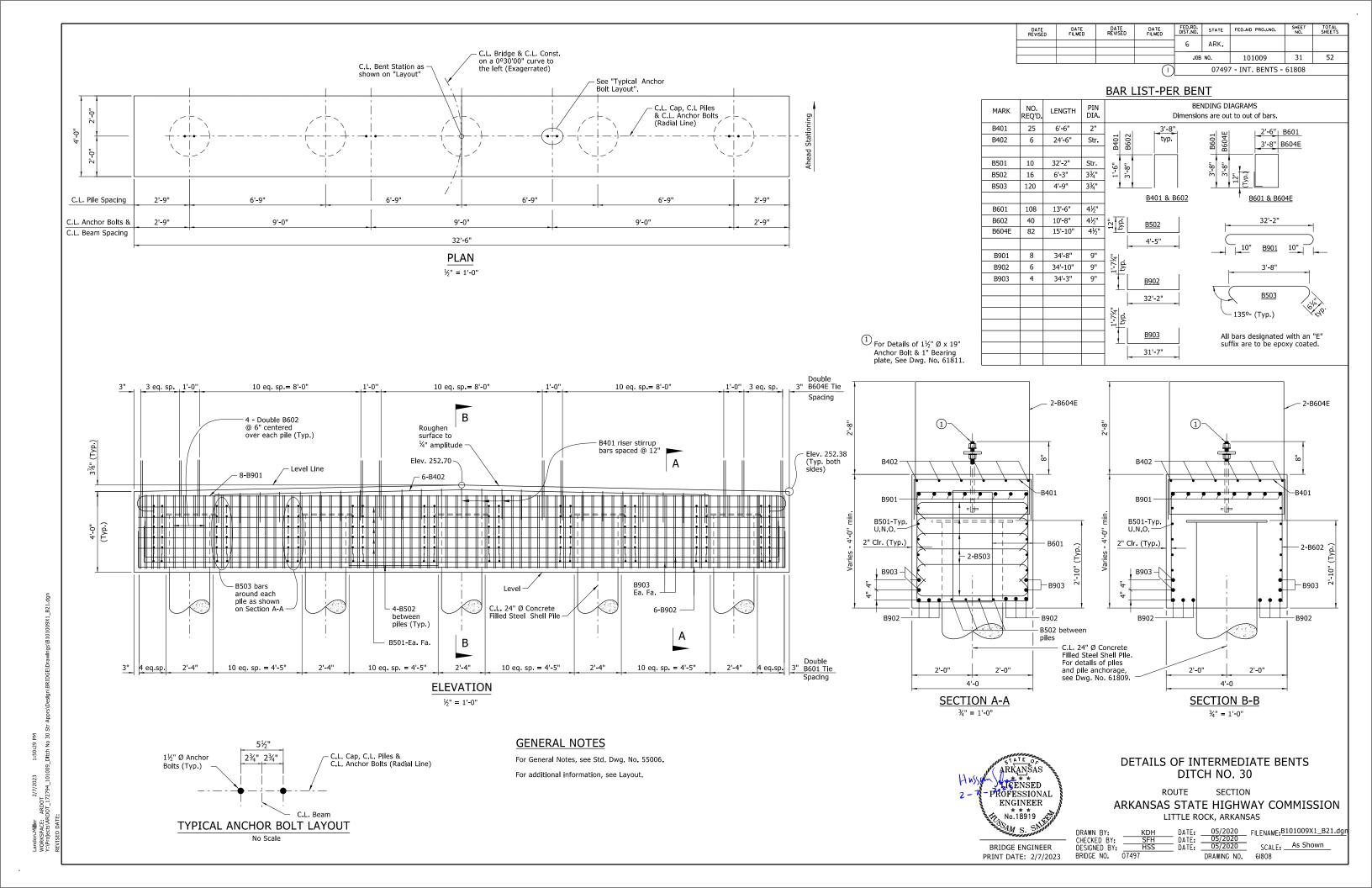
REFER TO SURVEY CONTROL DETAILS FOR HORIZONTAL AND VERTICAL CONTROLS.











Length of Pile

ALTERNATE VANED TIP DETAIL

TABLE OF VARIABLES

OUTSIDE DIAMETER STEEL SHELL PILE "D"	PILE EMBEDMENT "X"	PLATE THICKNESS "Y"	OUTSIDE DIAMETER ANNULAR RING "Z"	MINIMUM CONICAL TIP DESIGN LOAD (KIPS)
18"	2'-6"	1½"	26"	1,114
24"	2'-10"	1¾"	32"	1,495

GENERAL NOTES FOR CONCRETE FILLED STEEL SHEEL PILES

Steel shells shall conform ASTM A252, Grade 3 (Fy = 45,000 psi.)

Concrete used for filling of steel shell shall be Class S with a minimum 28-day compressive strength, f'c = 3,500 psi. and shall

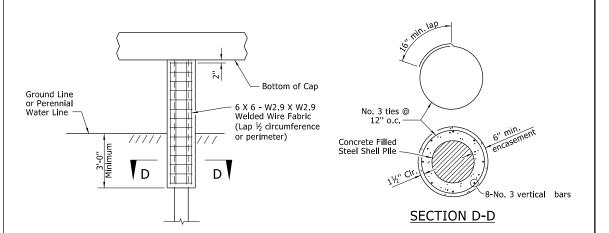
See Bridge Layout for size and estimated length of steel shell piles and for driving information.

Concrete, structural steel, reinforcing steel (including welding), and painting shall not be paid for directly, but shall be considered subsidiary to the item "Steel Shell Piling (18" Dia.)" or "Steel Shell Piling (24" Dia.)".

Steel pile tip will not be paid for directly, but shall be subsidiary to the item "Steel Shell Piling (18" Dia.)" or "Steel Shell Piling (24" Dia.)".

DATE REVISED DATE FILMED STATE ARK. 32 52 JOB NO. 101009

07497 - STEEL SHELL PILES - 61809



PILE ENCASEMENT DETAIL FOR STEEL SHELL PILES

GENERAL NOTES FOR PILE ENCASEMENTS

See Bridge Layout for additional notes, any pile encasement restrictions and required

Concrete shall be Class S with a minimum 28-day compressive strength, f'c = 3,500 psi. If concrete cannot be placed in the dry, Seal Concrete may be used from top to bottom

Reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A.

Welded wire fabric shall conform to AASHTO M 55 or M 221.

Concrete, welded wire fabric or reinforcing steel, and galvanized pipe shall not be paid for directly, but shall be considered subsidiary to the item "Pile Encasement".



DETAILS FOR CONCRETE FILLED STEEL SHELL PILES AND PILE ENCASEMENTS

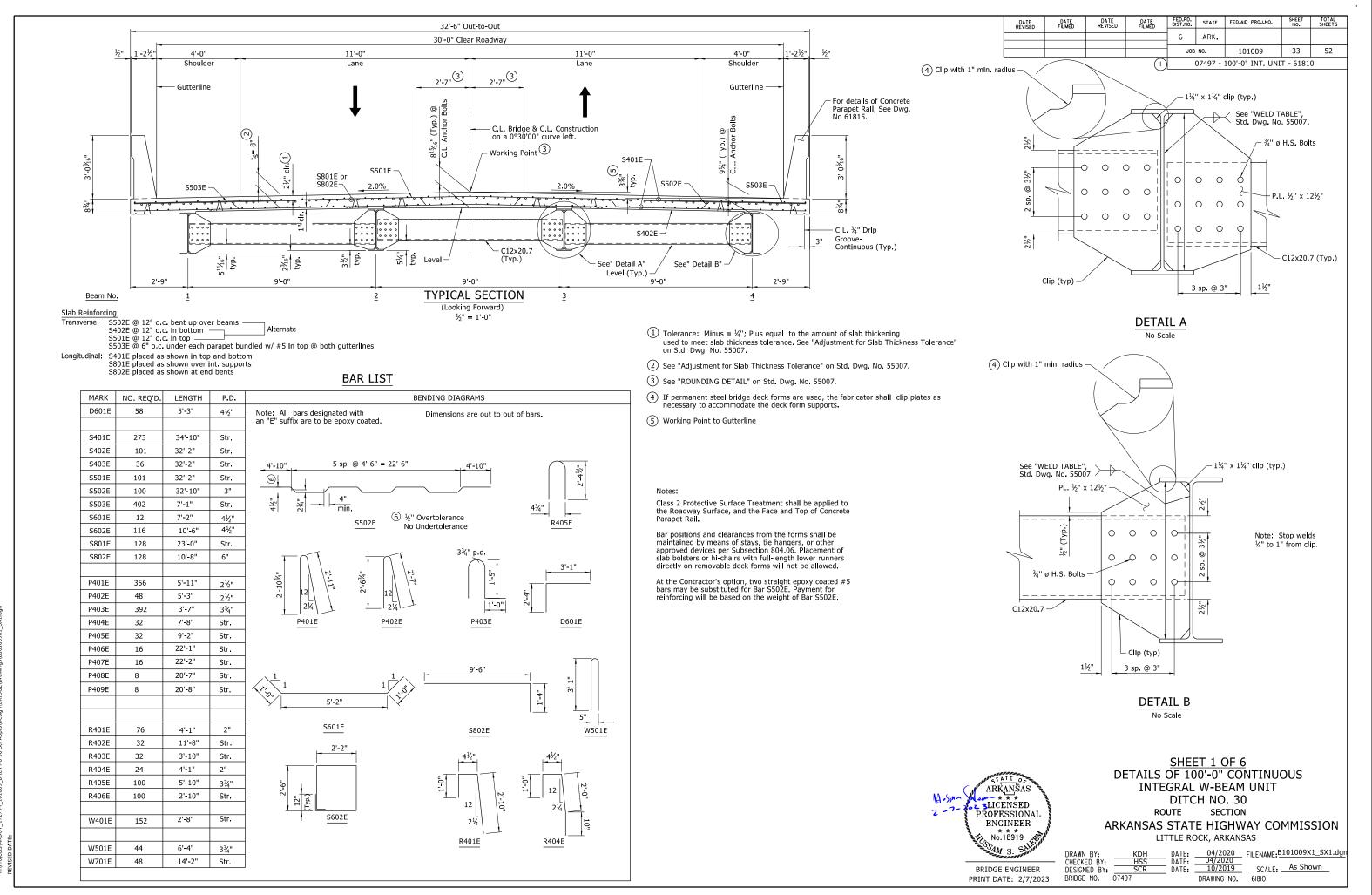
ROUTE SECTION

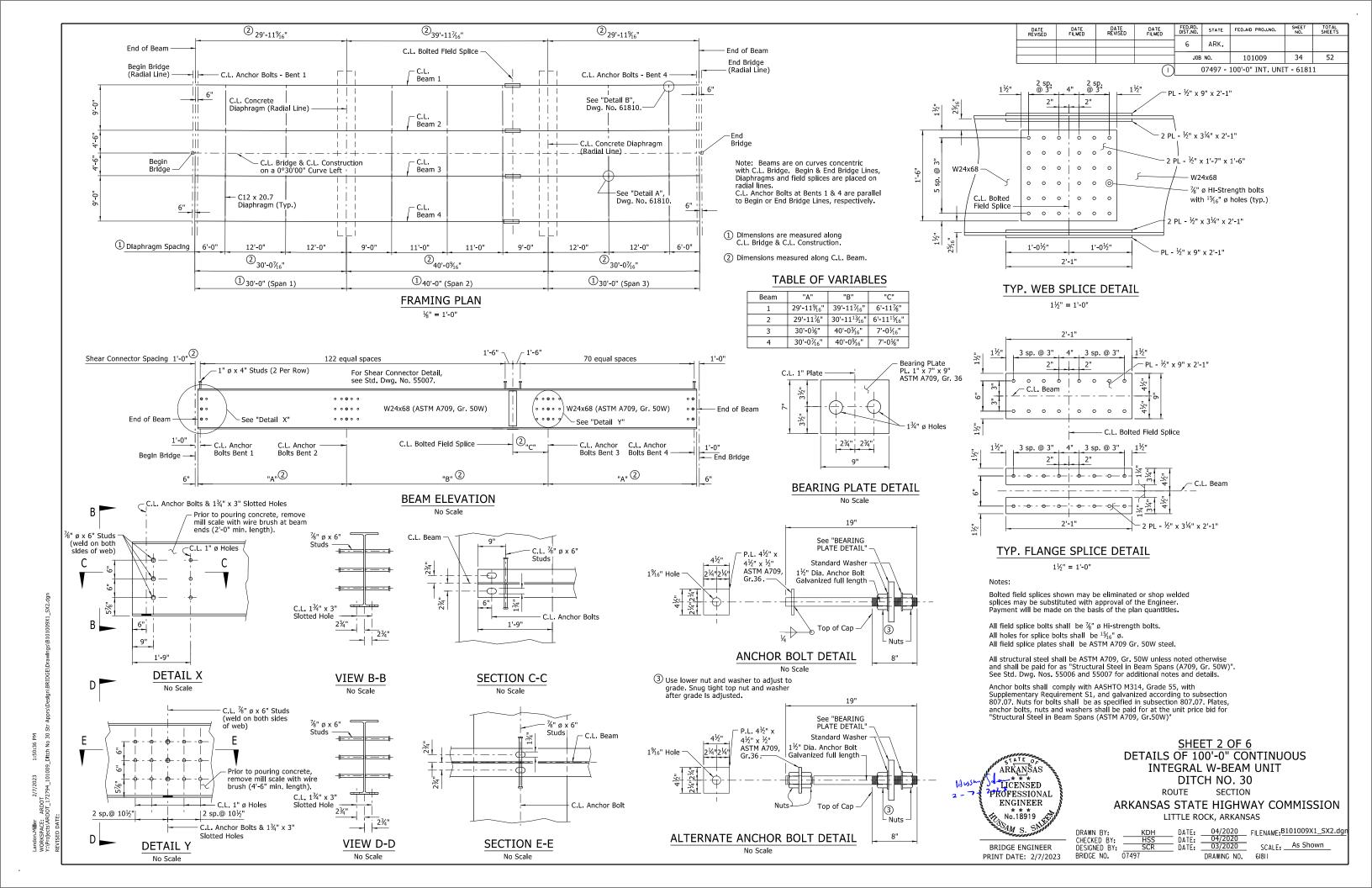
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARKANSAS DRAWN BY: _ FILENAME:B101009X1_B22.dgr

CHECKED BY: DESIGNED BY: BRIDGE ENGINEER PRINT DATE: 2/7/2023 BRIDGE NO.

DATE: 05/2020 DATE: 05/2020 SCALE: _ DRAWING NO. 61809







22'-5%" 22'-5%' Parapet Rail Joint Spacing 8'-0" 9'-6" 20'-11%' 9'-6" 8'-0" Closed Rail Closed Rail Closed Rail Closed Rai Measured along Gutterline Open Rail Open Rail Open Rail ② Required S503E -- Required ② S503E S501E Top, S402E Bottom - 100 sp. @ 12" o.c., See "REINFORCING DETAIL" 6" S502E - 99 sp. @ 12" o.c. (bent up over beams), See "REINFORCING DETAIL" 32'-6" Out-to-Out Pouring Sequence Const. Joint (2) — Pouring Sequence — C.L. Bridge & C.L. Const. Bridge B404E see Is on a 0°30'00" curve left Const. Joint (2) Bridge Dwg. Nos. 61807 9'-6" 11'-6" 11'-6" 11'-6" 11'-6" S801E(1) -0½e" 3 - S601E −D601E^③ 1'-11" ①S802E - S802E(1) - S801E (1) S401E(1) В min. lap S503E S503E S503E - 200 sp. @ 6" max. o.c. in top (both sides of Bridge Construction Joint in ③_{D601E} -(Lapped with Bars S501E & S502E, See "REINFORCING DETAIL") Parapet (Optional) 22'-61/3' 22'-61/5" Parapet Rail Joint Spacing 9'-6" 21'-0%' 8'-0" Open Rai Open Rail Open Rail Measured along Gutterline Closed Rail Closed Rai Closed Rail Closed Rai Pouring Sequence 5'-0" 17'-6" (Pour 1) 17'-6" (Pour 2) 21'-0" (Pour 1) 17'-6" (Pour 2) 17'-6" (Pour 1) 5'-0" (Pour 3) 30'-6" (Span 1) 40'-0" (Span 2) 30'-6" (Span 3) For Views A-A & B-B. C.L. Bent 2 - Radial C.L. Bent 3 - Radial -Α

REINFORCING PLAN AND POURING SEQUENCE

⅓" = 1'-0"

Partial depth parapet joint at this location. (Stop 1'-2" above top of slab)

Full depth parapet joint at this location. (Stop 4" above top of slab)

1 Placed as shown in "Typical Section", See Dwg. No. 61810.

(2) Align with parapet open joint unless noted otherwise See "Transverse Slab Joint Detail" on Std. Dwg. No. 55007.

③ Place as shown in "View A-A" on Dwg. No. 61813.

All transverse reinforcing steel shall be placed on radial lines to C.L. Bridge. Spacing shown is measured along C.L. Bridge & C.L. Construction.

All longitudinal lines and longitudinal reinforcing steel shall be spaced on curves concentric with C.L. Bridge & C.L. Construction.

Span lengths, slab pour lengths and transverse reinforcing spacing shown are measured along C.L. Bridge.

Pours with the same number may be placed simultaneously or separately. All pours (1) must be placed before pours (2) can be placed. All pours (2) must be placed before pours (3) can be placed. 48 hours shall elapse before the end of a pour and the start of the next pour. 72 hours shall elapse between the end of a pour and the start of an adjacent pour. 72 hours shall elapse between the completion of the entire deck and the pouring of the parapet. Any railing pours made before the entire slab unit has been placed must be approved by the Engineer. The Contractor must obtain approval from the Engineer for any deviations from the pouring sequence shown

Concrete in bridge superstructure shall be placed, consolidated and screeded off for the entire length of pour before any concrete has taken its initial set. This may

Concrete diaphragms at end bents shall be poured monolithically with the deck, A minimum of 48 hours shall elapse between the intermediate bent diaphragm pour and the deck slab pour.

require the use of a retarding agent.

- Pouring Pouring Sequence Const. Joint Const Joint Required 91'-0" (Pour 1) 5'-0" 5'-0" (Pour 2) (Pour 2)

ALTERNATE POURING SEQUENCE

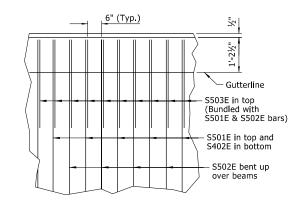
1/16" = 1'-0"

Pours with the same number may be placed simultaneously or separately. Pour (1) must be placed before pours (2) can be placed. 48 hours shall elapse before the end of a pour and the start of the next pour. 72 hours shall elapse between the end of a pour and the start of an adjacent pour. 72 hours shall elapse between the completion of the entire deck and the pouring of the parapet. Any railing pours made before the entire slab unit has been placed must be approved by the Engineer. The Contractor must obtain approval from the Engineer for any deviations from the pouring sequence shown.

Concrete in bridge superstructure shall be placed, consolidated and screeded off for the entire length of pour before any concrete has taken its initial set. This may require the use of a retarding agent.

Concrete diaphragms at end bents shall be poured monolithically with the deck. A minimum of 48 hours shall elapse between the Intermediate bent diaphragm pour and the deck slab pour.

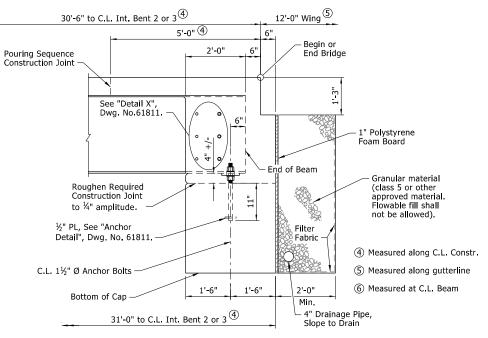
DATE FILMED DATE REVISED STATE DATE REVISED ARK. 6 101009 35 52 JOB NO. 07497 - 100'-0" INT. UNIT - 61812



REINFORCING DETAIL

No Scale

Note: Rails and wings are included in span construction and are included in span guantities.



SECTION AT END BENT

No Scale

Limits of concrete end diaphragm shall match plan dimension of end bent cap.

For additional details of pipe underdrain see Std. Dwg. PU-1 and Section 611. Pipe underdrains will not be measured or paid for separately, but will be considered subsidiary to the unit price bid for "Unclassified Excavation - Bridge"

1" Polystyrene Foam Board, Filter Fabric & Granular Material will not be paid for directly, but shall be considered subsidiary to various bid items.



SHEET 3 OF 6 DETAILS OF 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT DITCH NO. 30

ROUTE SECTION ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARKANSAS

_ FILENAME:B101009X1_SX3.dg DRAWN BY: 04/2020 04/2020 SCALE: As Shown DATE: DESIGNED BY: BRIDGE NO. 61812 DRAWING NO.

Camber for Dead Load Deflection plus Vertical curve +/- ½" tolerances. Deflections shown are along C.L. Beam from the plane perpendicular to the web extending from C.L. Anchor Bolts to C.L. Anchor Bolts. Vertical curve corrections not included. Negative sign (-) indicates upward deflection.

DEAD LOAD DEFLECTION DIAGRAM

TABLE OF DEAD LOAD DEFLECTIONS (INCHES)

Structural

Steel +

Slab+ Rail

0.000

0.050

0.091

0.118

0.126

0.117

0.093

0.059

0.024

0.000

0.000

0.056

0.145

0.233

0.296

0.318

Exterior Beams

Steel +

Slab

0.000

0.039

0.070

0.090

0.098

0.090

0.072

0.045

0.019

0.000

0.000

0.043

0.111

0.178

0.227

0.244

Table is symm, about the C.L. Unit.

