ARKANSAS DEPARTMENT OF TRANSPORTATION CONSTRUCTION PLANS FOR STATE HIGHWAY

JACKS CREEK STR. & APPRS. (S)

GREENE COUNTY ROUTE 135 SECTION 5 JOB 101013

FED. AID PROJ. NHPP-0028(54)

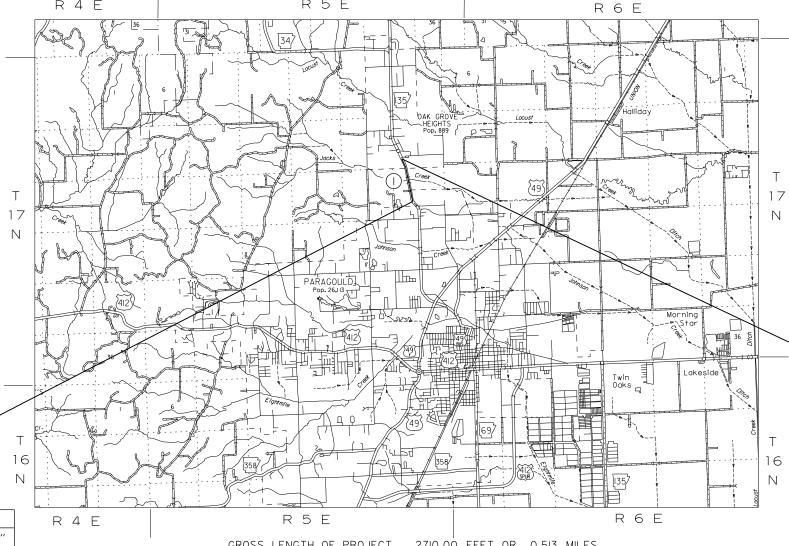
NOT TO SCALE

ARK. 101013 JACKS CREEK STR. & APPRS. (S)

ARKANSAS HIGHWAY DISTRICT 10

BRIDGE CONSTRUCTION DATA

() STA. II6+20.00 BRIDGE END BRIDGE NO. 07514 OVER JACKS CREEK I59'-0" INTEGRAL PRESTRESSED BOX BEAM UNIT (49.50', 60', 49.50') 40'-0" CLEAR ROADWAY 160'-0" BRIDGE LENGTH STA. 117+80.00 BRIDGE END



DESIGN TRAFFIC DATA DESIGN YEAR ----- 2043

DESIGN TEAN	201	_
2023 ADT		
2043 ADT	8,20	00
2043 DHV	902	
DIRECTIONAL DISTRIBUTION	60%	
TRUCKS	5%	
DESIGN SPEED	60	MPH

STA.129+60.00 END JOB 101013

N

STA. 102+50.00 BEGIN JOB 101013 L.M. 2.76

PROJECT COORDINATES

	BEGIN	MID-POINT	END
LATITUDE	N 36°06′13″	N 36°06′26″	N 36°06′39″
LONGITUDE	W 90°30′18″	W 90°30′20″	W 90°30′23″
STATION	102+50.00	116+05.00	129+60.00





R 4 E

" PROJECT

GROSS LENGTH OF PROJECT 2710.00 FEET OR 0.513 MILES NET " ROADWAY 2550.00 " " 0.483 MILES NET " PROJECT 2710.00 " " 0.030 MILES NET " PROJECT 2710.00 " " 0.513 MILES

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB	NO.	101013	2	70

2 INDEX OF SHEETS AND STANDARD DRAWINGS

ARKANSAS

LICENSED

PROFESSION ALTO

ENGINEER

No.14994

Digitally Signed 08/10/2023

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55005	STANDARD DETAILS FOR PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS	03-24-16
55010	STANDARD DETAILS FOR TYPE D BRIDGE NAME PLATE	04-14-23
55021	STANDARD DETAILS FOR CONCRETE FILLED STEEL SHELL PILES AND PILE ENCASEMENTS	03-24-16
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TC-4 STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION-TEMPO	
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WF-4 WIRE FENCE TYPE C AND D	08-22-02

JOB 101013__ WATER GATES

GOVERNING SPECIFICATIONS

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS:

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FHWA-1273_	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTICE TO CONTRACTORS
FHWA-1273_	_ SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 140)
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
FHWA-1273_	_ SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
FHWA-1273_	_ SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
FHWA-1273_	SUPPLEMENT - WAGE RATE DETERMINATION
100-3	_ CONTRACTOR'S LICENSE
100-4	_ DEPARTMENT NAME CHANGE
102-2	SSUANCE OF PROPOSALS
105-4	_ MAINTENANCE DURING CONSTRUCTION
107-2	RESTRAINING CONDITIONS
108-1	_ LIQUIDATED DAMAGES
108-2	_ WORK ALLOWED PRIOR TO ISSUANCE OF WORK ORDER
110-1	PROTECTION OF WATER QUALITY AND WETLANDS
210-1	UNCLASSIFIED EXCAVATION
303-1	_ AGGREGATE BASE COURSE
306-1	_ QUALITY CONTROL AND ACCEPTANCE
307-1	
308-1	_ CEMENT
	_ TACK COATS
400-4	_ DESIGN AND QUALITY CONTROL OF ASPHALT MIXTURES
400-5	PERCENT AIR VOIDS FOR ACHM MIX DESIGNS
400-6	_ LIQUID ANTI-STRIP ADDITIVE
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	BROADBAND INTERNET SERVICE FOR ASPHALT CONCRETE PLANT
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(2) GOVERNING SPECIFICATIONS AND GENERAL NOTES

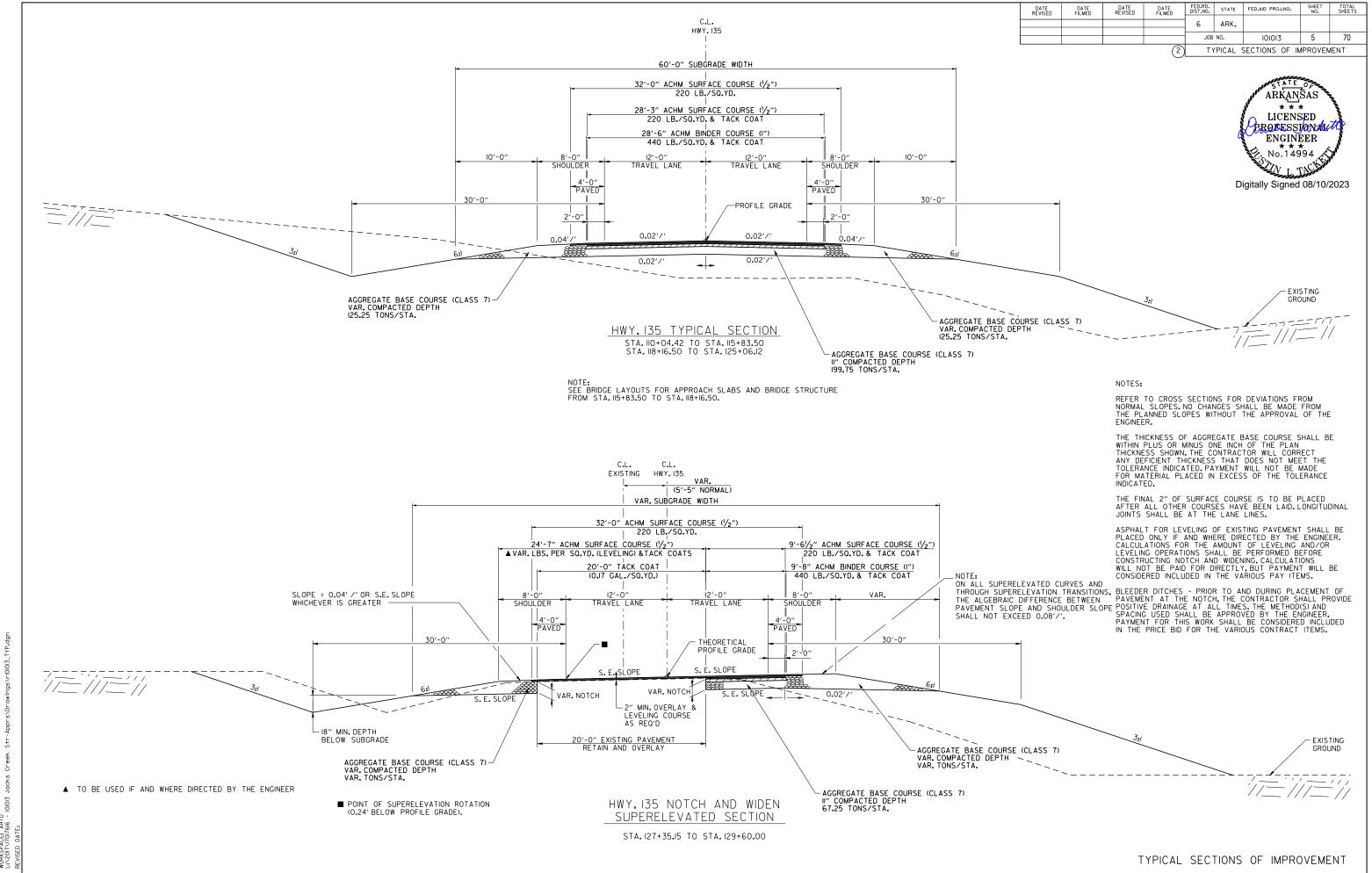


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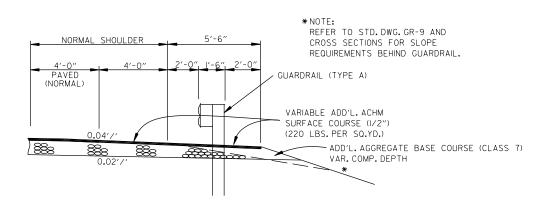
GENERAL NOTES

- 1. GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- 2. ALL PIPE LINES, POWER, TELEPHONE, AND TELEGRAPH LINES TO BE MOVED OR LOWERED BY THE RESPECTIVE OWNERS AS PER AGREEMENT WITH SUCH OWNERS.
- 3. ANY EQUIPMENT OR APPURTENANCE THAT INTERFERES WITH THE PROPOSED CONSTRUCTION AND WHICH MAY BE THE PROPERTY OF UTILITY SERVICE ORGANIZATIONS SHALL BE MOVED BY THE OWNERS UNLESS OTHERWISE PROVIDED.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING U. S. MAILBOXES WITHIN THE PROJECT LIMITS IN SUCH A MANNER THAT THE PUBLIC MAY RECEIVE CONTINUED MAIL SERVICE. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS BID ITEMS.
- 5. ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- 6. ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED AS DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO ENSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A FENCE TO CONTROL LIVESTOCK IN AREAS WHERE PASTURES ARE SEVERED. WIRE FENCE MAY BE CONSTRUCTED INITIALLY, OR IN LIEU THEREOF, THE CONTRACTOR AT HIS OWN EXPENSE, MAY ELECT TO PROVIDE TEMPORARY FENCING SUITABLE TO CONTAIN LIVESTOCK.
- 8. THE SEQUENCE AS SHOWN ON THE MAINTENANCE OF TRAFFIC PLANS IS A GENERAL OUTLINE FOR THE CONSTRUCTION OF THIS PROJECT, AND IN NO WAY IS IT INTENDED TO COVER EVERY ITEM IN THE PROJECT. ITEMS NOT CRITICAL TO THE CONSTRUCTION SEQUENCE MAY BE CONSTRUCTED IN ANY STAGE AS APPROVED BY THE RESIDENT ENGINEER.
- 9. ALL FLEXIBLE BASE AND ASPHALTIC PAVEMENTS REMOVED SHALL BE PAID FOR UNDER THE ITEM NO. 210 - UNCLASSIFIED EXCAVATION.
- 10. THE EXISTING ASPHALT PAVEMENT TO BE REMOVED FROM THE REMAINING PAVEMENT SHALL BE SEPARATED BY SAWING ALONG A NEAT LINE. AFTER SAWING, THE PAVEMENT TO BE REMOVED SHALL BE CAREFULLY REMOVED IN A MANNER THAT WILL NOT DAMAGE THE PAVEMENT THAT IS TO REMAIN. ANY DAMAGE OF THE ASPHALT PAVEMENT THAT IS TO REMAIN IN PLACE SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.
- 11. THIS PROJECT IS COVERED UNDER A SECTION 404 NATIONWIDE 23 PERMIT. REFER TO SECTION 110 OF THE STANDARD SPECIFICATIONS, EDITION OF 2014, FOR PERMIT REQUIREMENTS.

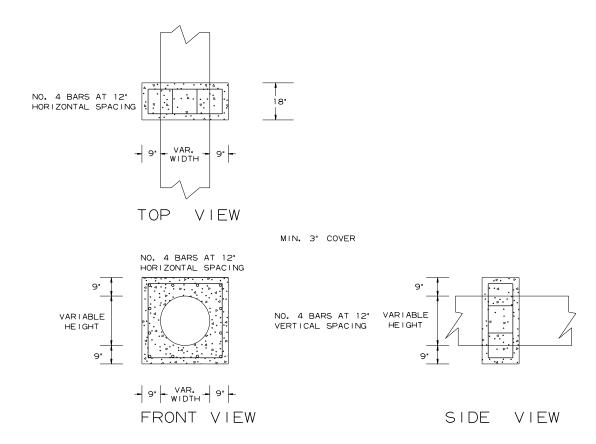
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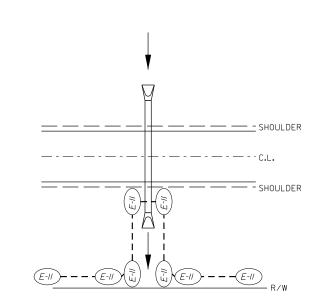


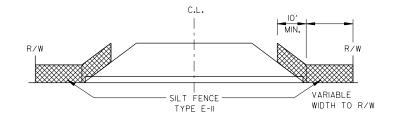


WIDENING FOR GUARDRAIL AT PROPOSED SHOULDER EDGE



SINGLE PIPE EXTENSION REINFORCED CONCRETE COLLAR DETAIL





DETAILS OF SILT FENCE AT CROSS DRAINS

SPECIAL DETAILS

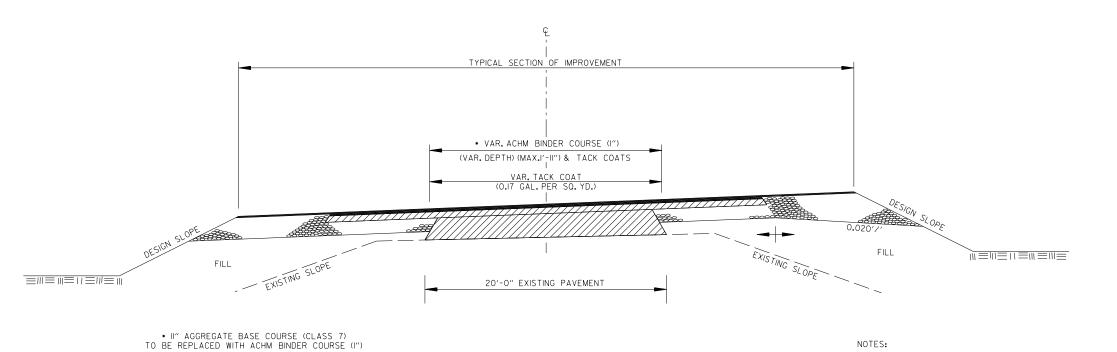
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SPECIAL DETAILS

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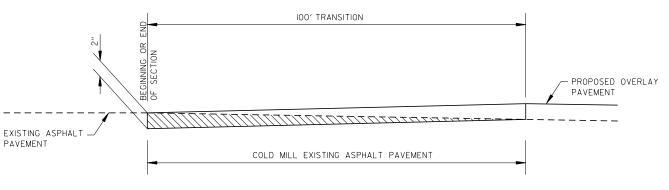
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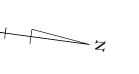
METHOD OF RAISING GRADE

NOTES:

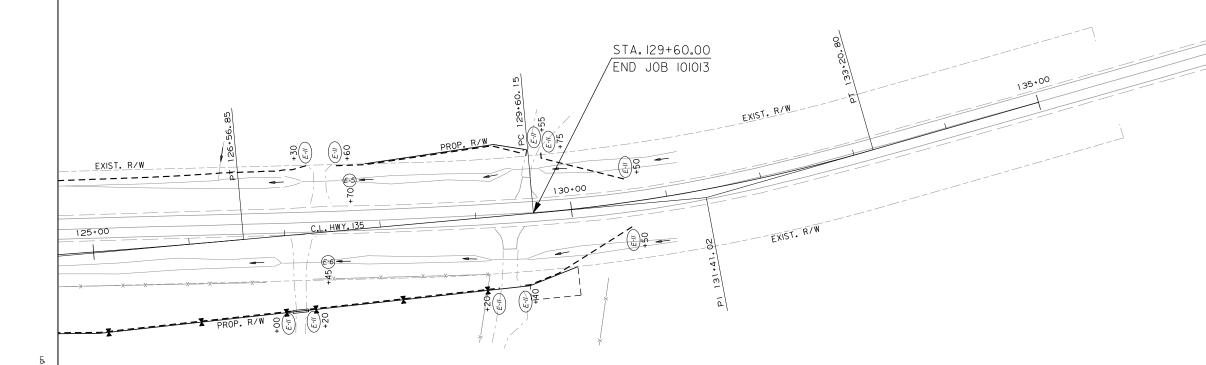
- (I) THIS DETAIL TO BE USED ONLY IF AND WHERE DIRECTED BY THE ENGINEER.
- (2) QUANTITIES FOR METHOD OF GRADE RAISE USING ASPHALT WERE CALCULATED ON THIS PROJECT AT LOCATIONS WHERE THE DISTANCE BETWEEN THE EXISTING ASPHALT ROADWAY AND THE PROPOSED SUBGRADE WAS ONE FOOT OR LESS.
- (3) IN LOCATIONS WHERE THE DISTANCE BETWEEN THE PROPOSED SUBGRADE AND THE EXISTING ASPHALT ROADWAY IS MORE THAN ONE FOOT, SCARIFICATION OF THE EXISTING ASPHALT ROADWAY WILL BE REQUIRED AS STATED IN SECTION 210, SUBSECTION 210.09, OF THE STANDARD SPECIFICATIONS.



DATE REVISED DATE FILMED DATE REVISED DATE FILMED FED.RD. STATE FED.AID PROJ.NO. ARK. JOB NO. 101013 10 70 TEMPORARY EROSION CONTROL DETAILS HWY.135 (E-//) SILT FENCE LIN. FT. STA. 127+20 TO 129+20 STA. 127+60 TO 129+55 STA. 129+40 TO 130+50 STA. 129+75 TO 130+50 200 200 132 96







REVISIONS

DATE	REVI SI ON

<u>LEGEND</u>

€-5 = SAND BAG DITCH CHECKS

€-6 = ROCK DITCH CHECKS

E-II = SILT FENCE

EROSION CONTROL MEASURES TO BE PLACED DURING APPROPRIATE STAGES.
THESE DEVICES SHALL BE LEFT IN PLACE AS LONG AS REQUIRED TO CONTROL EROSION.

SAND BAG DITCH CHECK

ROCK DITCH CHECK

STA. 127+70

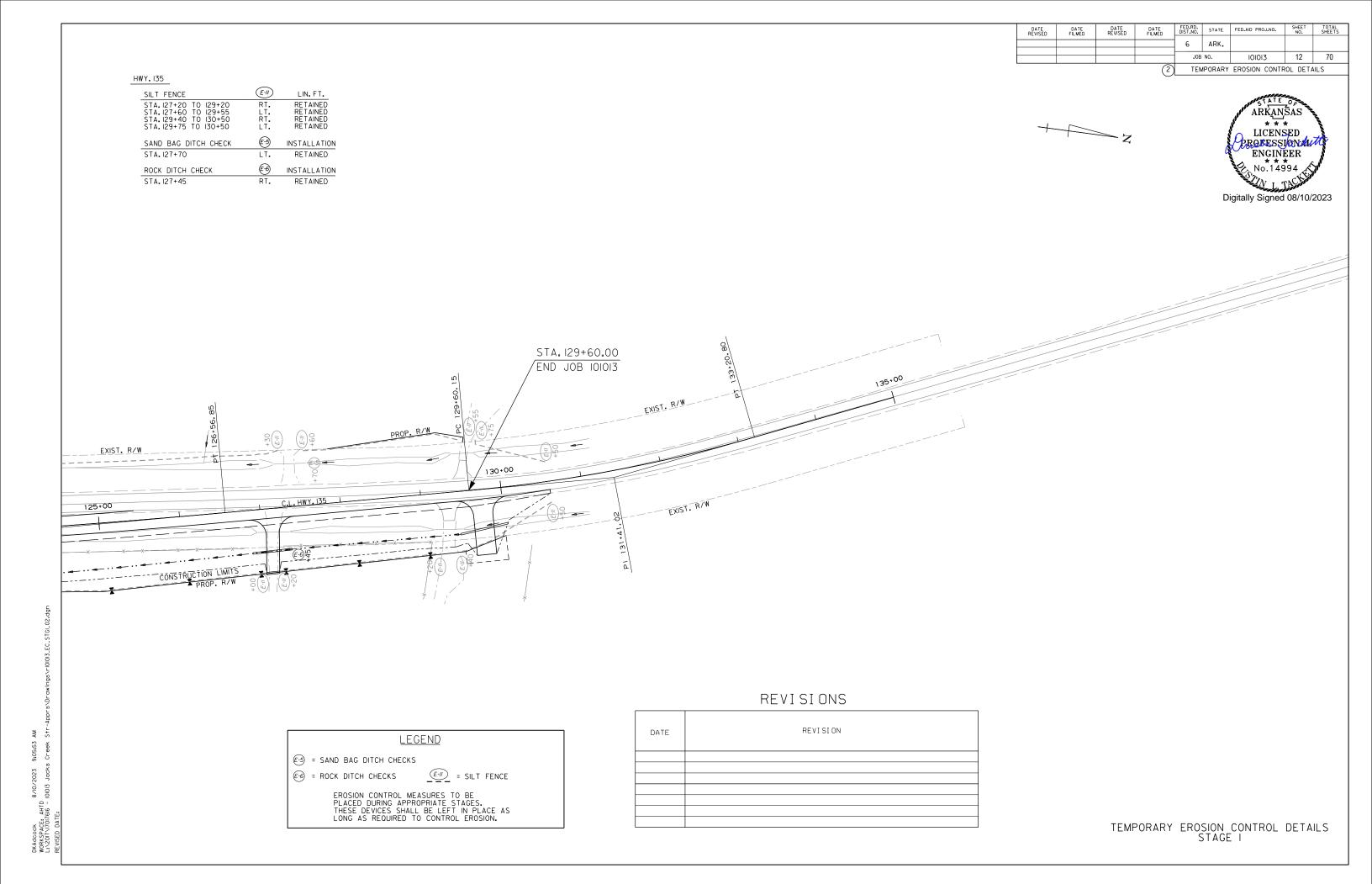
STA. 127+45

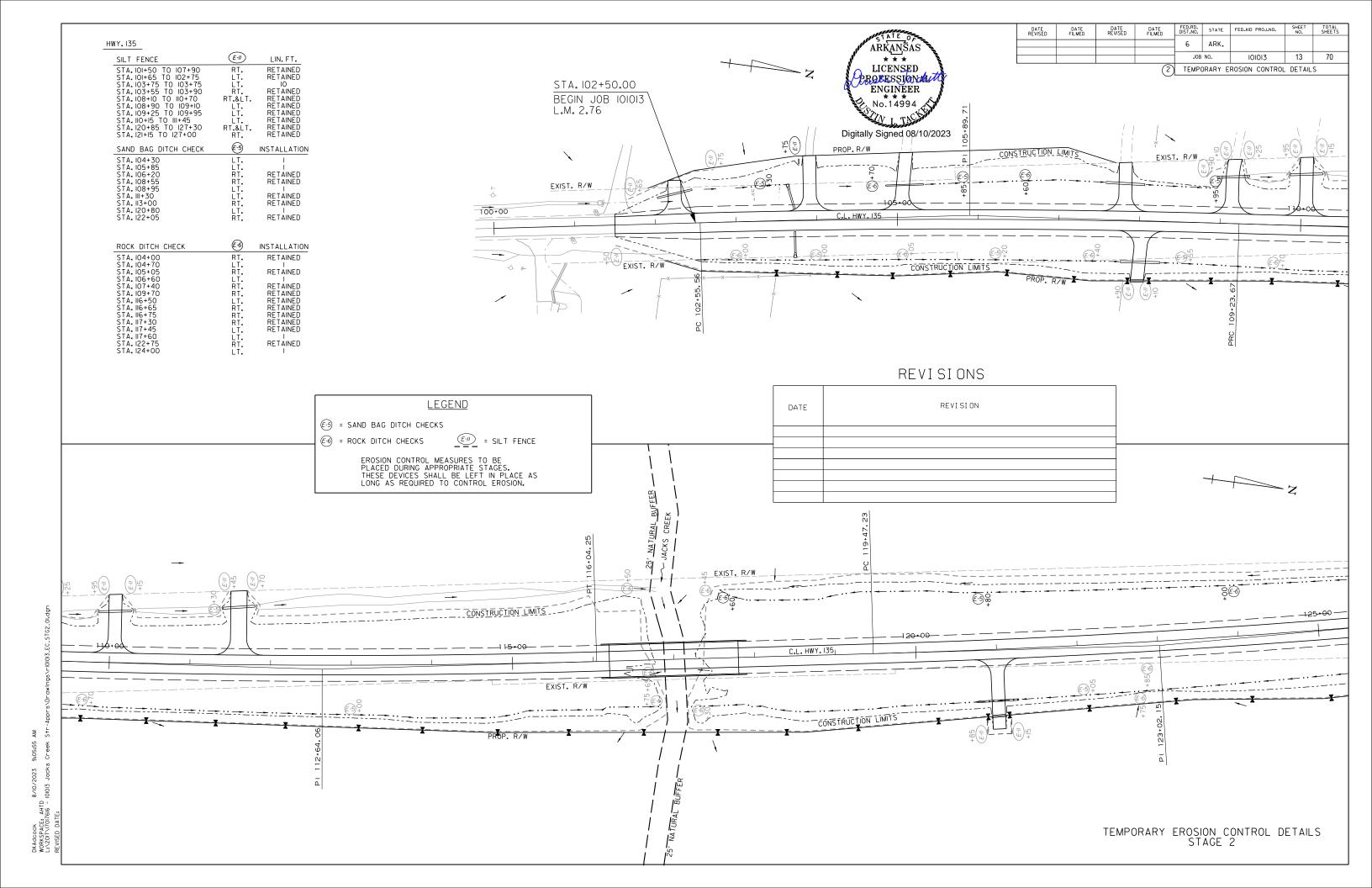
INSTALLATION

INSTALLATION

LT.

TEMPORARY EROSION CONTROL DETAILS CLEARING AND GRUBBING STAGE

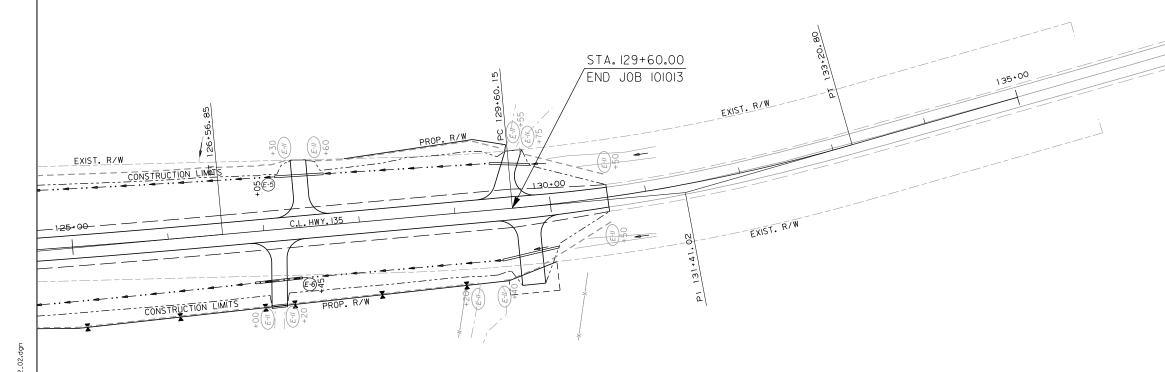




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€-5 = SAND BAG DITCH CHECKS

€-6 = ROCK DITCH CHECKS

E-II = SILT FENCE

EROSION CONTROL MEASURES TO BE PLACED DURING APPROPRIATE STAGES.
THESE DEVICES SHALL BE LEFT IN PLACE AS LONG AS REQUIRED TO CONTROL EROSION.

STA. 127+20 TO 129+20 STA. 127+60 TO 129+55 STA. 129+40 TO 130+50 STA. 129+75 TO 130+50

SAND BAG DITCH CHECK

STA. 127+05

ROCK DITCH CHECK

STA. 127+45

RETAINED RETAINED RETAINED RETAINED

INSTALLATION

INSTALLATION

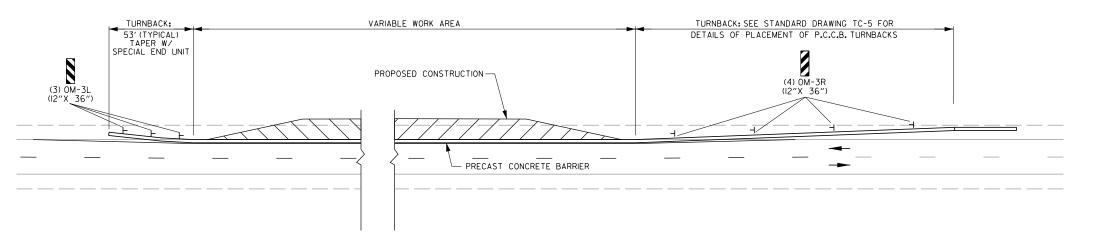
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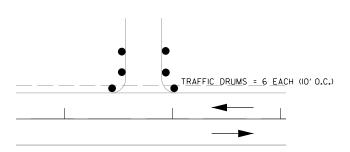
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				6	ARK.			
				JOB	NO.	101013	15	70
			MAINTENANCE OF TRAFFIC DETAILS			ALLC		

MAINTENANCE OF TRAFFIC DETAILS

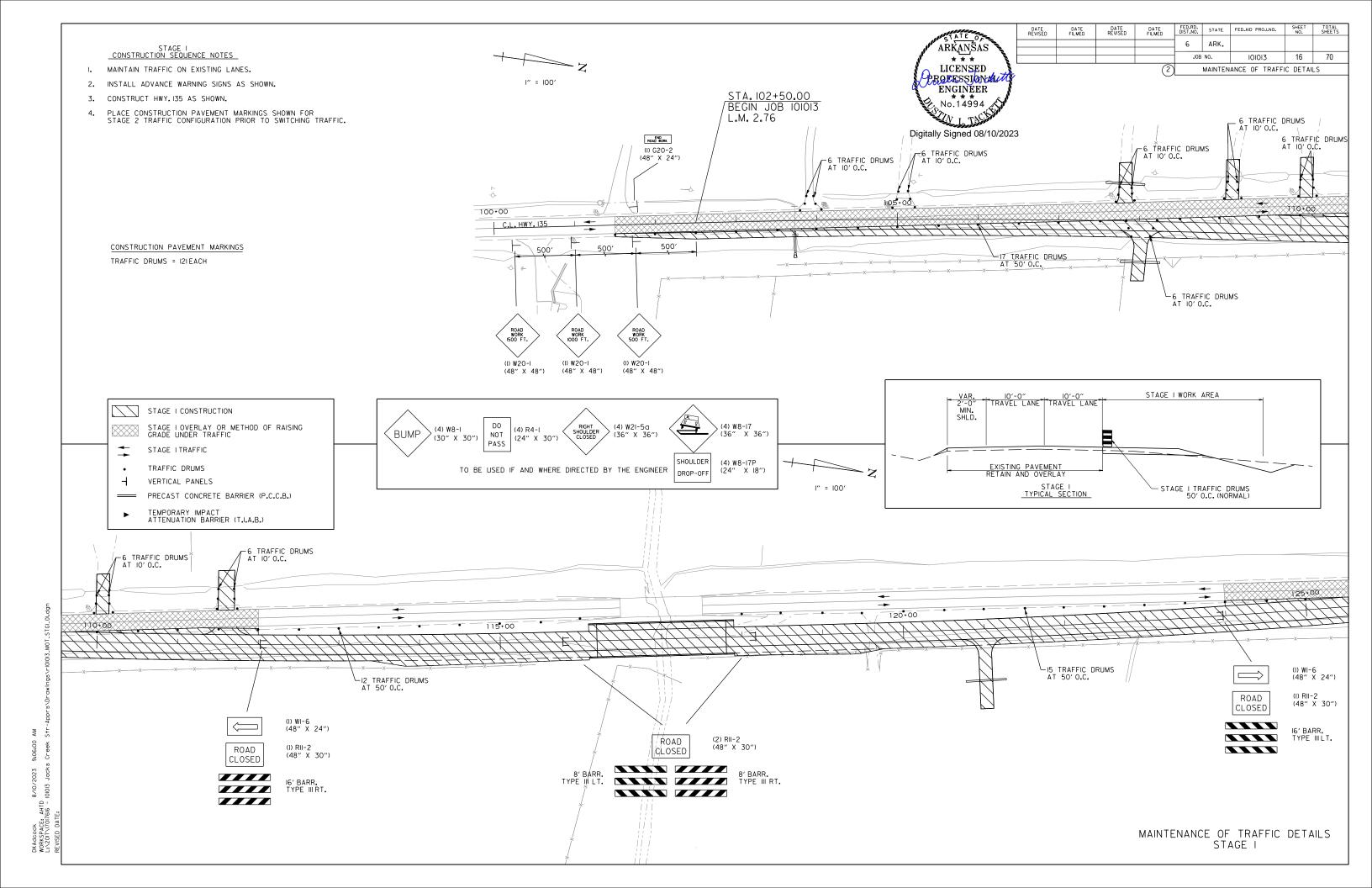


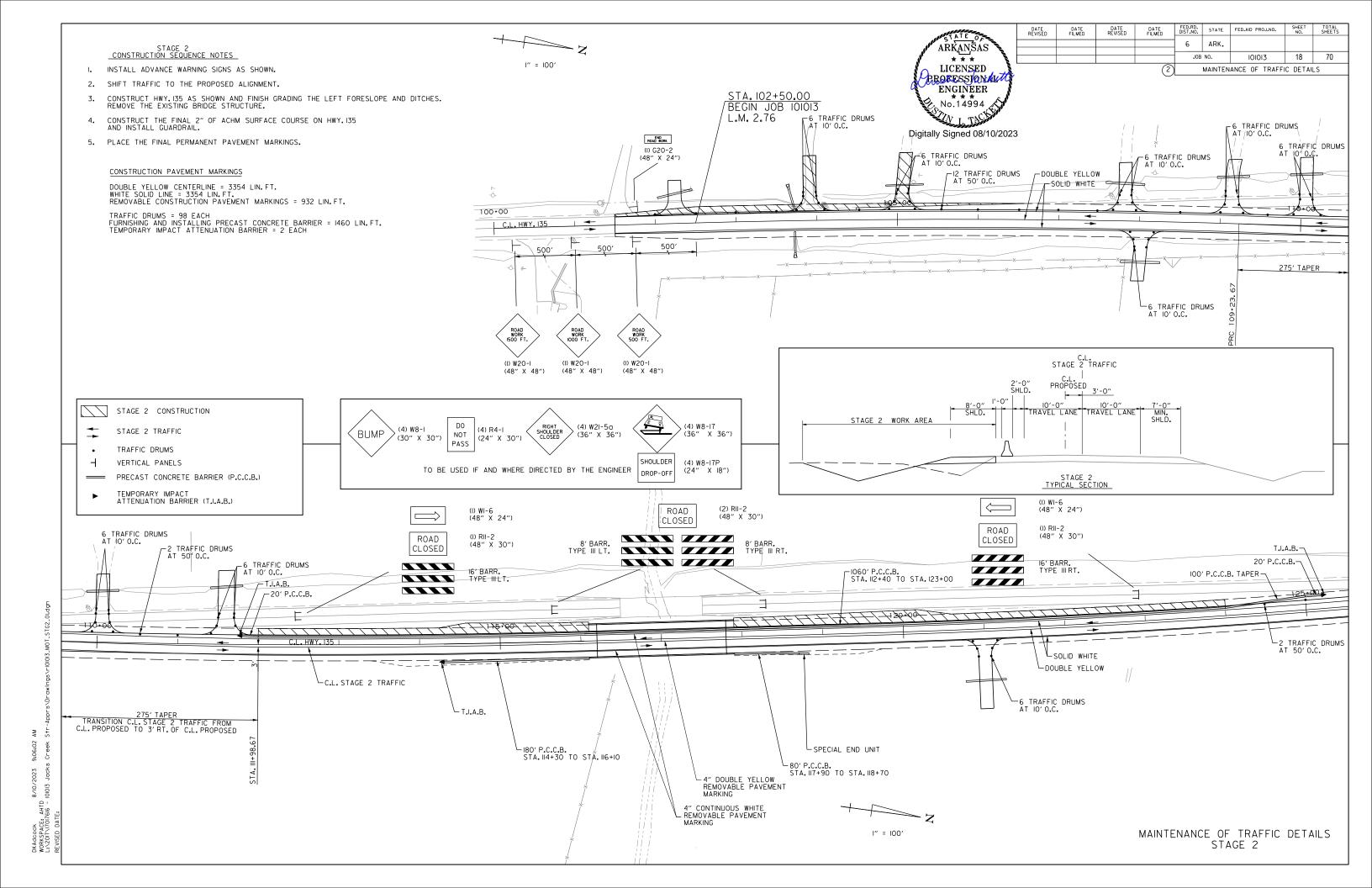


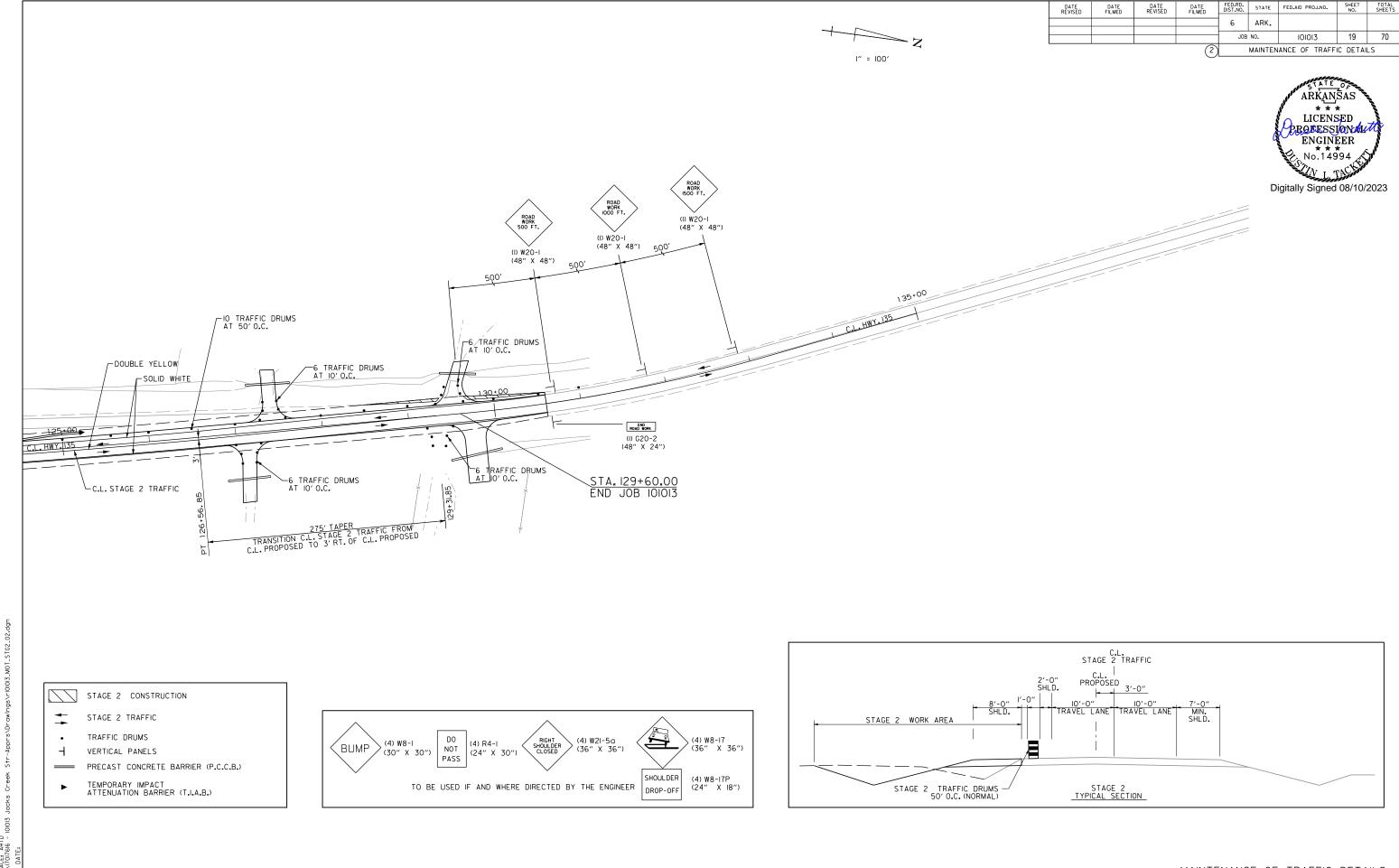
DETAIL OF OBJECT MARKERS AT PRECAST CONCRETE BARRIER TURNBACKS

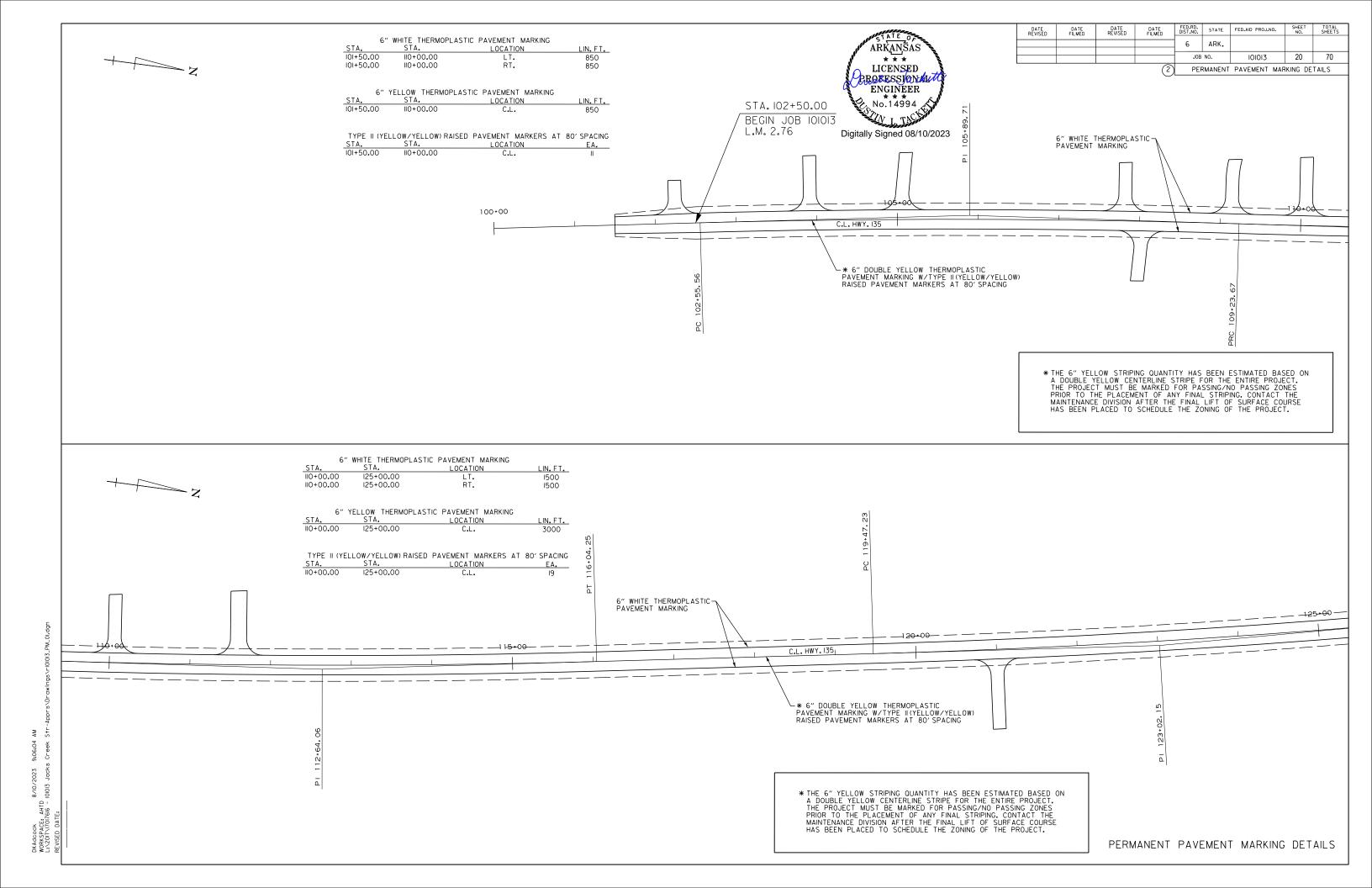


DRIVEWAY/TRAFFIC DRUM DETAIL









6	" WHITE TH	HERMOPLASTIC	PAVEMENT	MARKING	
STA.	STA.		LOCATIO	N	LIN. FT.
125+00.00	130+6	0.00	LT.		560
125+00.00	130+6	0.00	RT.		560

6′	YELLOW	THERMOPLASTIC	PAVEMENT	MARKING	
STA.	STA		LOCATIO	N	LIN. FT.
125+00.00	130+	60.00	C.L.		1120

TYPE II (YEL	LOW/YELLOW) RAISED	PAVEMENT MARKERS	ΑТ	80' SPACING
STA.	STA.	LOCATION		EA.
125+00.00	130+60.00	C.L.		7

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				J0B	NO.	101013	21	70
)					