0.2

½ Span 2

Structural

Steel

0.000

0.004

0.006

0.008

0.009

0.008

0.007

0.004

0.002

0.000

0.000

0.004

0.010

0.016

0.021

0.022

Structural | Structural

Steel +

Slab+ Rai

0.000

0.041

0.075

0.097

0.105

0.097

0.077

0.049

0.020

0.000

0.000

0.046

0.119

0.192

0.243

0.262

Symm. about C.L. Unit

Interior Beams

Structural

Steel +

Slab

0.000

0.048

0.087

0.112

0.120

0.111

0.089

0.056

0.023

0.000

0.000

0.053

0.137

0.222

0.281

0.303

Point of

Deflection

0.1

0.2

0.3

0.4

0.5

0.6

0.7

0.8

0.9

0.1

0.2

0.3

0.4

0.5

Structura

Stee

0.000

0.004

0.007

0.009

0.009

0.008

0.007

0.004

0.002

0.000

0.000

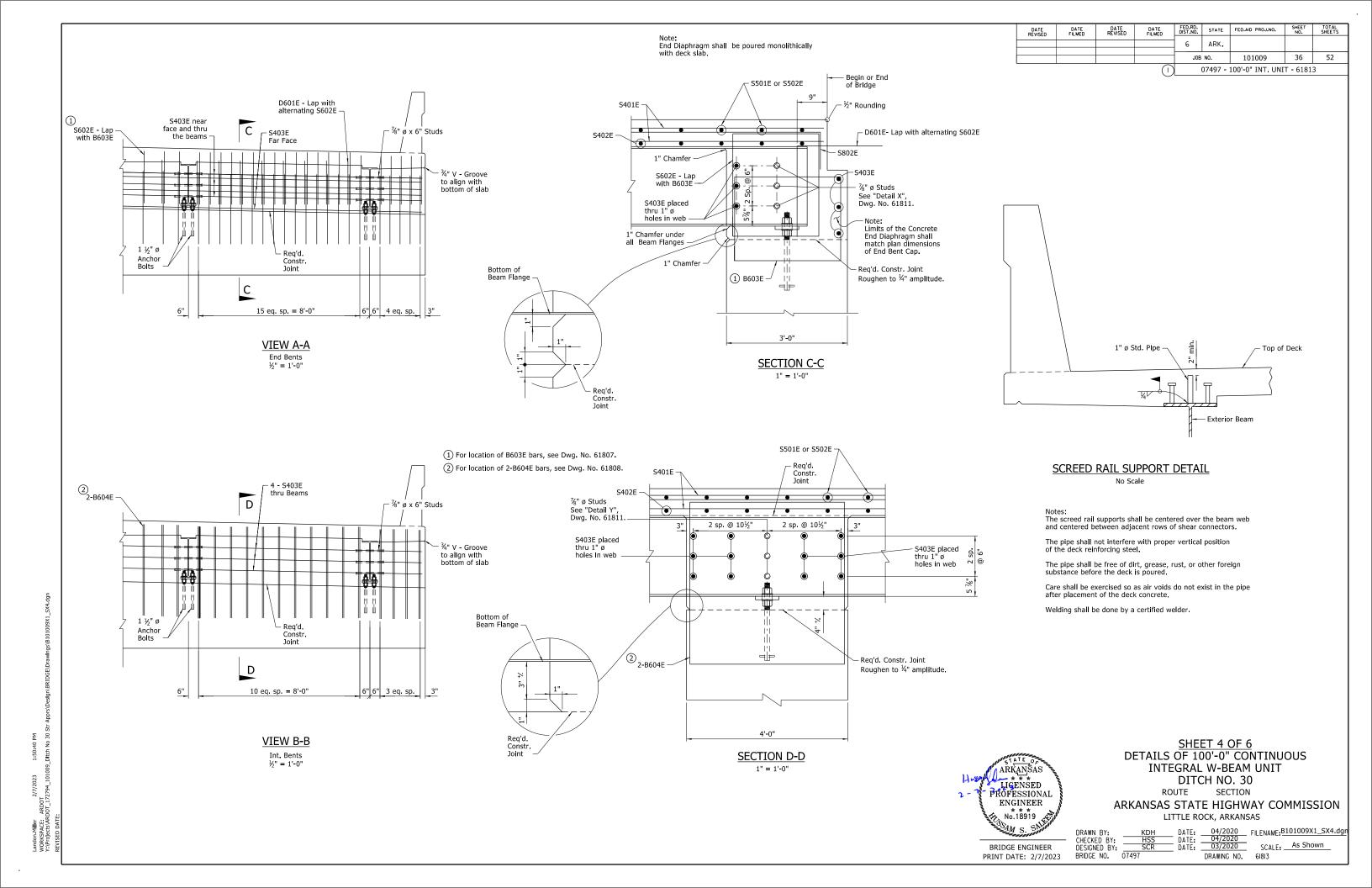
0.004

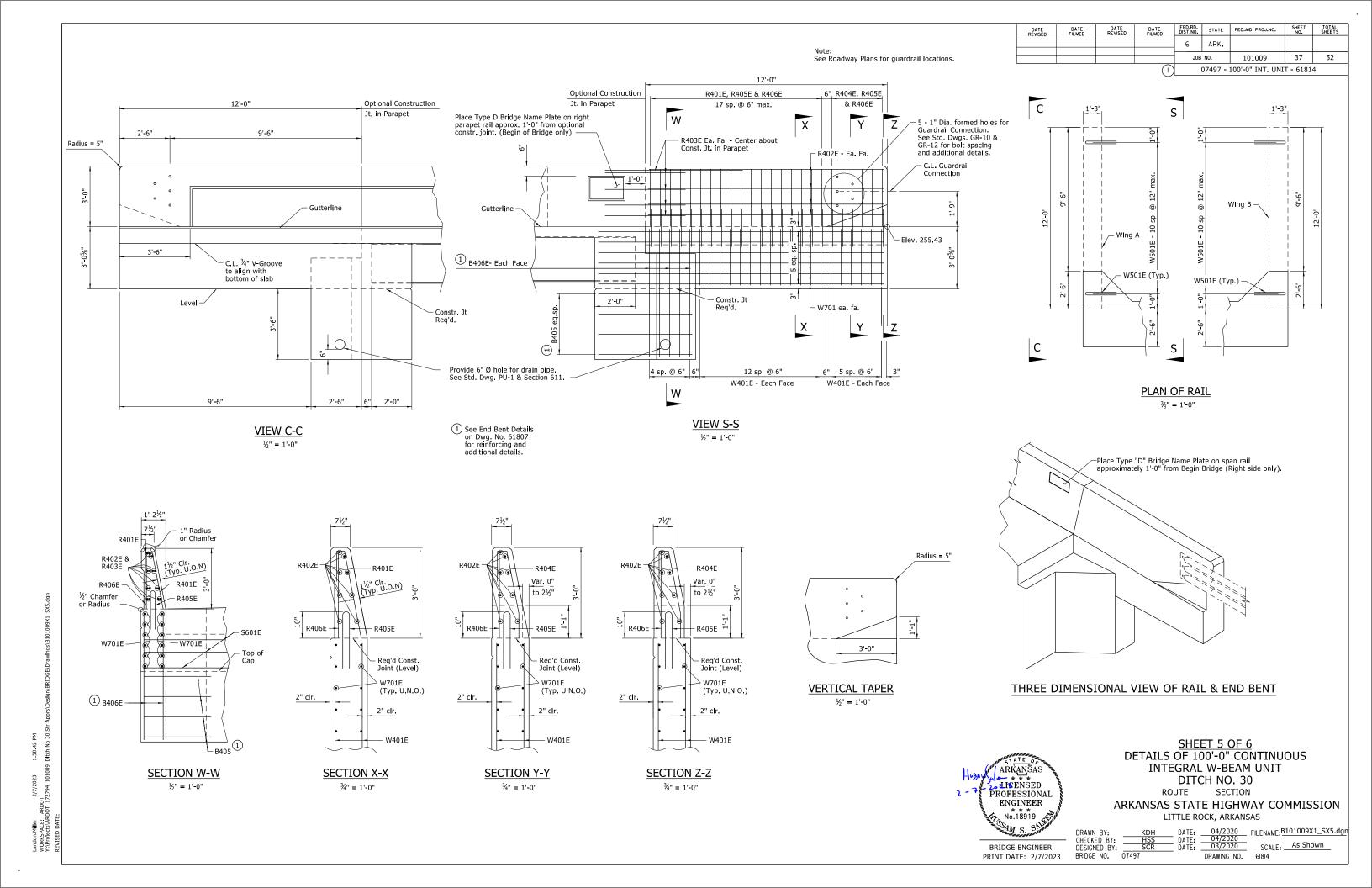
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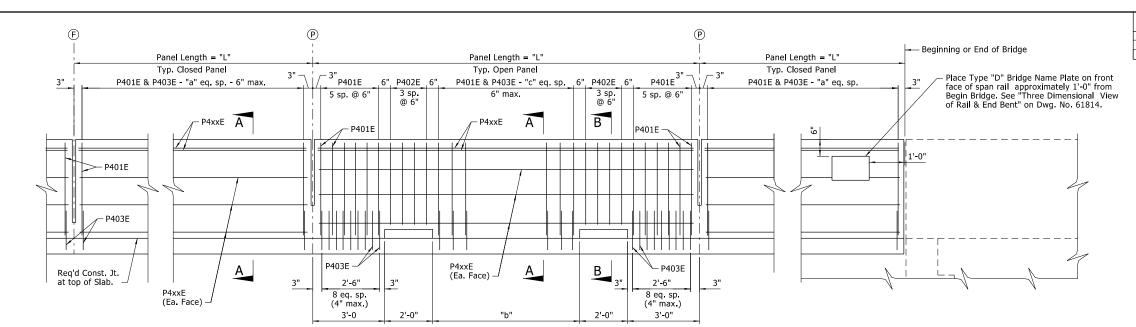
0.017

0.021

0.023



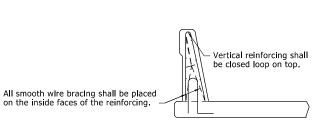




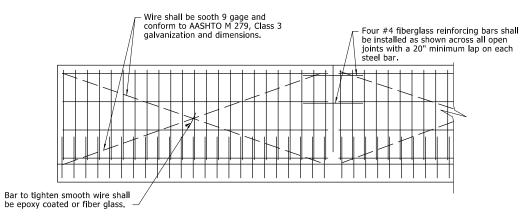
ELEVATION - CONCRETE PARAPET RAIL

No Scale

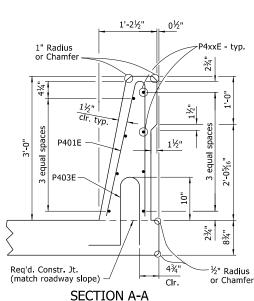
- \bigcirc C.L. $^{1}\!\!4$ " to 1" Full Depth Parapet Joint (Stop 4" from top of slab) (Typ. unless noted otherwise)
- (P) C.L. ¼" to 1" Partlal Depth Parapet Joint (Stop 1'-2" from top of slab) (Typ. unless noted otherwise)



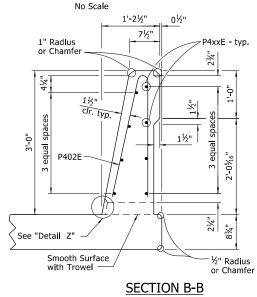
The extruded parapet shall conform to the horizontal and vertical lines shown on the plans or as directed by the Engineer and shall present a smooth, uniform appearance and texture. Exposed surfaces may be given a light brush finish or a Class 3. Textured Coating Finish, in place of the Class 2, Rubbed Finish

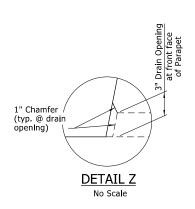


All panels shall be braced as shown to prevent racking. All open joints shall be sawed as soon as



½" = 1'-0"





ARKANŜAS LICENSED PROFESSIONAL ENGINEER No.18919 BRIDGE ENGINEER

SHEET 6 OF 6 DETAILS OF 100'-0" CONTINUOUS INTEGRAL W-BEAM UNIT DITCH NO. 30

ROUTE SECTION

ARKANSAS STATE HIGHWAY COMMISSION LITTLE ROCK, ARKANSAS

DRAWN BY: CHECKED BY: DESIGNED BY:

BRIDGE NO.

DATE: 04/2020 DATE: 04/2020 DATE: 03/2020 _ FILENAME:B101009X1_SX6.dgr SCALE: As Shown DRAWING NO. 61815

PRINT DATE: 2/7/2023

1/2" = 1'-0"

practical to a minimum width of $\frac{1}{4}$ ". To control cracking before sawing, all joints must be grooved before the concrete is set. Sawing of the joints must be controlled so it will follow the grooved joint. DETAILS OF OPTIONAL SLIPFORMING OF CONCRETE PARAPET RAIL

Panel P4xxE Length "L" "a" Bars Type 8'-0" 15 P404E closed

P405E 9'-6" closed 24 22'-5½" 12'-5%" P406E open 24 22'-6½" 12'-6½" P407E open 10'-11¾" 21 P408E 20'-11%" open 21 P409E 21'-0%" 11'-0%"

PARAPET RAIL VARIABLES

STATE

ARK.

101009

07497 - 100'-0" INT. UNIT - 61815

JOB NO.

52

38

DATE REVISED

DATE FILMED

For location of full and partial depth parapet joints, see Dwg. No. 61812.

For location of open and closed parapet panels, See

BAR LIST FOR ONE TYPE SPECIAL APPROACH SLAB

MARK	NO REQ'D	LENGTH	PIN DIA.
S401	31	21'-8''	Str.
S402	44	2'-8''	Str.
S403	44	3'-0''	Str.
S501	44	32'-8''	Str.

GENERAL NOTES

Concrete shall be Class S(AE) (f'c = 4,000 psi).

Reinforcing Steel shall conform to AASHTO M31 or M322, Type A with Mill Test Reports, Gr. 60 (fy = 60,000 psi).

Approach Slabs will be measured and paid for in accordance with Section 504 of the Standard Specifications.

Surface finish for Approach Slabs to match that used on the bridge

32'-11%" 16'-6" S403 Dowels Longitudinal 3'-0" at 18" sp. Constr. Joint Υ X S401 - 21 sp. @ 18" o.c. in bottom 16'-6" C.L. Bridge & C.L. Const. Is on a 0°30'00" curve left Required S401 in botton Sawed Joint В End Bridge S401 in footing (typ.)-S402 @ 12" o.c. in footing (typ.)-Υ S403 Dowels at 18" sp. Longitudinal Constr. Joint 16'-6" 33'-0¾"

QUANTITIES FOR ONE

TYPE SPECIAL APPROACH SLAB

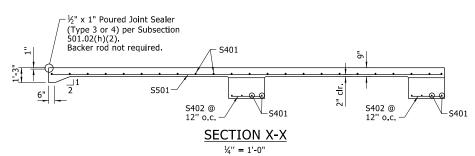
1 Longitudinal Sawed Jt. (Place as a continuation

of the roadway longitudinal

Slab Width	Reinforcing Steel	Concrete	
wiath	(lbs.)	(cu. yds.)	
22'-0"	2,115	29.50	

All transverse reinforcing shall be placed on radial lines.

All longitudinal lines and reinforcing steel shall be placed on curves concentric with C.L. Bridge & C.L. Construction.

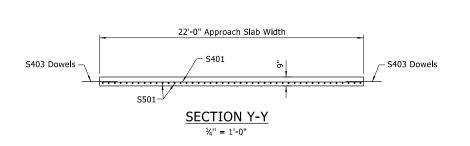


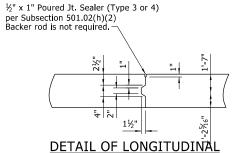
PLAN

(Shown at End Bridge,

Begin Bridge similar)

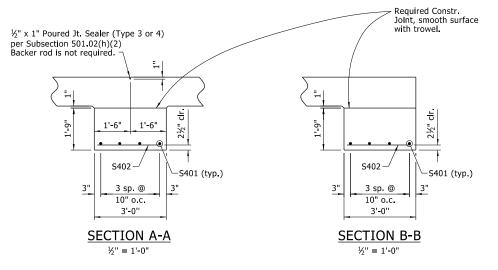
1/4" = 1'-0"