2 PERMANENT PAVEMENT MARKING DETAILS

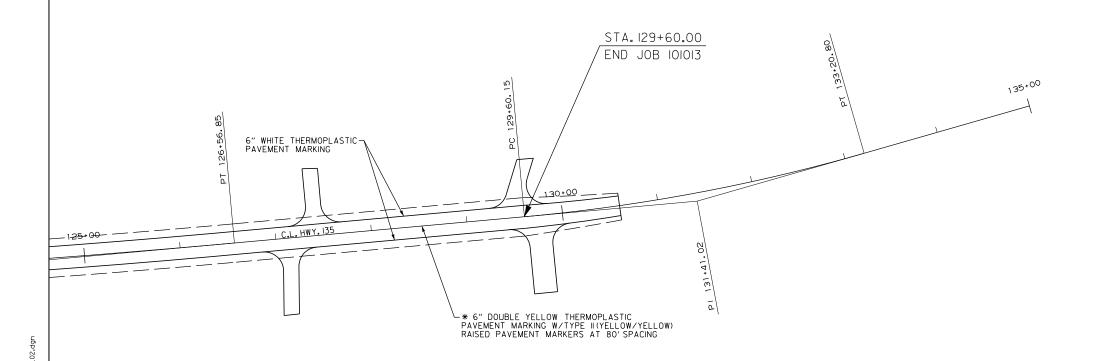
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PROFESSIONAL
ENGINEER

No.14994

Digitally Signed 08/10/2023



*THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT. THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING. CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				J0B	NO.	101013	22	70
			2			SOIL BORING LO)G	



SOIL BORING LOG										
BORING	APPROX. STATION	SAMPLE	WATER		TTERBERG		PERCENT	UNIFIED	AASHTO	
or TEST PIT NO.	APPROX. STATION	DEPTH (ft)	CONTENT (%)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	PASSING #200, %	CLASS.	CLASS.	
1	115+62, 36' LT	6	. ,				94	ML	A-4	
1	115+62, 36' LT	8.5					98	ML	A-4	
1	115+62, 36' LT	13		25	22	3	77	ML	A-4	
1	115+62, 36' LT	23		35	22	13	81	CL	A-6	
1	115+62, 36' LT	33.5		21	16	5	42	SC-SM	A-4	
1	115+62, 36' LT	58.5		31	16	15	82	CL	A-6	
3	118+43, 34' LT	3.5		37	21	16	74	CL	A-6	
3	118+43, 34' LT	6					77	ML	A-4	
3	118+43, 34' LT	8.5					81	ML	A-4	
3	118+43, 34' LT	13.5		28	20	8	86	CL	A-4	
3	118+43, 34' LT	18.5		48	21	27	95	CL	A-7-6	
3	118+43, 34' LT	23.5					29	SM	A-2-4	
3	118+43, 34' LT	28.5		35	22	13	99	CL	A-6	
3	118+43, 34' LT	38.5					4	SP	A-1-b	
3	118+43, 34' LT	48.5					32	SM	A-2-4	
3	118+43, 34' LT	98.5		24	13	11	81	CL	A-6	

SOIL CHARACTERISTICS TABULATED ABOVE ARE REPRESENTATIVE AT THE LOCATION OF THE SAMPLE, AND FROM SURFACE INDICATIONS ARE TYPICAL FOR THE LIMIT SHOWN. THESE DATA ARE SHOWN FOR INFORMATION ONLY. THE STATE WILL NOT BE RESPONSIBLE FOR VARIATIONS IN THE SOIL CHARACTERISTICS AND/OR EXTENT OF SAME DIFFERING FROM THE ABOVE TABULATIONS.

	CONCINCOTION I AVEINENT MANNING AND I ENMANENT I AVEINENT MANNING										
DESCRIPTION	STAGE 2 END OF PAVEMENT P		REMOVABLE CONSTRUCTION PAVEMENT	RAISED PAVEMENT MARKERS	THERMOPLASTIC PAVEMENT MARKING						
			WARRINGS	MARKINGS	TYPE II	6	3"				
					(YELLOW/YELLOW)	WHITE	YELLOW				
	LIN. FT	EACH	LIN. FT.	LIN. FT.	EACH	LIN. FT.					
CONSTRUCTION PAVEMENT MARKINGS	6708		6708								
REMOVABLE CONSTRUCTION PAVEMENT MARKINGS	932			932							
RAISED PAVEMENT MARKERS TYPE II (YELLOW/YELLOW)		37			37						
THERMOPLASTIC PAVEMENT MARKING WHITE (6")		5820				5820					
THERMOPLASTIC PAVEMENT MARKING YELLOW (6")		4970					4970				
TOTALS:			6708	932	37	5820	4970				
NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTIO	N 604 03 STAN	IDARD SPECI	FICATIONS FOR HIGHV	VAY CONSTRUCTION							

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION

NOTE: THE 6" YELLOW STRIPING QUANTITY HAS BEEN ESTIMATED BASED ON A DOUBLE YELLOW CENTERLINE STRIPE FOR THE ENTIRE PROJECT.

THE PROJECT MUST BE MARKED FOR PASSING/NO PASSING ZONES PRIOR TO THE PLACEMENT OF ANY FINAL STRIPING.

CONTACT THE MAINTENANCE DIVISION AFTER THE FINAL LIFT OF SURFACE COURSE HAS BEEN PLACED TO SCHEDULE THE ZONING OF THE PROJECT.

DATE REVISED DATE REVISED DATE FILMED FED.RD. STATE FED.AID PROJ.NO. SHEET NO. SHEET N

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PROFESSIONALO

ENGINEER

No.14994

Digitally Signed 08/10/2023

ADVANCE WARNING SIGNS AND DEVICES

				ANOL WA	KINING SIGI	10 AND DE	VIOLO								
SIGN NUMBER	DESCRIPTION	SIGN SIZE	STAGE 1	STAGE 2	MAXIMUM NUMBER REQUIRED	TOTAL SIGN	IS REQUIRED	TRAFFIC DRUMS			BARRICADES (TYPE III) INSTA		FURNISHING & INSTALLING PRECAST CONC. BARRIED	INSTALLING IMPACT	
					KLQUIKLD				RIGHT	LEFT					
			LIN. FT	EACH		NO.	SQ. FT.	EACH		LIN. F	Т.	EACH	EACH		
W20-1	ROAD WORK 1500 FT.	48"x48"	2	2	2	2	32.0								
W20-1	ROAD WORK 1000 FT.	48"x48"	2	2	2	2	32.0								
W20-1	ROAD WORK 500 FT.	48"x48"	2	2	2	2	32.0								
G20-2	END ROAD WORK	48"x24"	2	2	2	2	16.0								
R11-2	ROAD CLOSED	48"x30"	4	4	4	4	40.0								
OM-3L	OBJECT MARKER	12"x36"		6	6	6	18.0								
OM-3R	OBJECT MARKER	12"x36"		8	8	8	24.0								
W1-6	LARGE ARROW	48"x24"	2	2	2	2	16.0								
R4-1	DO NOT PASS	24"x30"	4	4	4	4	20.0								
W21-5a	RIGHT SHOULDER CLOSED	36"x36"	4	4	4	4	36.0								
W8-1	BUMP	30"x30"	4	4	4	4	25.0								
W8-17	SHOULDER DROP-OFF	36"X36"	4	4	4	4	36.0								
W8-17P	SHOULDER DROP-OFF	24"X18"	4	4	4	4	12.0								
	TRAFFIC DRUMS		121	98	121			121							
	TYPE III BARRICADE-RT. (8')		1	1	1				8						
	TYPE III BARRICADE-LT. (8')		1	1	1					8					
	TYPE III BARRICADE-RT. (16')		1	1	1				16						
	TYPE III BARRICADE-LT. (16')		1	1	1					16					
	FURNISHING AND INSTALLING PRECAST CONCRETE BARRIER			1460	1460						1460				
	TEMPORARY IMPACT ATTENUATION BARRIER			3	3	1						3			
	TEMPORARY IMPACT ATTENUATION BARRIER (REPAIR)			3	3								3		
TOTALS:							339.0	121	24	24	1460	3	3		

NOTE: THIS IS A HIGH TRAFFIC VOLUME ROAD AS DEFINED IN SECTION 604.03, STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.

CLEARING AND GRUBBING

STATION	STATION	LOCATION	CLEARING	GRUBBING	
			STATION		
101+50	130+60	HWY. 135	30	30	
TOTALS:			30	30	

REMOVAL AND DISPOSAL OF FENCE

STATION	N STATION LOCATION		FENCE	GATES
			LIN. FT.	EACH
103+49	107+82	HWY. 135 RT.	450	
107+83	109+00	HWY. 135 RT.	117	1
108+32	108+53	HWY. 135 RT.	31	
109+00	115+43	HWY. 135 RT.	644	
116+28	117+24	HWY. 135 RT.	142	
117+34	124+00	HWY. 135 RT.	727	1
124+00	129+05	HWY. 135 RT.	510	2
TOTALS:			2621	4

REMOVAL AND DISPOSAL OF CULVERTS

STATION	DESCRIPTION	PIPE CULVERTS					
		EACH					
105+03	HWY. 135 LT 18" SIDE DRAIN	1					
107+82	HWY. 135 LT 18" SIDE DRAIN	1					
108+05	HWY. 135 RT 18" SIDE DRAIN	1					
109+14	HWY. 135 LT 18" SIDE DRAIN	1					
110+06	HWY. 135 LT 30" SIDE DRAIN	1					
111+60	HWY. 135 LT 30" SIDE DRAIN	1					
127+16	HWY. 135 RT 24" SIDE DRAIN	1					
127+42	HWY. 135 LT 24" SIDE DRAIN	1					
129+31	HWY. 135 RT 24" SIDE DRAIN	1					
129+42	HWY. 135 LT 24" SIDE DRAIN	1					
TOTAL:	TOTAL: 10						

NOTE: QUANTITIES SHOWN ABOVE SHALL INCLUDE REMOVAL & DISPOSAL OF ALL HEADWALLS AND FLARED END SECTIONS IF APPLICABLE.

REMOVAL AND DISPOSAL OF ITEMS

REMOVAL AND DISPOSAL OF ITEMS								
STATION	STATION	LOCATION	MAILBOXES	GUARDRAIL	SIGNS			
			EACH	LIN. FT.	EACH			
103	3+73	HWY. 135 LT.						
103	3+73	HWY. 135 RT.						
103	3+80	HWY. 135 LT.	1					
104	+10	HWY. 135 LT.	2					
104+87		HWY. 135 LT.	2					
107	'+65	HWY. 135 LT.	1					
108	3+43	HWY. 135 RT.			1			
109)+12	HWY. 135 RT.	1					
109)+87	HWY. 135 LT.	1					
115+70	116+50	HWY. 135 LT.		80				
115+70	116+50	HWY. 135 RT.		80				
117+52	118+32	HWY. 135 RT.		80				
117+52	118+32	HWY. 135 LT.		80				
129)+77	HWY. 135 LT.	1					
129)+81	HWY. 135 RT.	1					
TOTALS:		0)4414	10	320	1			

NOTE: THE QUANTITY SHOWN ABOVE FOR THE REMOVAL AND DISPOSAL OF GUARDRAIL SHALL INCLUDE THE REMOVAL AND DISPOSAL OF ALL GUARDRAIL TERMINALS AND TERMINAL ANCHOR POSTS.

Ditti i i i i i i i i i i i i i i i i i										
STATION	STATION	LOCATION / DESCRIPTION	UNCLASSIFIED EXCAVATION	COMPACTED EMBANKMENT						
			CU. YDS.							
ENTIRE	PROJECT	STAGE 1 - HWY. 135	2616	23641						
ENTIRE	PROJECT	STAGE 2 - HWY. 135	7904	988						
ENTIRE	PROJECT	DRIVEWAYS	130	695						
TOTALS:	•		10650	25324						

NOTE: EARTHWORK QUANTITIES SHALL BE PAID AS PLAN QUANTITY

DRIVEWAYS & TURNOUTS

DRIVEWAYS & TURNOUTS										
STATION	SIDE	LOCATION	WIDTH		URFACE 2") 220 LBS.). (PG 64-22)	AGGREGATE BASE COURSE (CLASS 7)	RSE SIDE DRAINS		ıs	STANDARD DRAWINGS
							18"	24"	30"	
			FEET	SQ. YD.	TON	TON		LIN. FT.		
102+25	LT.	HWY. 135	16	93.24	10.26	38.07		44		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
103+92	LT.	HWY. 135	16	140.36	15.44	57.31				PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
105+03	LT.	HWY. 135	16	146.05	16.07	59.64	47			PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
107+82	LT.	HWY. 135	16	126.27	13.89	51.56			48	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
108+05	RT.	HWY. 135	16	130.26	14.33	53.19			49	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
109+14	LT.	HWY. 135	16	139.47	15.34	56.95			40	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
110+06	LT.	HWY. 135	16	148.05	16.29	60.45			40	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
111+60	LT.	HWY. 135	20	195.74	21.53	79.93			40	PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
121+00	RT.	HWY. 135	16	173.30	19.06	70.76		50		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
127+16	RT.	HWY. 135	16	140.20	15.42	57.25		49		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
127+42	LT.	HWY. 135	16	122.68	13.49	50.09		51		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
129+42	LT.	HWY. 135	16	117.73	12.95	48.07		42		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
129+75	RT.	HWY. 135	24	202.80	22.31	82.81		60		PCC-1, PCM-1, PCP-1, PCP-2, PCP-3
ENTIRE PRO	JECT TEMPO	RARYDRIVES			10.00	100.00				
TOTALS	OTALS				216.38	866.08	47	296	217	
DACK OF FO	DTALS 1876.15 216.38 866.08 47 296 217									

ACHM SURFACE COURSE (1/2")......94.9% MIN. AGGR.......5.1% ASPHALT BINDER MAXMUM NUMBER OF GYRATIONS = 115 FOR PG 64-22

TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

* NOTE: QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS.

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED. NOTE: FOR C.M. PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.

COLD MILLING ASPHALT PAVEMENT

GOED MILEURO AOI HAET I AVEMENT									
STATION	STATION	LOCATION	AVG. WIDTH	COLD MILLING ASPHALT PAVEMENT					
			FEET	SQ. YD.					
101+50.00	102+50.00	HWY. 135	26.00	288.89					
128+60.00	129+60.00	HWY. 135	26.00	288.89					
TOTAL:				577.78					

NOTE: AVERAGE MILLING DEPTH 1".

COORDINATE COLD MILLING STOCKPILE LOCATIONS WITH DISTRICT ENGINEER. STOCKPILE LOCATIONS SHALL BE NO FURTHER THAN FIVE MILES FROM EACH SITE.

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC

LOCATION	TON	ТАСК СОАТ	
		GALLON	
ENTIRE PROJECT - TO BE USED IF AND WHERE	12	24	
DIRECTED BY THE ENGINEER			
TOTALS:	12	24	
DASIS OF ESTIMATE:			

ASPHALT CONCRETE PATCHING FOR MAINTENANCE OF TRAFFIC...25 TON/MILE

TACK COAT FOR MAINTENANCE OF TRAFFIC...

ACHM PATCHING OF EXISTING ROADWAY

DESCRIPTION	TON
ENTIRE PROJECT - TO BE USED IF AND WHERE	100
DIRECTED BY THE ENGINEER	
TOTAL:	100

NOTE: QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

STATE DATE FILMED ARK. 24 70 JOB NO. 101013 QUANTITIES

FLOWABLE SELECT MATERIAL

,	STATION	LOCATION	CU. YD.	
*EN	NTIRE PROJ	JECT	4	
TC	TOTAL:			

* NOTE: QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

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PROBESSIONAL
ENGINEER
No.14994
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APPROACH GUTTERS AND SLABS

STATION	STATION	LOCATION	APPROACH GUTTER (TYPE SPECIAL)	APPROACH SLABS	REINFORCING STEEL-RDWY. (GR. 60)	AGGREGATE BASE CRS. (CLASS 7)
			CU.YD.	CU.YD.	POUND	TON
115+83.50	116+20.00	HWY. 135		49.15	5775	28.00
115+83.50	116+20.00	HWY. 135 LT.	14.20		768	
115+83.50	116+20.00	HWY. 135 RT.	14.20		768	
117+80.00	118+16.50	HWY. 135		49.15	5775	28.00
117+80.00	118+16.50	HWY. 135 LT.	14.20		768	
117+80.00	118+16.50	HWY. 135 RT.	14.20		768	
TOTALS:			56.80	98.30	14622	56.00

STRUCTURES

	0111001011E0							
STATION	DESCRIPTION			WATER	STD. DWG. NOS.			
		(CLASS III)						
		24"	24"	12"				
		LIN. FT.	EACH	LIN. FT.	SQ.YD.	M.GAL.		
103+73	HWY. 135 EXTEND 24" PIPE CULVERT	52	2		16	0.20	FES-1, FES-2, PCC-1	
ENTIRE PROJECT				60			PCC-1, PCM-1, PCP-1, PCP-2, PCP-3	
TOTALS:	TOTALS:		2	60	16	0.20		

BASIS OF ESTIMATE:

..12.6 GAL. / SQ. YD. OF SOLID SODDING

NOTE: FOR R.C. PIPE CULVERT INSTALLATIONS USE TYPE 3 BEDDING UNLESS OTHERWISE SPECIFIED.

* NOTE: QUANTITY ESTIMATED SEE SECTION 104.03 OF THE STD. SPECS.

TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.

SELECTED PIPE BEDDING

SELECTED I II E DEDDI	10
LOCATION	SELECTED PIPE BEDDING
	CU.YD.
ENTIRE PROJECT TO BE USED IF	
AND WHERE DIRECTED BY THE	10
ENGINEER	
TOTAL:	10

NOTE: QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

SOIL STABILIZATION

	0012 017 12112121 111011								
STATION	STATION	LOCATION / DESCRIPTION	SOIL STABILIZATION TON						
ENTIRE	PROJECT	TO BE USED IF AND WHERE	100						
		DIRECTED BY THE ENGINEER							
TOTAL:			100						

QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS

4" PIPE UNDERDRAIN

		4 FIFE UNDERDRAIN			
STATION	STATION	LOCATIONS	4" PIPE UNDERDRAINS	UNDERDRAIN OUTLET PROTECTORS	
			LIN. FT.	EACH	
ENTIRE PROJECT TO BE USED IF AND		500	4		
WHERE DIRECTED BY THE ENGINEER					
TOTALS:			500	4	

* NOTE: QUANTITY ESTIMATED.

SEE SECTION 104.03 OF THE STD. SPECS.

FENCING

FENCING							
STATION	STATION	LOCATION	WIRE FENCE	* 16'-0" GATES	WATER		
STATION		LOCATION	(TYPE D-1)	GATES	GATE		
			LIN. FT.	EACH	EACH		
102+10	107+90	HWY. 135 RT.	580	1			
108+06	120+85	HWY. 135 RT.	1290	1	1		
121+15	127+01	HWY. 135 RT.	596				
127+17	130+00	HWY. 135 RT.	285	1			
-							
TOTALS:			2751	3	1		

* DENOTES ALTERNATE BID ITEM.

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EROSION CONTROL

STATION STATION LOCATION LOCATION SEEDING LIME SEEDING APPLICATION ACRE M.GAL. ACRE ACRE M.GAL. BAG CU.YD. LIN.FT. CU.YD.	OF SEDIMENT R	*SEDIMENT REMOVAL & DISPOSAL
STATION STATION LOCATION LOCATION SEEDING LIME MULCH COVER WATER SEEDING APPLICATION SEEDING APPLICATION SEEDING APPLICATION SEEDING APPLICATION SEEDING SEEDING SEEDING COVER WATER DITCH CHECKS SILT FENCE SEDIMENT BASIN (E-14)	OF SEDIMENT R	REMOVAL &
(E-5) (E-11) (E-14)	DASIN	DISPUSAL
ACDE TON ACDE MON ACDE ACDE MON DAG CHAYD LINET CHAYD		
ACRE TON ACRE WIGHL ACRE ACRE ACRE WIGHL BAG CO.TD. LIN.FT. CO.TD.	CU.YD.	CU. YD.
ENTIRE PROJECT CLEARING AND GRUBBING 132 105 5565		206
ENTIRE PROJECT STAGE 1 110 105		
ENTIRE PROJECT STAGE 2 110 60 10		1
*ENTIRE PROJECT TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER. 5.41 10.82 5.41 551.8 5.41 5.41 5.41 110.4 75 100	100	100
TOTALS: 5.41 10.82 5.41 551.8 5.41 5.41 5.41 110.4 352 345 5575 100	100	307

BASIS OF ESTIMATE:

ROCK DITCH CHECKS...

..15 CU.YD./LOCATION

NOTE: THE TEMPORARY EROSION CONTROL DEVICES SHOWN ABOVE AND ON THE PLANS SHALL BE INSTALLED IN SUCH A SEQUENCE AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM PERMIT.

*NOTE: QUANTITIES ESTIMATED. SEE SECTION 104.03 OF THE STD. SPECS.

BENCH MARKS

	DENOTI MARKO	
STATION	LOCATION	BENCH MARKS
		EACH
116+10	SW CORNER OF BRIDGE NO. 07514	1
TOTAL:	1	
NOTE: CHO	MAN FOR INFORMATION ONLY RENCHIMAR	VC

NOTE: SHOWN FOR INFORMATION ONLY. BENCH MARKS SHALL BE FURNISHED AND PLACED BY STATE FORCES.

EROSION CONTROL MATTING

STATION	STATION	LOCATION	LENGTH	CLASS 3
			LIN. FT.	SQ. YD.
102+50.00	103+83.00	HWY. 135 LT.	133.00	118.22
102+50.00	107+77.00	HWY. 135 RT.	527.00	468.44
104+00.00	104+86.00	HWY. 135 LT.	86.00	76.44
105+33.00	107+58.00	HWY. 135 LT.	225.00	200.00
108+06.00	108+95.00	HWY. 135 LT.	89.00	79.11
108+26.00	116+84.00	HWY. 135 RT.	858.00	762.67
109+35.00	109+86.00	HWY. 135 LT.	51.00	45.33
110+26.00	111+40.00	HWY. 135 LT.	114.00	101.33
117+20.00	120+75.00	HWY. 135 RT.	355.00	315.56
117+59.00	127+16.00	HWY. 135 LT.	957.00	850.67
121+26.00	126+87.00	HWY. 135 RT.	561.00	498.67
127+36.00	129+06.00	HWY. 135 RT.	170.00	151.11
127+67.00	129+33.00	HWY. 135 LT.	166.00	147.56
129+56.00	129+80.00	HWY. 135 RT.	24.00	21.33
TOTAL:				3836.44
NOTE: AVED	ACE WIDTH -	0! 0!!		

NOTE: AVERAGE WIDTH = 8'-0"

MAILBOXES

	MAILBOXES	MAILBOX	SUPPORTS
LOCATION	WAILBUXES	(SINGLE)	(DOUBLE)
		EACH	
ENTIRE PROJECT	10	6	2
TOTALS:	10	6	2

ARKANSAS LICENSED PROFESSION ATT ENGINEER No.14994	
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BASE AND SURFACING

					ATE BASE (CLASS 7)				TACK COAT				Д	CHM BINDE	R COURSE (1'	")				ACHM SU	JRFACE COUI	RSE (1/2")			
STATION	STATION	LOCATION	LENGTH	TON /	TON	(0.05) TOTAL WID.	GAL. PER SO	i - ,	(0.17 (GAL. PER SC	1 '	TOTAL	AVG. WID.	SQ.YD.	POUND /	PG 64-22	AVG. WID.	SQ.YD.	POUND /	PG 64-22	AVG. WID.	SQ.YD.	POUND /	PG 64-22	TOTAL PG 64-22
			FEET	STATION		FEET	SQ.YD.	GALLON	FEET	SQ.YD.	GALLON	GALLONS	FEET		SQ.YD.	TON	FEET		SQ.YD.	TON	FEET	- 4	SQ.YD.	TON	TON
MAIN	LANES																								
101+50.00	102+50.00	HWY. 135 - TRANSITION	100.00	VAR.	120.00												29.94	332.67	220.00	36.59					36.59
102+50.00	105+42.67	HWY. 135 - NOTCH AND WIDEN	292.67	307.50	899.96	16.75	544.69	27.23	20.00	650.38	110.56	137.79	8.50	276.41	440.00	60.81	32.00	1040.60	220.00	114.47	8.25	268.28	220.00	29.51	143.98
105+42.67	110+04.42	HWY. 135 - NOTCH AND WIDEN	461.75	252.25	1164.76	36.04	1849.05	92.45	16.17	829.61	141.03	233.48	18.08	927.60	440.00	204.07	32.00	1641.78	220.00	180.60	17.96	921.45	220.00	101.36	281.96
110+04.42	115+83.50	HWY. 135 - FULL DEPTH	579.08	450.25	2607.31	56.75	3651.42	182.57				182.57	28.50	1833.75	440.00	403.43	32.00	2058.95	220.00	226.48	28.25	1817.67	220.00	199.94	426.42
118+16.50	125+06.12	HWY. 135 - FULL DEPTH	689.62	450.25	3105.01	56.75	4348.44	217.42				217.42	28.50	2183.80	440.00	480.44	32.00	2451.98	220.00	269.72	28.25	2164.64	220.00	238.11	507.83
125+06.12	127+35.15	HWY. 135 - NOTCH AND WIDEN	229.03	252.25	577.73	36.04	917.14	45.86	16.17	411.49	69.95	115.81	18.08	460.10	440.00	101.22	32.00	814.33	220.00	89.58	17.96	457.04	220.00	50.27	139.85
127+35.15	129+60.00	HWY. 135 - NOTCH AND WIDEN	224.85	317.75	714.46	19.21	479.93	24.00	24.58	614.09	104.40	128.40	9.67	241.59	440.00	53.15	32.00	799.47	220.00	87.94	9.54	238.34	220.00	26.22	114.16
129+60.00	130+60.00	HWY. 135 - TRANSITION	100.00	VAR.	120.00												30.09	334.33	220.00	36.78					36.78
ADD	ITIONAL FOR										_														
102+50.00	105+42.67		292.67			20.00	650.38	32.52				32.52					20.00	650.38	VAR.	86.83					86.83
105+42.67	110+04.42		461.75			16.17	829.61	41.48				41.48					16.17	829.61	VAR.	240.51					240.51
125+06.12	127+35.15	HWY. 135	229.03			16.17	411.49	20.57				20.57					16.17	411.49	VAR.	105.14					105.14
127+35.15	129+60.00	HWY. 135	224.85			24.58	614.09	30.70				30.70					24.58	614.09	VAR.	66.52					66.52
ADD	ITIONAL FOR	SUPERELEVATION							1						l				I				I.		-
127+35.15	129+60.00	HWY. 135	224.85	VAR.	496.41																				
ADD	I ITIONAL FOR	I R METHOD OF RAISING GRADE							1 1								<u> </u>								
102+50.00	105+42.67		292.67			20.00	650.38	32.52				32.52	20.00	650.38	VAR.	65.16									$\overline{}$
105+42.67	110+04.42		461.75			16.17	829.61	41.48				41.48	16.17	829.61	VAR.	25.80									
125+06.12	127+35.15		229.03			16.17	411.49	20.57				20.57	16.17	411.49	VAR.	89.82									
127+35.15	129+60.00		224.85			24.58	614.09	30.70				30.70	24.58	614.09	VAR.	4.10									
12. 22.112																									
ADD	TIONAL FOR	GUARDRAIL WIDENING		•							•				•					•	•		•		
113+48.85	116+10.60	HWY. 135	261.75	VAR.	132.76												VAR.	256.35	220.00	28.20					28.20
114+23.85	116+10.60		186.75	VAR.	81.12						1						VAR.	159.35	220.00	17.53	1		1		17.53
117+89.40	119+76.15		186.75	VAR.	81.35												VAR.	158.93	220.00	17.48					17.48
117+89.40	120+51.15	HWY. 135	261.75	VAR.	132.80												VAR.	257.09	220.00	28.28					28.28
ADD	ITIONAL FOR	MOT VERTICAL TRANSITIONS						1								<u> </u>				1					
102+50.00	114+00.00		1150.00	ı		24.00	3066.67	153.33	24.00	3066.67	521.33	674.66			1	1	24.00	3066.67	VAR.	529.57	1		1		529.57
122+75.00	129+60.00		685.00			24.00	1826.67	91.33	24.00	1826.67	310.53	401.86				 	24.00	1826.67	VAR.	359.06					359.06
122173.00	128100.00	111001. 100	003.00			24.00	1020.07	91.00	24.00	1020.07	310.33	401.00					24.00	1020.07	VAIX.	333.00					338.00
TOTALS:	•	•	•	•	10233.67		21695.15	1084.73		7398.91	1257.80	2342.53		8428.82		1488.00		17704.74		2521.28		5867.42		645.41	3166.69

				JOB NO.		101013	27	70
				6	ARK.			
EVISED	FILMED	REVISED	FILMED	D131. NO.			NO.	SHEETS
DATE	DATE	DATE	DATE	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS

O 07514 QUANTITIES 63787

SCHEDULE OF BRIDGE QUANTITIES - JOB. NO. 101013

			ITEM NO.	205	801	SP, SS, & 802	SP, SS, & 802	SP, SS, & 802	SP & 803	SS & 804	SS & 804	SS & 805	SS & 805	SS & 805	SS & 805	812	SS & 816	SS & 816
BRIDGE NO.	NAME PLATE TITLE	UNIT OF STRUCTURE	ITEM UNIT	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO)	UNCLASSIFIED EXCAVATION FOR STRUCTURES - BRIDGE	CLASS S CONCRETE - BRIDGE	CLASS S(AE) CONCRETE - BRIDGE	PRESTRESSED CONCRETE BOX BEAMS (27"x48")	CLASS 2 PROTECTIVE SURFACE TREATMENT	REINFORCING STEEL - BRIDGE (GRADE 60)	EPOXY COATED REINFORCING STEEL (GRADE 60)	STEEL SHELL PILING (16" DIA.)	STEEL SHELL PILING (20" DIA.)	PILE ENCASEMENT	PREBORING	BRIDGE NAME PLATE (TYPE D)	FILTER BLANKET	DUMPED RIPRAP
				LUMP SUM	CU. YD.	CU. YD.	CU. YD.	LIN. FT.	SQ. YD.	LB.	LB.	LIN. FT.	LIN. FT.	LIN. FT.	LIN. FT.	EACH	SQ. YD.	CU. YD.
		DENT NO. 1			11	20.70				6.210	1.602	220			F0		261	102
	¥	BENT NO. 1 BENT NO. 2			11	20.70 25.00				6,310 7,925	1,602 606	320	370	34	50		361	192
	じい	BENT NO. 3				25.00				7,925	606		370	29				
47	1228	BENT NO. 4			17	20.70				6,310	1,602	335			50		309	165
15	ξS									·								
ľ	\$\$	159'-0" INTEGRAL PRESTRESSED CONCRETE BO	X BEAM UNIT				265.00	785.0	859.9		66,544					1		
	HIGHWAY 135 OVER JACKS CREEK	SITE NO. 1 (EXISTING BR. NO. 02717)		1														
		TOTALS FOR JOB NO. 101013			28	91.40	265.00	785.0	859.9	28,470	70,960	655	740	63	100	1	670	357

¹⁾ Steel shell piles shall conform to ASTM A252, Grade 3, Fy = 45 ksi.



SCHEDULE OF BRIDGE QUANTITIES JACKS CREEK STR. & APPRS. (S) GREENE COUNTY

ROUTE 135 SEC. 5

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

 DRAWN BY:
 JME
 DATE:
 JUNE 2020
 FILENAME:
 b101013 q1.dgn

 CHECKED BY:
 JHR
 DATE:
 JUNE 2020
 SCALE:
 As Shown

 DESIGNED BY:
 JME
 DATE:
 JUNE 2020
 SCALE:
 As Shown

 BRIDGE NO.
 07514
 DRAWING NO.
 63787

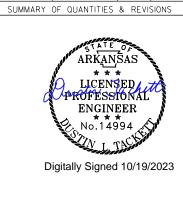
⁽²⁾ The top of the 20" steel shell piling shall be fitted with an Annular Ring Plate in accordance with the details shown on Dwg. No. 63793. The cost of all labor and materials required to fabricate and install the Annular Ring will not be paid for directly but shall be considered subsidiary to the item "STEEL SHELL PILING (20" DIA)".