DETAIL OF LONGITUDINAL **CONSTRUCTION JOINT**

No Scale



ARKANSAS *** BICENSED PROFESSIONAL ENGINEER No.18919 BRIDGE ENGINEER

PRINT DATE: 2/7/2023

DETAILS OF TYPE SPECIAL APPROACH SLAB DITCH NO. 30

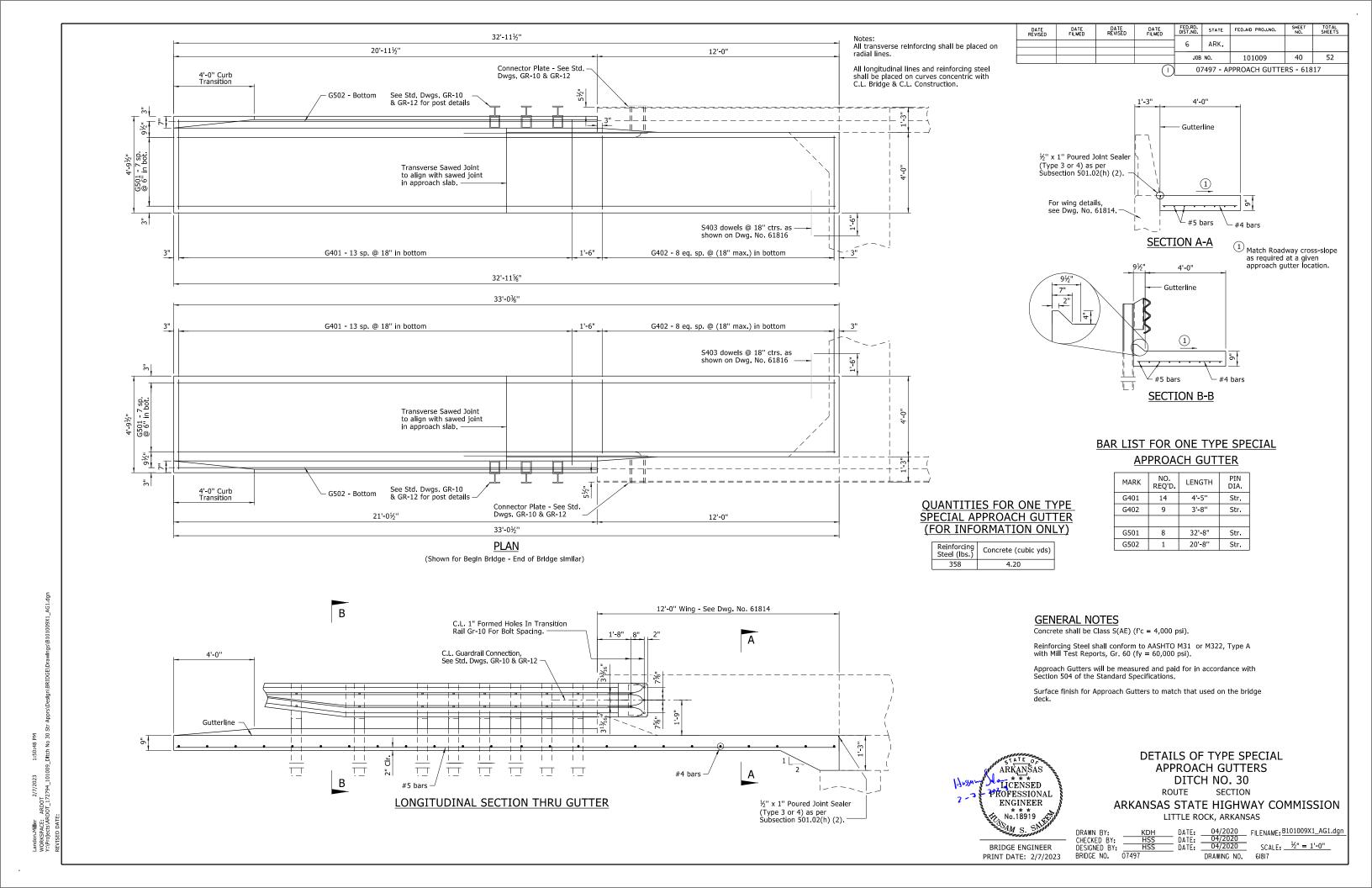
SECTION ROUTE

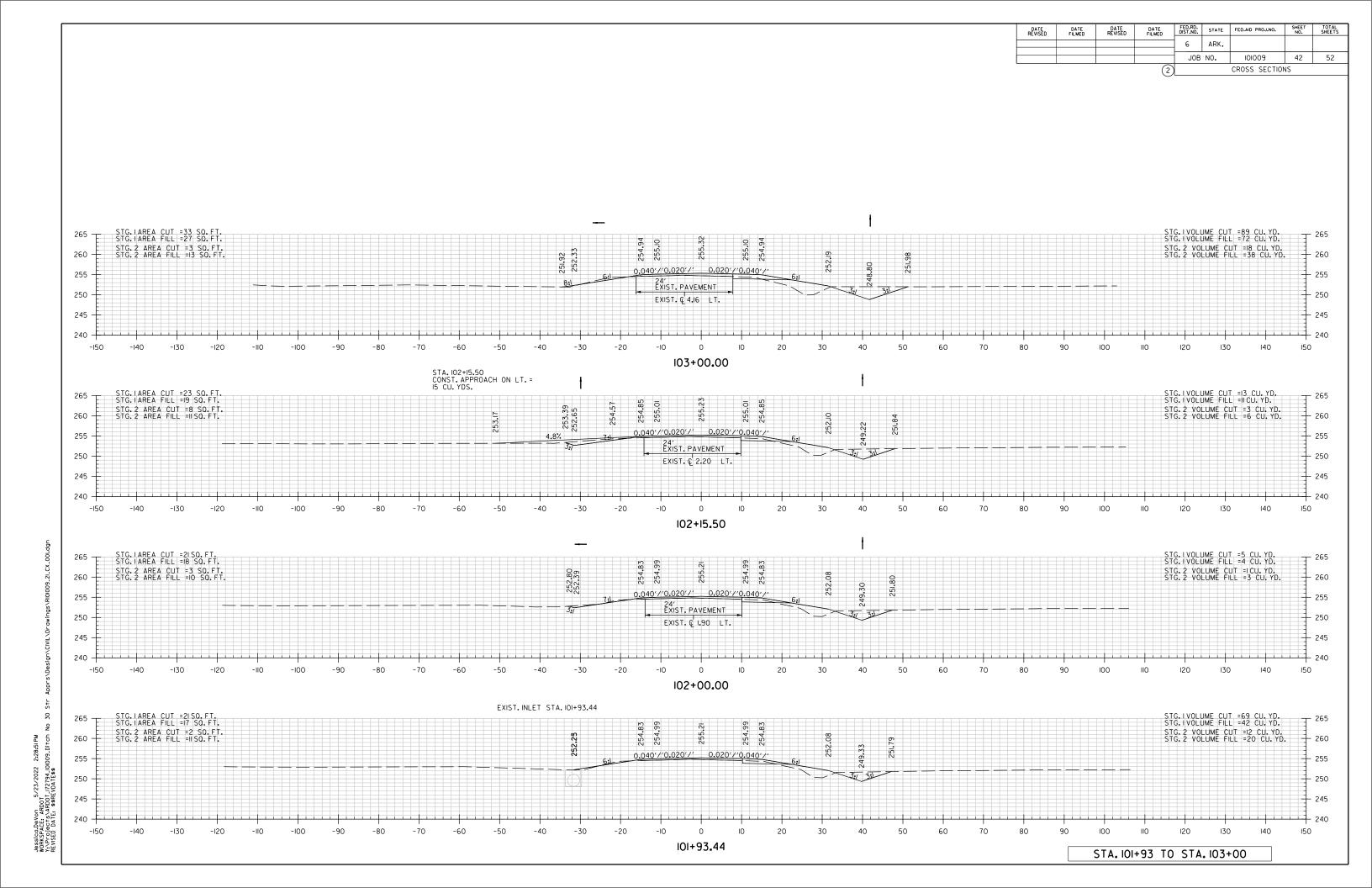
ARKANSAS STATE HIGHWAY COMMISSION

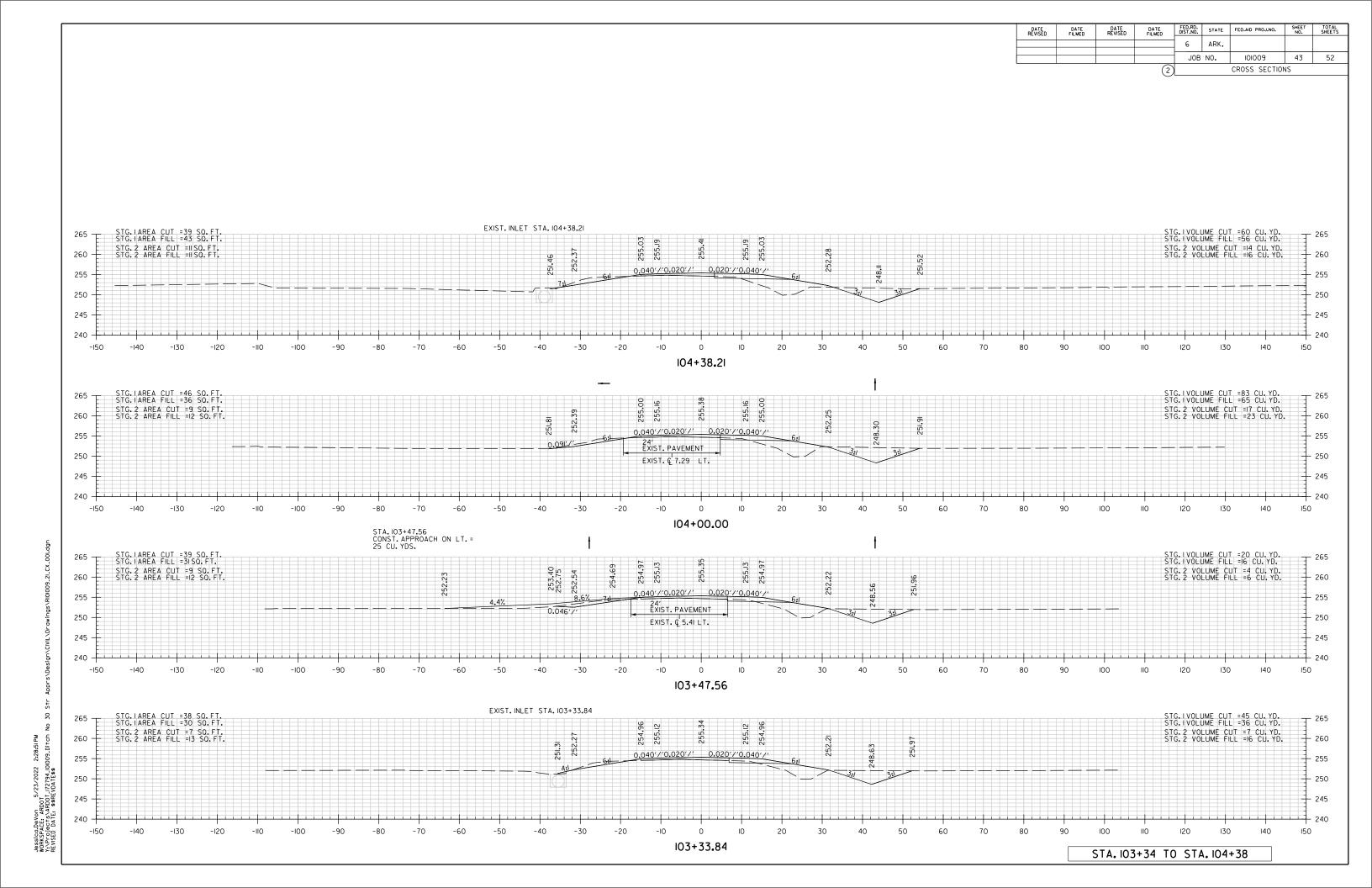
LITTLE ROCK, ARKANSAS

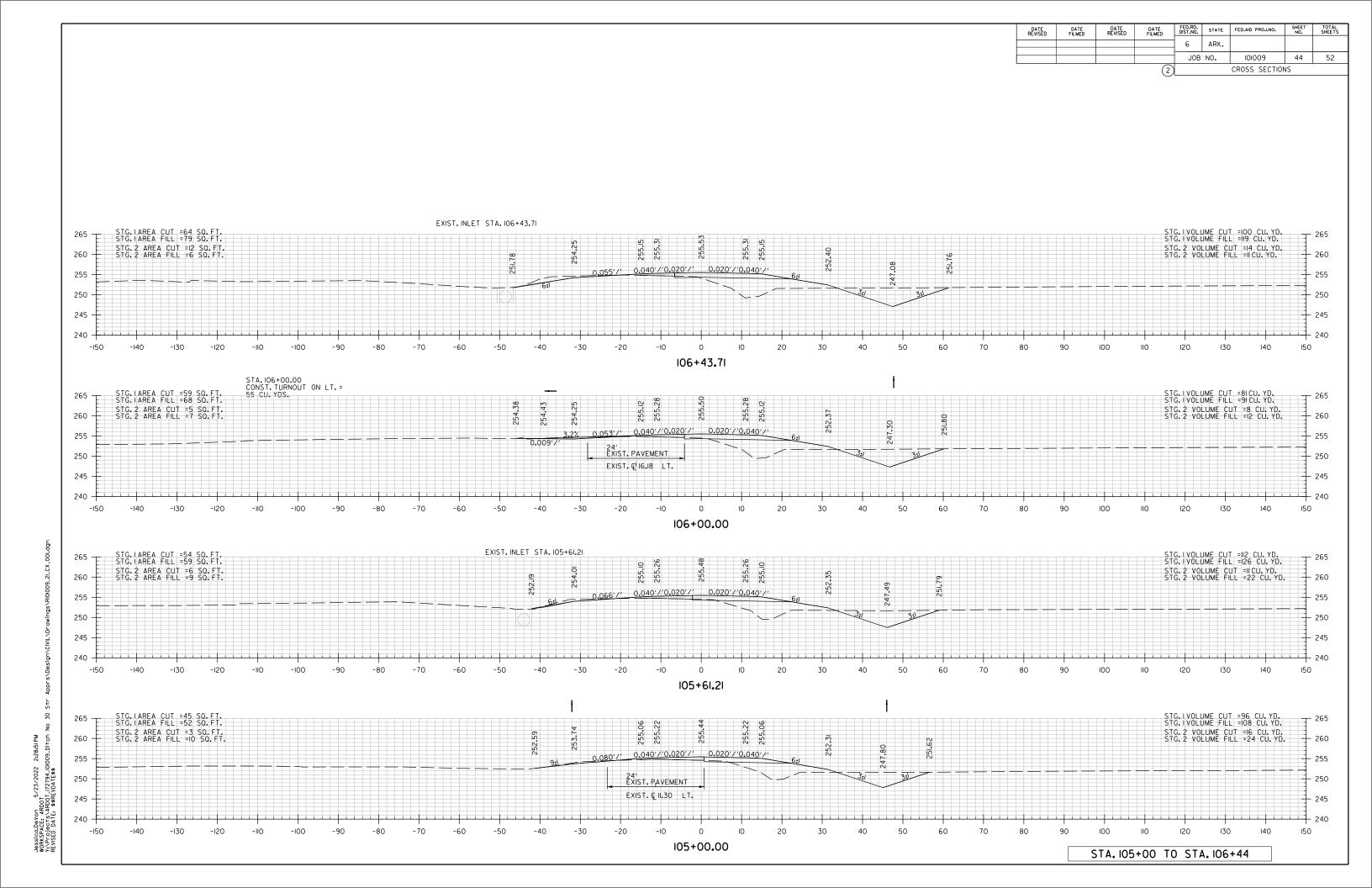
DRAWN BY: CHECKED BY: DESIGNED BY: BRIDGE NO.

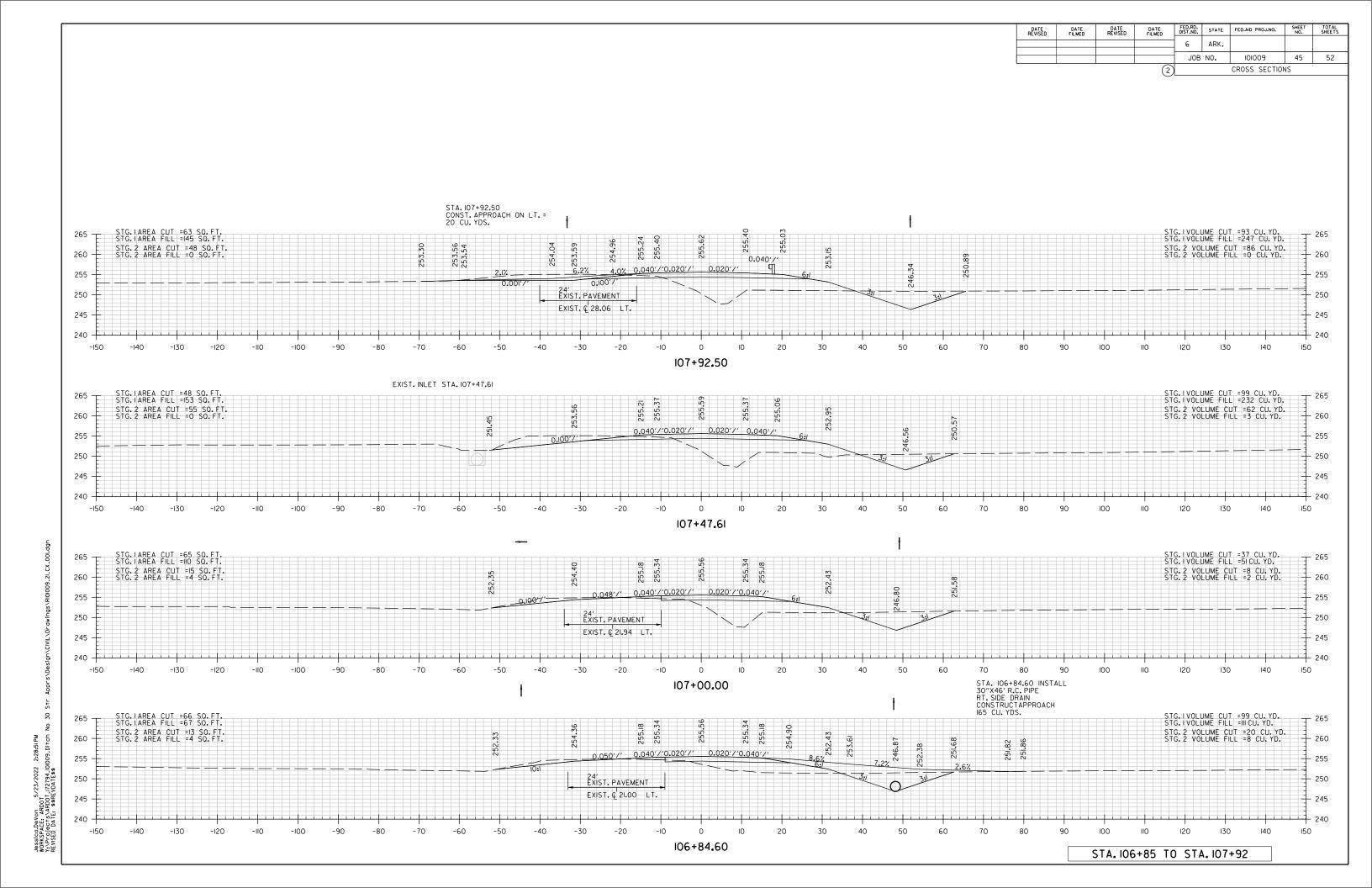
DATE: 03/2020 DATE: 04/2020 DATE: 04/2020 _ FILENAME: B101009X1_AS.dgn SCALE: As shown DRAWING NO. 61816

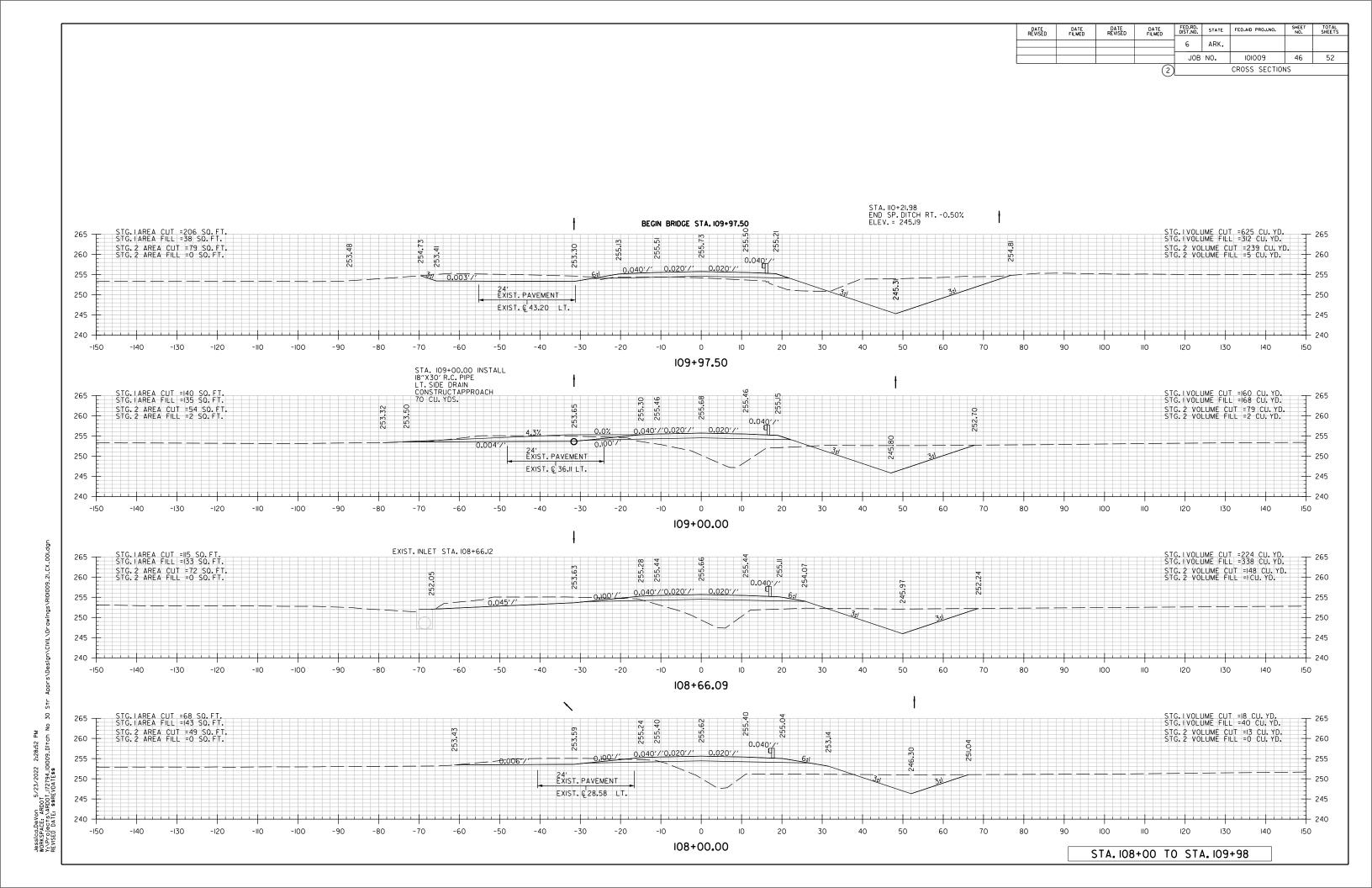


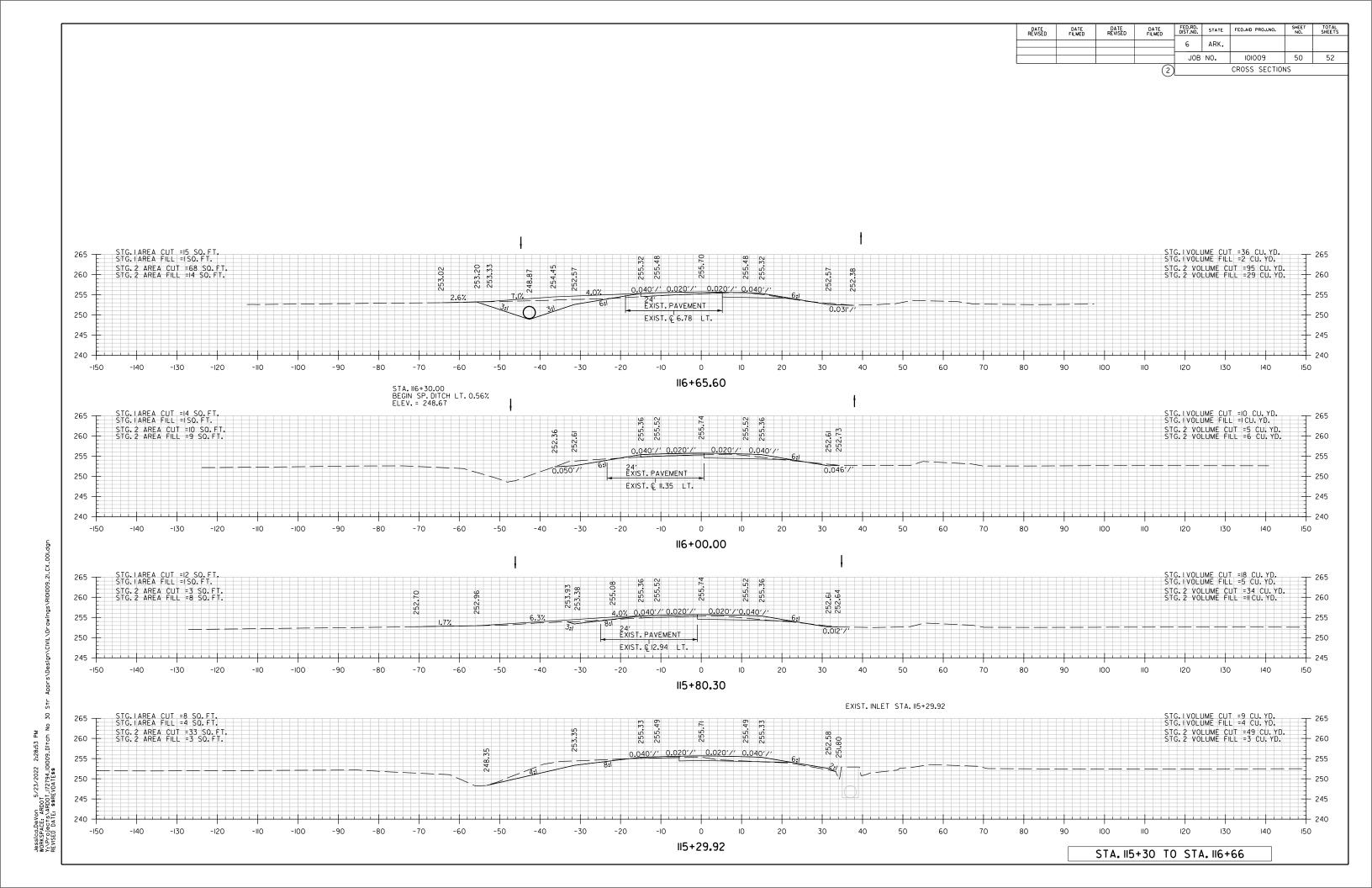


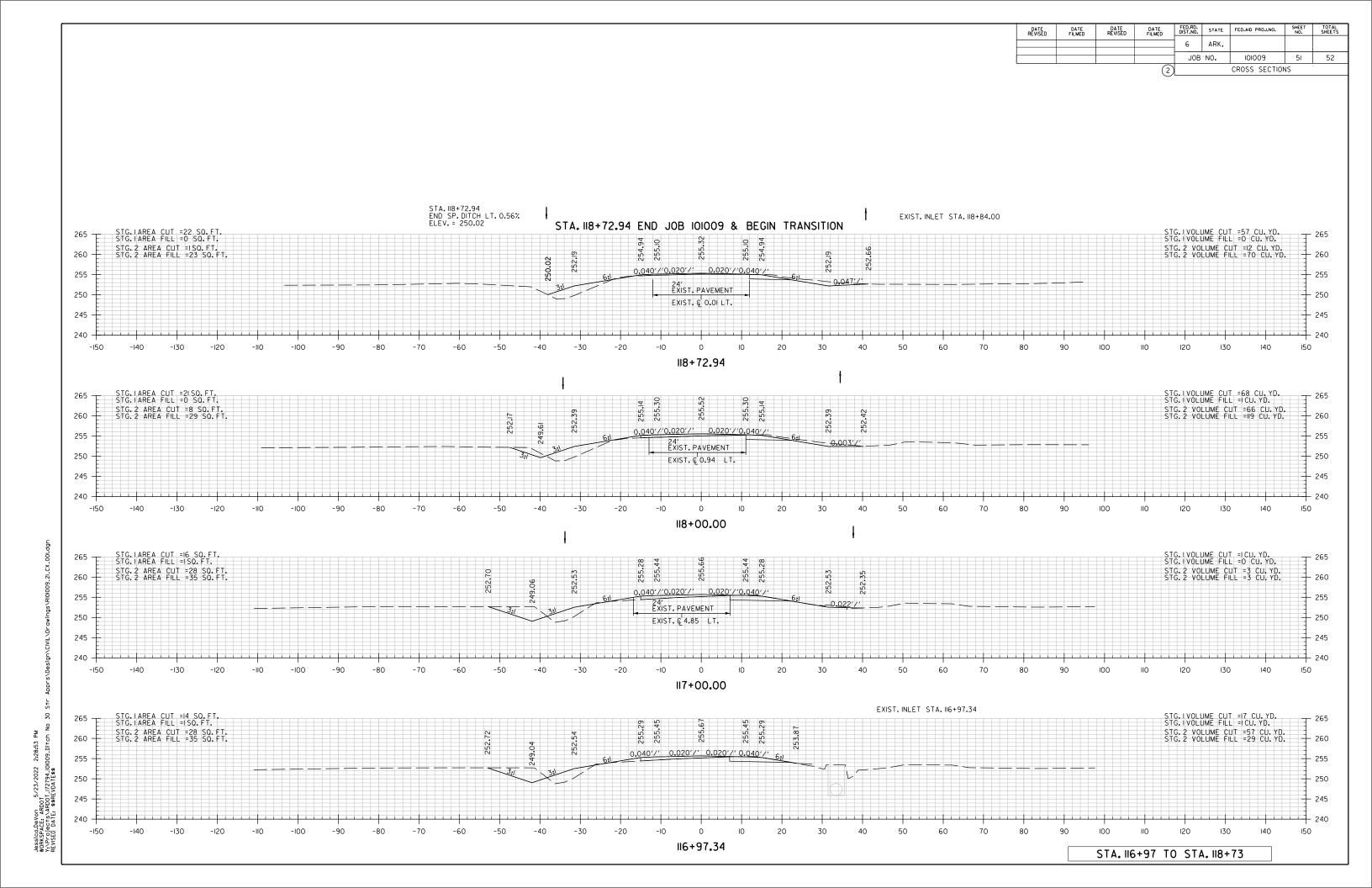












STA. II9+72.94 END IOO' TRANSITION STG. IVOLUME CUT =40 CU. YD. STG. IVOLUME FILL =0 CU. YD. STG. 2 VOLUME CUT =2 CU. YD. STG. 2 VOLUME FILL =43 CU. YD. - 260 255 -255 EXIST. Q 0.00 - 250 119+72.94 STA. II9+73 TO STA. II9+73

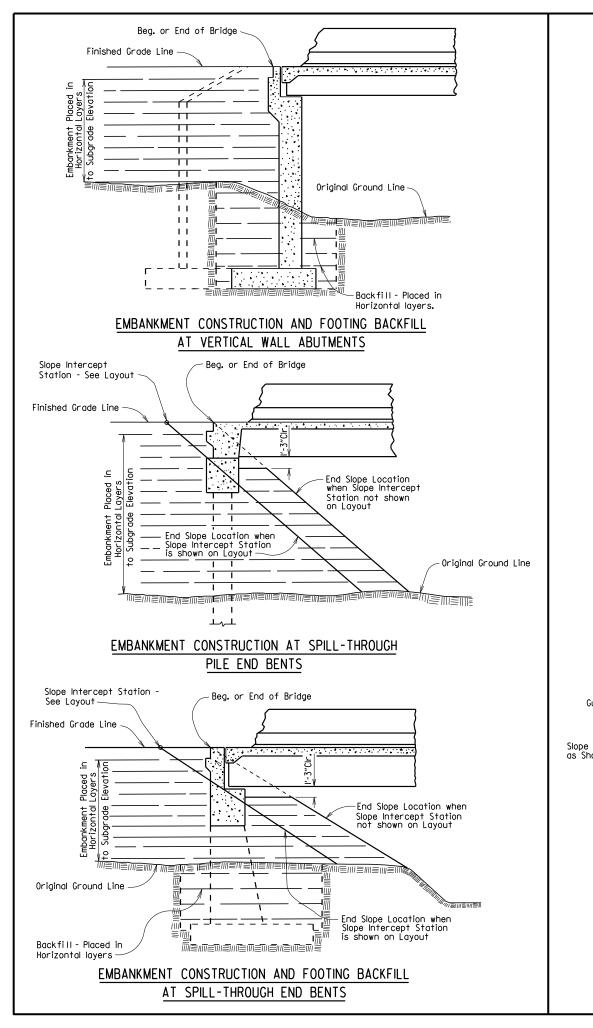
DATE REVISED

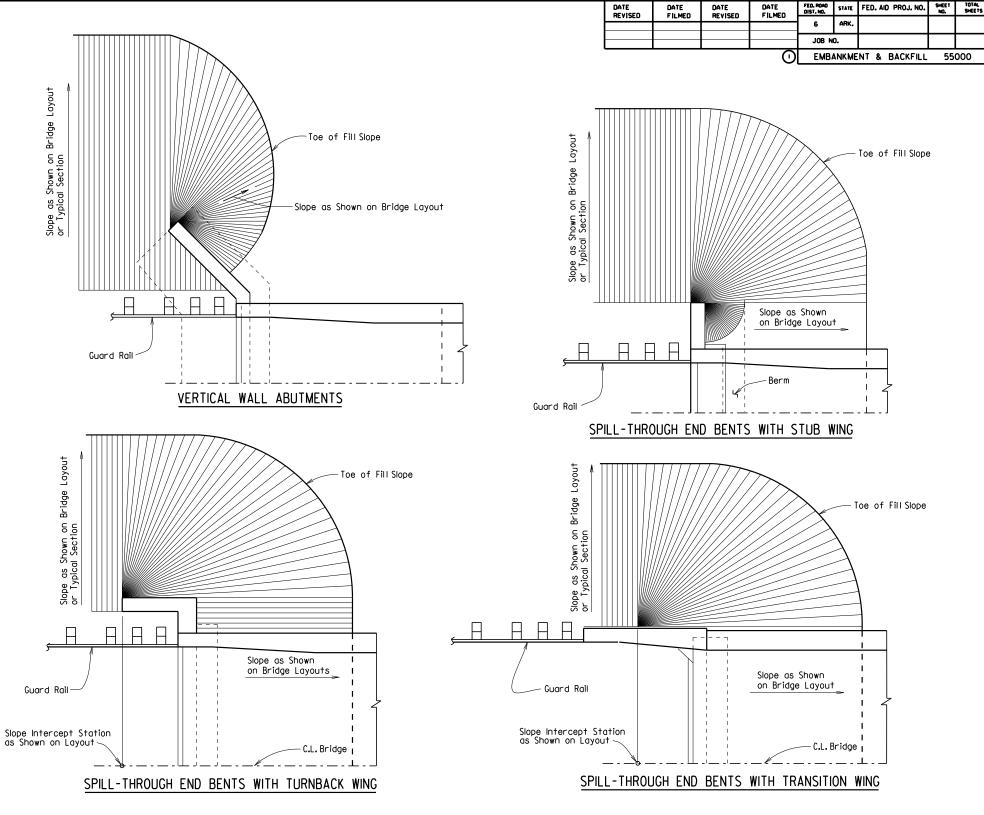
FED.RD. DIST.NO. STATE FED.AID PROJ.NO. ARK. JOB NO.