Part	ITEM NUMBER	ITEM	OUANTITY	LNO
0.00000000000000000000000000000000000				
CONDESTRATES DESIGNATIONS 1.0.1 EDITORAL AND SERSAL OF ERCES 1.0.1 EDITORAL AND SERSAL OF SERSAL OF MARKED 1.0.1 EDITORAL AND SERSAL OF SERSAL OF MARKED 1.0.1 EDITORAL AND SERSAL OF SERSAL OF MARKED 1.0.1 SOLIS SERSAL OF SERSAL OF MARKED 1.0.1 SOLIS SERSAL OF SERSAL OF MARKED 1.0.1 SOLIS SERSAL OF SERSAL OF MARKED 1.0.1 MERCHAN ELOSIS SERVED 1.0.1 AND SERVED SERVED SERVED SERVED 1.0.1 AND SERVED	SP & 201 CL	EARING	30	STATION
Control Actor Designed, or preferences Control Actor Designed, or preferen	201 GR	UBBING	30	STATION
Appropriate Processing Content of Content	202 RE	MOVAL AND DISPOSAL OF FENCE	2621	LN. FT.
ADDITION OF TABLE O	202 RE	MOVAL AND DISPOSAL OF GATES	4 ;	EACH
Compact	202 KE	MOVAL AND DISPOSAL OF FIRE CULVEKTS MOVAL AND DISPOSAL OF GLIADDDAIL	000	EACH
10	202 202 RE	MOVEL AND DISTORAL OF STANDARD OF STANDARD AND DISPOSAL OF STANDARD AND	1	FACH
ADMINISTRATION ADMINISTRATION	202 RE	MOVAL AND DISPOSAL OF MAILBOXES	. 10	EACH
1985 JOHNSTON OF STREET DE CANADITA JOHNSTON OF STREET JOHNSTON OF STR	SS & 206 FL(OWABLE SELECT MATERIAL	4	CU. YD.
CORDONALIZED BANKWIRTH SASSA CORDONALIZED BANKWIRTH SASSA ASSECTION CORRECTION CONSTITUTION 1.127 WINTER ASSECTION CONSTITUTION 1.127 MARINAL CONSTITUTION SIGNATURE STREET CONSTITUTION 1.127 ASSECTION CONSTITUTION SIGNATURE STREET STREET CONSTITUTION SIGNATURE STREET STR	SP, SS, & 210 UN	CLASSIFIED EXCAVATION	10650	CU. YD.
CASE OFFICE ALL SEAR OFFICE COARSE (CASS.) AND ENGINEER COARSE (CASS.)<	SP & 210 CO	MPACTED BRANKMENT	25324	CU. YD.
MICH DATE DATE DATE DATE DATE DATE DATE DATE	SP & 210 SO	IL STABLIZATION	100	NO P
MATERNALE BERGERE CANSELTY 617 MASHALL SECRETE BACHER BERGERSELTY 617 MASHALL SECRETE BACHER BERGERSELTY 617 MASHALL SECRETE BACHER BERGER CANSELTY 617 ASSALL SECRETE BACHER BERGER CANSELTY 617 ASSALL SECRETE BACHER BERGER CANSELTY 617 ASSALL SECRETE BACHER SECRETE CANSELTY 617 ASSALL SECRETE BACHER SECRETE CANSELTY 617 ASSALL SECRETE BACHER SECRETE BA	SP, SS, & 303 AG	GREGATE BASE COURSE (CLASS 7)	11156	2 5
ASSESTATION OF STEAD STAN ACTIVATE TO COURSE (LT.) 2011 ASSESTATION ACTIVATE TO COURSE (LT.) 2011 ASSESTATION ACTIVATION ACTIVATE COURSE (LT.) 2011 ASSESTATION ACTIVATION ACTIVATE COURSE (LT.) 2011 ASSESTATION ACTIVATION ACTIVATE ACTIVATE (LT.) 2011 ASSESTATION ACTIVATION ACTIVATE ACTIVATE (LT.) 2011 ASSESTATION ACTIVATE ACTIV	55 & 401 1A1	LECTOR ACCRECATE IN ACCURATION OF UPON THE ACCURATION OF THE ACCUR	1407	GAL
ASPENDED PARTY DESCRIPTION CONTRICTORS	5P, 55, & 406 WIII	JET ALGORICO DE LA CHIMA DINIDER COLONDE (1.1) DINITATE AUGUSTICA (2.4 20) IN ACCIMA DINIDED COLIDEE (4.1)	1427	
PARTICATION OF EASTERNAMENT SIRPCAGE OUTSITE AND ANAMENT SIRPCAGE OUTSITE ANAMENT	3P SC & 400 MIN	TI IALL DINOLIN (FOR ACTION OF INTERNAL COORDINATION OF INTERNAL O	3211	
ASTACOLOGIESTIC PARCES ASTACOLOGIESTIC PAR	3P SS & 407	RELATE COURT AND IN A CHIM SIDE ACTION TO THE COURT (I.E.). PHAT TRINDER (PC 64.29) IN A CHIM SIDE ACE COIRSE (1/2").	173	Ž
ACM PARTED CONSTRUCTIONS CONTRAFTED 1.0. ACM MAY PATCH CONCESTED 1.0. ACM MAY PATCH CONCESTED 1.0. ACM MAY PATCH CONCESTED 1.0. ACM PATCH CONCESTED 1.0. ACM PATCH CONTRACT 1.0.	SP & 412	ID MILI ING ASPHALT PAVEMENT	578	CX OS
APPENDIX SETS DESIGNADED DESIGNA	3P SS & 414 ASI	LED MICELATORY OF TALL I AVENUEVI. PHAILT CONCRETE PARTITIONING FOR MAINTENANCE OF TRAFFIC.	12	
APPERADES EN DESCRIPTIONS 98.90 APPERADES EN DESCRIPTIONS 1.01 BORDING SECURIOR SOFTED PREDICTIONS 1.01 AFEB UNDER DAYS 1.01	3P. SS. & 415 AC	HM PATCHING OF EXISTING ROADWAY	100	NOT
MORE DESCRIPTIONS 6.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	SP. SS. & 504 AP	PROACHSLABS	98.30	CU.YD
ACMINGMENTON CONTROL ED ACMI	SP, SS, & 504 AP	PROACH GUTTERS	56.80	CU. YD.
Control Cont	601 MC	BILIZATION	1.00	LUMP SUN
AMATER CARE DESCRIPTIONS CONCERTE BARRER 28.29 SAME SAME SAME SAME SAME SAME SAME SAME	SP & 602 FUI	RNISHING FIELD OFFICE	1	EACH
QUARDENIA DESCRIPTION DE SERVER SER	SS & 603 MA	INTENANCE OF TRAFFIC	1.00	LUMP SUN
SECOND SEED NOT SECOND SEED NOT SEED	603 12"	TEMPORARY CULVERT	09	LIN. FT.
A STATE CONTROL ES AND ESTATE CONTROL E SANDER	SS & 604 SIG	SNI	339	SQ. FT.
TRAFFIC DATE OF THE BARRER 1721 TRAFFIC DATE OF THE BARRER	SS & 604 BA	RRICADES	48	LIN. FT.
FURNISHING CONSTRUCTOR NATE RATE RATE RATE RATE RATE RATE RATE R	SS & 604 TR	AFFIC DRUMS	121	EACH
CONSTRUCTOR NAME ENERGY MARKEN MASS PRINT MARKEN MASS PRINT MARKEN MASS PRINT MARKEN MASS PRINT MASS	SS & 604 FU	RNISHING AND INSTALLING PRECAST CONCRETE BARRIER	1460	LN.
REPORTORIED CONSTRUCTION PARKETIN MARKINGS 25.2 AT FERENDEZ CONSTRUCTION PARKETIN MARKINGS 21.4 AT FERENDEZ CONSTRUCTION PARKETIN MARKINGS 21.7 AT FERENDEZ CONSTRUCTION PARKETIN PARKET	604 CO	NSTRUCTION PAVEMENT MARKINGS	6708	LN. FT.
18. THE WINDORED CONCRETE FIPE CLIVERTS CLASS III) 2.75 18. SIDE DAWN 2.75 28. SEED DAWN 2.75 29. SEED DAWN 2.75 29. SEED DAWN 2.75 20. SEED DAWN 2.75	604 RE	MOVABLE CONSTRUCTION PAVEMENT MARKINGS	932	LN. FT.
47 47 26 47 26 26 27 26 27 27 27 27 28 27 29 27 20 27 21 27 22 <	SS & 606 24"	REINFORCED CONCRETE PIPE CULVERTS (CLASS III)	52	LIN. FT.
2.96 2.97 30. SIGE DABAN 2.7 30. SIGE DABAN 2.7 30. SIGE DABAN 2.7 30. SIGE DABAN 4 ALECTED DASIONS CRICAGES CRICAGES 4 ALMARDAR TENDER CRICAGES 4 ALMARDAR TENDER CRICAGES 4 ALMARDAR TENDER CRICAGES 4 THERE DEBONG 4 THE DEBONG 4 THE DEBONG 4	3P, SS, & 606 18"	SIDE DRAIN	47	LIN. FT.
27.7 STATE OF SCHOOL STATE PIPE COLVERTS 27.7 28.7 SEED DAMA 4.0 4.0 28.6 SEED DAMA SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.0 28.6 SEED DAMA SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.0 28.6 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.0 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.0 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.0 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 4.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONCRETE PIPE COLVERTS 5.1 29.7 SEED DAG OF REENFORCED CONTROL 5.1 29.7 SEED DAG OF REENFORCED CONTROL 5.1 29.8 DAG OF REENFORCED CONTROL 5.1 29.8 DAG	3P, SS, & 606 24"	SIDE DRAIN	296	LIN. FT.
1. AT ARDED ROD SETUDAS FOR REINFORCED CONCRETE PRE CULVERTS 1.0 2. AT ARDED ROS SETUDAS FOR REINFORCED CONCRETE PRE CULVERTS 1.0 1. AT ARDED ROS SETUDAS SETUDAS FOR REINFORCED CONCRETE PRE CULVERTS 1.0 1. AT ARDED ROS SETUDAS SETUD	3P, SS, & 606 30"		217	LIN. FT.
10.00 SELECTOR DEP REDDING 10.00 SELECTOR DEP REDDING 4.00 UNDERDINGUETE PROJECTORS 50.00 UNDERDINGUETE PROJECTORS 4.00 GUARDARAL (THER AL 4.00 GUARDARAL (THER AL 4.00 GUARDARAL (THER AL 1.00 GUARDARAL (THER AL 1.00 MALE DATE (ALMAN CARE TERMANAL 1.00 MALE DATE (ALMAN CARE TERMANAL TERMANAL 1.00 MALE DATE (ALMAN CARE TERMANAL AND DEPOCATION 1.00 SEDING 1.00 BALL DATE (ALMAN CARE TERMANAL AND DEPOCATION 1.00 SEDING CARL (THERMANAL AND DEPOCATION 1.00 SEDING SEDING CARLOL (ALSO SA) 1.00 SEDING SEDING CARLOL (ALA AND DEPOCATION) 1.00 SEDING SEDING CARLOL (ALA AND DEPOCATION) 1.00 SEDING SEDING CARLOL (ALMAN CARE TERMAN CARLOL (ALMAN CARLO	SS & 606 24"	PE	2	EACH
4. PAD LONG PARAMAL (PRE 2) 4 90 9. OLAND EDER PROJECTIONS 1 1 9. STEEL CAPIES 1 1 <	SS & 606 SE		10	CU. YD.
AUMENDARY OUT CHEST PROTECTORS AUMANDARY ITRIANIA AUG AUMANDARY ITRIANIA AUG AUMANDARY ITRIANIA AUG AUMENDARY ITRIANIA AUMANDARY IT	SS & 611 4" F	PIPE UNDERDRAINS	200	LIN. FT.
AURICATION OF TITE NAME (TITE AUGUSTANCE) AURICATION OF TITE AUGUSTANCE (TITE AUGUSTANCE) AUGUSTANCE (TITE AUGUSTANCE)	SS & 611 UN	DERDRAIN OUTLET PROTECTORS	4	EACH
AUTOCAME PART	SS & 617 GU	ARDRAIL (TYPE A)	450	LIN. FT.
The ERA GLANDAL TENNIAL MARE ENDER TO THE ERAMIE NO.1 2 2751	SS & 617 GU	ARDRAIL TERMINAL (TYPE 2)	4	EACH
VANTER CANTER	SS & 617 TH	RE BEAM GUARDRAIL TERMINAL	4	EACH
VALER GAPE 10 VALER GAPE 10 VALER GAPE 11 VALER GAPE 12 VALER GAPE 13 VALER GAPE 14 VALER GAPE 15 VALER GAPE 16 VALER GAPE 17 VALER GAPE 18 VALER GAPE 18 VALER GAPE 19 VALER GAPE 10 VALER GAPE	SS & 619 WIF	REFENCE (TYPE D-1)	2751	LIN. FT.
16 16 16 16 16 16 16 16	SP WA		_	EACH
INTERNATION OF ENTRY OF ENTR	SS & 619 16'	(ALTERNATE NO	က	EACH
MAIL CH COVER AMADE AND	91. 81.9 SS	(ALIEKNAIE NO.	ω ;	FACH
MALECH COVER WATER CHOOLE WATE	620 CE	L C	F 74	
WATER WATER 662.4 SLITENDCRARY SEEDING 514.1 514.1 SLITENDCRARY SEEDING 514.1 514.1 SAND BACD DITCH CHECKS 352.2 352.2 SEDING HALL BASIN 307 307 SEDING HALL OF SEEDING APPLICATION 100 307 SECOND SEEDING APPLICATION 345 345 SECOND SEEDING APPLICATION 346 347 ROCK DETICAL CHECKS 346 347 SECOND SEEDING APPLICATION 347 345 RANGE DATE SEEDING APPLICATION 100 348 MALL BOX SUPPORTS (SINGLE) 100 448 PREMOPIAL SITE PARAMENT MARKING WHITE (ST) 100 100 PARSED PAYAEWENT WARKING YELLOW (ST) 100 100 PREMOPLA SITE DECALLY ALTERNUAL (ST) FOR ESID 100 REMOVED CONSTILE BRINGE (ST)<	SS & 620 MI	CHCOVER	10.82	ACRE
Interproperate See DING	620 800		662.4	M GA
SATE FROEZ \$575 SAND BACD DITCH CHECKS 100 SEDIMENT BASIN 100 SEDIMENT BASIN 100 SEDIMENT BASIN 100 SEDIMENT BASIN 307 SEDIMENT BASIN 345 SECOND SEDIMENT BASIN 345 SECOND SEDIMENT BASIN 100 BASIN SECOND SERVICE ON SERVICE AND	621 TEI	MPORARY SEEDING	5.41	ACRE
SED MEND BAGO DITCH CHECKS 382 SEDIMENT BASIN 100 OBLITERATION OF SEDIMENT BASIN 100 OBLITERATION OF SEDIMENT BASIN 100 OBLITERATION OF SEDIMENT BASIN 345 BOCK DITCH CHECKS 16 SCOLD SECRIFICATION 16 SCOLD SECRIFICATION 16 SCOLD SECRIFICATION 16 ROAD DAY 16 SCOLD SECRIFICATION 16 MALE BOX SUPPORTS (SINGLE) 170 PRENDEC CONSTRUCTION (SINGLE) 170 PRENDEC CONSTRUCTION SURFRES (PRENDE SITEL SANDER 170 CLASS SEED CONCRETE BRIDGE (SANDE SITEL SANDER S	621 SIL	T FENCE	5575	L
SEDIMENT BASIN 100 DELITERATION OF SEDIMENT BASIN 100 DELITERATION OF SEDIMENT BASIN 307 RECOMD SEEDING APPLICATION 345 SECOND SEEDING APPLICATION 100 SECOND SEEDING APPLICATION 100 MALE SOEDING 100 MALE BOXIES 100 MALE BOXIES 100 MALE BOXIES 100 MALE BOX SUPPORTS (SINGLE) 100 HERMOPLAST (MAPCI ATTENIATION BARRIER RELEASED STANGLE) 100 REINFORCING STEEL ROADWAY (GRADE 60) STRUCTURES SINGLE BOX STEEL RELEASED CONCRETE BOX SEAL STRUCTURE SINGLE	621 SA	ND BAG DITCH CHECKS	352	BAG
OBLITERATION OF SEDMENT BASIN 100 SEDMENT FERMOVAL AND DISPOSAL 347 ROCK DITCH CHECKS 347 SEDMENT FERMOVAL AND DISPOSAL 146 ROCK DITCH CHECKS 341 SOLD SCODING 341 READ SOLD MONEY 110 READ SOLD CANTEN CLAND AND THE CANADAMA AND AND AND AND AND AND AND AND AND AN	621 SE	DIMENT BASIN	100	CU. YD.
SEDMENT REMOVAL AND DISPOSAL 367 ROCK DITCH CHECKS 367 SECONO SEEDING APPLICATION 16 SCOLOS SOODING 3836 ROSDIANG 100 MALE BOXES ON CONTROL 100 MALE BOXES SOLD SCORDING 100 MALE BOXES SUPPORTS (SINGLE) 100 RAISED PAYEMENT MARKING YHITE (SIT 100 RAISED PAYEMENT MARKING YHITE (SIT 100 REINFORKING SITELL RADOGE 100 REINFORKING SITELL RADOGE 100 CLASS SIAEL ONCHETE BOXES SITEL (SIADLE 60) 100 SITEL SHELL PLING (GV DAMETER) 100 SITEL SHELL PLING (GV DAMETER) 100	621 OB	LITERATION OF SEDIMENT BASIN	100	CU. YD.
SECOND SEEDING APPLICATION SECOND SEEDING APPLICATION SECOND SEEDING APPLICATION SOLID SOOD DING SOLID SOOD DING SOLD SOOD DING SOLID SOOD DING SOON SETE LEARD DING SOON SET LEARD DING	621 SE	DIMENT REMOVAL AND DISPOSAL	307	CU. YD.
SECOND SEEDING APPLICATION 16 SOLLD SODD NIG 16 ENCOSION CONTROL MATTING (CLASS 3) 10 SOLLD SODD NIG 100 MALE BOX SUPPORTS (CONTROL 10 MALE BOX SUPPORTS (DOUBLE) 6 MALE BOX SUPPORTS (DOUBLE) 6 MALE BOX SUPPORTS (DOUBLE) 6 THE RANGO PLASTIC PANCHEMEN IN MARKING WHITE (6°) 37 THE RANGO PLASTIC PANCHEMEN IN MARKING YELLOW (6°) 37 THE RANGO PLASTIC PANCHEMEN IN MARKING YELLOW (6°) 37 THE MODEARY WHACT ATTENUATION BARRIER (REPAIR) 37 TEMPORARY WHACT ATTENUATION BARRIER (REPAIR) 37 TEMPORARY WHACT ATTENUATION BARRIER (REPAIR) 14622 REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1) 100 BRIDGE CONSTRUCTION CONTROL 28 CLASS SCHOOKETE THE BRIDGE 28 CLASS SCHOOKETE THE BRIDGE 28 CLASS SCHOOKETE THE BRIDGE 28 CLASS S PROTECTIVE SURFACE THE AMBINIT 68 PRESIDENCE SURFACE THE AMBINIT 68 PRESIDENCE SURFACE THE REMOVED STEEL (GRADE 60) 28 PREE INCASE MINIT	621 RO	CK DITCH CHECKS	345	CU. YD.
CALCASS SIGNED	623 SE	COND SEEDING APPLICATION	5.41	ACRE
REMONAL OF CONTROL MATTING CLASS 3) 100 ROADWAY CONSTRUCTION CONTROL 100 RAME BOX SIPPORTS (SINGLE) 2 RAME BOX SUPPORTS (SINGLE) 6 RAME BOX SUPPORTS (SINGLE) 7 THE RAMOPLASTIC PARAEMENT MARKING WHITE (\$") 7 THE RAMOPLASTIC PARAEMENT MARKING SILLOW (\$") 7 THE RAMOPLASTIC PARAEMENT MARKERS (TYPE (") 7 THE MODE ANY IMPACT ATTENUATION BARRIER (REPAIR) 3 THE MODE ANY IMPACT ATTENUATION BARRIER (REPAIR) 7 THE MODE CONTROL STELL ROADWAY (GRADE 60) 7 REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1) 7 REMOVAL OF EXISTING BRIDGE STRUCTURE SIRDGE 7 CLASS SICHE CONCRETE BRIDGE 7 CLASS SICHE CONCRETE BOX BEAMS (27'X48') 7 REFERENCE OF CONCRETE BRIDGE 7 CLASS SICHE SIGNE SITE (SANDE	624 SO	FID SODDING	16	SQ. YD.
RADADWAY CONS TRUCTION CONTROL 100 MALIB DOXES 100 MALIB DOXES 100 MALIB DOXES 100 MALIB DOX SUPPORTS (SINGLE) 6 MALIB DOX SUPPORTS (SINGLE) 6 MALIB DOX SUPPORTS (SINGLE) 100 THERWOPLA STIC PAVEMENT MARKING VIPTE (D) 13 TEARED PAYEMENT MARKING STORE (NPE I) 3 TEMPORARY MIRACETS (TYPE II) 3 TEMPORARY MIRACET STRUCTURE (SITE NO. 1) STRUCTURES OVER 20'S PAN REINFORCING STELL ROADWAY (GRADE 60) 1100 BRIDGE CONSTRUCTION CONTROL 1100 BRIDGE CONSTRUCTION CONTROL 1100 BRIDGE CONSTRUCTION CONTROL 1100 BRIDGE CONSTRUCTION CONTROL 1100 CLASS S (AE) CONCRETE BRIDGE 1265.00 CLASS S (AE) CONCRETE BRIDGE 1265.00 CLASS S (AE) CONCRETE BRIDGE (GRADE 60) 1265.00 CLASS S PROTECTIVE SURFACE TREATMENT 1265.00 STEEL SHELL PILL NG (3° DAMETER) 1265.00 STEEL SHELL PILL NG (3° DAMETER) 100 BRIDGE NAME PLATE (TYPE D) 100 <	626 ER	OSION CONTROL MATTING (CLASS 3)	3836	SQ. YD.
MALIBOX SUPSIDED	635 RO	ADWAY CONSTRUCTION CONTROL	1.00	LUMP SUN
MALIBOX SUPPORTS (SINGLE)	637 MA	ILBOXES	10	EACH
MALIBOX SUPFORISE (DOUBLE)	637 MA	ILBOX SUPPORTS (SINGLE)	9	EACH
THERMOPLASTIC PAVEMENT MARKING WHITE (6°) THERMOPLASTIC PAVEMENT MARKING YELLOW (6°) 4370 THERMOPLASTIC PAVEMENT MARKING YELLOW (6°) THERMOPLASTIC PAVEMENT MARKING YELLOW (6°) 4370 TEMPORARY MAPACT ATTENUATION BARRIER (REPAIR) 3 TEMPORARY MIPACT ATTENUATION BARRIER (REPAIR) 3 TEMPORARY MIPACT ATTENUATION BARRIER (REPAIR) 100 REINFORCING STEEL-ROADWAY (GRADE 60) 1100 BRIDGE CONSTRUCTORE (SITE NO. 1) 1100 BRIDGE CONSTRUCTORE (SITE NO. 1) 1100 UNCLASS SCONCRETE BRIDGE 265.00 CLASS S(AE) CONCRETE BRIDGE 265.00 CLASS S(AE) CONCRETE BRIDGE 265.00 CLASS SLAD CONCRETE BRIDGE 265.00 STEEL SHELL PRING (1° DAMETER) 265.00 STEEL SHELL PRING (1° DAMETER) 265.00 PILE ENCASEMENT 265.00 PILE ENCASEMENT 265.00 PILE ENCASEMENT 265.00 PILE ENCASEMENT 266.00 BRIDGE CONSTRUCTORE STEEL	637 MA	ILBOX SUPPORTS (DOUBLE)	2	EACH
THERMOPLASTIC PAVEMENT MARKING YELLOW (6") THERMOPLASTIC PAVEMENT MARKING YELLOW (6") THERMOPLASTIC PAVEMENT MARKING STILEM (1") THERMOPLASTIC PAVEMENT MARKING STATEM (1") THE MINIOR DATA TITEM (1") THE MINI	719 THI	ERMOPLASTIC PAVEMENT MARKING WHITE (6")	5820	LIN. FT.
RAJSED PAVEMBENT MARKERS (TYPE II) 37 TEMPORARY IMPACT ATTENUATION BARRIER 3 TEMPORARY IMPACT ATTENUATION BARRIER 3 TEMPORARY IMPACT ATTENUATION BARRIER 3 REINFORDCING STELL-ROADWAY (GRADE 60) 1.00 REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1) 1.00 BRIDGE CONSTRUCTON CONTROL 28 UNCLASSIFIED CONCRETE-BRIDGE 265.00 CLASS SCANCRETE-BRIDGE 265.00 CLASS SCANCRETE-BRIDGE 265.00 CLASS SCANCRETE-BRIDGE 265.00 CLASS SCANCRETE-BRIDGE 265.00 CLASS SCANCRETE BRIDGE 267.00 CLASS SCANCRETE BRIDGE 267.00 CLASS SCANCRETE BLANKET 100 BRIDGE NAME TERN 100 BRIDGE NAME TERN 100 BRIDGE NAME TERN 267.00 BRIDGE NAME TERN 267.00 <	719 TH	ERMOPLASTIC PAVEMENT MARKING YELLOW (6")	4970	LIN. FT.
TEMPORARY MIPACT ATTENUATION BARRIER 3 TEMPORARY MIPACT ATTENUATION BARRIER (REPAIR) 3 TEMPORARY MIPACT ATTENUATION BARRIER (REPAIR) 14622 REINFORCING STEEL-ROADWAY (GRADE 60) 100 REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1) 100 BRIDGE CONSTRUCTION CONTROL 28 CLASS SCONCRETE-BRIDGE 26 CLASS SCONCRETE-BRIDGE 265.00 PRESSED CONCRETE BOX BEAMS (27.X48") 284.70 PRESSED CONCRETE BOX BEAMS (27.X48") 284.70 CLASS 2 PROTECTIVE SURFACE TREATMENT 284.70 REINFORCING STEEL (GRADE 60) 284.70 STEEL SHELL PLING (3° DIAMETER) 740 PREDOXY COATED REINFRORDING STEEL (GRADE 60) 700 STEEL SHELL PLING (3° DIAMETER) 740 PREDOX NOTED REINFRORDING STEEL (GRADE 60) 863 PREDOXE COATED REINFRORDING STEEL (GRADE 60) 863 BRIDGE NAME PLATE (TYPE D) 740	721 RA	ISED PAVEMENT MARKERS (TYPE II)	37	EACH
TEMPORARY MIPACT ATTENUATION BARRIER (REPAIR) 3 REINFORCING STEEL-ROADWAY (GRADE 60) STRUCTURES OVER 20' SPAN 14622 REINFORCING STEEL-ROADWAY (GRADE 60) 100 100 BRIDGE CONSTRUCTURE (SITE NO. 1) 1 00 1 00 BRIDGE CONSTRUCTURES-BRIDGE 28 91.40 CLASS SCONCRETE-BRIDGE 265.00 265.00 CLASS SCONCRETE BRIDGE (GRADE 60) 265.00 265.00 CLASS SCONCRETE BRIDGE (GRADE 60) 2847.0 2847.0 CLASS SLED CONCRETE BRIDGE (GRADE 60) 285.0 2847.0 CLASS SLEI-BRIDGE (GRADE 60) 265.0 2847.0 REINFORCING STEEL (BRIDGE (GRADE 60) 2847.0 2847.0 STEEL SHELL PILING (20' DAMETER) 655 387.0 PREBORING BRIDGE (ANME TER) 400 400 PREBORING BRIDGE (ANME TER) 400 400 BRIDGE (ANME TER) BRIDGE (ANME TER) 400 400 BRIDGE (ANME TER) 400 400 400 BRIDGE (ANME TER) 400 400 400 BRIDGE (ANME TER)	SS & 731 TEI	MPORARY IMPACT ATTENUATION BARRIER	3	EACH
STRUCTURES OVER 20' SPAN 14622 STRUCTURE (SITE NO. 1) 1.00 1.00 BRIDGE CONSTRUCTION CONTROL 1.00 1.00 UNCL ASSIFIED EXAZANATION FOR STRUCTURES-BRIDGE 2.8 CLASS SCONCRETE-BRIDGE 2.65.00 CLASS SCONCRETE BRIDGE 2.65.00 CLASS SCAEL CONCRETE BRIDGE 2.65.00 CLASS SCAEL SCONCRETE BRIDGE 2.65.00 CLASS SCAEL SCAEL STREAMENT 8.59.9 REINFORCING STEEL-BRIDGE (GRADE 60) 2.67.00 EPOXY COATED REINFRER 7.0960 STEEL SHELL PLING (20" DAMETER) 6.55 STEEL SHELL PLING (20" DAMETER) 6.55 PREBORING PREBORING BRIDGE NAME LATE (TYPE D) 6.70 BRIDGE NAME ET RAINET 6.70 BLIFER BLANKET 6.70 DUMPED RIPRAP 6.70	SS & 731 TEI	MPORARY IMPACT ATTENUATION BARRIER (REPAIR)	3	EACH
REMOVAL OF EXISTING BRIDGE STRUCTURES ONER 20' SPAN 1.00 BRIDGE CONSTRUCTION CONTROL 1.00 UNCLASSIFED EXCAVATION FOR STRUCTURES-BRIDGE 2.8 CLASS CONCRETE BRIDGE 2.8 CLASS SCANCRETE BRIDGE 26.50 CLASS SAGE CONCRETE BRIDGE 26.50 PRESTRESSED CONCRETE BOX BEAMS (27"X48") 28.50 CLASS 2 PROTECTIVE SURFACE TREATMENT 859.9 REINFORCING STEEL (GRADE 60) 28.470 ECLASS 2 PROTECTIVE SURFACE TREATMENT 28.470 ENEL SHELL PILING (16" DIAMETER) 740 PRED STEEL SHELL PILING (16" DIAMETER) 740 PRED STEEL SHELL PILING (16" DIAMETER) 655 STEEL SHELL PILING (16" DIAMETER) 100 PRED STEEL SHELL PILING (16" DIAMETER) 655 STEEL SHELL PILING (18" DIAMETER) 670 BRIDGE NAME PLATE (TYPE D) 670 FILTER BLANKET 670 DUMPED RIPRAP 357	SS & 804 RE	INFORCING STEEL-ROADWAY (GRADE 60)	14622	POUND
BRIDGE CONSTRUCTORE (SITE NO. 1) STRUCTURES OVER 20' SPAN 1.00 BRIDGE CONSTRUCTION CONTROL 1.00 28 UNCLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE 28 CLASS CONCRETE BRIDGE 26.00 PRESS (AED CONCRETE BRIDGE 26.00 PRESS (AED CONCRETE BRIDGE 26.00 PRESS S(AE) CONCRETE BOX BEAMS (27"X48") 26.00 CLASS 2 PROTECTIVE SURFACE TREATMENT 85.00 REINFORCING STEEL BRIDGE (GRADE 60) 85.00 REINFORCING STEEL (GRADE 60) 28.470 STEEL SHELL PILING (16" DAMMETER) 740 PILE ENCASEMENT 65 STEEL SHELL PILING (16" DAMMETER) 66 PREBOGE NAME PLATE (TYPE D) 10 FILTER BLANKET 670 DUMPED RIPRAP 357				
NEWDYAL OF EXISTING SKIDES MOUTURE (SITE NO.1)	100	T (1)		
DECLASS END END CONTROL DECLASS END	205 RE	TURE (SITE	1.00	LUMP SUN
UNCLASSIFED EXCAVATION FOR STRUCTURES-BRIDGE 28 CLASS SCHOCKETE-BRIDGE 265.00 CLASS SCHOCKETE-BRIDGE 265.00 CLASS SCE) CONCRETE BOX BEAMS (27"X48") 785.0 CLASS 2 PROTECTIVE SURFACE TREATMENT 859.9 RENIFORCING STEEL, BRIDGE (GRADE 60) 859.9 RENIFORCING STEEL (GRADE 60) 655 STEEL SHELL PILING (16" DAMETER) 655 STEEL SHELL PILING (20" DAMETER) 740 PREBORNING 63 PREBORNING 100 BRIDGE NAME PLATE (TYPE D) 1 FILTER BLANKET 670 DUMPED RIPRAP 357	636 BR	DGE CONSTRUCTION CONTROL	1.00	LUMP SUN
CLASS S CONCRETE BRDGE 9140 CLASS S CONCRETE BRDGE 265.00 CLASS S(AE) CONCRETE BRDGE 785.0 PRESTRESSED CONCRETE BRDGE 785.0 CLASS 2 PROTECTIVE SURFACE TREATMENT 859.9 REINFORCING STEEL BRDGE (GRADE 60) 2847.0 REINFORCING STEEL GRADE 60) 7096.0 STEEL SHELL PILING (16" DIAMETER) 655 STEEL SHELL PILING (16" DIAMETER) 63 PILE ENCASEMENT 63 BRDGE SAME PATE (TYPE D) 10 FILTER BLANKE THERE BLANKET 670 DUMPED RIPRAP 357	801 UN	CLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE	28	CU. YD.
CLASS S(AE) CONCRETE BRIDGE 265.00 PRESTRESSED CONCRETE BRIDGE 265.00 CLASS 2 PROTECTIVE SURFACE TREATMENT 859.9 REINFORCING STEEL BRIDGE (GRADE 60) 2847.0 ENDXY COATED REINFORCING STEEL (GRADE 60) 2647.0 STEEL SHELL PILING (3° DAMETER) 740 PILE ENCANSEMENT 63 PREBORING 63 BRIDGE NAME PLATE (TYPE D) 10 FILTER BLANKET 670 DUMPED RIPRAP 357	P, SS, & 802 CL,	ASS S CONCRETE-BRIDGE	91.40	CU. YD.
PRES TRESSED CONCRETE BOX BEAMS (27'X48') PRES TRESSED CONCRETE BOX BEAMS (27'X48') PRES TRESSED CONCRETE BOX BEAMS (27'X48') 859 859 859 850	P, SS, & 802 CL	ASS S(AE) CONCRETE-BRIDGE	265.00	CU. YD.
CLASS 2 PROTECTIVE SURFACE IREA MMENT 859.9 CLASS 2 PROTECTIVE SURFACE IREA MMENT 28470 RENFORCING STEEL (GRADE 60) 28470 REPOXY COATED REINFORCING STEEL (GRADE 60) 655 STEEL SHELL PILING (16" DIAMETER) 65 STEEL SHELL PILING (20" DIAMETER) 63 PREBORNO 63 BRIDGE NAME PLATE (TYPE D) 10 FILTER BLANKET 670 DUMPED RIPRAP 357	P, SS, & 802 PR	ESTRESSED CONCRETE BOX BEAMS (27"X48")	785.0	LIN FT.
REINFORCING STEEL-BRIDGE (GRADE 60) 28470 270960	SP & 803 CL	ASS 2 PROTECTIVE SURFACE TREATMENT	829.9	SQ. YD.
EPOXY COATED REINFORCING STEEL (GRADE 60) 70960 STEEL SHELL PILING (1° DIAMETER) 655 STEEL SHELL PILING (2° DIAMETER) 740 PILE ENCASEMENT 63 PREBORING 100 BRIDGE NAME PLATE (TYPE D) 1 FILTER BLANKET 670 DUMPED RIPRAP 357	SS & 804 RE	INFORCING STEEL-BRIDGE (GRADE 60)	28470	POUND
STEEL SHELL PILING (16" DAMETER) 655 STEEL SHELL PILING (20" DAMETER) 740 FILE ENCASEMENT 63 PREBORNIG 100 BRIDGE NAME PLATE (TYPE D) 1 FILTER BLANKET 670 DUMPED RIPRAP 357	SS & 804 EP	OXY COATED REINFORCING STEEL (GRADE 60)	70960	POUND
STEEL SHELL PILING (20" DAMETER) 740 PILE ENCASEMENT 63 PREBORNG 100 BRIDGE NAME PLATE (TYPE D) 1 FILTER BLANKET 670 DUMPED RIPRAP 357	SS & 805 STI	EEL SHELL PILING (16" DIAMETER)	655	LIN. FT.
PILE ENCASEMENT 63 PREDED GING 100 BRIDGE NAME PLATE (TYPE D) 1 FIL TER BLANKET 670 DUMPED RIPRAP 357	SS & 805 STI	EEL SHELL PILING (20" DIAMETER)	740	LIN. FT.
PREBORING 100 BRIDGE NAME PLATE (TYPE D) 1 FILTER BLAINET 670 DUMPED RIPRAP 357	SS & 805 PIL	E ENCASEMENT	63	LIN. FT.
BRIDGE NAME PLATE (TYPE D) 1 FILTER BLANKET 670 DUMPED RIPRAP 357	SS & 805 PR	FRORNS	100	N F
FILTER BLANKET DUMPED RIPRAP 357	20 & 000 842	DOWNING DI ATE (TVDE D)	- 5	- L
PILLER BLAINKE I DUMPED RIPRAP 357	00 9 040 FIII.	TION AND TAKE (TYPE D)	- 60	EACH
DUMPED KIPKAP	SS & 816 FIL	IEX BLANKE I	0/9	SQ. YD.
	SS & 816 DU	MPED RIPRAP	32/	CU. YD.

REVISIONS

10-19-23

SHEETNUMBER	3 & 28						
REVISION	10-19-2023 ADDED "CONTACT INFORMATION FOR MOTORIST DAMAGE CLAIMS" SPECIAL PROVISION.						
DATE	10-19-2023						



101013 28 70

FED.RD. STATE FED.AID PROJ.NO.

ARK. JOB NO.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS	
				6	ARK.				
					AINIX.				
				JOB NO.		101013	29	70	
<u></u>					CHRICK CONTROL DETAILS				

ARKANŠAS LICENSED PROFESSIONAL ENGINEER No.14994 Digitally Signed 08/10/2023

SURVEY CONTROL COORDINATES

Project Name: s101013 Date: 6/6/2019

Coordinate System: ARKANSAS STATE PLANE - NORTH ZONE BASED ON GPS CONTROL. PROJECTED TO GROUND.

lo i ter	11 C	SURVEY	FOOT
JIII (.5.	0. 3.	SURVE	F 00 1

Point. Name	Northing	Easting	Elev Feature	Description
1	647397, 5609	1754027, 7157	327. 427 CTL	STD ARDOT CAP STAMPED PN: 1 STD ARDOT CAP STAMPED PN: 2 STD ARDOT CAP STAMPED PN: 3 STD ARDOT CAP STAMPED PN: 4 STD ARDOT CAP STAMPED PN: 5 SQUARE CUT N EDGE DI X CUT ON BOLT OF FH SOUARE CUT E EDGE CA
2	648242, 2626	1753930, 9805	311. 856 CTL	
3	649074, 4669	1753790, 7918	310. 914 CTL	
4	650137, 3731	1753580, 1429	309. 027 CTL	
5	651047, 7496	1753323, 9664	308. 485 CTL	
900	645376, 4225	1757446, 1735	307. 653 TBM	
901	646422, 9614	1756133, 2782	356. 242 TBM	
902	647581, 4387	1753993, 1455	322. 831 TBM	
903	648369, 4041	1753817, 6537	313.102 TBM	X CUT IN BOLT OF FH SQUARE CUT ON NE CRNR BR X CUT IN BOLT OF FH SQUARE CUT CNTR W HW USGS DISK STMPD R 188 PARAGOULD
904	649186, 3163	1753773, 4183	311.815 TBM	
905	650351, 2368	1753499, 4932	310.352 TBM	
906	650914, 6332	1753340, 9480	308.024 TBM	
999	645308, 2159	1760115, 0616	296.450 BM	

Note - Rebar and Cap - Standard - 5/8 Rebar with 2* Aluminum Cap stamped *(standard markings common to all caps), or as indicated (other markings indicated in the point description of the individual point).

USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT
A PROJECT CAF OF 0.99966999 HAS BEEN USED TO COMPUTE THE ABOVE GROUND COORDINATES.
THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS.
GRID DISTANCE = GROUND DISTANCE X CAF.
GRID COORDINATES ARE STORED UNDER FILE NAME \$101013gi.ct.!
HORIZONTAL DATUM: NAD 83 (2011)

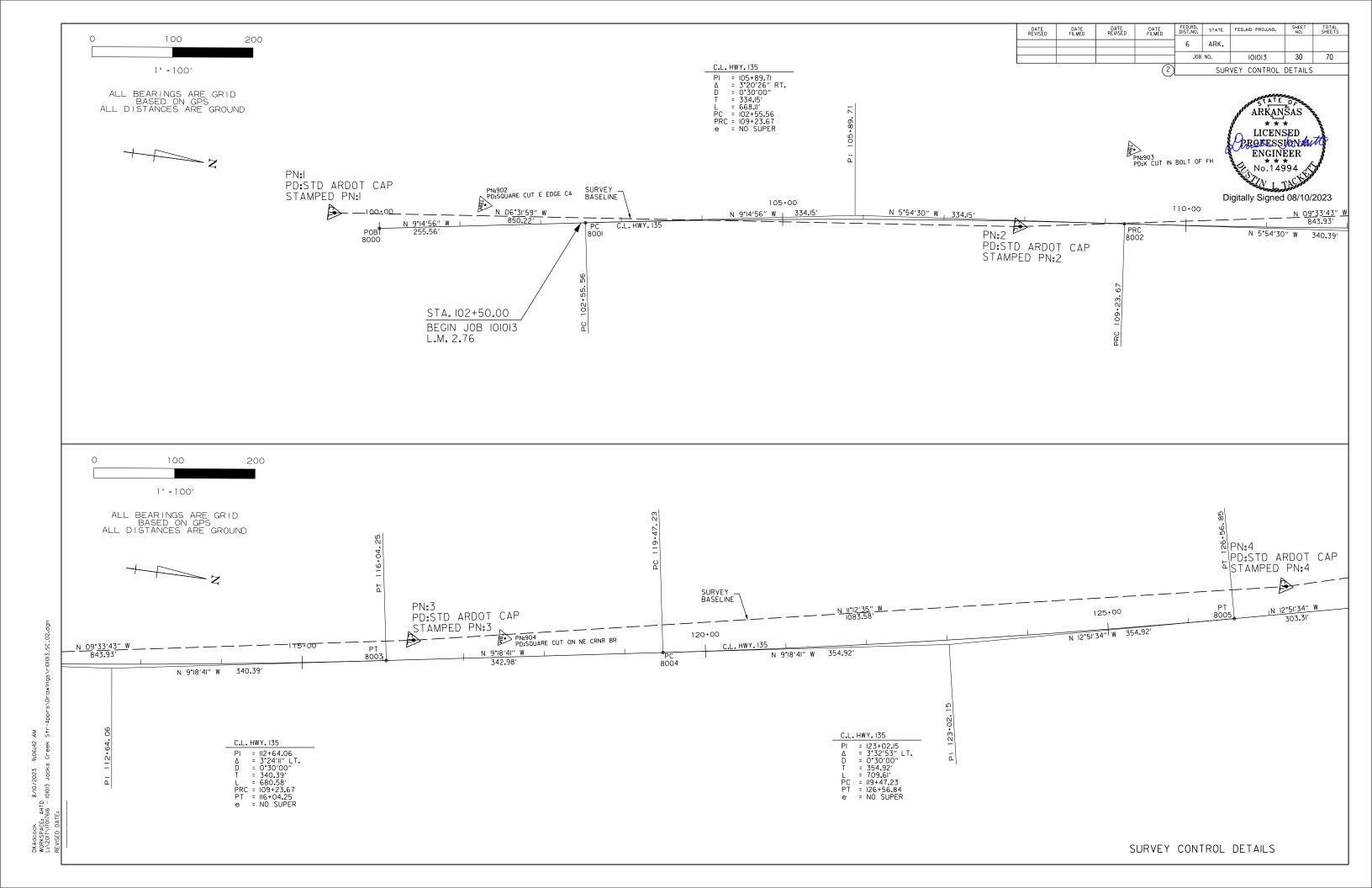
VERTICAL DATUM: NAVD 88 POSITIONAL ACCURACY THIRD ORDER, UNLESS SPECIFIED OTHERWISE AT A SPECIFIC POINT.

REFERENCE POINTS (1500 SERIES) ARE TO BE USED TO ESTABLISH CONTROL IF THE PRIMARY CONTROL POINTS LISTED ABOVE HAVE BEEN DESTROYED. REFERENCE POINTS ARE NOT TO BE USED FOR VERTICAL CONTROL

BASIS OF BEARING:
ARKANSAS STATE PLANE GRID BEARINGS - 0301-NORTH ZONE
DETERMINED FROM STATIC GPS OBSERVATIONS
CONVERGENCE ANGLE: 00-52-10 RIGHT AT LAT N 36-06-26 LON W090-30-20
GRID AZIMUTH = ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

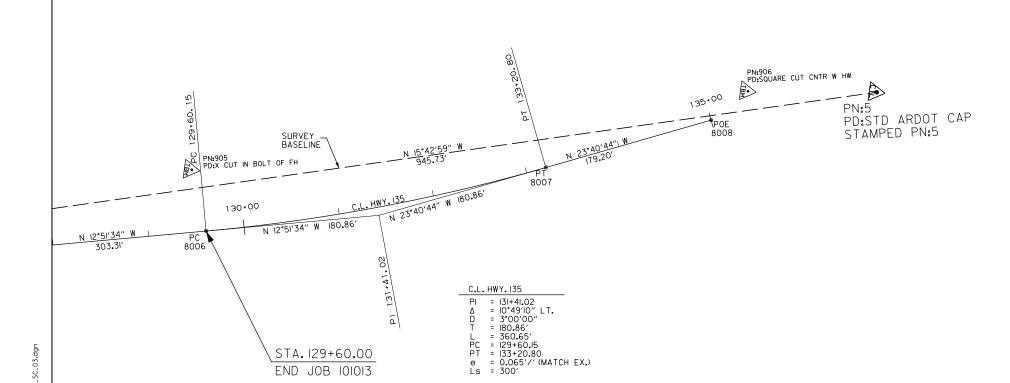
ALIGNMENT NAME: HWY. 135

POINT	STATION	TYPE	NORTHING	EASTING			
8000	100+00.00	POB	647455.1955	1754039.4459			
8001	102+55.56	PC	647707.4310	1753998.3714			
8002	109+23.67	PRC	648369.6114	1753910.2682			
8003	116+04.25	PT	649044.1018	1753820.1535			
8004	119+47.23	PC	649382.5639	1753764.6594			
8005	126+56.85	PT	650078.8265	1753628.2426			
8006	129+60.15	PC	650374.5253	1753560.7389			
8007	133+20.80	PT	650716.4887	1753447.8494			
8008	135+00.00	POE	650880.5980	1753375.8824			



			DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS
0	100	200					6 ARK.		'	
	100						JOB NO.	101013	31	70
						(2)SUI	RVEY CONTROL	DETAILS	
	1" = 100′									





ALL BEARINGS ARE GRID BASED ON GPS ALL DISTANCES ARE GROUND

FED. AID PROJ. NO JOB NO 101013 35 70 07514 LAYOUT 63788

BENCH MARK: Vertical Control Data shown on the Survey Control Data Sheets.

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications For Highway Construction (2014 Edition) with Applicable Supplemental Specifications And Special Provisions. Unless otherwise noted in the plans, Section and Subsection refer to the Standard Construction Specifications.

DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications (2017, 8th Edition)

LIVE LOADING: HL-93

SEISMIC ZONE: 3 S $_{D1}$ = 0.44g Site Class = D

SEISMIC OPERATIONAL CLASSIFICATION: Essential

MATERIALS AND STRENGTHS: Class S(AE) Concrete (Superstructure) f'c = 4,000 psif'c = 8,000 psi f'c = 3,500 psi Class S Concrete (Prestressed Concrete Box Beams) Class S Concrete (Substructure) Prestressing Strands (AASHTO M 203, Gr. 270) Reinforcing Steel (AASHTO M 31 or M 322, Type A) fpu = 270,000 psi fy = 60,000 psi Fy = 50,000 psStructural Steel (ASTM A709, Gr. 50) Structural Steel (ASTM A709, Gr. 36) Fy = 36,000 ps

BORING LOGS: Boring Logs may be obtained from the Construction Contract Procurement Section of the Program Management Division

STEEL SHELL PILING: Piling in Bents 1 & 4 shall be 16" diameter concrete filled steel shell piles and shall be driven to a minimum ultimate bearing capacity of 185 tons and 230 tons per pile, respectively, and to a minimum tip elevation of 250 or lower. Piling in Bents 2 & 3 shall be 20" diameter concrete filled steel shell piles and shall be driven to a minimum ultimate bearing capacity of 375 tons per pile and to a minimum tip elevation of 250 or lower. All piling shall be driven with an approved air, steam, or diesel hammer. Piling in end bents shall be driven after embankment to bottom of cap is in

Lengths of piling shown are assumed for estimating quantities only, Actual lengths are to be determined in the field. No additional payment will be made for cut-off or build-up. Test piles are not required but may be driven for the Contractor's information in accordance with Subsection 805.08(g). No piles will be paid for as Test Piles.

Water jetting or other methods as approved by the Engineer may be required to achieve minimum penetration. This work shall not be paid for directly but shall be considered incidental to the items "STEEL SHELL PILING (16" DIA.)" and "STEEL SHELL PILING (20" DIA.)".

PREBORING: Preboring is required for all piling at Bents 1 and 4. Prebored holes shall have a diameter 6" greater than the diameter of the pile for a depth of 10' below the bottom of cap. The void space around the pile after completion of driving shall be backfilled with sand or pea gravel. The Contractor shall be responsible for keeping prebored holes free of debris prior to backfilling which may require the use of temporary casings or other approved methods. Any related cost for backfilling and temporary casing will not be paid for directly but shall be considered subsidiary to the item "PREBORING"

DRIVING SYSTEM: The driving system approval and the ultimate bearing capacity determination for piling shall be based on the requirements of Subsection 805.09(b), "Method B - Wave Equation Analysis (WEAP)". It is estimated that the minimum rated hammer energy required to obtain the minimum ultimate bearing capacity for 16" piles at Bents 1 & 4 shall be 42,000 and 50,000 foot pounds per blow, respectively, and for 20" piles at Bents 2 & 3 shall be 71,000 foot pounds per

PILE ENCASEMENT: Pile Encasement for Bents 2 and 3 shall extend from bottom of cap to 3 feet below natural ground. See Std. Dwg. No. 55021 for additional information.

BRIDGE DECK: The concrete bridge deck shall be given a tine finish as specified for final finishing in Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish.

PROTECTIVE SURFACE TREATMENT: Class 2 Protective Surface Treatment shall be applied to the roadway surface and to the roadway face and top of the concrete parapet rail in accordance with Section 803.

EXISTING BRIDGE: Existing Bridge No. 02717 (Log Mile 3.08) is 105.0' in length, 26.8' wide (24.0' clear roadway) and consists of a concrete slab on I-beam spans (3 spans total) supported by concrete pile bents. Plans of the existing structure, if available, may be obtained upon request to the Construction Contract Procurement Section of the Program

REMOVAL AND SALVAGE: After the new bridge is open to traffic, the Contractor shall remove existing Bridge No. 02717, including existing haul road and any exposed substructures from previous construction in accordance with Section 205. Exposed substructures from previous construction shall be removed to a depth of 2' below subgrade or final ground exposed substructures from previous construction shall be removed to a depth of 2 below subgrade or infall ground surface. All material from the existing haul road, previous construction, and existing bridge shall become property of the Contractor except the steel beams, including diaphragms and all accessories, which shall remain the property of the Department. The Contractor shall notify the Department prior to removal to coordinate with the Engineer for removal and delivery of the salvage items to District 10 Headquarters, 2510 West Kingshighway, Paragould, AR 72450. This work shall be considered incidental to the item "REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1)".

MAINTENANCE OF TRAFFIC: See Roadway Plans.

(6) Proposed Low Bridge Chord Elev. = 309.76 (Sta. 116+23.00 & Sta. 117+77.00)

For "ELEVATION OF SOIL BORINGS", "BORING LEGEND", "N-VALUES", and "HYDRAULIC DATA", See Dwg. No. 63789.

R = 11,459.16'

PROFESSIONAL **ENGINEER** * * * No.8017

Digitally Signed 08/10/2023

BRIDGE ENGINEER

Stations shown are along CL Construction. Flevations shown are theoretical working point elevations at CL Bridge. Any vertical dimension referenced to CL Deck is based on theoretical working point elevation at CL Bridge. See "ROUNDING DETAIL" on Dwg. No. 63794 for additional information.

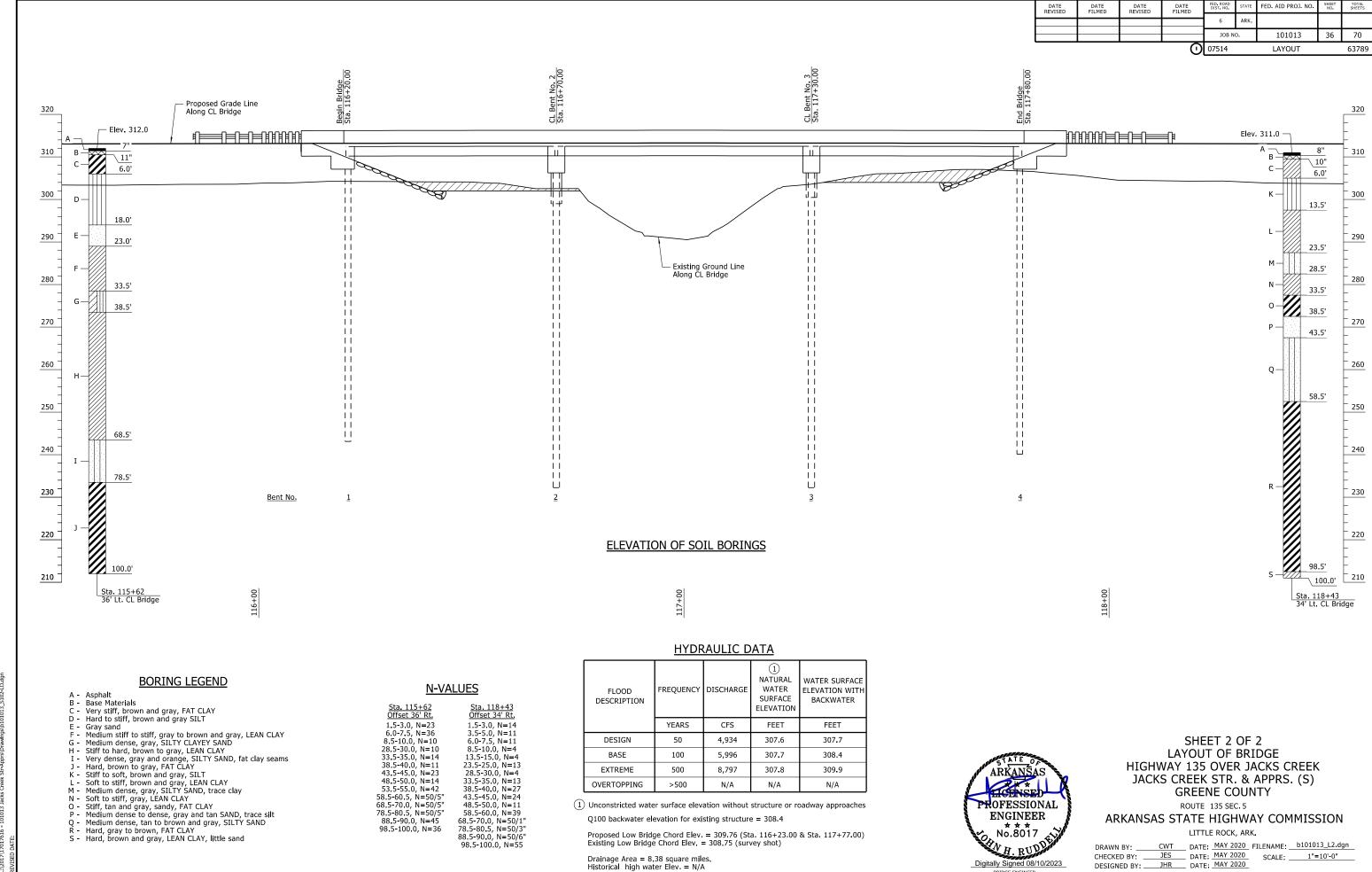
SHEET 1 OF 2 LAYOUT OF BRIDGE HIGHWAY 135 OVER JACKS CREEK JACKS CREEK STR. & APPRS. (S) **GREENE COUNTY**

ROUTE 135 SEC. 5

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

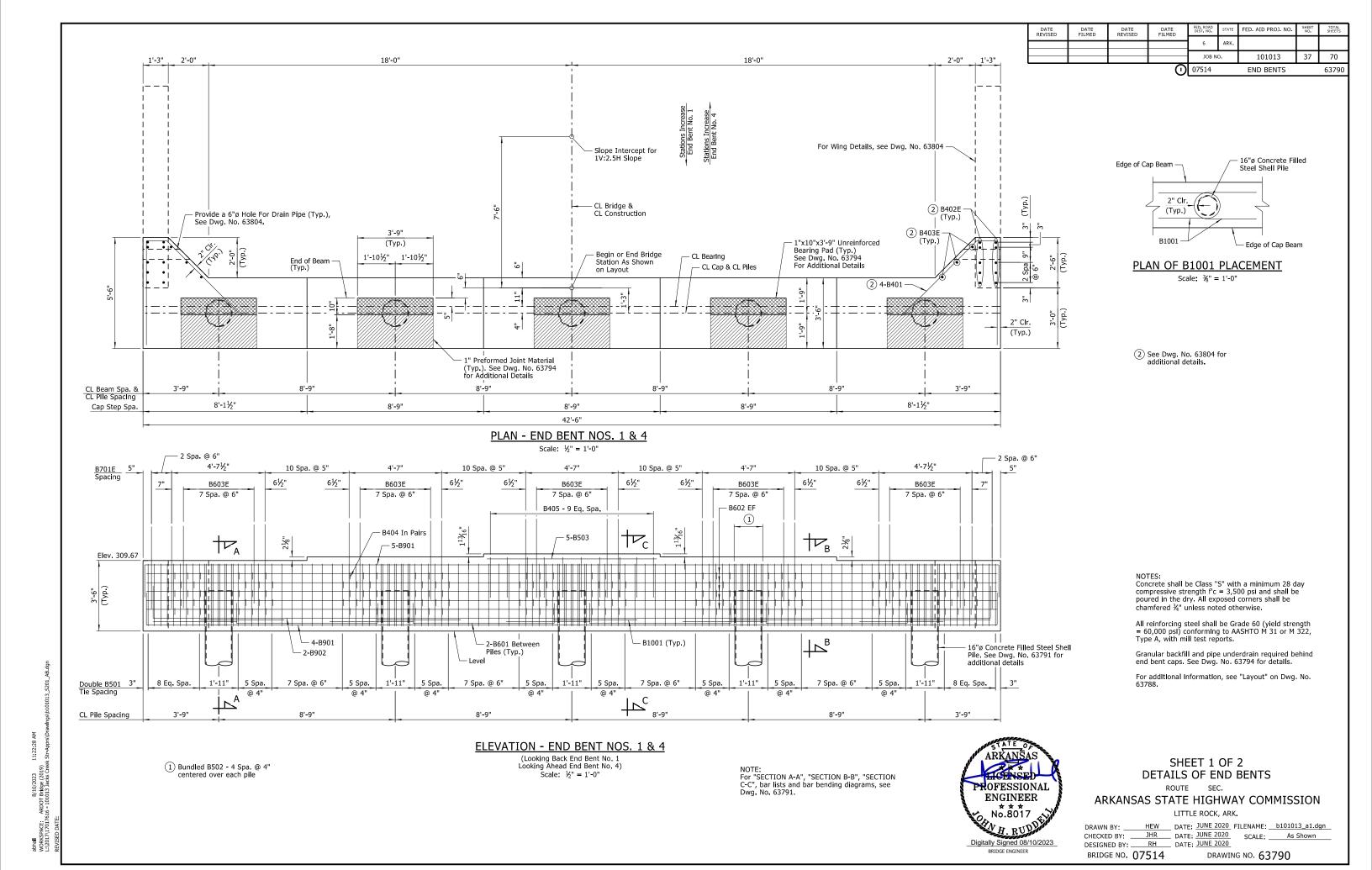
HEW DATE: NOV. 2019 FILENAME: b101013_L1.dgn JES DATE: JAN. 2020 CHECKED BY: SCALE: 1"=20'-0" JHR DATE: NOV. 2019 DESIGNED BY: BRIDGE NO. 07514 DRAWING NO. 63788



BRIDGE ENGINEER

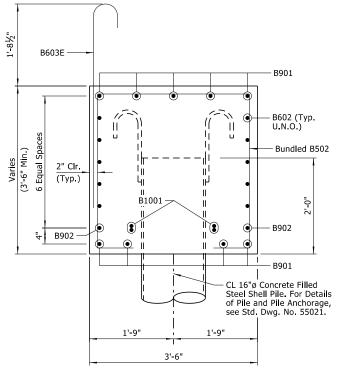
BRIDGE NO. 07514

DRAWING NO. 63789

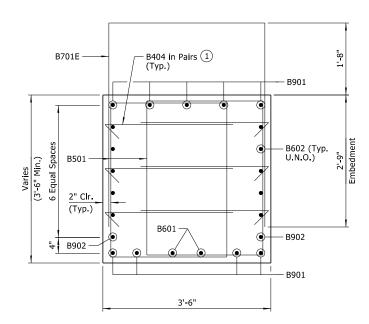


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		101013	38	70

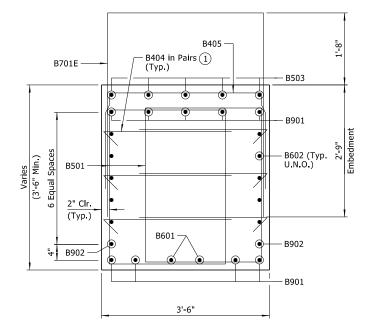
07514 END BENTS 63791



Scale: 1'' = 1'-0''



Section B-B Scale: 1'' = 1'-0''



SECTION C-C Scale: 1" = 1'-0"

 $\begin{tabular}{ll} \hline \end{tabular}$ See "ELEVATION - END BENTS NOS. 1 & 4" on Dwg. No. 63790 for placement.