101009

CROSS SECTIONS

52 52





METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 6 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to Subsections 210.09, 210.10 and 801.08 for construction requirements.

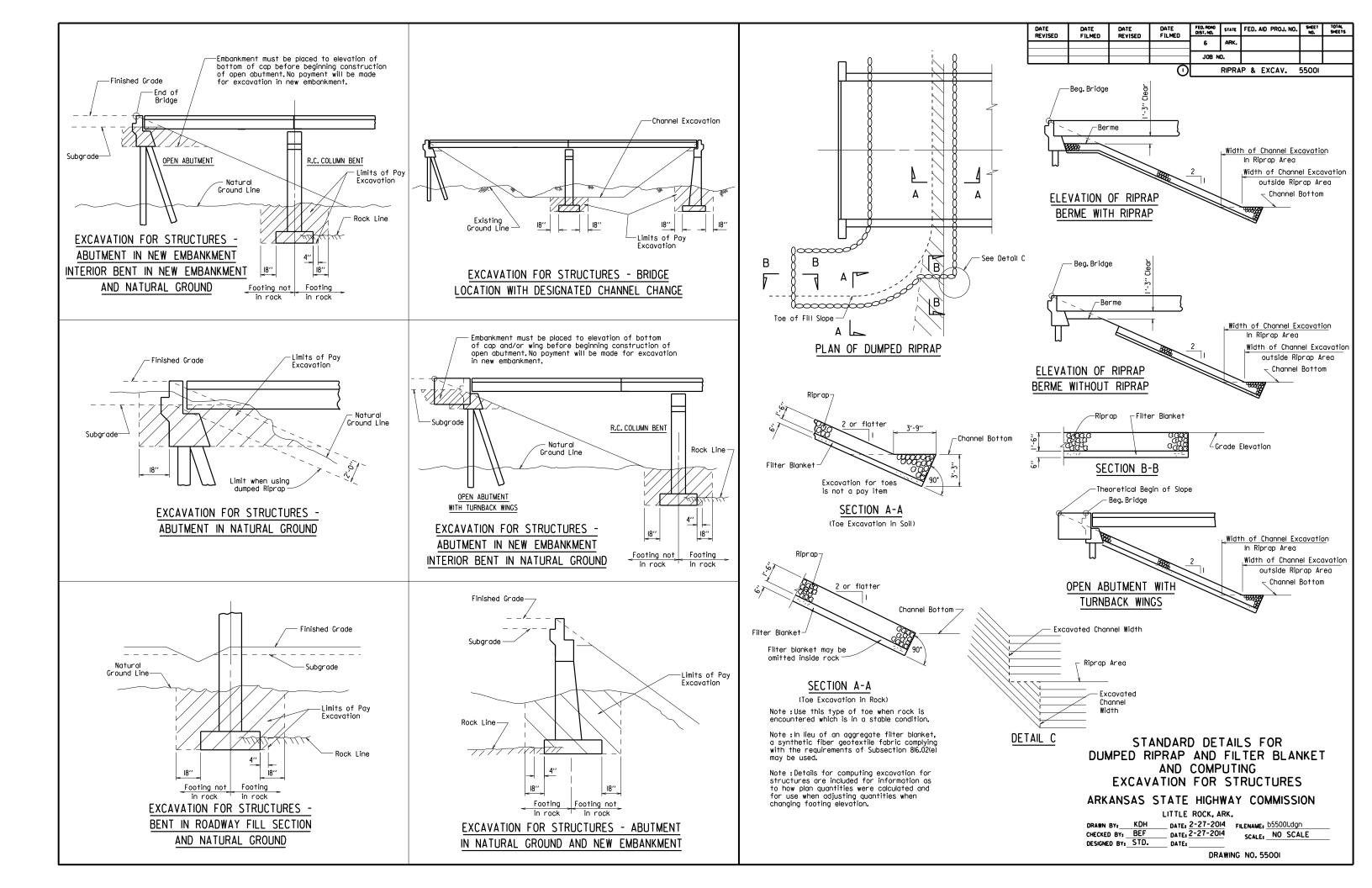
STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

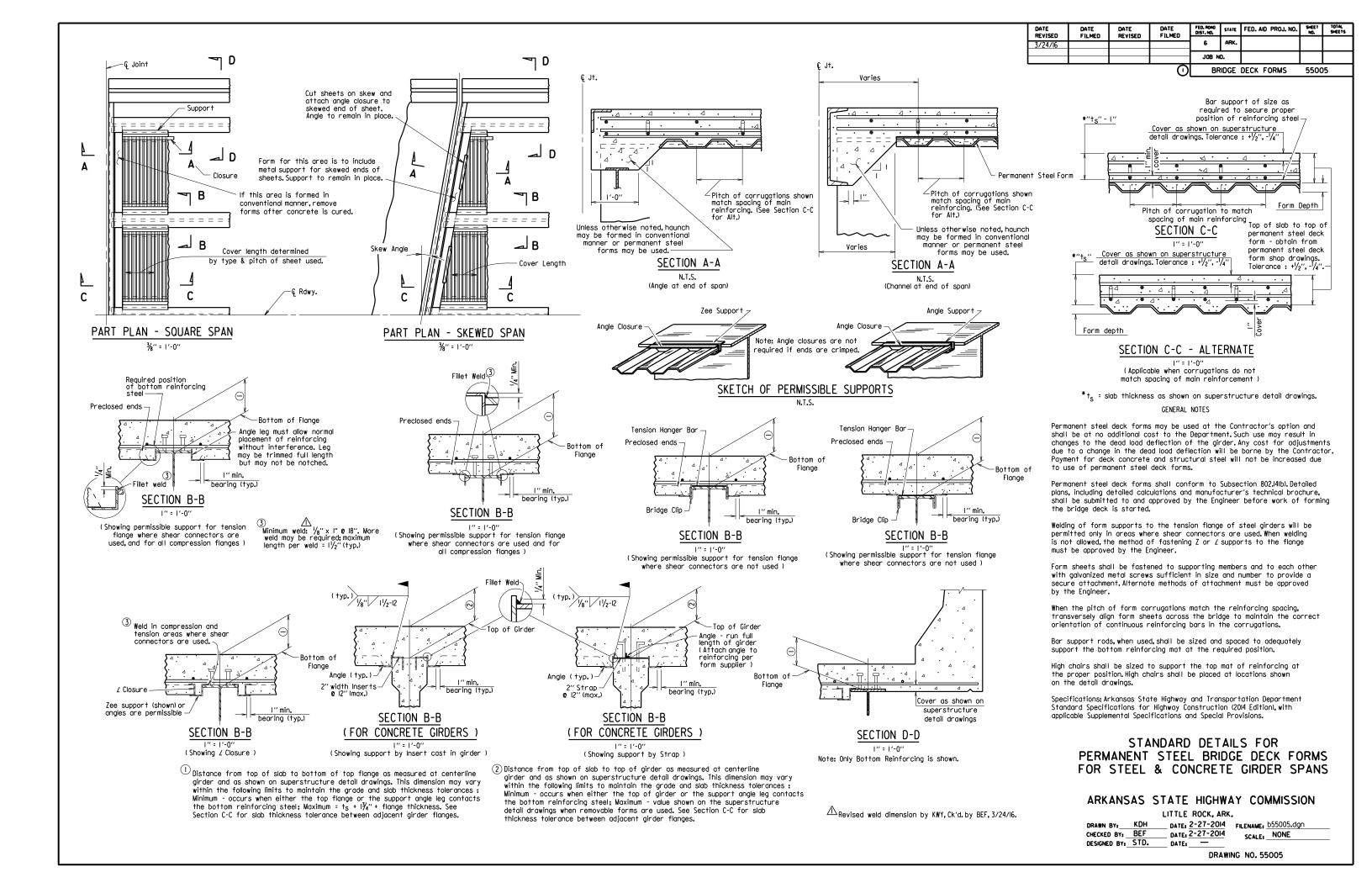
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK. KDH

DATE: 2-27-2014 FILENAME: <u>b55000.dgn</u> DRAWN BY:__ CHECKED BY: BEF
DESIGNED BY: STD. SCALE: NO SCALE DATE: 2-27-2014

DRAWING NO. 55000





GENERAL NOTES

These GENERAL NOTES are applicable unless otherwise shown in the Plan Details, Special Provisions, or Supplemental Specifications.

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 Edition) with applicable Supplemental Specifications and Special Provisions. Section and Subsection refer to the Standard Specifications.

DESIGN SPECIFICATIONS: See Bridge Layout(s).

SUPERSTRUCTURE NOTES:

MATERIALS AND STRENGTHS:

Class S(AE) Concrete	fʻc	=	4 , 000 psi
Reinforcing Steel (Gr. 60, AASHTO M 31 or M 322, Type A)	fy	=	60,000 psi
Structural Steel (AASHTO M 270, Gr. 36)	Fy	=	36,000 psi
Structural Steel (AASHTO M 270, Gr. 50)	Fy	Ξ	50,000 psi
Structural Steel (AASHTO M 270, Gr. 50W)	Fy	=	50,000 psi
Structural Steel (AASHTO M 270 Gr HPS70W)	Fy	=	70,000 psi

See Plan Details for Grade(s) of Structural Steel required.

CONCRETE:

All concrete shall be Class S(AE) with a minimum 28 day compressive strength f'c = 4,000 psi. Concrete shall be poured in the dry and all exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

The superstructure details shown are for use when removable deck forming is used and are the basis for measurement of Class S(AE) Concrete. See Standard Drawing No. 55005 for allowable modifications and for tolerances when Permanent Steel Bridge Deck Forms are used.

Use of a longitudinal screed is not permitted on any span of a bridge deck with horizontal curvature.

The concrete deck (roadway surface) shall be given a tine finish in accordance with Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Sidewalks shall receive a broomed finish as specified for final finishing in Subsection 802.19 for Class 6 Broomed Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour. Sufficient concrete must be placed ahead of the strike-off to fully load the beam or girder. When permitted, the use of a longitudinal strike-off will require that a vertical camber adjustment be made in the strike-off to account for the future dead load deflection due to any railings, median barrier, and sidewalks.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A, with mill test reports and shall be epoxy coated. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Epoxy Coated Reinforcing Steel (Grade 60)".

STRUCTURAL STEEL (COMMON TO W-BEAMS AND PLATE GIRDERS):

Structural steel shall be AASHTO M 270 with grade and payment as specified in the plans. Grade 50W steel shall not be painted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84(e). Grade 36 and Grade 50 steel shall be painted unless otherwise noted and all exposed surfaces shall be cleaned in accordance with Subsection 807.84. Structural steel completely embedded in concrete may be AASHTO M 270, Gr. 36, Gr. 50 or Gr. 50W unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with the specifications, submitted and approval secured before fabrication is begun.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval; however, additional welds used for attaching falsework support devices or screed rail supports to the structural steel that do not exceed the limitations of Subsection 802.13 will not require approval prior to construction. All welding shall conform to Subsection 807.26.

Unless otherwise noted, field connections shall be bolted with $\frac{3}{4}$ " Ø high-strength bolts using $\frac{13}{6}$ " Ø open holes. Holes for $\frac{3}{4}$ " Ø high-strength bolts may be $\frac{15}{6}$ " Ø if a washer is supplied for use under both the nut and head of the bolt. The use of oversized holes will not be allowed on main members unless otherwise noted. Bolts shall be placed with heads on the outside face of the exterior beam or girder webs and on the bottom of the beam or girder flanges.

All stud shear connectors shall be granular flux filled, solid fluxed, or equal and shall be automatically end welded in accordance with recommendations of the Manufacturer.

When painting is required, all structural steel except galvanized steel and steel completely encased in concrete shall be painted in accordance with Subsection 807.75. The color of paint shall be as specified in the plans.

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				100.1	<u> </u>			
				JOB N	u.			
			\odot			GENERAL NOTES	55	5006

STRUCTURAL STEEL (W-BEAMS):

All beams and field splice plates, and all diaphragms and connection plates attached to horizontally curved beams are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Beam Spans (M 270, Gr.___)".

All beams in continuous units and simple spans with field splices shall be blocked in their true position in the shop in groups as specified in Subsection 807.54(b)(2) with the webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All beams in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Flange field splice plates shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

All beam dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for camber.

Bent plate diaphragms for horizontally curved beams shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight beams may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved beams.

Unless otherwise noted, diaphragms shall be installed as beams are erected. All bolts in diaphragms and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

STRUCTURAL STEEL (PLATE GIRDERS):

All references to cross-frames shall include "X" or "K" types.

All girder web and flange plates, all field splice plates, and all diaphragms, cross-frames and connection plates attached to horizontally curved girders are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr.___)".

All girders in continuous units and simple spans with field splices shall be assembled in the shop as specified in Subsection 807.54(b)(2) and blocked in their true position with webs horizontal. The camber, length of sections, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram.

All girders in simple spans without field splices shall be blocked in their true position with webs horizontal. The camber, distance between bearings, and openings of joints shall be measured and this information shall become part of the permanent records.

Web and flange plates for main members and flange splice plates for main members shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

Girder webs may be made by shop splicing with minimum lengths of 25 feet for sections. Flange plates longer than 50 feet may be made by shop splicing with minimum lengths of 25 feet for sections. No additional payment will be made for shop welded splices.

All girder dimensions are based on a temperature of 60 degrees F. A tolerance of $^{1}\!/_{4}"$ +/- is allowed for camber.

Groove welds in web and flange plates shall be Quality Control (Q.C.) tested by nondestructive testing, as required in Subsection 807.23(b). Fillet welds at flange to web plate connections shall be Q.C. tested by the magnetic particle method. All Q.C. testing shall be considered subsidiary to the item "Structural Steel in Plate Girder Spans (M 270, Gr,...)".

Bent plate diaphragms for horizontally curved girders shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses. Bent plate diaphragms for straight girders may be cut and fabricated in accordance with Subsection 807.35 or as required for horizontally curved girders.

Unless otherwise noted, cross-frames and diaphragms shall be installed as girders are erected. All bolts in cross-frames, diaphragms, and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

SUBSTRUCTURE NOTES:

CONCRETE:

Unless otherwise noted, concrete in caps, columns and footings (except seal footings) shall be Class "S" with a minimum 28 day compressive strength f'c = 3,500 psi and shall be poured in the dry. Seal Concrete for footings shall have a minimum 28 day compressive strength f'c = 2,100 psi.

Concrete in drilled shafts shall be Class "S" as modified by Job SP "Drilled Shaft Foundations".

All exposed corners shall be chamfered $\frac{3}{4}$ " unless otherwise noted.

REINFORCING STEEL:

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322, Type A, with mill test reports.

Top reinforcing bars in cap shall be properly placed to avoid interference with anchor bolts or sheet metal sleeves.

STRUCTURAL STEEL:

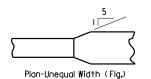
Structural steel in end bents shall be AASHTO M 270 with grade and payment as specified in the plans

FOR ADDITIONAL INFORMATION AND NOTES, SEE LAYOUT(S) AND PLAN DETAILS.

STANDARD GENERAL NOTES FOR STEEL BRIDGE STRUCTURES

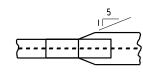
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWING NO. 55006

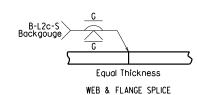


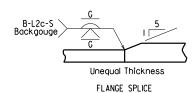
FLANGE SPLICE

Plate Girder Spans (____)".

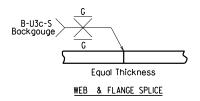


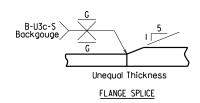
FLANGE SPLICE AT UNEQUAL BOTTOM FLANGE WIDTHS





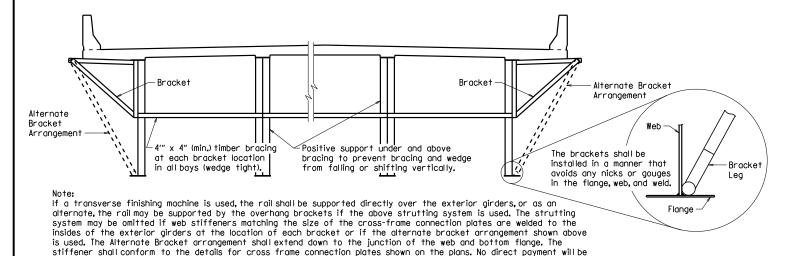
(Use when Base Metal Thickness is Equal to or Less than 2")





(Use when Base Metal Thickness is Greater than 2")

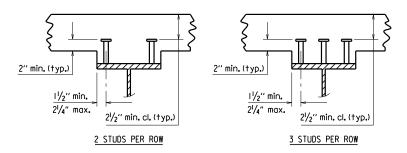
DETAILS OF WELDED SPLICES FOR PLATE GIRDERS



SCREED RAIL SUPPORT FOR PLATE GIRDERS

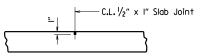
(USE WHEN WEB DEPTHS ARE 48" OR GREATER)

made for brackets, timber bracing, supports, or welded stiffeners. Payment shall be subsidiary to "Structural Steel in



Stud Shear Connectors shall be automatically end welded to the beam or girder flange in accordance with the recommendations of the Manufacturer. See plan details for number and size.

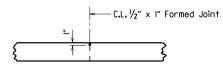
SHEAR CONNECTOR DETAIL



Use Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint Sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. Slab Joints shall extend to the outside edge of the deck slab and shall align with open joints at the front face of the parapet. Slab joints shall be installed before the parapet railing is poured. If slab joints are to be sawed, they shall be sawed as soon as the concrete has sufficiently set to allow sawing of the joint without damage to the slab. Slab joints shall be placed at all pouring sequence construction joints and required slab joint locations. The joint sealer shall extend across the deck from gutterline to gutterline.

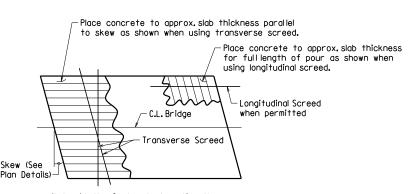
ADDITIONAL NOTES IF SIDEWALKS OR RAISED MEDIANS ARE REQUIRED: Slob Joints shall be installed before the sidewalk or raised median is poured. After installation of the joint in the sidewalk or raised median and prior to pouring the porapet rail, the joint sealer shall be placed extending across the deck slab from gutterline to gutterline and acrosss the top of the sidwalk or raised median to the edge of the slab. No joint sealer shall be placed on the deck slab under the sidewalk or raised median.

TRANSVERSE SLAB JOINT DETAIL



Use $\frac{1}{2}$ " x I" Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint sealer shall be measured and paid for as Class S(AE) Concrete-Bridge. This joint shall be formed. Seal color shall be gray or other color similar to concrete.

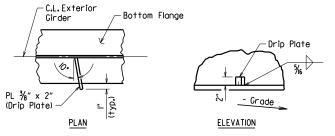
LONGITUDINAL CONSTRUCTION JOINT



Note: At the Contractor's option, the transverse screed may be placed parallel to the skew or perpendicular to C.L. Bridge.

CONCRETE PLACEMENT PROCEDURE

FOR BRIDGES WITH SKEW



Drip Plate to be welded to the outer side of the bottom flange of the exterior girders.

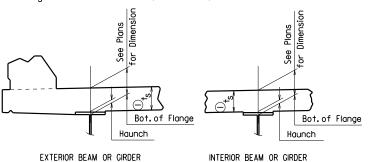
Locate drip plate 5'-0" from C.L. Bearing on high side of each Bent, unless otherwise noted in the plans.

BOTTOM FLANGE DRIP PLATE

(USE WHEN WEB DEPTHS ARE 54" OR GREATER AND UNIT OR SPAN IS NOT IN LEVEL GRADE)

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FEO. ROAO DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
NEVISED	FILMED	REVISED		6	ARK.			
				JOB N	0.			
		-	$\overline{\Box}$		STE	EL RRINCE STRUCTI	IDES	55007

 $t_{\rm S}$ = slab thickness. See "Typical Roadway Section" in the plans.



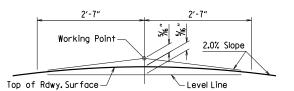
Tolerance when removable deck forming is used is + $\frac{1}{2}$ ", - $\frac{1}{4}$ ". Haunch forming is required and shall be adjusted to maintain slab thickness tolerance.

NOTES:

Haunch dimension may vary within the following limits to maintain the grade and slab thickness tolerance: Minimum occurs when top flange contacts bottom reinforcing steel; Maximum = top flange thickness plus $1\frac{3}{4}$ " unless otherwise noted in the plans. No increase in concrete and structural steel quantities will be made to maintain tolerances.

Tolerances shown are applicable only when removable deck forming is used. See Std. Dwg. No. 55005 for tolerances when permanent steel deck forms are used. Payment for concrete shall be based on removable deck forming.

ADJUSTMENT FOR SLAB THICKNESS TOLERANCE



NOTE: Working Point matches Theoretical Roadway Grade.

ROUNDING DETAIL BRIDGES IN NORMAL CROWN

WELD TABLE

Material Thickness of Thicker Part Joined (Inches)	Minimum Size of Fillet Weld (Inches)	Single Pass Weld Must
To ¾" Inclusive	1/4"	Be
0ver ¾′′	%6′′	Used

NOTE: When a fillet weld size, as shown on the plans, is larger than the minimum, the first pass shall be that specified for minimum size of fillet weld.

SECTION AND SUBSECTION REFER TO THE ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2014 EDITION).

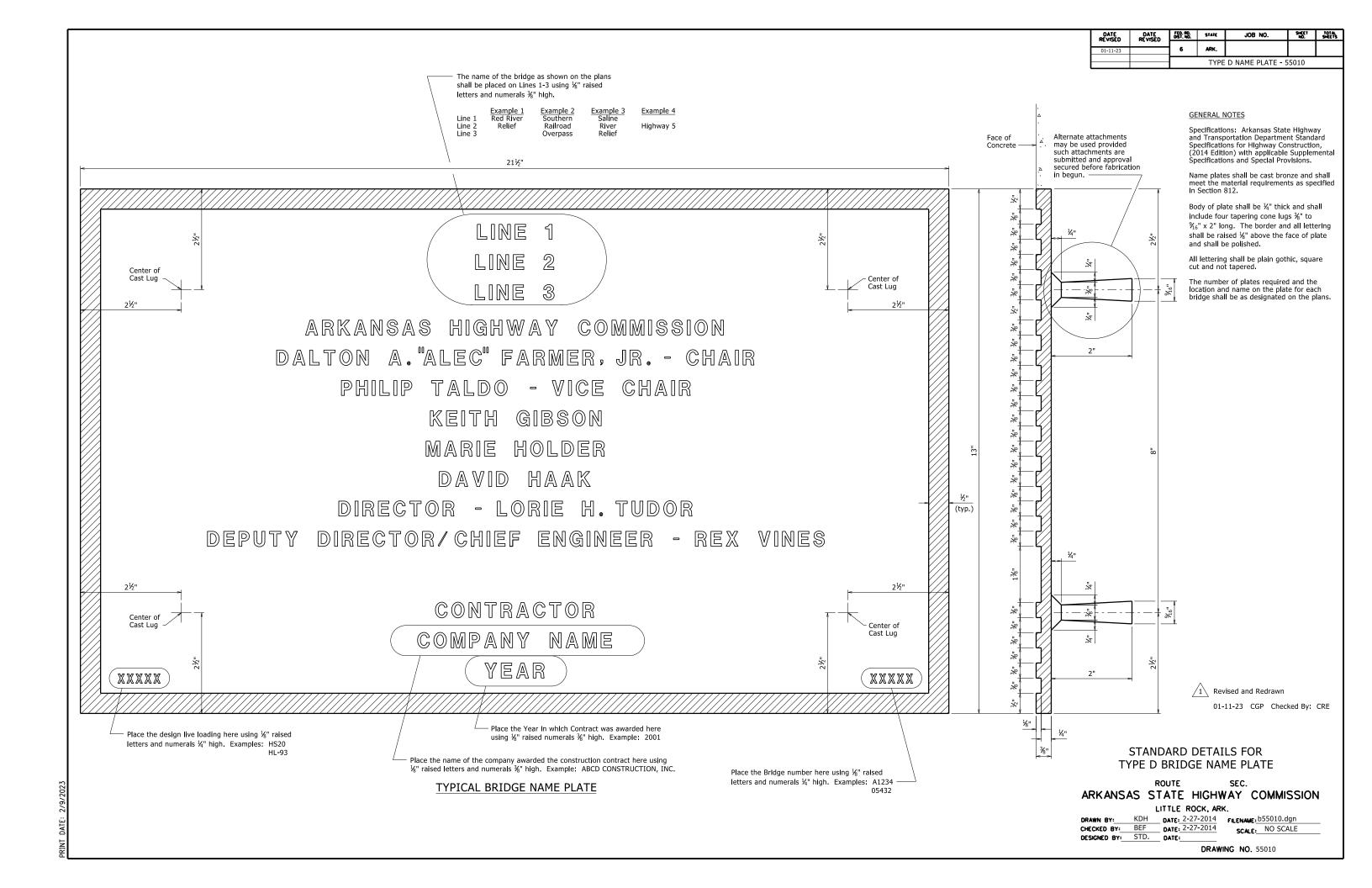
THESE DETAILS ARE APPLICABLE UNLESS OTHERWISE SHOWN IN THE PLAN DETAILS, SPECIAL PROVISIONS, OR SUPPLEMENTAL SPECIFICATIONS.