LEGEND

U.N.O. - Unless Noted Otherwise

BAR LIST (PER BENT)					BAR BENDING DIAGRAMS		
MARK	NO. REQ'D	LENGTH	"A"	"B"	P.D.	. 1'-0" .	
B401	8	11'-3"			2"		
B402E	20	6'-3"			Str.	6 MIn. (Typ.)	
B403E	6	5'-0"			Str.		
B404	300	3'-2"			3"	1	
B405	10	7'-0"	3'-2"	2'-0"	2"	3-7-5	
B501	180	11'-6"			2½"		
B502	50	10'-7"			2½"	<u>B401</u> <u>B501</u>	
B503	5	8'-5"			Str.	2'-8"	
						<u> </u>	
B601	8	8'-9"	7'-0"	1'-0"	4½"	l — — — — — — — — — — — — — — — — — — —	
B602	10	42'-2"			Str		
B603E	40	5'-2"			4½"	B404 N	
		140			-1	<u>B404</u>	
B701E	50	11'-7½"	3'-2"	4'-5"	5¼"		
L		441.401	401.011	41 =140	0"		
B901	9	44'-10"	42'-2"	1'-7¼"	9"	[a] (d) (Typ.)	
B902	2	44'-2"	41'-6"	1'-7¼"	9"	"A"	
B1001	10	9'-4"			1'-8"		
B1001	10	,			1 0	4'-2½"	
						' 	
						B405, B601, B701E,	
						B901 & B902	
						B405, B601, B701E, B901 & B902	
						4'-6" <u>B1001</u>	
						6"	
						DENZE	
						<u>B603E</u>	
						1	

NOTE: Dimensions of bars are out-to-out.

Bars designated with "E" suffix shall be epoxy coated.



SHEET 2 OF 2 DETAILS OF END BENTS

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

 DRAWN BY:
 HEW
 DATE:
 JUNE 2020
 FILENAME:
 b101013_a2.dgn

 CHECKED BY:
 JHR
 DATE:
 JUNE 2020
 SCALE:
 As Shown

 DESIGNED BY:
 RH
 DATE:
 JUNE 2020
 SCALE:
 As Shown
 BRIDGE NO. 07514 drawing no. 63791

abhall 8/10/2023 11:22:29 AM WORKSPACE: ARDOT Bridge (2019) L:\2017\17017616 - 101013 Jacks Greek Str-Apprs\Dra

8'-9"

PLAN - INT. BENT NOS. 2 & 3 Scale: ½" = 1'-0"

8'-9"

8'-9

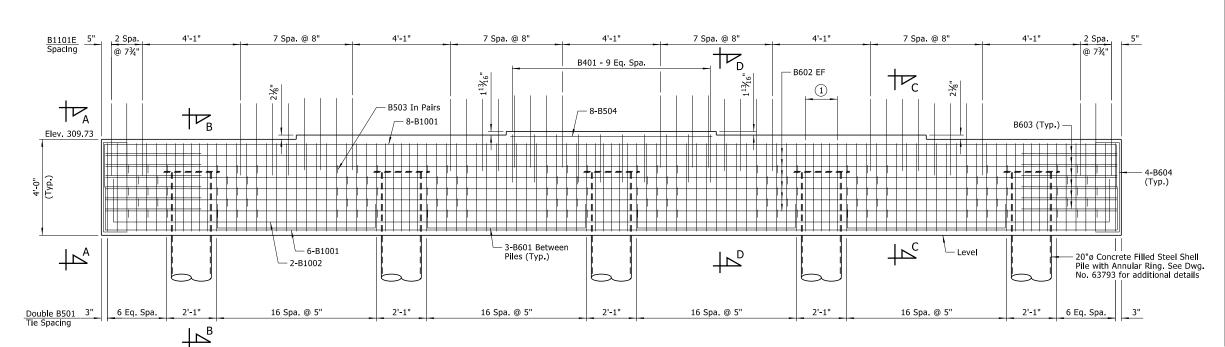
8'-9"

1 B502 - 4 Spa. @ 4" centered over each pile (Typ.)

8'-1½"

1'-10½" | 1'-10½"

3'-9'



ELEVATION - INT. BENT NOS. 2 & 3

(Looking Ahead) Scale: ½" = 1'-0"

8'-1½"

Concrete shall be Class "S" with a minimum 28 day compressive strength f'c = 3,500 psl and shall be poured in the dry. All exposed corners shall be chamfered ¾" unless noted otherwise.

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psj) conforming to AASHTO M 31 or M 322, Type A, with mill test reports.

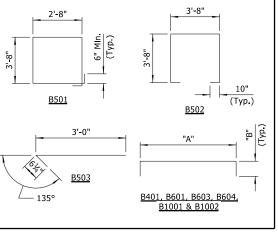
For additional information, see "Layout" on Dwg. No. 63788.

For "VIEW A-A", "SECTION B-B", "SECTION C-C" and "SECTION D-D", see Dwg. No. 63793.

LEGEND EF = Each Face



BAR LIST (PER BENT) MARK NO. REQ'D LENGTH "A" P.D. "B" B401 2½" 2½" 3¾" 12'-3" B502 25 B503 260 3'-7" Str. B504 B601 8'-5½" 1'-0" 41/5" B602 B603 42'-2" Str. 4½" 10'-3" 1'-0" 4½" 3'-6½" B604 B1001 45'-11/ 42'-2" 41'-6" 1'-9½" 1'-9½" B1002 44'-5% B1101E BAR BENDING DIAGRAMS



- 1" Preformed Joint Material (Typ.). See Dwg. No. 63795 for additional details

Dimensions of bars are out-to-out.

Bars designated with "E" suffix shall be epoxy coated.

SHEET 1 OF 2 **DETAILS OF INTERMEDIATE BENTS**

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

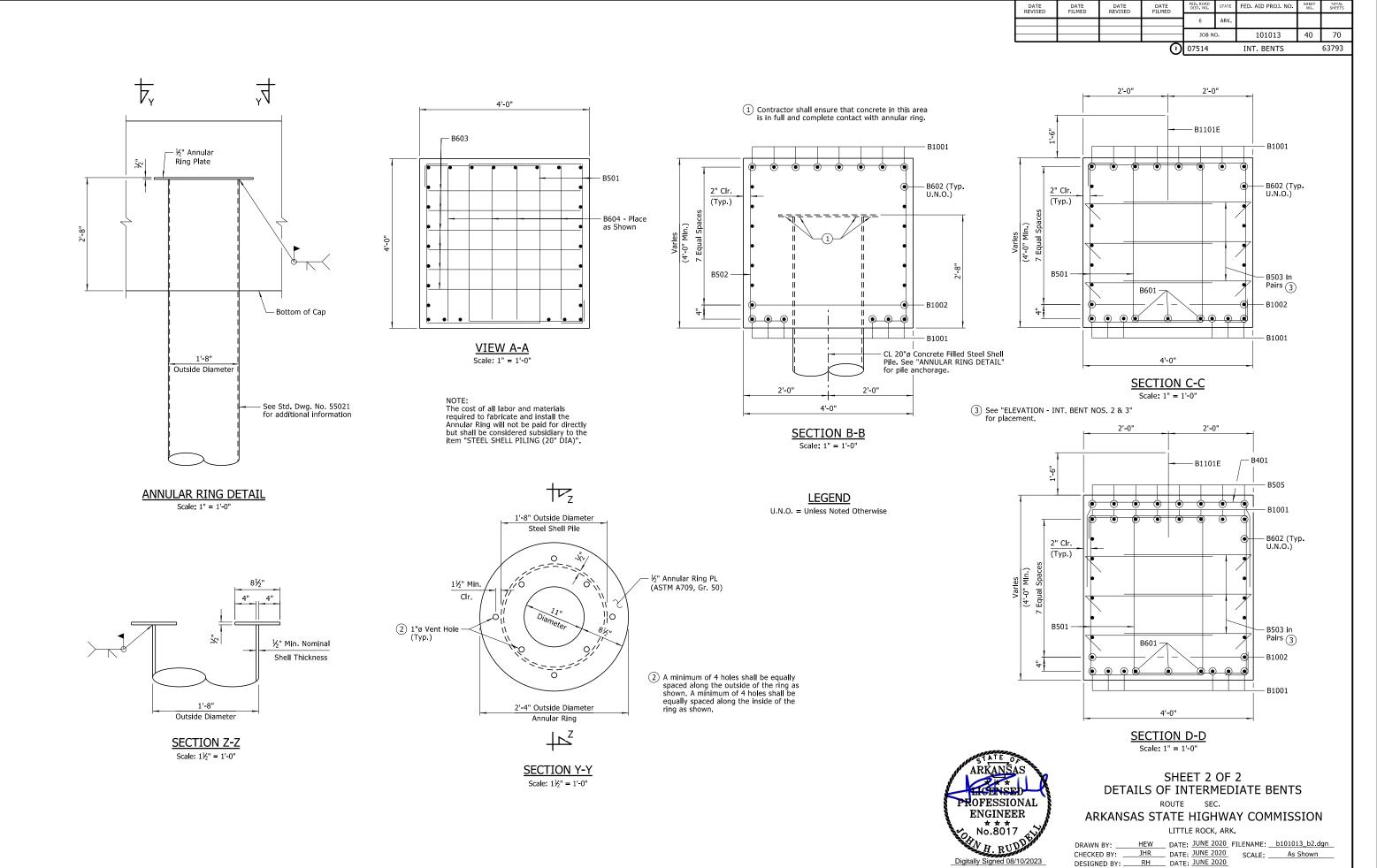
HEW DATE: JUNE 2020 FILENAME: b101013_b1.dgn
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 DATE:
 JUNE 2020
 SCALE:
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 DESIGNED BY:
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 DATE:
 JUNE 2020
 SCALE:
 As Shown
 BRIDGE NO. **07514** DRAWING NO. 63792

CL Beam Spacing &

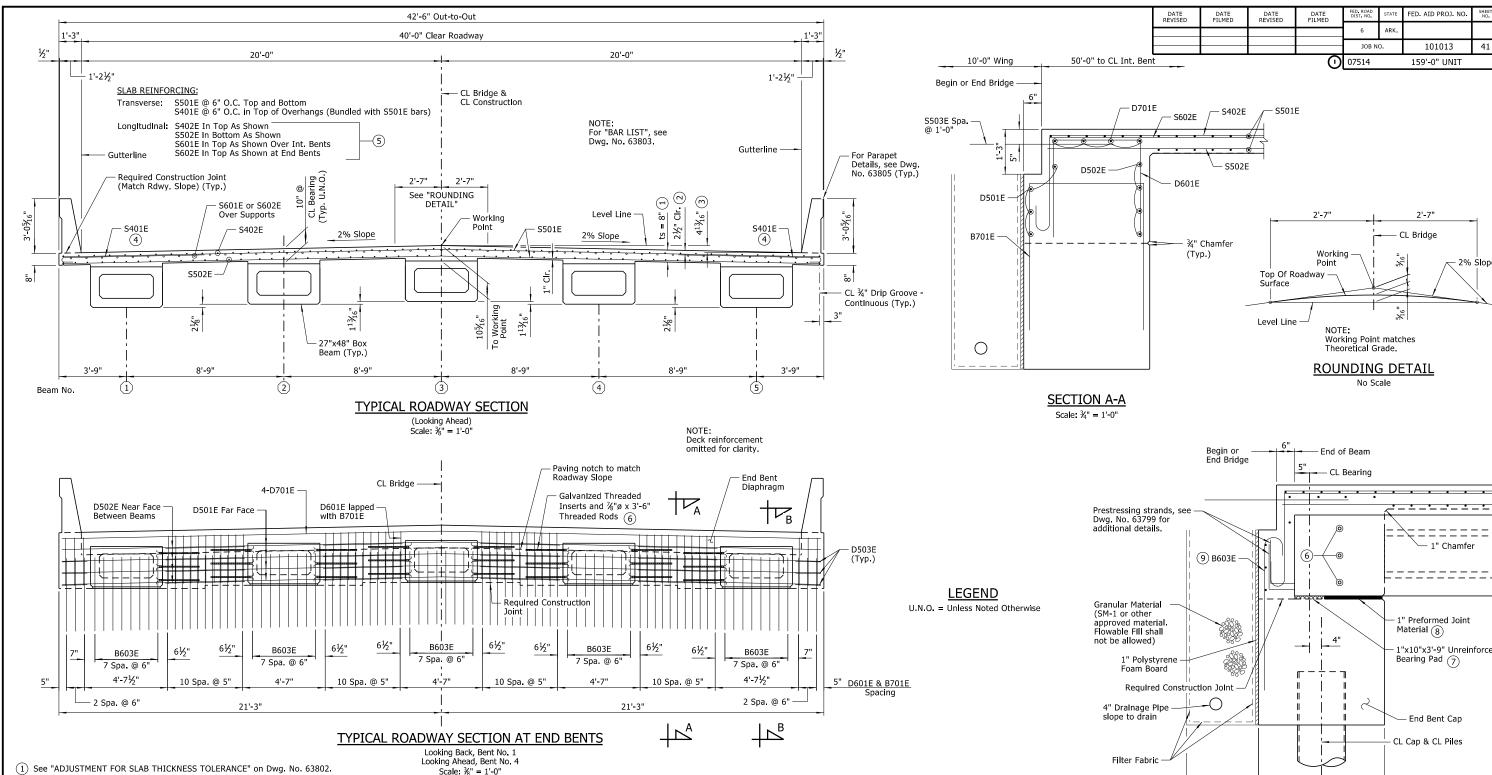
Cap Step Spacing

CL Pile Spacing



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BRIDGE NO. 07514



(1) See "ADJUSTMENT FOR SLAB THICKNESS TOLERANCE" on Dwg. No. 63802.

(3) Working Point to Gutterline

4 Bundle with S501E bars in top. Rotate as needed to avoid interference with bottom mat of deck reinforcement.

Plus = to the amount of slab thickening used to meet slab thickness tolerance. See "ADJUSTMENT FOR SLAB THICKNESS TOLERANCE" on Dwg. No. 63802.

- 5 See "HALF REINFORCING PLAN & DECK POURING SEQUENCE" on Dwg. No. 63797.
- ⑥ Galvanized Threaded Inserts shall be Dayton-Richmond F-42 Loop Ferule Inserts or approved equal. ⅙"ø Galvanized Threaded Rods shall be ASTM A709, Grade 36 or AASHTO M 31 or M 322 Type A, Grade 60. Galvanizing shall be in accordance with AASHTO M 232, Class C or ASTM B695, Class 50. These items will not be paid for directly but shall be considered subsidiary to the item "PRESTRESSED CONCRETE BOX BEAMS (27"x48")".
- (7) Unreinforced bearing pads shall meet the requirements of Section 808 with the exception that hardness shall be 50 durometer. Unreinforced bearing pads shall not be paid for directly, but shall be considered subsidiary to the item "CLASS S(AE) CONCRETE BRIDGE".
- (8) Preformed Joint Material shall conform to AASHTO M 153 Type I. See "PLAN END BENT NOS. 1 & 4" on Dwg. No.
- 9 See End Bent Details on Dwg. Nos. 63790 and 63791.

Limits of the concrete End Bent Diaphragm shall match plan dimension of End Bent Cap.

Preformed Joint Material will not be paid for directly, but shall be considered subsidiary to the Item "CLASS S(AE)

For additional details of pipe underdrain see Std. Dwg. PU-1 and Section 611. Pipe underdrains will not be measured or paid for separately, but shall be considered subsidiary to the unit price bid for "UNCLASSIFIED EXCAVATION".

1" Polystyrene Foam Board, Filter Fabric and Granular Material shall not be paid for directly, but shall be considered subsidiary to the various bid items.

Class 2 Protective Surface Treatment shall be applied to the roadway surface and to the roadway face and top of the concrete parapet rall.

Bar positions and clearances from the forms shall be maintained by means of stays, ties, hangers or other approved devices sufficient in size and number to prevent displacement during construction, per Subsection 804.06. Placement of slab bolsters or hi-chairs with full-length lower runners directly on removable deck forms will not be allowed.

For "GENERAL NOTES - SUPERSTRUCTURE", see Dwg. No. 63802.



SHEET 1 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT ROUTE SEC.

SECTION B-B

Scale: ¾" = 1'-0"

FED. AID PROJ. NO

101013

159'-0" UNIT

2'-7"

" Preformed Joint

– End Bent Cap

CL Cap & CL Piles

- 1"x10"x3'-9" Unreinforced Bearing Pad 7

Material (8)

2% Slope

CL Bridge

70

63794

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

CWT DATE: MAY 2020 FILENAME: b101013_s1.dgn JHR DATE: JUNE 2020

JME DATE: MAY 2020 CHECKED BY: ___ SCALE: As Shown DESIGNED BY: BRIDGE NO. **07514** DRAWING NO. 63794

BRIDGE ENGINEER

① Galvanized Threaded Inserts shall be Dayton-Richmond F-42 Loop Ferule Inserts or approved equal. ⅙"ø Galvanized Threaded Rods shall be ASTM A709, Grade 36 or AASHTO M 31 or M322 Type A, Grade 60. Galvanizing shall be in accordance with AASHTO M 232, Class C or ASTM B695, Class 50. These items will not be paid for directly but shall be considered subsidiary to the item "PRESTRESSED CONCRETE BOX BEAMS (27"x48")".

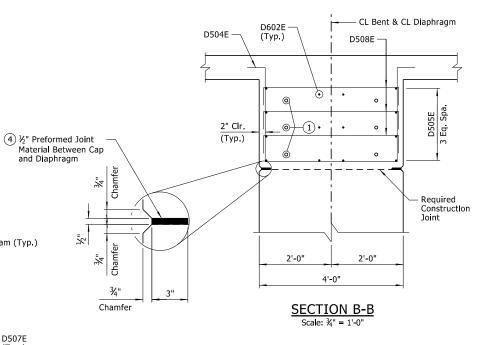
2 For additional details of B1101E bars, see Dwg. No. 63792.

- 3 Unreinforced bearing pads shall meet the requirements of Section 808 with the exception that hardness shall be 50 durometer. Unreinforced bearing pads shall not be paid for directly, but shall be considered subsidiary to the item "CLASS S(AE) CONCETTE BRIDGE"
- 4 Preformed Joint Material shall conform to AASHTO M 153 Type 1. See "PLAN INT. BENT NOS. 2 & 3" on Dwg. No. 63792.

IOTES:

Limits of Intermediate Bent Diaphragm shall match plan dimension of Intermediate Bent Cap.

Preformed Joint Material will not be paid for directly, but shall be considered subsidiary to the item "CLASS S(AE) CONCRETE-BRIDGE".



U.N.O. = Unless Noted Otherwise

ARKANŠAS ARKANŠAS ARKANŠAS ARKANŠAS PROFESSIONAL ENGINEER No.8017 Digitally Signed 08/10/2023

BRIDGE ENGINEER

SHEET 2 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

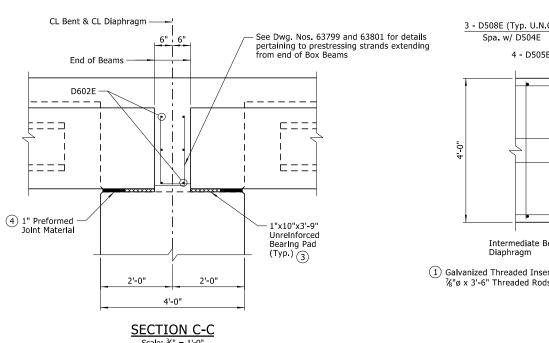
LITTLE ROCK, ARK.