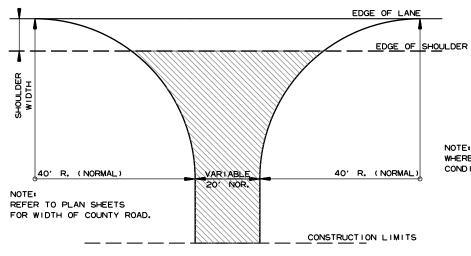
STANDARD DETAILS FOR STEEL BRIDGE STRUCTURES

ARKANSAS STATE HIGHWAY COMMISSION

		LITTLE	ROCK, AF	₹K.		
DRAWN BY:	JYP	DATE:	2/11/2016	FILENAME:	b550	007.
CHECKED BY:	AMS	DATE:	2/11/2016	SCALE:	No	Sca
DESIGNED BY:	STD.	DATE:	_			

DRAWING NO. 55007

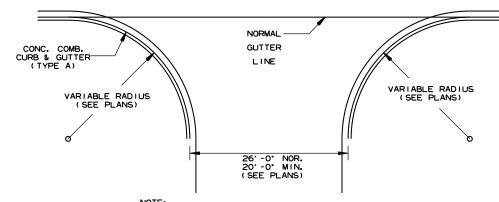




NOTE: TURNOUTS SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

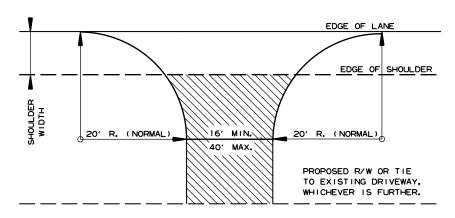
ACHM SURFACE COURSE (1/2") (220 LBS. PER SQ. YD.) AND AGGREGATE BASE COURSE (CLASS 7) 7" COMP. DEPTH, UNLESS OTHERWISE SPECIFIED IN PLANS.





NOILE PAVEMENT STRUCTURE FOR STATE HIGHWAYS, CITY STREETS, & COUNTY ROADS TO BE SAME AS MAIN LANES.

DETAIL OF TURNOUTS, ASPHALT STREETS, COUNTY ROADS & STATE HIGHWAYS CURB & GUTTER SECTION

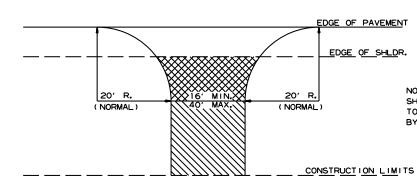


NOTE: TURNOUTS AND PRIVATE DRIVES SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.



ACHM SURFACE COURSE (1/2") (220 LBS. PER SQ. YD.) AND AGGREGATE BASE COURSE (CLASS 7) 7" COMP. DEPTH IF ASPHALT OR GRAVEL DRIVE EXISTING: OR 6" CONCRETE IF CONCRETE DRIVE





NOTE: TURNOUTS AND PRIVATE DRIVES SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

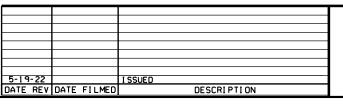


ASPHALT CONCRETE HOT MIX SURFACE COURSE (220 LBS, PER SQ, YD.) AGGREGATE BASE COURSE (CLASS 7) 7' COMP. DEPTH IF ASPHALT DRIVE EXIST OR 6' CONCRETE IF CONCRETE DRIVE EXIST.



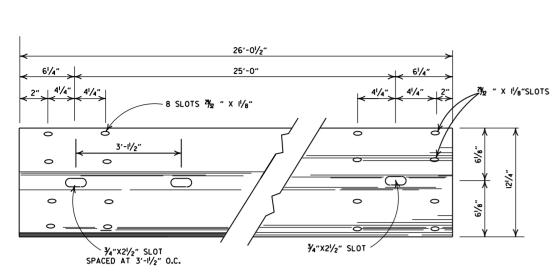
AGGREGATE BASE COURSE (CLASS 7)
9° COMP. DEPTH OR CONFORM
TO EXISTING DRIVEWAY

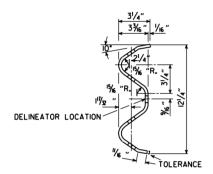
DETAIL FOR DRIVEWAY TURNOUTS (COLLECTORS)



ARKANSAS STATE HIGHWAY COMMISSION DETAILS OF DRIVEWAYS & STREET TURNOUTS

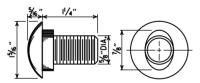
STANDARD DRAWING DR-2



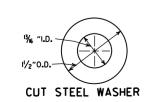


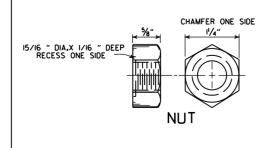
DETAILS OF W-BEAM GUARDRAIL

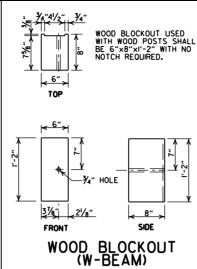
RAIL SECTION OF CLOSELY SIMILAR DIMENSIONS AND COMPARABLE STRENGTH MAY BE SUBSTITUTED IF APPROVED BY THE ENGINEER.

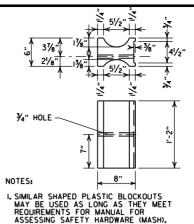


SPLICE BOLT
POST BOLT - SAME EXCEPT LENGTH



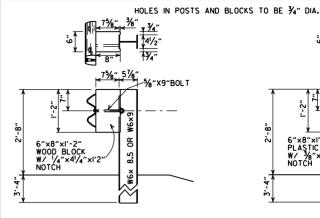




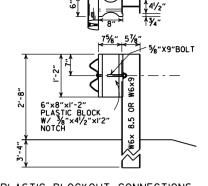


2.DIMENSIONS ARE SUBJECT TO MANUFACTURERS TOLERANCES.

PLASTIC BLOCKOUT (W-BEAM)

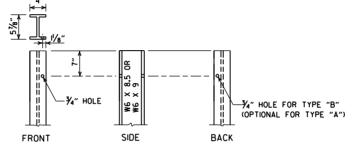


WOOD BLOCKOUT CONNECTIONS

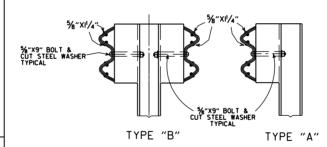


PLASTIC BLOCKOUT CONNECTIONS

DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)



STEEL POST



DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)

-GENERAL NOTES-

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $\frac{1}{4}$ " BEYOND IT.

WHERE W-BEAM GUARDRAIL CONTINUES, THE INTERMEDIATE SECTIONS
SHALL HAVE A POST SPACING OF 6'-3" UNLESS OTHERWISE NOTED.
W-BEAM GUARDRAIL REPRESENTING INTERMEDIATE SECTIONS
WILL BE MEASURED ALONG THE ROADWAY FACE FROM CENTERLINE OF
POST TO CENTERLINE OF POST.

USE W-BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. FOR EXTENSIONS OR MODIFICATION OF EXISTING GUARDRAIL, W-BEAM GUARDRAIL COMPONENTS OF THE SAME TYPE AS THOSE EXISTING SHALL BE USED.

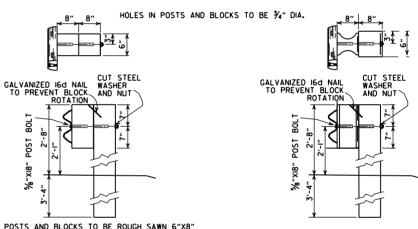
ANY BACKFILLING UNDER OR AROUND POST SHALL BE DAMP SAND THOROUGHLY TAMPED IN PLACE.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO. I 1350 f SOUTHERN PINE.

CONTRACTOR SHALL HAVE THE OPTION OF USING WOOD BLOCKOUTS FOR W-BEAM GUARDRAIL OR PLASTIC BLOCKOUTS, AS LONG AS BLOCKOUT USED MEETS REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) FOR W-BEAM GUARDRAIL.

TO MAINUAL FOR ASSESSING SAFEIT HARDWARE MASHIFUR WELAM GUARDRAIL.

DELINEATORS SHALL BE MOUNTED AT 37,5' SPACING ON THE FRONT FACE OF THE GUARDRAIL. SPACING MAY BE REDUCED IN CURVES, AS DIRECTED BY THE ENGINEER. COLOR SHALL BE IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, PAYMENT FOR DELINEATORS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID PER LIN.FT.FOR GUARDRAIL.



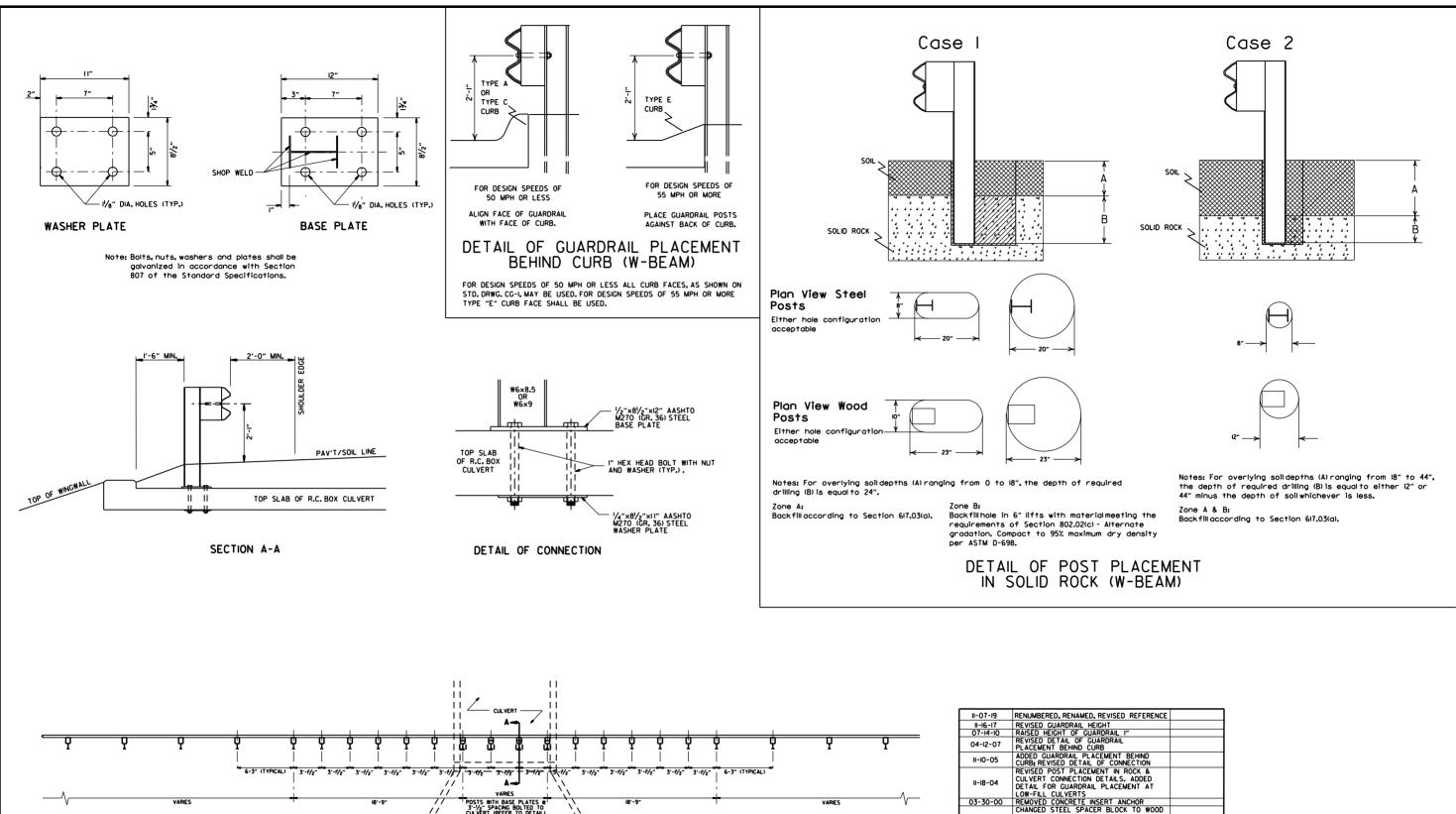
POSTS AND BLOCKS TO BE ROUGH SAWN 6"X8" WITH A TOLERANCE OF + OR - 1/4".

WOOD BLOCKOUT CONNECTIONS

CTIONS PLASTIC BLOCKOUT CONNECTIONS

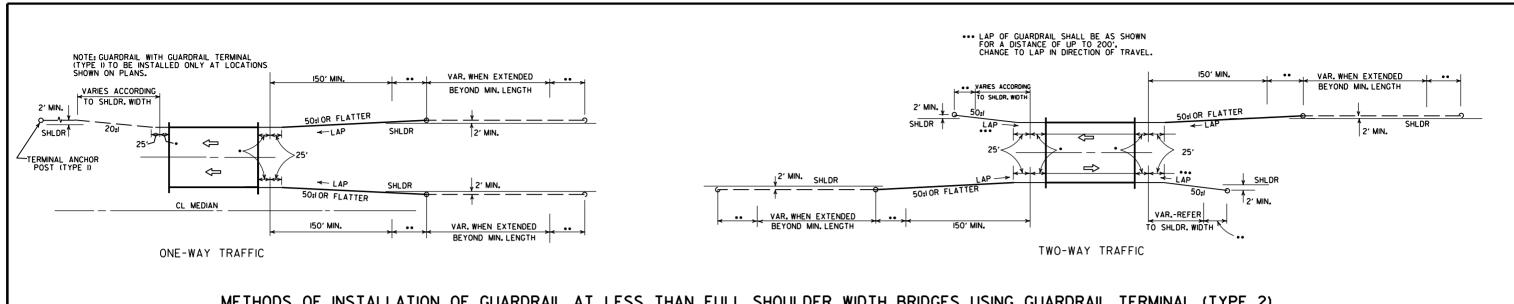
DETAILS OF WOOD LINE POST CONNECTIONS (W-BEAM)

05	5-19-22	REVISED GENERAL NOTES. ADDED DELINEATOR LOCATION.		
11-	-07-19	RENUMBERED AND RENAMED		
11-	-16-17	REVISED GENERAL NOTES AND RAISED GUARDRAIL HEIGHT 3"		
0.	7-14-10	RAISED HEIGHT OF GUARDRAIL I"		
	-15-09	ADDED REFERENCE TO MASH		
04	1-10-03	REVISED GENERAL NOTES		
08	-22-02	REVISED DIMENSION ON WOOD & PLASTIC BLOCKOUT CONNECTIONS & STEEL POST		
ı	I-I6-0I	REVISED WOOD BLOCKOUT & DETAILS OF WOOD LINE POST CONNECTIONS		
03	-30-00	REMOVED GUARDRAIL AT BRIDGE ENDS		
01	I-I2-00	ADDED PLASTIC BLOCKOUT		
08	3-12-98	REV. BLOCKOUTS TO WOOD, DELETED CONC. POST & REV. GENERAL NOTE.DELETED DET. OF GUARDRAIL REPLACE. BEHIND CURB & DET. OF POST PLACE. IN SOLID ROCK, & ADDED DETAILS OF STEEL LINE POST CONN. REMOVED BACK-UP PLATE, REVISED HOLES IN STEEL POLES		
	-03-97	REMOVED "LAP IN DIRECTION OF TRAFFIC" NOTE & PLACED ARROWS ON WASHERS		
	-18-96	REVISED WOOD POST NOTE		
	-02-94	ADDED ALT. STEEL POST SIZE		
	-05-93	REVISED STEEL POST SIZE	8-5-93	ARKANSAS STATE HIGHWAY COMMISSION
	-01-92	REDRAWN & REVISED	10-1-92	ALLANDAS STATE HIGHWAT COMMISSION
0	8-15-91	REVISED WASHER NOTE	8-15-91	
	-02-90	REV. GEN. NOTE & DEPTH OF ANC. POST IN ROCK	8-2-90	CHARDEDAN DETANC
07	7-15-88	REVISED SECTION 3 & GENERAL NOTES		GUARDRAIL DETAILS
1	-04-88	REV. ANCHOR POST "ELEV. NOTES & POST IN ROCK	780-3-4-88	
	-30-87	REVISED WOOD LINE POST DETAIL	546-10-30-87	
10	-09-87	REDRAWN & REVISED	802-10-9-87	STANDARD DRAWING GR-6
	DATE	REVISION	FILMED	STANDAND DINAWING ON G

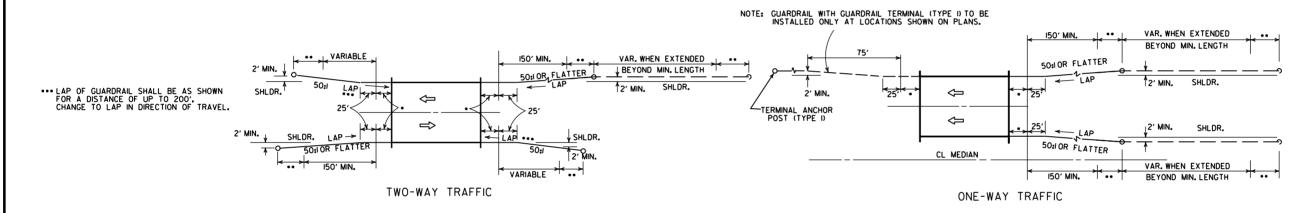


`		BY INE	ENGINEER.		
PLAN LAY	OUT OF	TYPE	A GUARDRAIL	AT LOW-FILL	CULVERTS
			ONLY WHEN THE COV		

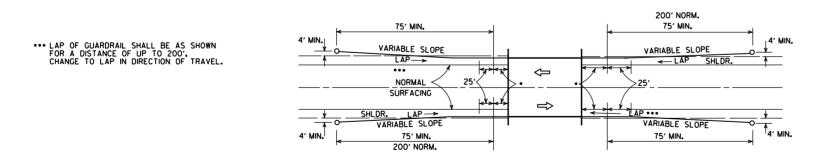
07-14-10	KAISED HEIGHT OF GUARDRAIL I		
04-12-07	REVISED DETAIL OF GUARDRAIL PLACEMENT BEHIND CURB		
11-10-05	ADDED GUARDRAIL PLACEMENT BEHIND CURB; REVISED DETAIL OF CONNECTION		
11-18-04	REVISED POST PLACEMENT IN ROCK & CULVERT CONNECTION DETAILS. ADDED DETAIL FOR GUARDRAIL PLACEMENT AT LOW-FILL CULVERTS		
03-30-00	REMOVED CONCRETE INSERT ANCHOR		
08-12-98	CHANGED STEEL SPACER BLOCK TO WOOD BLOCKOUT, ADDED DET, OF GUARDRAIL CONNECTION TO R.C. BOX CULY'T., DELETED DET, OF STEEL LINE POST CONN. & ADDED DET, OF GUARDRAIL PLACE, BEHIND CURB & DET, OF POSTPLACE, IN SOLID ROCK		
04-03-96	PLACED ARROWS AT CUT STEEL WASHERS	4-3-96	
10-18-96	REV. ASTM REF. TO AASHTO		
II-22-95	ADDED OPTIONAL HOLES		ABUANCAS STATE INCIDENTAL COLUMNS
06-02-94	REVISED ALTERNATE POST SIZE		ARKANSAS STATE HIGHWAY COMMISSION
08-05-93	REVISED STEEL POST SIZE		
10-01-92	REDRAWN & REVISED	10-1-92	
08-02-90	DEL. WASHER ON ANCHOR ASSEMBLY	8-2-90	
07-15-88	CONFORMED TO 1988 SPECS		I GUARDRAIL DETAILS
03-04-88	REVISED ANCHOR NOTE		
10-30-87	REVISED ANCHOR ASSEMBLY	712-10-30-87	
10-30-87	REVISED PLACEMENT BEHIND CURB	547-10-30-87	
10-09-87	REDRAWN & REVISED	803-10-9-87	STANDARD DRAWING GR-7
DATE	REVISION	FILMED	STANDAND DINAWING ON T



METHODS OF INSTALLATION OF GUARDRAIL AT LESS THAN FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



METHOD OF INSTALLATION OF GUARDRAIL AT FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



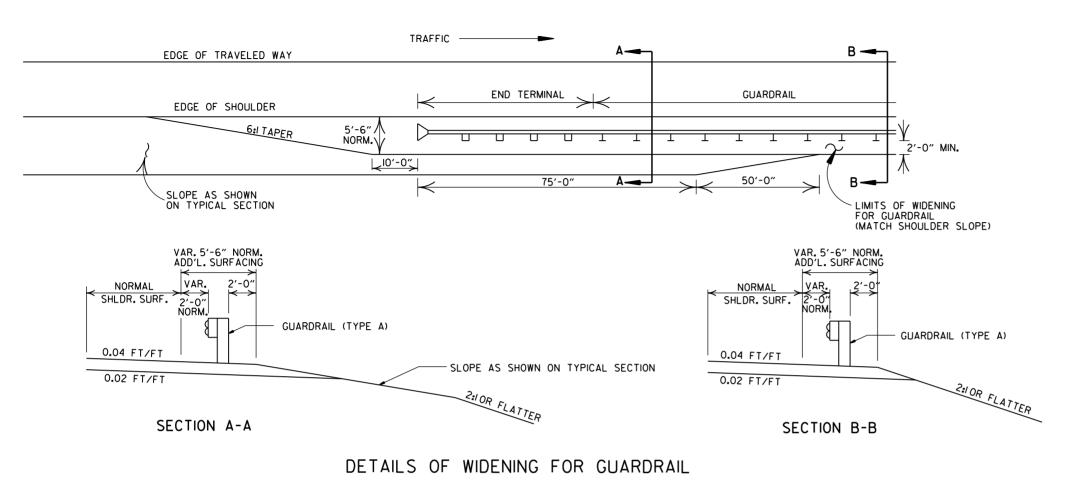
METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERMINAL (TYPE I) (FULL SHOULDER WIDTH OR LESS BRIDGES)

			ARKANSAS STATE HIGHWAY COMMISSION
11-07-19	RENUMBERED AND RENAMED		
4-17-08	REVISED LAYOUTS		
11-10-05	REMOVED GUARDRAIL NOTES AND DETAILS		
11-16-01	DELETED NOTE-METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERM. (TY. I)		GUARDRAIL DETAILS
1-12-00	ADDED CONSTRUCTION NOTE	1-12-00	
6-26-97	REVISED LAYOUT		
10-1-92	REDRAWN & REVISED	10-1-92	
	ADDED NOTE		
10-9-87	REDRAWN & REVISED		STANDARD DRAWING GR-8
DATE	REVISION	DATE FILM	

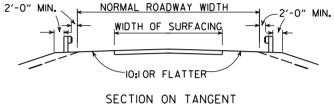
LEGEND

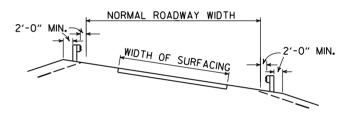
.. GUARDRAIL TERMINAL (TYPE 2)

THRIE BEAM GUARDRAIL TERMINAL



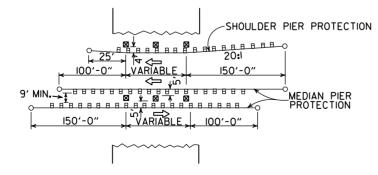
NOTE: NORMAL SECTION TO BE WIDENED APPROX. 5'-6" EACH SIDE TO SUPPORT GUARDRAIL.





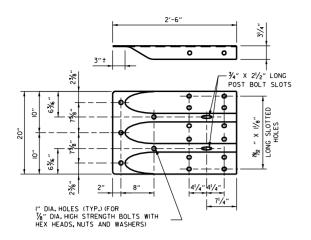
SECTION ON CURVE

DETAILS SHOWING POSITION OF GUARDRAIL ON HIGHWAY

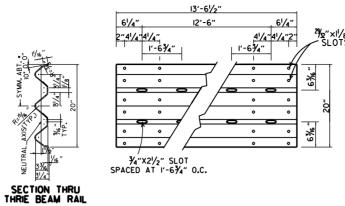


METHOD OF INSTALLATION OF GUARDRAIL AT FIXED OBSTACLE

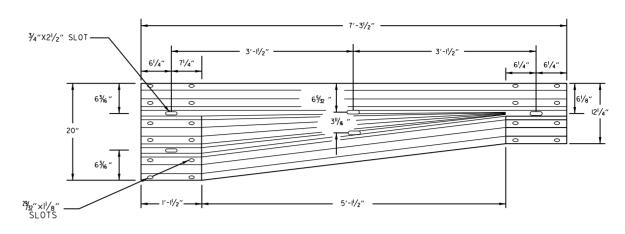
			ARKANSAS STATE HIGHWAY COMMISSION
			GUARDRAIL DETAILS
			OUANDINAL DETAILS
11-07-19	RENUMBERED AND RENAMED		
4-17-08	MINOR REVISION		
11-10-05	DRAWN		STANDARD DRAWING GR-9
DATE	REVISION	DATE FILM	



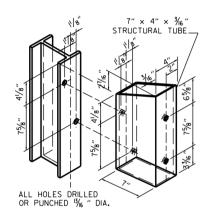
SPECIAL END SHOE



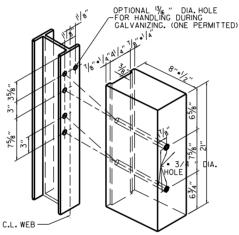
THRIE BEAM RAIL



TRANSITION SECTION



ATTACH BLOCKOUT TO POST USING %" DIA. HEX HEAD BOLTS WITH $1\frac{1}{2}$ " O.D. CUT STEEL WASHERS AND NUT.

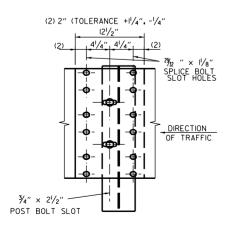


ALL HOLES 13/16 " DIAMETER EXCEPT AS NOTED

HOLE PUNCHING DETAIL FOR STEEL POST & WOOD OR PLASTIC BLOCKOUTS

NOTE: BLOCKS SHALL BE THE SAME TYPE THROUGHOUT THE PROJECT LIMITS.

STRUCTURAL STEEL TUBING BLOCKOUT DETAIL



THRIE BEAM RAIL SPLICE AT POST

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I.

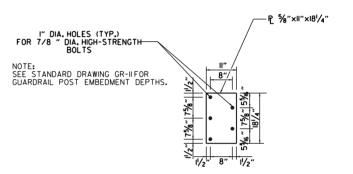
 $\mbox{\sc Rail}$ posts shall be set perpendicular to the roadway profile grade and vertically in cross section.

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $3^{\rm *4}{\rm ''}$ Beyond it.

ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-13.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

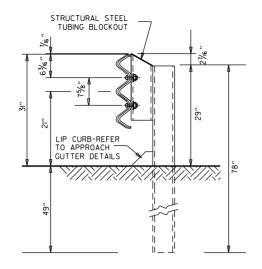
USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.
THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR
BETTER 9.7f (1400 f) OR NO.1 1350 f SOUTHERN PINE.



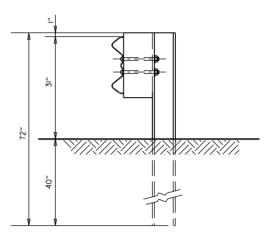
CONNECTOR PLATE

CONNECTOR PLATE SHALL BE AASHTO M270, CR. 36 AND SHALL BE GALVANIZED AFTER FABRICATION. GALVANIZING SHALL CONFORM TO SUBSECTION 807.19 OF THE STANDARD SPECIFICATIONS. CONNECTOR PLATE TO BE BOLTED TO SPECIAL END SHOE USING 1/8" DIA. HIGH STRENGTH BOLTS, WITH THE HEADS PLACED ON THE TRAFFIC FACE. WASHERS SHALL BE USED UNDER THE HEAD AND NUT. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND SHALL CONFORM TO SUBSECTION 807.06.

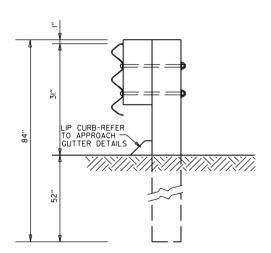
11-07-19	RENAMED AND REVISED REFERENCES		
11-16-17	REVISED TRANSITION SECTION, GUARD RAIL HEIGHT, AND GENERAL NOTES; MOYED THRIE BEAM GUARD RAIL CONNECTIONS AT BRIDGES ENDS TO STD. DRWG. GR-12		
07-14-10	RAISED HEIGHT OF W-BEAM I"		
11-29-07	ADDED PLASTIC BLOCKOUTS		ADVANCAC CTATE HICHWAY COMMICCION
11-10-05	ADDED NOTE FOR ATTACHING STEEL BLOCKOUT		ARKANSAS STATE HIGHWAY COMMISSION
11 10 04			
11-18-04	REVISED GENERAL NOTES		
10-9-03	REVISED GENERAL NOTES	1	l
04-10-03	REVISED GENERAL NOTES		I GUARDRAIL DETAILS I
08-22-02	REVISED NOTE (2)		
06-29-00	MOVED DIMENSION LINES		
05-18-00	ADDED NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-10
DATE	REVISION	FILMED	STANDAND DIVAMINO ON TO



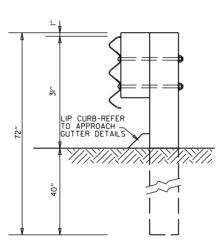
THRIE BEAM RAIL WITH STEEL TUBING BLOCKOUT AND STEEL POST POSTS 1-7



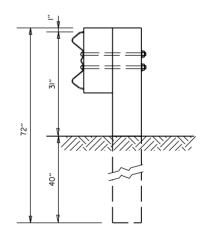
W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT AND STEEL POST POST 8



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUTS & WOOD POSTS
POSTS I-6



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUT & WOOD POST
POST 7

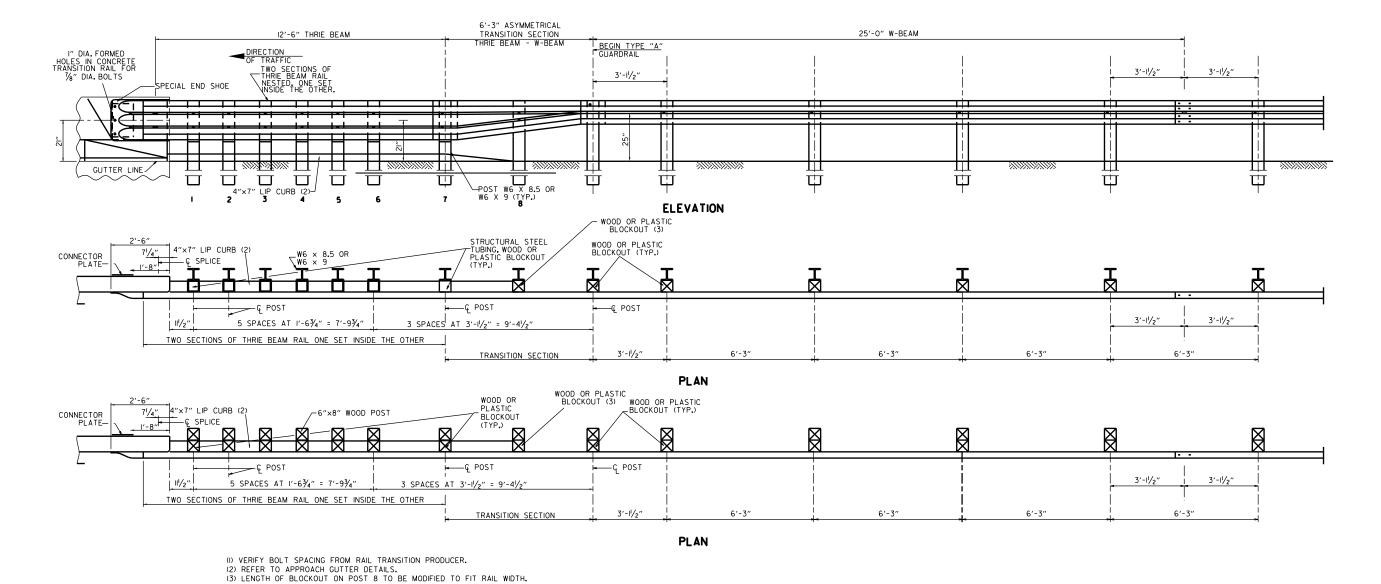


W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT & WOOD POST POST 8

GENERAL NOTES:
RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO. I 1350 f SOUTHERN PINE.

			ARKANSAS STATE HIGHWAY COMMISSION
11-07-19	RENAMED		
11-16-17	REVISED GUARDRAIL HEIGHT, CHANGED STD. DWG. NUMBER FROM GR-IOA TO GR-II		GUARDRAIL DETAILS
07-14-10	REVISED POST 8 DIMENSIONS		1
II-29-07	ADDED PLASTIC BLOCKOUTS		
08-22-02	REVISED LIP CURB NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-II
DATE	REVISION	FILMED	STANDARD DRAWING OR II



THRIE BEAM GUARDRAIL CONNECTION AT BRIDGE ENDS

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I.

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

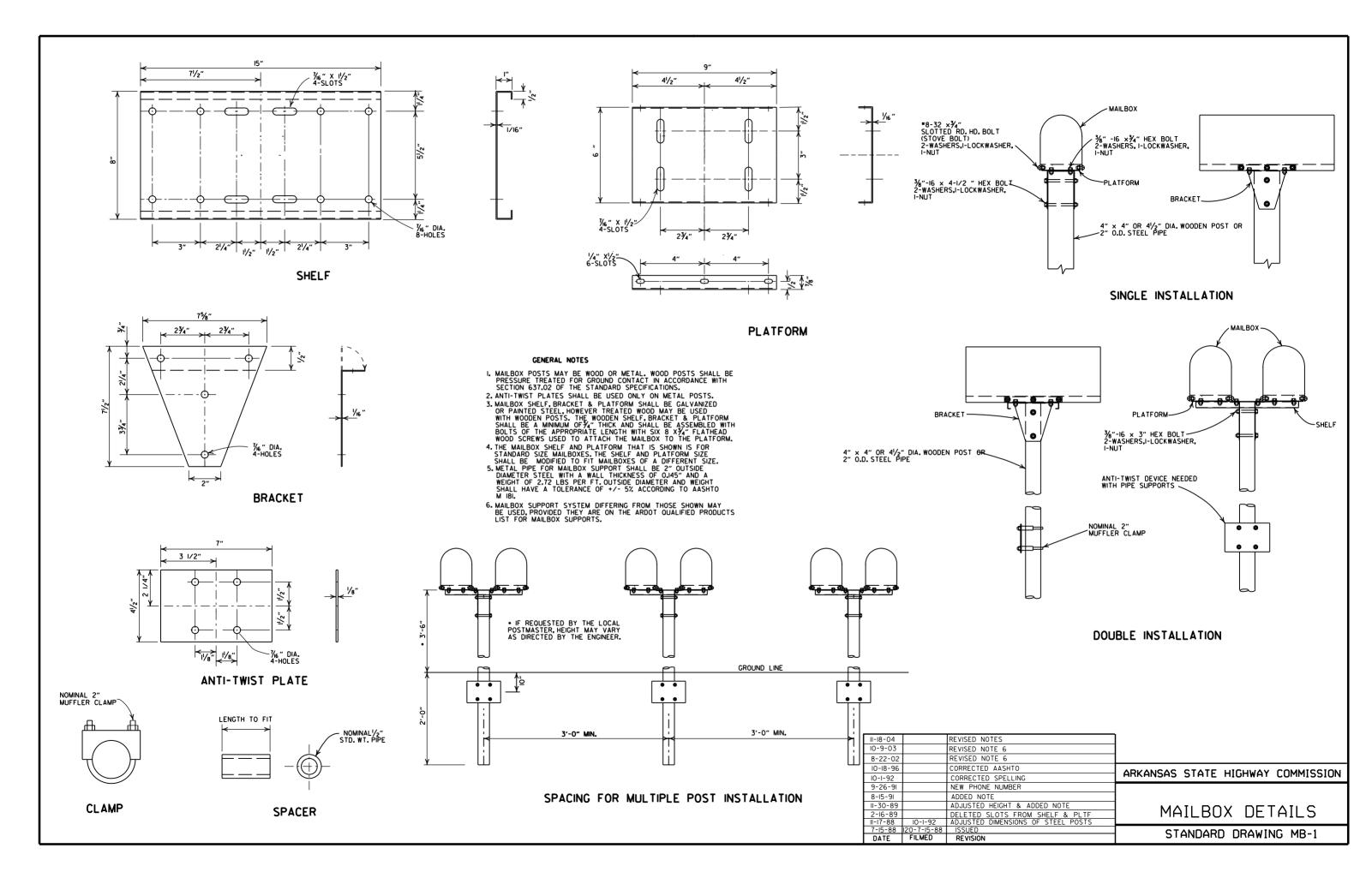
ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $3/4^{\prime\prime}$ BEYOND IT.

ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-3.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.
THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.
POSTS SHALL NOT BE PLACED AT SPLICE LOCATIONS ALONG W-BEAM RAILS.
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR

				ARKANSAS STATE HIGHWAY COMMISSION
				GUARDRAIL DETAILS
	-14-20 -07-19	REVISED NOTES RENAMED & REVISED REFERENCES		
l.	-16-17 DATE	RE-DRAWN FROM STD. DWG. GR-IO & ISSUED REVISION	FILMED	STANDARD DRAWING GR-12



REINFORCED CONCRETE ARCH PIPE DIMENSIONS

EQUIV.	SP	AN	RISE		
DIA.	AASHTO ARDOT M 206 NOMINAL		AASHTO ARDOT M 206 NOMINAI		
INCHES		INC	HES		
15 18 21 24 30 36 42 48 54 60 72 84 90 96 108 120 132	18 22 26 28½ 36¼ 43¾ 51½ 65 73 88 102 115 122 138 154 168¾	18 22 26 29 36 44 51 59 65 73 88 102 115 122 138 154 169	11 13½ 15½ 18 22½ 26% 31% 36 40 45 54 62 77½ 87½ 96% 106½	11 14 16 18 23 27 31 36 40 45 54 62 77 87 97	

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN + 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE DIMENSIONS

'	I I C DINCIADIONS					
	EQUIV.	AASHTO M 207				
	DIA.	SPAN	RISE			
	INCHES	INC	HES			
	18	23	14			
	24	30	19			
	27	34	22			
	30	38	24			
	33	42	27			
	36	45	29			
	39	49	32			
	42	53	34			
	48	60	38			
	54	68	43			
	60	76	48			
	66	83	53			
	72	91	58			
	78	98	63			
	84	106	68			

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN
± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE.
 5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(I).

NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE

- LEGEND -

D₁ = NORMAL INSIDE DIAMETER OF PIPE
D₀ = OUTSIDE DIAMETER OF PIPE
H = FILL COVER HEIGHT OVER PIPE (FEET)
MIN. = MINIMUM
STATE = UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3**	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

- *SM-3 WILL NOT BE ALLOWED.
- ** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.

MINIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

	CLASS OF PIPE					
	CLASS	III	CLASS IV	CLASS V		
INSTALLATION TYPE	TYPE 1 OR 2	TYPE 3	ALL	ALL		
PIPE ID (IN.)		FEE	Т			
12-15	2	2.5	2	1		
18-24	2.5	3	2	1		
27-33	3	4	2	1		
36-42	3 . 5	5	2	1		
48	4.5	5.5	2	1		
54-60	5	7	2	1		
66-78	6	8	2	1		
84-108	7.5	8	2	1		

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MINIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS OF PIPE			
INSTALLATION TYPE	CLASS III CLASS I			
	FEET			
TYPE 2 OR TYPE 3	2.5	1.5		

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

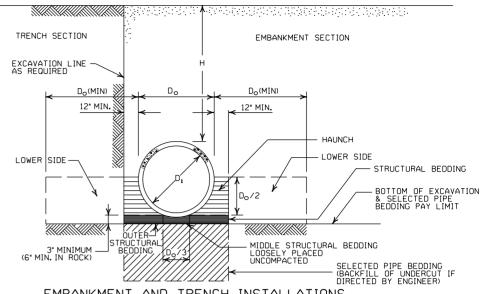
	CLASS OF PIPE				
INSTALLATION TYPE	CLASS III	CLASS IV	CLASS V		
1175	FEET				
TYPE 1	21	32	50		
TYPE 2	16	25	39		
TYPE 3	12	20	30		

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MAXIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS OF PIPE				
INSTALLATION	CLASS III	CLASS IV			
ITPE	FEET				
TYPE 2	13	21			
TYPE 3	10	16			

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH, IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND RECOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
- 3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMIIM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

- I. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. ALL PIPE SHALL CONFORM TO SECTION 606. CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO MI70, R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING, HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SOUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
- 9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- IO. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

BEDDING

			ARKANSAS STATE HIGHWAY COMMISSION
			CONCRETE PIPE CULVERT
2-27-14 12-15-11 5-18-00	REVISED GENERAL NOTE I. REVISED FOR LRFD DESIGN SPECIFICATIONS REVISED TYPE 3 BEDDING & ADDED NOTE		FILL HEIGHTS & BEDDING
3-30-00 II-06-97	REVISED INSTALLATIONS ISSUED		STANDARD DRAWING PCC-1
DATE	REVISION	DATE FILMED	

CORRUGATED STEEL PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX.FILL	HEIGHT "	H" ABOVE	TOP OF PI	PE (FEET)
DIAMETER	PIPE TO TOP OF GROUND		METAL	THICKNESS	(INCHES)	
(INCHES)	"H" (FEET)	0.064	0.079	0.109	0.138	0.168
	2% RIVET		½ INCH D, OR HEL	CORRUGATI		
12 15 18 24 30 36 42 48	2 2 2	84 67 56 42 34	9I 73 6I 46 36 30 43 37	59 47 39 67 58	41 70 61	73 64
	2 3 INCH BY RIVETE	D, WELDED		I BY 1 INCI OR HELICA		AM
36 42 48 54 60 66 72 78 84 90 96 102 108 II4	1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	48 41 36 32 29 26 24	60 51 45 40 36 33 30 28 26 24 22	88 72 64 59 53 47 44 41 38 35 33 31 30 28 27	III 90 77 71 64 53 49 45 43 40 38 35 34 32	118 102 85 79 71 64 59 54 45 44 42 37 35

CORRUGATED ALUMINUM PIPE (ROUND)

DIDE	① MINUMUM	MAX. FILL	HEIGHT '	'H'' ABOVE	TOP OF F	PIPE (FEET
PIPE DIAMETER	PIPE TO TOP		METAL TH	HICKNESS I	IN INCHES	
(INCHES)	OF GROUND "H" (FEET)	0.060	0.075	0.105	0.135	0.164
		2 ² / ₃		Y ½ INCH R HELICAL	CORRUGA LOCK-SEA	
12 18 24 30 36 42 48 54 60 66	1 2 2 2.5 2 2 2 2 2 2 2	45 30 22	45 30 22 18 15	52 39 31 26 43 40 35	41 32 27 43 41 37 33	34 28 44 43 38 34 31 29

CONSTRUCTION SEQUENCE

- 1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. COMPLETE STRUCTURAL BACKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE,
- NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

3 SM-3 WILL NOT BE ALLOWED.

EQUIVALENT METAL THICKNESSES AND GAUGES

METAL			
ST	EEL		GAUGE NUMBER
ZINC COATED	UNCOATED	ALUMINUM	
0.064	0.0598	0.060	16
0.079	0.0747	0.075	14
0.109	0.1046	0.105	12
0.138	0.1345	0.135	10
0.168	0.1644	0.164	8

ALUMINUM

FILL. "H" (FT.)

INSTALL ATTON

TYPE 1

1 MIN. HEIGHT OF MAX. HEIGHT OF

2 3 INCH BY 1/2 INCH CORRUGATION

RIVETED OR HELICAL LOCK-SEAM

INSTALLATION

TYPF 1

2.25

MIN.

CORRUGATED METAL PIPE ARCHES

STEEL
MINUMUM MIN. (1) MIN. HEIGHT OF MAX. HEIGHT OF

FOUTV	LILE	MINOMOM	MIIN.	FILL, "		MAX. HE		MIIN.	
EQUIV.	DIMENSION SPAN X RISE	CORNER RADIUS	THICKNESS					THICKNESS	H
			REQUIRED	INSTAL			LATION	REQUIRED	L
(INCHES)	(INCHES)	(INCHES)	INCHES	TYPE		TYPE	1	INCHES	
			2		BY ½ INCH (
						AL LOCK-SEA			_
15	17×13	3	0.064	2		15		0.060	ı
18	21×15	3	0.064	2		15		0.060	ı
21	24×18	3	0.064	2.2		15 15		0.060	ı
24	28×20 35×24	3 3	0.064 0.079	2.)	I2		0.075 0.075	ı
30 36	42×29	31/2	0.079	3		12		0.015	ı
42	49×33	4	0.079	3		12		0.105	ı
48	57×38	5	0.109	3		13		0.135	ı
54	64×43	6	0,109	3		13		0.135	ı
60	71×47	7	0.138	3		15		0.164	ı
66	77×52	8	0,168	3		l i5			_
72	83×57	9	0.168	3		15			
						BY 1 INCH CO]	
			RIVE	TED, WELDE	D, OR HELIC	AL LOCK-SE	AM	1	
				INSTAL	LATION	INSTAL	LATION	1	F
				TYPE 2	TYPE 1	TYPE 2	TYPE 1	2	W
36	40×31	5	0.079	3	2	12	15	1	W
42	46×36	6	0.079	3	2	13	15	1	0
48	53×4I	7	0.079	3	2	13	15		
54	60×46	8	0.079	3	2	13	15		
60	66×5I	9	0.079	3	2	13	15		
66	73×55	12	0.079	3	2	15	15		
72	81×59	14	0.079	3	2	15	15		
78 84	87×63 95×67	14 16	0.079	3 3	2	15 15	15 15		
90	103×71	16		3	2 2 2 2 2 2	15	15 15		
96	103×71	18	0.109 0.109	3	2	15	15		
102	117×79	18	0.109	3	2	15	15		
108	128×83	18	0.138	3	2	15	15		
	LONGS	-	0.00		_			J	

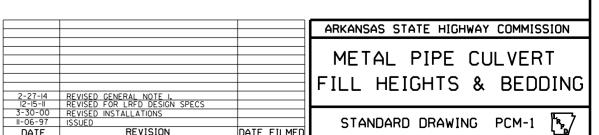
① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE. ② WHERE THE STANDARD 2 2/3'x ½ CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3'x 1'OR 5'x 1'CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS GAUGED FOR A FILL HEIGHT CONDITION EQUAL TO

OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.

- EXCAVATION LINE AS REQUIRED - LEGEND -Do = OUTSIDE DIAMETER OF PIPE Do(MIN) 12" MIN. X MAX. = MAXIMUM MIN. = MINIMUM 12" MIN. = STRUCTURAL BACKFILL MATERIAL = UNDISTURBED SOIL STRUCTURAL BACKFILL EQUIV. DIA. = EQUIVALENT DIAMETER EMBANKMENT H = FILL COVER HEIGHT OVER PIPE (FEET) STRUCTURAL BEDDING -BOTTOM OF EXCAVATION & SELECTED PIPE BEDDING PAY LIMIT MIDDLE STRUCTURAL BEDDING
 - LOOSELY PLACED
 UNCOMPACTED TRIJICTURAL Ł IN SOIL-MIN. EQUALS TWICE CORRUGATION DEPTH IN ROCK-MIN. EQUALS GREATER OF; I/2°PER FOOT OF FILL OVER PIPE (24°MAX.) TWICE CORRUGATION DEPTH SELECTED PIPE BEDDING (BACKFILL OF UNDERCUT DIRECTED BY ENGINEER)
 - EMBANKMENT AND TRENCH INSTALLATIONS
 - I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
 - 2. INSTALLATION TYPE IOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
 - 3. INSTALALTION TYPE I SHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 23" X 1/2"
 - 4. INSTALLATION TYPE IOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X I" OR 5" X I" CORRUGATION.

GENERAL NOTES

- I. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE, REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING, THE OUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."



INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	•SELECTED MATERIALS (CLASS SM-I, SM-2 OR SM-4)

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INNCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF HOPE PIPE.

MULTIPLE INSTALLATION OF HIGH DENSITY POLYETHYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2'-6"
36"	3′-0″
42"	3′-6″
48"	4′-0″

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"	
18"	4′-6″	4′-6″	
24"	5′-0″	6'-0"	
30"	5′-6″	7′-6″	
36"	6′-0″	9'-0"	
42"	7′-0″	10'-6"	
48"	8'-0"	12'-0"	

18" MIN. (18" - 30" DIAMETERS) 24" MIN. (36" - 48" DIAMETERS) MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

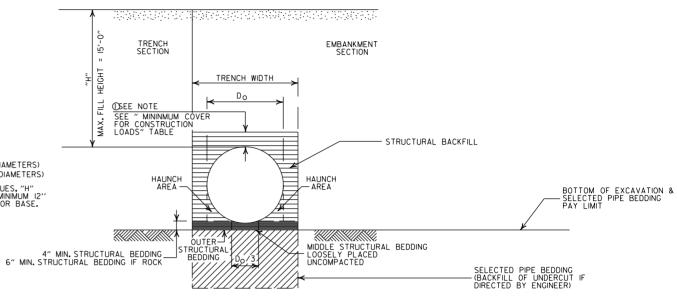
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. 0	OVER (FEET CONSTRUCT		ATED
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	IIO.0-175.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3′-0″	3′-0″
42" OR GREATER	3'-0"	3′-0″	3′-6″	4'-0"

OMINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M294, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. HIGH DENSITY POLYETHYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR HDPE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I, STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I.
12-15-11 REVISED GENERAL NOTES & MINIMUM COVER NOTE
II-I7-I0 ISSUED
DATE REVISION DATE FILM

PLASTIC PIPE CULVERT

(HIGH DENSITY POLYETHYLENE)

STANDARD DRAWING PCP-1

INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	SELECTED MATERIALS(CLASS SM-I, SM-2, OR SM-4)

AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF PVC PIPE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

		H WIDTH ET)
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0'
18"	4′-6″	4′-6″
24"	5′-0″	6′-0″
30"	5′-6"	7′-6″
36"	6'-0"	9'-0"

MULTIPLE INSTALLATION OF PVC PIPES

PIPE	CLEAR DISTANCE
DIAMETER	BETWEEN PIPES
DIAMETER	DETWEEN FIFES
18"	1′-6″
24"	2'-0"
30"	2′-6″
36"	3′-0"

MAXIMUM FILL HEIGHT BASED ON STRUCTURAL BACKFILL

PIPE DIAMETER	"H"
18"	45'-0"
24"	45'-0"
30"	40'-0"
36"	40'-0"

① NOTE:

12" MIN. (18" - 36" DIAMETERS)

MINIMUM COVER VALUE, "H"

SHALL INCLUDE A MINIMUM 12"

OF PAVEMENT AND/OR BASE.

MINIMUM COVER FOR CONSTRUCTION LOADS

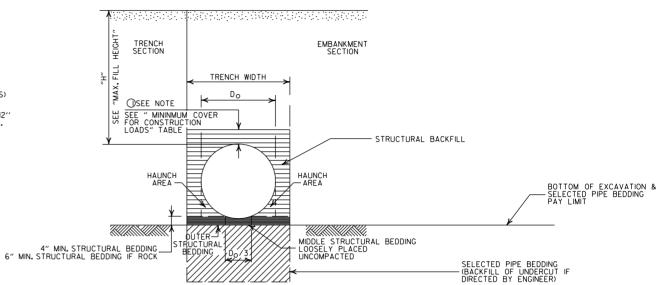
	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS				
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	II0.0-175.0 (KIPS)	
18" THRU 36"	2'-0"	2'-6"	3'-0"	3'-0"	

∅ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE

MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO ASTM F949, CELL CLASS 12454. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULYERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. PVC PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR PVC PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

H = FILL HEIGHT (FT.)
Do = OUTSIDE DIAMETER OF PIPE

MAX. = MAXIMUM

MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I. 12-15-II REV GENERAL NOTES & MINIMUM COVER NOTE; DELETED SM3 MATERIAL II-17-10 ISSUED DATE REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (PVC F949)

STANDARD DRAWING PCP-2



INSTALLATION TYPE	**MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE I	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	*SELECTED MATERIALS (CLASS SM-I, SM-2 OR SM-4) OR TYPE I INSTALLATION MATERIAL

*SM3 WILL NOT BE ALLOWED.

** STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF POLYPROPYLENE PIPE.

MULTIPLE INSTALLATION OF POLYPROPYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	l'-6"
24"	2'-0"
30"	2'-6"
36"	3'-0"
42"	3′-6″
48"	4'-0"
60"	5′-0″

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0'	
18"	4'-6"	4'-6"	
24"	5′-0″	6′-0″	
30"	5′-6″	7′-6″	
36"	6'-0"	9'-0"	
42"	7'-0"	10'-6"	
48"	8'-0"	12'-0"	
60"	10'-0"	15'-0"	

12" MIN. (18" - 42" DIAMETERS) 24" MIN. (60" DIAMETER) MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

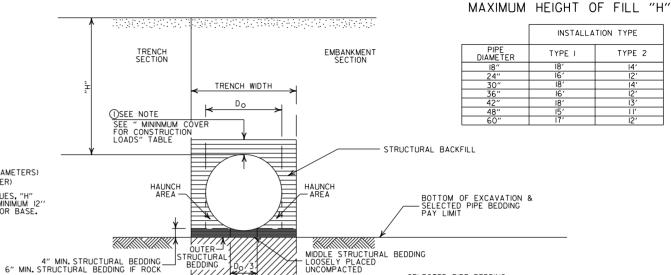
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	II0.0-I50.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3′-0″	3′-0″
42" OR GREATER	3'-0"	3'-0"	3′-6″	4'-0"

②MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M330, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SIXTH EDITION (2012) WITH 2013 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. POLYPROPYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR POLYPROPYLENE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN SECTION 26.4.2.4 AND 30.4.2 OF THE AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS 3RD EDITION (2010) WITH 2012 INTERIMS. JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



EMBANKMENT AND TRENCH INSTALLATIONS

I, STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND -

TYPE 2

H = FILL HEIGHT (FT.) Do = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

SELECTED PIPE BEDDING -(BACKFILL OF UNDERCUT IF DIRECTED BY ENGINEER)

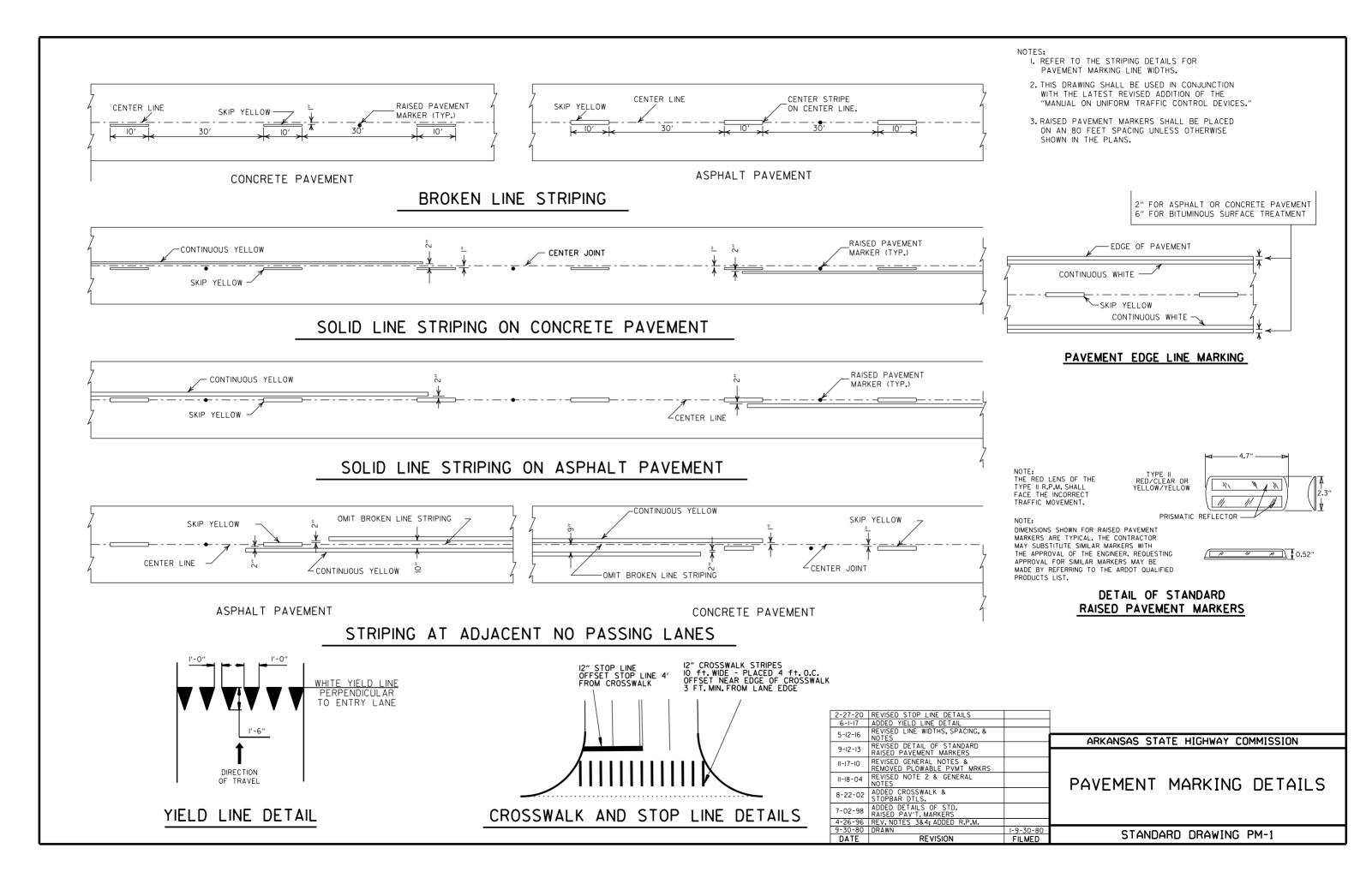
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02-27-20	REVISED		
11-07-19	ISSUED		
DATE		DATE	FILMED

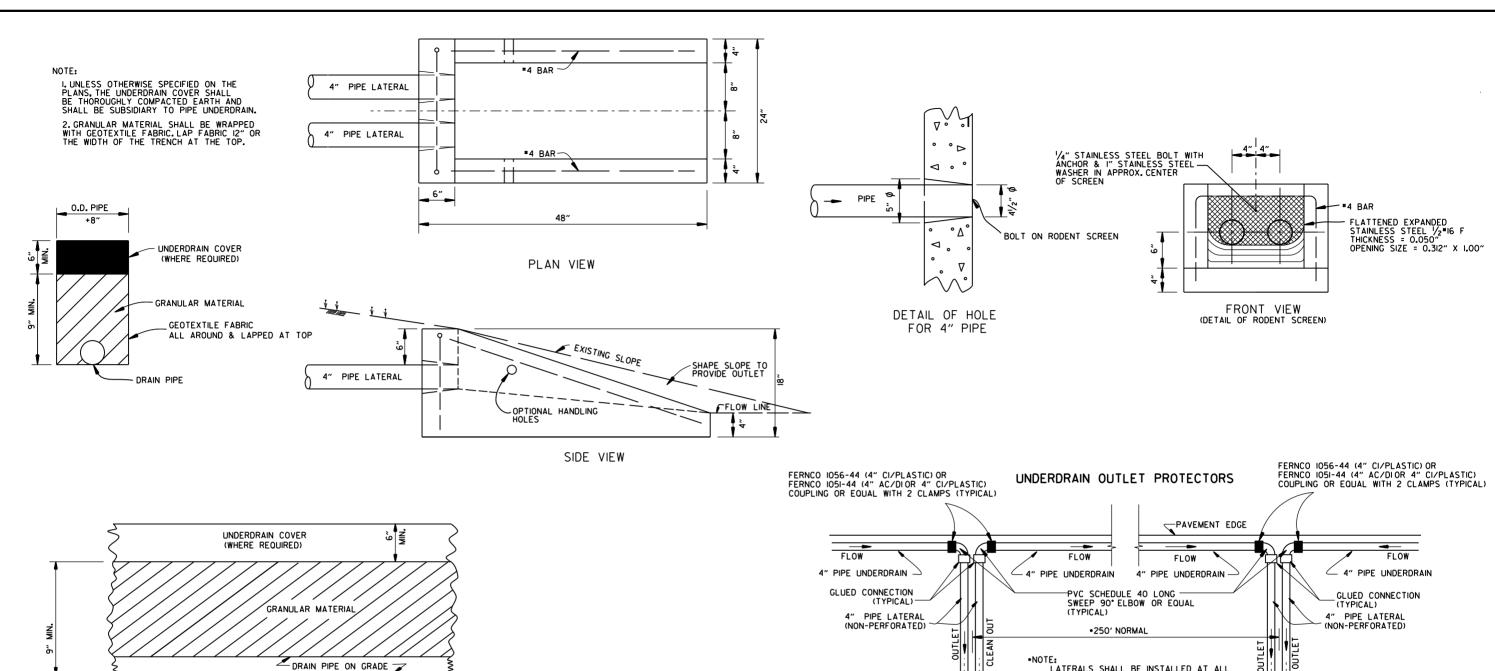
ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (POLYPROPYLENE)

STANDARD DRAWING PCP-3







DETAILS OF PIPE UNDERDRAIN

NOTES FOR PIPE UNDERDRAINS

I. GEOTEXTILE FABRIC SHALL MEET THE REQUIREMENTS OF SECTION 625 FOR TYPE I. PAYMENT FOR GEOTEXTILE FABRIC AND GRANULAR FILTER MATERIAL SHALL BE INCLUDED IN THE PRICE BID PER LIN. FT. FOR "4" PIPE UNDERDRAINS" IN ACCORDANCE WITH SECTION 611 OF THE STANDARD SPECIFICATIONS.

2.4" NON-PERFORATED SCHEDULE 40 PVC PIPE LATERALS WITH OUTLET PROTECTORS SHALL BE INSTALLED AS SHOWN HEREON, LATERALS WILL BE MEASURED AND PAID FOR AS "4" PIPE UNDERDRAINS." UNDERDRAIN OUTLET PROTECTORS WILL BE MEASURED AND PAID FOR BY THE UNIT IN ACCORDANCE WITH SECTION 611 OF THE STANDARD SPECIFICATIONS.

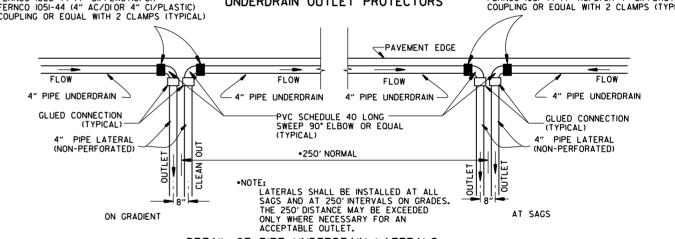
3. EXISTING 4" PIPE UNDERDRAINS MAY BE CONNECTED TO PROPOSED DROP INLETS OR EXTENDED WHERE DIRECTED BY THE ENGINEER. PAYMENT FOR CONNECTING TO DROP INLETS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR "4" PIPE UNDERDRAINS."

4. THE LOCATION OF ALL LATERALS SHALL BE MARKED WITH 4" X 12" PERMANENT PAVEMENT MARKING TAPE (TYPE III WHITE) AT THE OUTSIDE EDGE OF THE SHOULDER, PLACED TRANSVERSE TO TRAFFIC. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

5. PAYMENT FOR THE RODENT SCREEN SHALL BE INCLUDED IN THE PRICE BID PER EACH FOR "UNDERDRAIN OUTLET PROTECTORS."

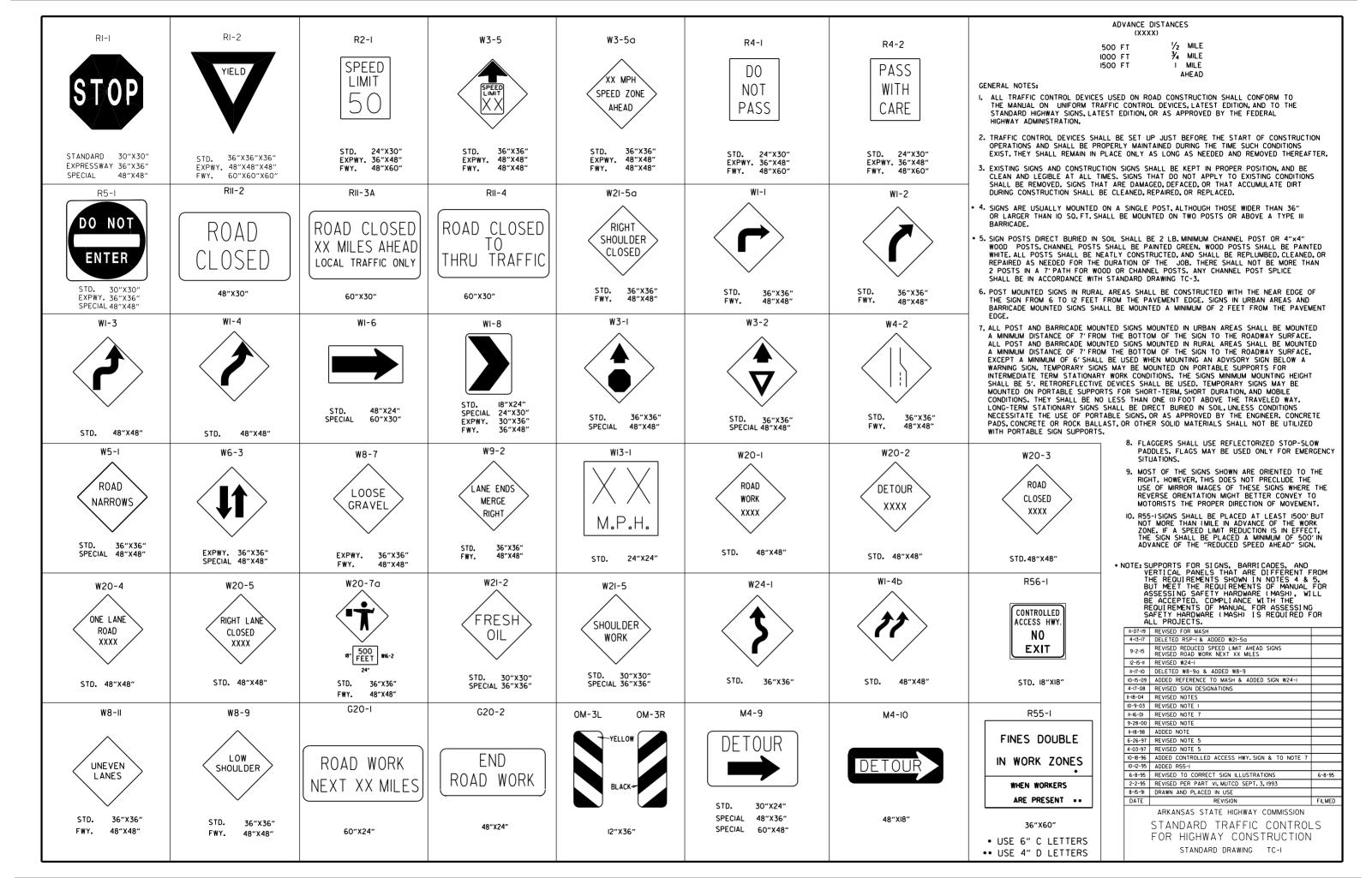
6. ANY EXISTING UNDERDRAINS THAT INTERFERE WITH INSTALLATION OF THE NEW UNDERDRAIN SYSTEM SHALL BE REMOVED AND DISPOSED OF AS DIRECTED BY THE ENGINEER, PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS. EXISTING UNDERDRAIN OUTLET PROTECTORS SHALL BE REMOVED UNDER THE ITEM "REMOVAL AND DISPOSAL OF UNDERDRAIN OUTLET PROTECTORS."

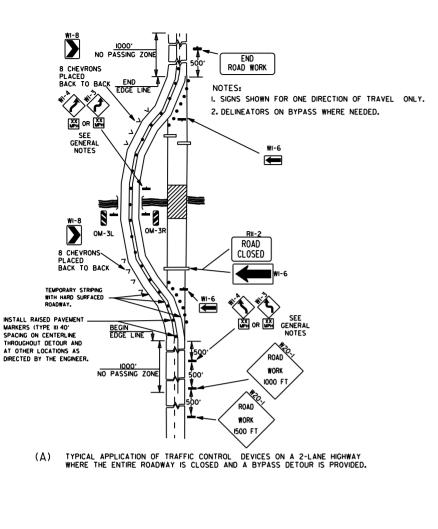
7. AT LOCATIONS WHERE A SINGLE LATERAL IS USED THE CONTRACTOR SHALL HAVE THE FOLLOWING OPTIONS: I, INSTALL OUTLET PROTECTOR AS SHOWN ON STANDARD DRAWING PU-I AND GROUT THE UNUSED HOLE OR 2. INSTALL AN OUTLET PROTECTOR WITH A SINGLE HOLE.

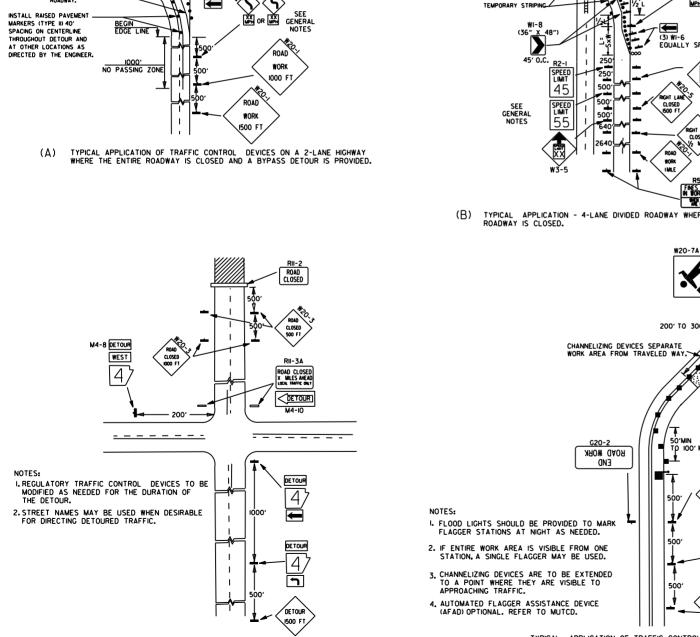


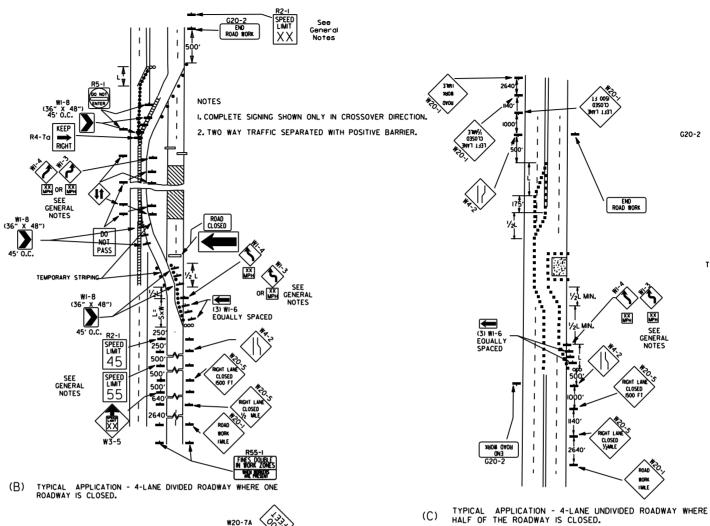
DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.

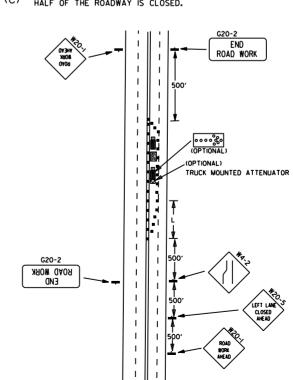
12-8-16	ADDED NOTES FOR PIPE UNDERDRAINS, REVISED RODENT SCREEN DETAIL AND NOTES, REMOVED NOTE IFOR GRANULAR MATERIAL, ADDED NOTE FOR GEOTEXTILE FABRIC		
4-10-03	REVISED NOTE 3		
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS		
11-18-98	REVISED NOTE		
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC		
4-26-96	ADDED LATERAL NOTE; 51/2" TO 5"		
II-22-95	REVISED LATERALS		
7-20-95	REVISED LATERALS & ADDED NOTE		ADVANCAC CTATE HIGHWAY COMMICCION
II- 3-94	REVISED FOR DUAL LATERALS	II- 3-94	ARKANSAS STATE HIGHWAY COMMISSION
10- 1-92	SUBSTITUTED GEOTEXTILE	10- 1-92	
8-15-91	ADDED POLYEDTHYLENE PIPE	8-15-91	DETAIL C OF DIDE LINDEDDDAIN
II- 8-90	DELETED ALTERNATE NOTE	II- 8-90	DETAILS OF PIPE UNDERDRAIN
1-25-90	ADDED 4" SNAP ADAPTER	1-25-90	
11-30-89	DEL. (SUBGRADE); ADDED (WHERE REQUIRED)	II-30-89	
7-15-88	ISSUED P.L.M.	647-7-15-88	STANDARD DRAWING PU-I
DATE	REVISION	DATE FILMED	555 5











(F) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WITH INSIDE LANE CLOSED.

POSITIVE BARRIER G20-I ARROW PANEL (IF REQUIRED) TYPE I BARRICADE CHANNELIZING DEVICE TRAFFIC DRUM RAISED PAVEMENT MARKER TYPE II A YELLOW/YELLOW PRISMATIC 0.52" DETAIL OF RAISED PAVEMENT MARKERS

KEY:

FLAGGER

TYPICAL ADVANCE WARNING SIGN PLACEMENT

TAPER FORMULAE:

L=SXW FOR SPEEDS OF 45MPH OR MORE.

 $L = \frac{WS}{60}^2$ FOR SPEEDS OF 40MPH OR LESS.

WHERE:

L= MINIMUM LENGTH OF TAPER.

S= NUMERICAL VALUE OF POSTED SPEED LIMIT PRIOR TO WORK OR 85TH PERCENTILE SPEED.

W= WIDTH OF OFFSET.

GENERAL NOTES:

I. THE MAINTENANCE DIVISION SHALL CONDUCT A BALL BANK STUDY TO DETERMINE THE ADVISORY SPEED LIMIT PRIOR TO OPENING TO TRAFFIC. THE ADVISORY SPEED WILL BE POSTED ON WI-3 OR WI-4 CURVE WARNING SIGNS. USE WI-4 WHEN SPEED IS GREATER THAN 30MPH AND WI-3 WHEN 30MPH OR LESS

30MPH OR LESS
2. WHEN THE EXISTING SPEED LIMIT IS 55MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 45MPH, THE R2-K55) SHALL BE
OMITTED AND THE W3-5 SHALL BE INSTALLED AT THAT
LOCATION, ADDITIONAL R2-145MPH SPEED LIMIT SIGNS SHALL BE
INSTALLED AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK AREA A R2-KXX)
SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

3. WHEN THE EXISTING SPEED LIMIT IS 65MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 55MPH, THE R2-145) SHALL BE OMITTED.
ADDITIONAL R2-155MPH SPEED LIMIT SIGNS SHALL BE INSTALLED
AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK

AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK
AREA A R2-(XX) SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

4. THE MAXIMUM SPACING BETWEEN CHANNELIZING DEVICES IN A TAPER
SHOULD BE APPROXIMATELY EQUAL IN FEET TO THE SPEED LIMIT.
BEYOND THE TAPER, MAXIMUM SPACING SHALL BE TWO TIMES
THE SPEED LIMIT, OR AS DIRECTED BY THE ENGINEER.

5. WARNING LIGHTS AND/OR FLAGS MAY BE MOUNTED
TO SIGNS OR CHANNELIZING DEVICES AT NIGHT AS NEEDED.

6. PAVEMENT MARKINGS NO LONGER APPLICABLE WHICH MIGHT CREATE CONFUSION IN THE MINDS OF VEHICLE OPERATORS SHALL BE REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

7. TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE DELINEATED BY AFFIXING CONSPICUITY MATERIAL IN A CONTINUOUS LINE ON THE FACE OF THE TRAILER, WHEN PLACED ON ON A DAJACENT TO THE SHOULDER AND NOT BEHIND A POSITIVE BARRIER, THESE DEVICES SHALL BE DELINEATED BY PLACING FIVE (5) TRAFFIC DRUMS, EQUALLY SPACED ALONG THE TRAFFIC SIDE OF THE DEVICE, PAYMENT FOR TRAFFIC DRUMS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR VARIOUS TRAILER MOUNTED DEVICES.

B. DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL.THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE ARDOT QUALIFIED PRODUCTS LIST.

ALL TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL MEET THE REQUIREMENTS OF THE MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).

05-20-21	REVISED NOTE 7	
II-07-I9	REVISED NOTE I, ADDED NOTE 9	
9-2-15	REVISED NOTE 2, ADDED NOTE 8, REVISED DRAWING (A) & REPLACED R2-5A WITH W3-5	
9-12-13	REVISED DETAIL OF RAISED PAVEMENT MARKERS	
3-11-10	ADDED (AFAD)	
II-20-08	REVISED SIGN DESIGNATIONS	
II-I8-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-I	
4-26-96	CORRECTED (a) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON WI-4A	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED

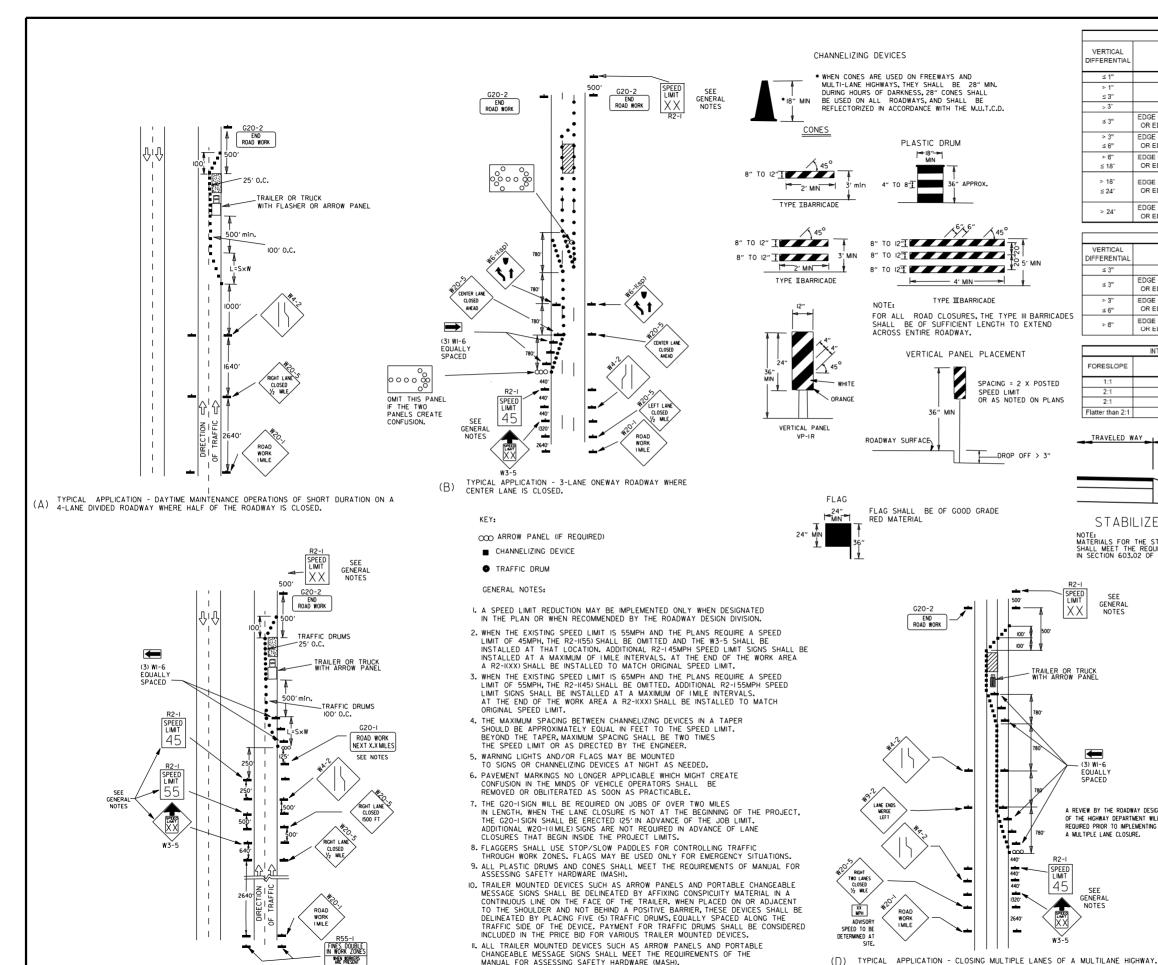
ARKANSAS STATE HIGHWAY COMMISSION

STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION

STANDARD DRAWING TC-2

TYPICAL APPLICATION - ROADWAY CLOSED BEYOND DETOUR POINT.

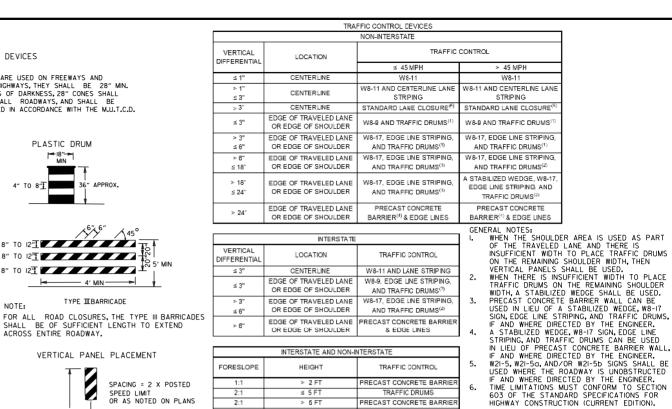
(E) TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES ON 2-LANE HIGHWAY WHERE ONE LANE IS CLOSED AND FLAGGING IS PROVIDED.



TYPICAL APPLICATION - CONSTRUCTION OPERATIONS OF INTERMEDIATE TO LONG TERM

DURATION ON A 4-LANE DIVIDED ROADWAY WHERE HALF OF THE ROADWAY IS CLOSED.

MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).



TRAFFIC DRIIMS

PRECAST CONCRETE BARRIE

HIGHWAY CONSTRUCTION (CURRENT EDITION).

≤ 5 FT

> 5 FT

TYPE III BARRICADE

VERTICAL PANEL PLACEMENT

END ROAD WORK

SPACING = 2 X POSTED

OR AS NOTED ON PLANS

DROP OFF > 3"

= 100°

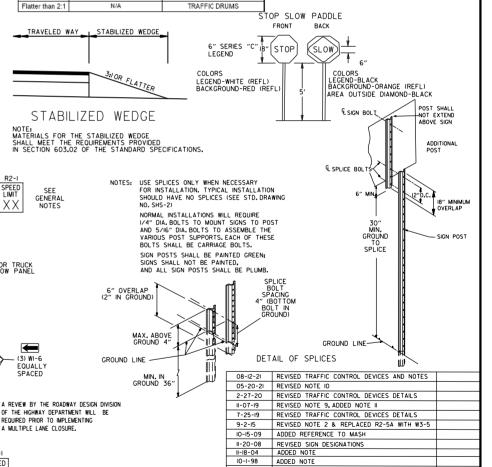
000 V

SPEED

45

NOTES

SPEED LIMIT



4-03-97

DATE

DEVICES NOTE

6-8-95 REVISED SPLICE DETAIL, TEXT

STANDARD DRAWING

8-I5-9I DRAWN AND PLACED IN USE

10-12-95 MOVED UPPER SPLICE

IO-I8-96 ADDED R55-I

ADDED (SP) TO W6-I & REVISED TRAFFIC CONTROL

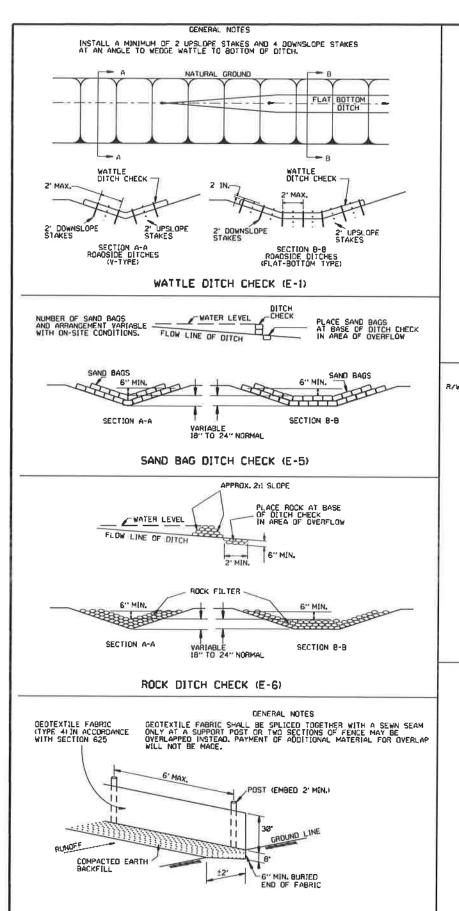
ARKANSAS STATE HIGHWAY COMMISSION

FOR HIGHWAY CONSTRUCTION

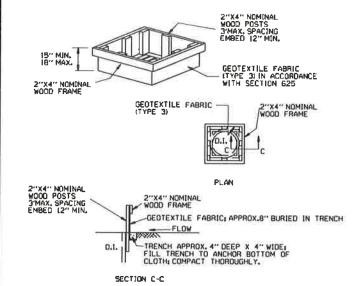
STANDARD TRAFFIC CONTROLS

2-2-95 REVISED PER PART VI, MUTCD, SEPT. 3, 1993

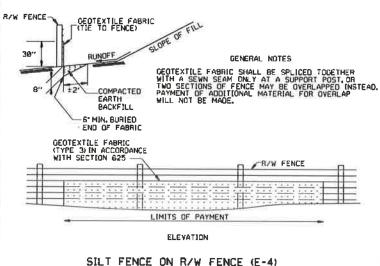
6-8-95



SILT FENCE (E-11)



DROP INLET SILT FENCE (E-7)

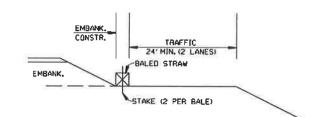


GENERAL NOTES

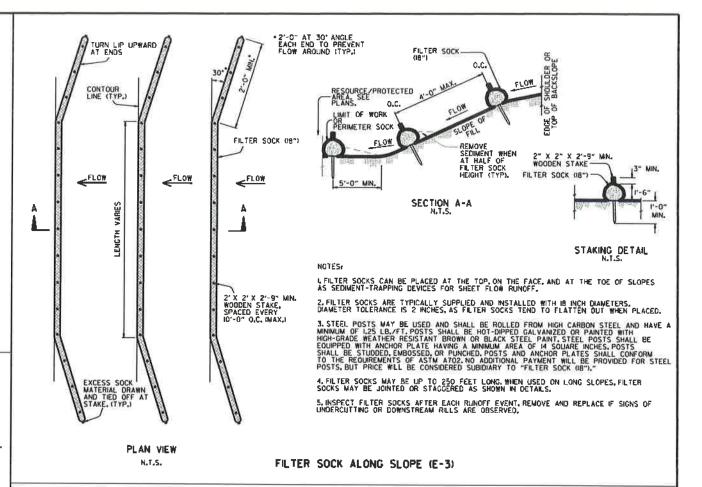
I.STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.

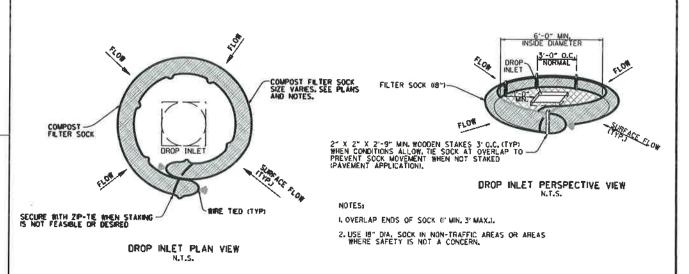
2. NO GAPS SHALL BE LEFT BETWEEN BALES.

3. BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.



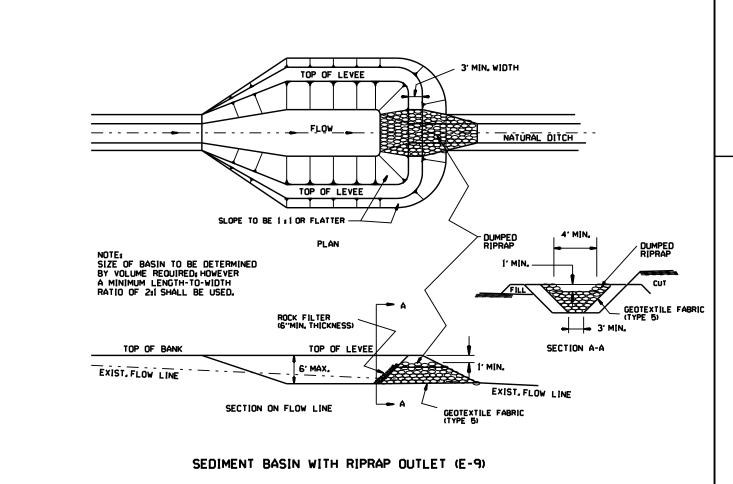
BALED STRAW FILTER BARRIER (E-2)

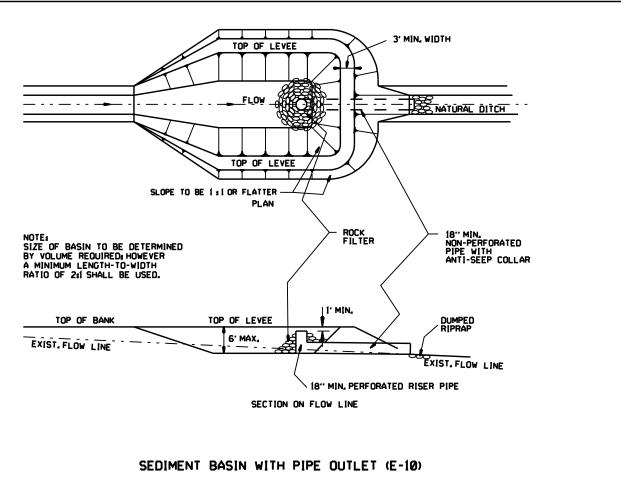


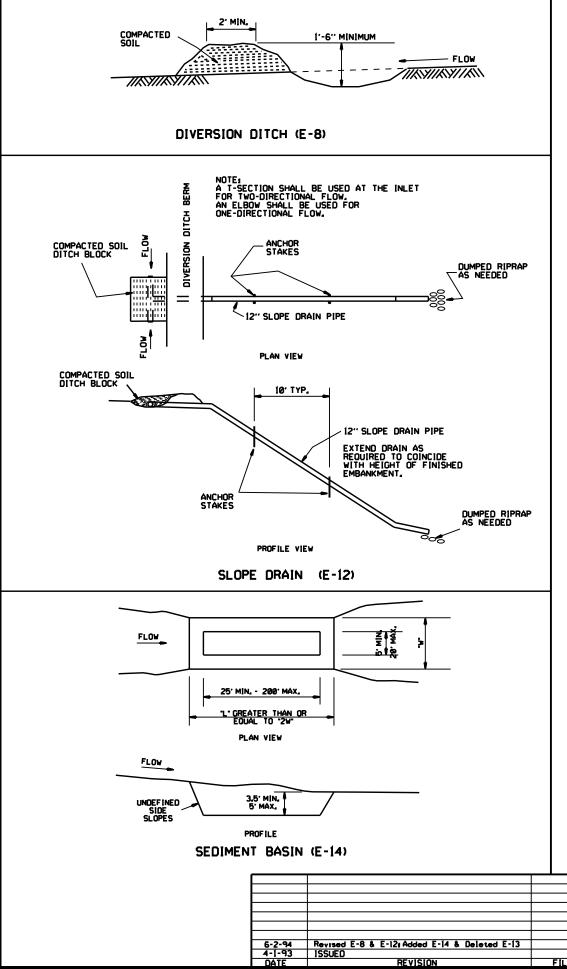


COMPOST FILTER SOCK DROP INLET PROTECTION (E-I3)

11-16-17	ADDED FILTER SOCK E-3 AND E-13		
12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ADVANCAS STATE INCUMENT CONTRACTION
11-18-98	ADDED NOTES		ARKANSAS STATE HIGHWAY COMMISSION
07-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)		
07-20-95	REVISED SILT FENCE E-4 AND E-II	7-20-95	TEMPORARY EROSION
07-15-94	REV. E-4 & E-II MIN. 13" BURIED END OF FABRIC		I LIMI ONANTI LINGSION
06-02-94	REVISED E-1,4.7 & II; DELETED E-2 & 3	6-2-94	CONTROL DEVICES
04-01-93	REDRAWN		CONTROL DEVICES
10-01-92	REDRAWN		
08-02-76	ISSUED R.D.M.	298-7-28-76	STANDARD DRAWING TEC-I
DATE	REVISION	FILMED	STANDARD DRAWING TECT







ARKANSAS STATE HIGHWAY COMMISSION

TEMPORARY EROSION CONTROL DEVICES

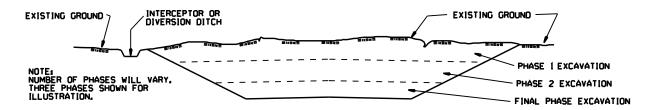
STANDARD DRAWING TEC-2

CLEARING AND GRUBBING

CONSTRUCTION SEQUENCE

- 1. PLACE PERIMETER CONTROLS (I.E. SILT FENCES , DIVERSION DITCHES, SEDIMENT BASINS, ETC.)
- 2. PERFORM CLEARING AND GRUBBING OPERATION.

EXCAVATION



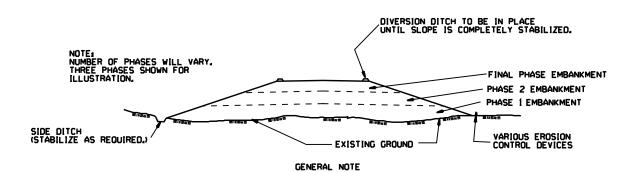
GENERAL NOTE

ALL CUT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

- 1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
- 2. PERFORM PHASE 1 EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING.
- 3. PERFORM PHASE 2 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
- 4. PERFORM FINAL PHASE OF EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING, STABILIZE DITCHES, CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

EMBANKMENT



ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.

2. PLACE PHASE 1 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING.
PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION
IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

			ADV
			AKK
11-03-94	CORRECTED SPELLING		
6-2-94	Drawn & Issued	6-2-94	
			l
DATE	REVISION	FILMED	

KANSAS STATE HIGHWAY COMMISSION

TEMPORARY EROSION CONTROL DEVICES

STANDARD DRAWING TEC-3