 DRAWN BY:
 CWT
 DATE:
 MAY 2020 MAY 2020

Intermediate Bent Galvanized Threaded Inserts And (1) CL Bridge -Diaphragm (Typ.) $\frac{7}{8}$ ø x 3'-6" Threaded Rods (Typ.) D505E Each Face -Between Beams 1亿 2 - D602E -(Typ.) (Typ.) 3 - D507F (Typ.) D506E (Typ.) Required Construction D504E & B1101E 5" 7 Spa. @ 8 7 Spa. @ 8" 7 Spa. @ 8' 7 Spa. @ 8 4'-1" @ 7¾" @ 7¾" 3'-9" 8'-9" 8'-9" 8'-9" 8'-9" 3'-9" 21'-3" 21'-3"

TYPICAL ROADWAY SECTION AT INTERMEDIATE BENTS

(Looking Ahead)



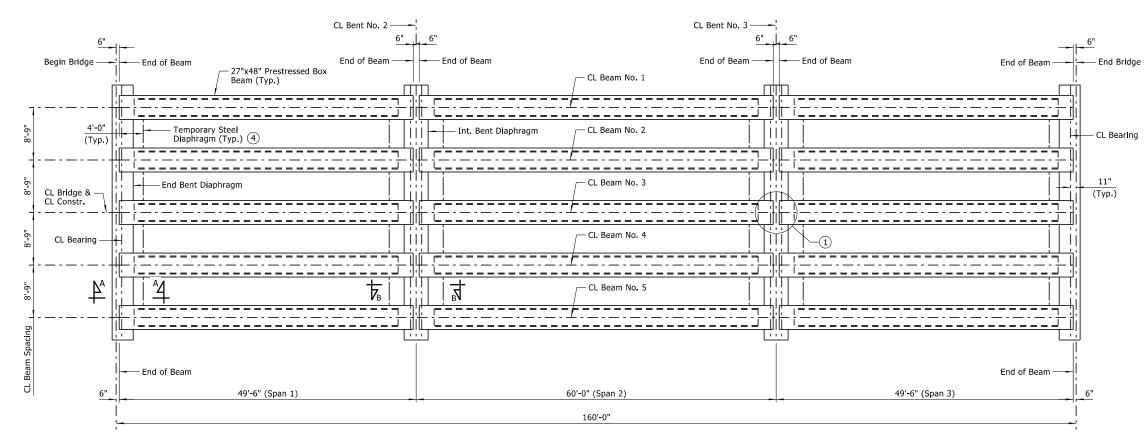
3 - D508E (Typ. U.N.O.)
Spa. w/ D504E
4 - D505E

Intermediate Bent
Diaphragm

1 Galvanized Threaded Inserts and
%"ø x 3'-6" Threaded Rods (Typ.)

Scale: 3/4" = 1'-0"

07514 159'-0" UNIT 63796



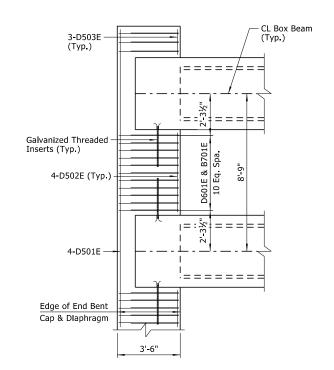
NOTE: For "SECTION A-A", see Dwg. No. 63794.

For "SECTION B-B", see Dwg. No. 63795.

- (1) After erection, the ends of the beams at all bents shall be blocked using temporary blocking to maintain proper location on bent caps. The ends of beams shall remain blocked until the temporary steel diaphragms are
- \bigodot The #5 bar shall be connected to Bar B401 within the limits of the End or Int. Bent Dlaphragm.
- ③ Galvanized Threaded Inserts shall be Dayton-Richmond F-42 Loop Ferrule Inserts or approved equal. ¾"ø Galvanized Bolts shall be Hi. Str. bolts galvanized In accordance with AASHTO M 232, Class C or ASTM B695, Class 50. These items will not be paid for directly but shall be considered subsidiary to the Item "PRESTRESSED CONCRETE BOX BEAM (27"x48")".
- (4) The Temporary Steel Diaphragms shall be installed prior to commencing deck forming. After the concrete deck construction and curing are complete, the temporary steel diaphragms and connecting elements may remain in place or be removed and become property of the contractor and the holes in the box beam web filled with a QPL approved non-shrink epoxy grout.

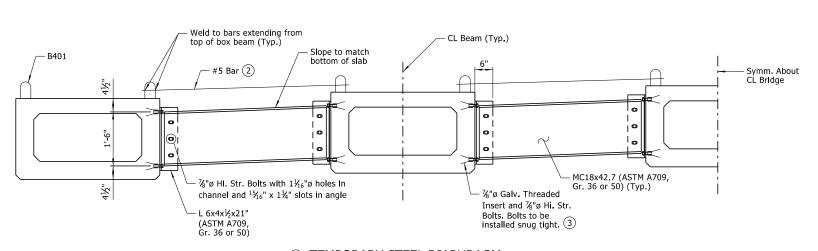
FRAMING PLAN Scale: 1/8" = 1'-0"

A standard washer shall be supplied under both the nut and the head of the $\frac{7}{8}$ "ø Hi. Str. Bolts. An additional plate washer shall cover the angle slots.



PLAN OF END BENT DIAPHRAGM

Scale: ¾" = 1'-0"



(4) TEMPORARY STEEL DIAPHRAGM

Steel Diaphragms shall be used at locations noted as "Temporary Steel Diaphragm". The Temporary Steel Diaphragm and components will not be paid for directly, but shall be considered subsidiary to the item "PRESTRESSED CONCRETE BOX BEAM (27"x48")".

All components of Temporary Steel Diaphragms shall be galvanized. Channels and angles shall be galvanized in accordance with Subsection 807.19.



SHEET 3 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

CWT DATE: MAY 2020 FILENAME: b101013_s3.dgn
 CHECKED BY:
 JHR
 DATE:
 JUNE 2020
 SCALE:
 As Shown

 DESIGNED BY:
 JME
 DATE:
 MAY 2020
 MAY 2020</td

BRIDGE NO. 07514

No Scale

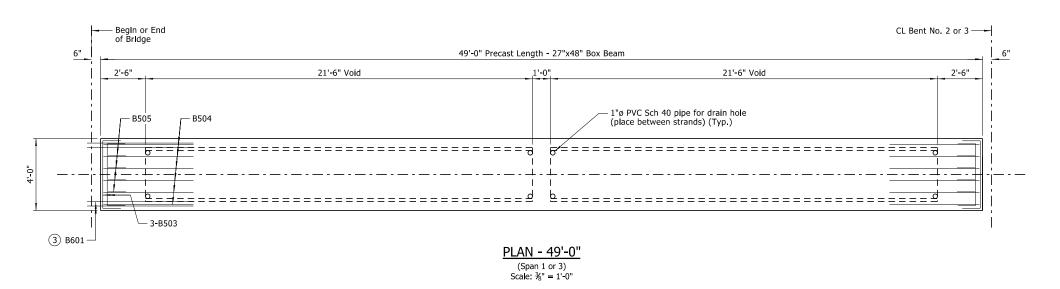
BRIDGE ENGINEER

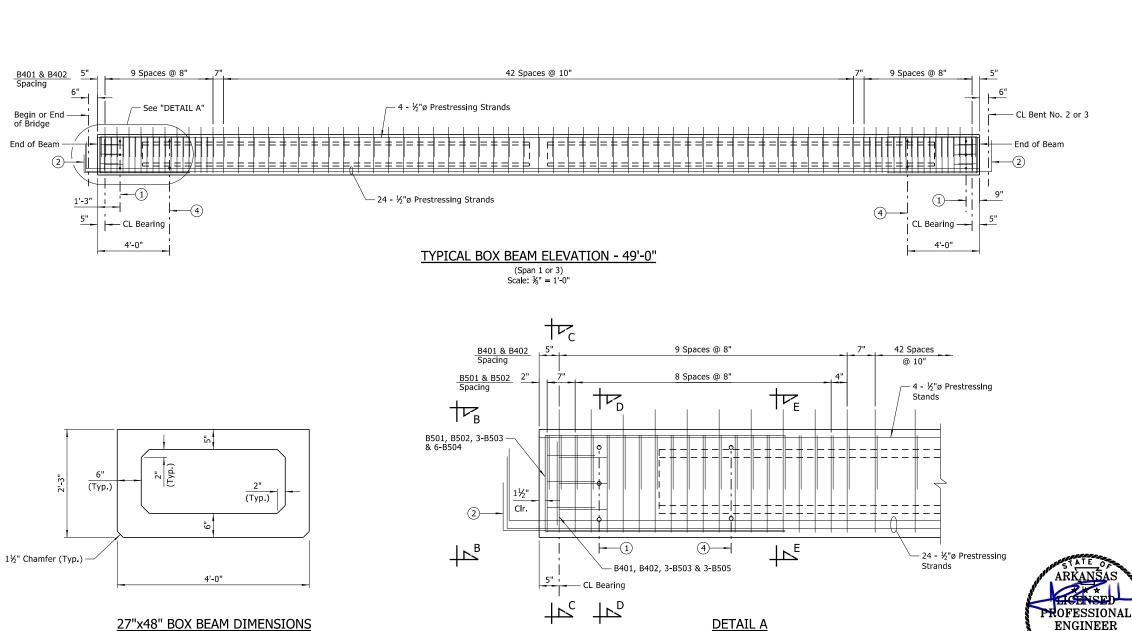
BRIDGE NO. **07514**

DRAWING NO. 63797

FED. AID PROJ. NO

gutterline to gutterline.





Scale: 1" = 1'-0"

FED. AID PROJ. NO 101013 45 70 JOB NO.

159'-0" UNIT 63798 07514

- 1 CL %"ø Threaded Inserts at interior face of exterior beams and both faces of interior beams. See Dwg. Nos. 63794 and 63795 for additional details.
- (2) See "END OF BEAM VIEW AT BENT NOS. 1 & 4 49'-0" BEAM" and "END OF BEAM VIEW AT BENT NOS. 2 & 3 49'-0" BEAM" on Dwg. No. 63799 for details of reinforcing extending from end of beam.
- 3 B601 required at Bent Nos. 1 and 4 only
- (4) CL %"ø Threaded Inserts for Temporary Steel Diaphragms. See Dwg. No. 63796 for additional details.

NOTES:

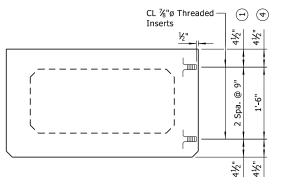
Dimensions are measured along CL Beam.

Prestressing strands and PVC pipe will not be paid for directly, but will be considered subsidiary to the item "PRESTRESSED CONCRETE BOX BEAMS (27"x48")".

Prestressing strands shall be bonded along the entire length of beam.

See Dwg. No. 63799 for "VIEW B-B", "SECTION C-C", "SECTION D-D", and "SECTION E-E".

For "GENERAL NOTES - SUPERSTRUCTURE", see Dwg. No. 63802.



INSERT DETAIL Scale: 1" = 1'-0"

* * * No.8017

Digitally Signed 08/10/2023 BRIDGE ENGINEER

SHEET 5 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

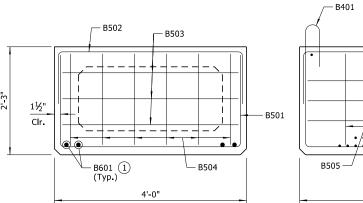
DRAWN BY:	CWT	DATE:	MAY 2020	FILENAME:	b101013_s5.dgn
CHECKED BY:	JHR	DATE:	JUNE 2020	SCALE:	As Shown
DESIGNED BY:	JME	DATE:	MAY 2020		
BRIDGE NO.	07514		DRAWI	ng no. 63	798

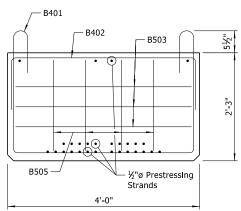
Scale: 1" = 1'-0"

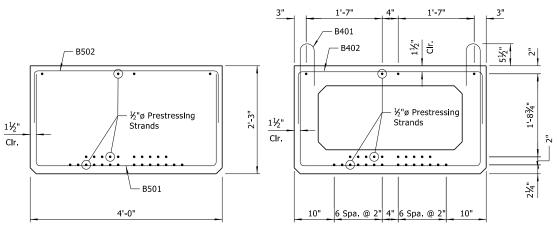
63799 07514 159'-0" UNIT

"B" P.D.

1) B601 required at End Bent only







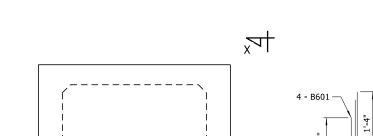


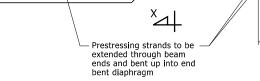
NOTE: Extended strands omitted

SECTION C-C Scale: 1" = 1'-0"

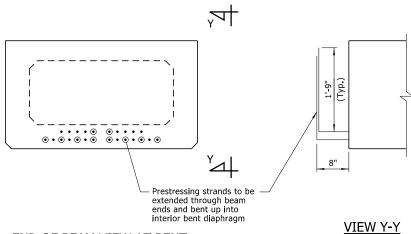
SECTION D-D Scale: 1" = 1'-0"

SECTION E-E (Showing spacing of prestressing strands)
Scale: 1" = 1'-0"









END OF BEAM VIEW AT BENT NOS. 2 & 3 - 49'-0" BEAM

Scale: 1" = 1'-0"

Scale: 1" = 1'-0"

END OF BEAM VIEW AT BENT NOS. 1 & 4 - 49'-0" BEAM Scale: 1" = 1'-0"

① B601 (Typ.)

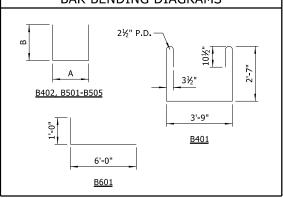
At End Bents, saw and shop bend 18 prestressing strands as shown. Cut or grind remaining strands to within 1" of the end of

At Intermediate Bents, saw and shop bend 10 bottom prestressing strands from end of beam into intermediate bent diaphragm as shown.

B401	63	10'-8½"			2"			
B402	63	6'-7"	3'-9"	1'-6"	2"			
B501	20	7'-6½"	3'-9"	2'-0"	2½"			
B502	20	7'-0½"	3'-9"	1'-9"	2½"			
B503	12	5'-5"	3'-7½"	1'-0"	2½"			
B504	12	11'-8"	1'-10½"	5'-0"	2½"			
B505	8	3'-8"	1'-10½"	1'-0"	2½"			
B601	4	6'-10"			4½"			
BAR BENDING DIAGRAMS								

BAR LIST - PER BEAM

MARK NO. REQ'D LENGTH "A"



NOTE: All bars In the Bar List will not be paid for directly, but will be considered subsidiary to the item "Prestressed Concrete Box Beams (27"x48")".

VIEW X-X

Scale: 1" = 1'-0"

Details on this drawing are applicable



SHEET 6 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

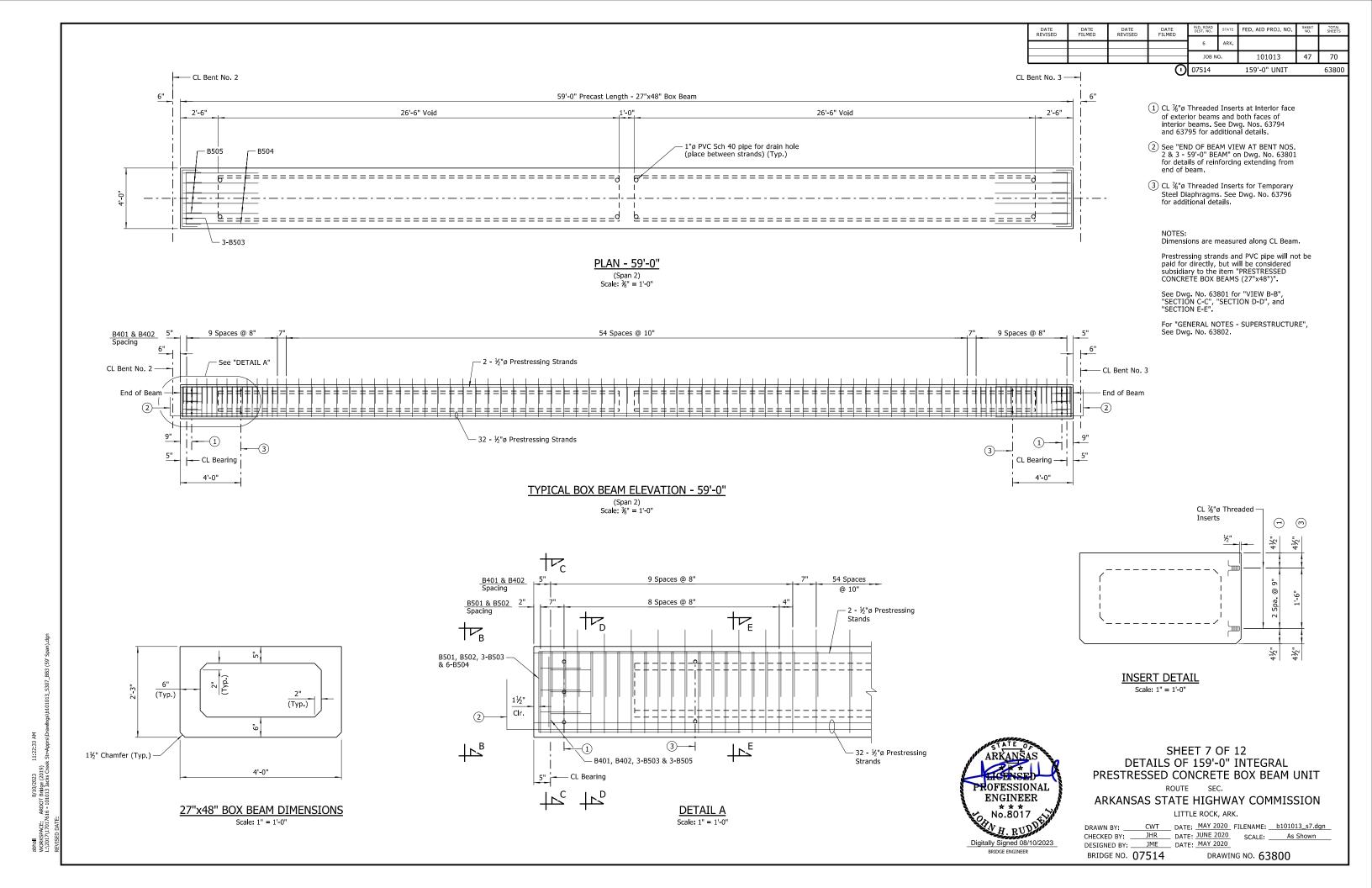
CWT DATE: MAY 2020 FILENAME: b101013_s6.dgn
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 DATE:
 JUNE 2020
 SCALE:
 As Shown

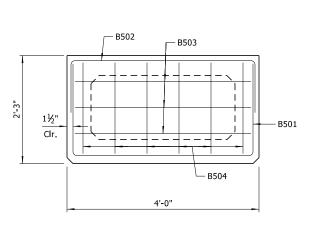
 DESIGNED BY:
 JME
 DATE:
 MAY 2020

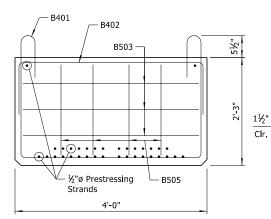
abha**ll** 8/10/2023 11:22:32 AM WORKSPACE: ARDOT Bridge (2019) L:\2017\17017616 - 101013 Jacks Creek Str-Apprs\Dr

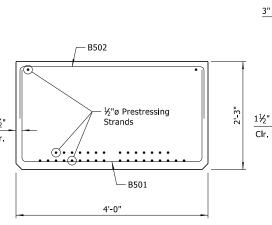
BRIDGE ENGINEER

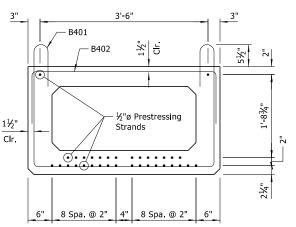
BRIDGE NO. 07514











VIEW B-B Scale: 1" = 1'-0"

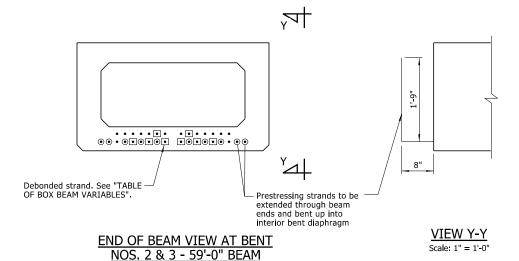
SECTION C-C Scale: 1" = 1'-0"

SECTION D-D Scale: 1" = 1'-0"

SECTION E-E

(Showing spacing of prestressing strands)
Scale: 1" = 1'-0"

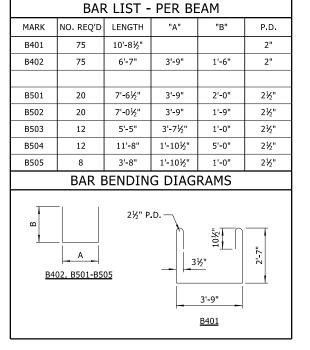
NOTE: Extended strands omitted from this view for clarity.



Scale: 1" = 1'-0"

At Intermediate Bents, saw and shop bend 10 prestressing strands from end of beam into intermediate bent diaphragm as shown.

> NOTE: Details of this drawing are applicable to Span 2.



All bars in the Bar List will not be paid for directly, but will be considered subsidiary to the item "Prestressed Concrete Box Beams (27"x48")".

TABLE OF BOX BEAM VARIABLES

	STRAND DESIGNATION		RIABLES NG/DEBO		"A"
ROW	LINE	"A"	"B"	"C"	Bonded ROW 3
1	A,B,C,D,F,H,K,M,O,P,Q,R	59'-0"			
1	G,I,J,L		3'-0"	53'-0"	"B" "C" "B"
1	E,N		6'-0"	47'-0"	Debonded Bonded Debonded
2	C,D,E,F,G,I,J,L,M,N,O,P	59'-0"			
2	H,K		6'-0"	47'-0"	BONDING/DEBONDING
3	A',R'	59'-0"			DIAGRAM ROW 1
					ROW 1
					A' A B C D E F G H I J K L M N O P Q R R'



SHEET 8 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC. ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

CWT DATE: MAY 2020 FILENAME: b101013_s8.dgn DRAWN BY: ____
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 SCALE:
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 DESIGNED BY:
 JME
 DATE:
 MAY 2020

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BRIDGE NO. 07514

Pretensioning steel shall be ½"ø low relaxation strands with a minimum ultimate strength of 270 ksi and shall conform to AASHTO M 203.

Distances from the forms and spacing of the prestressing steel shall be maintained by stays, ties, hangers, spacers, or other approved supports which shall be shown on

All beams shall be 27" x 48" Box Beams as noted on the details. All beams shall be cast in floored pallets and in metal forms. All work and materials shall be as

Concrete shall be Class S and shall have a minimum 28-day compressive strength f'c = 8.000 psi. The initial tensile force applied to each \(\frac{1}{2} \)" dia. strand shall be 31.000 lbs. except as noted. Transfer of this tensioning load to the beam shall not be done until the compressive strength of the concrete is 6,000 psi.

Dimensions shown are to the center of the strands.

The contractor shall submit the method and sequence for release of strands to the Engineer for approval prior to casting of the beams.

Holes and inserts shall be cast into the beams. Field drilling of holes shall not be permitted.

The tops of the beams shall be rough floated at approximately the time of set. This portion of the tops of beams shall be scrubbed transversely with a coarse wire brush to remove all laitance and to produce a roughened surface with an amplitude of \(\frac{1}{2} \)" to produce an adequate surface for bonding the slab.

Extreme care shall be exercised in handling and moving precast prestressed concrete beams. Beams must be maintained in an upright position at all times and must be picked up from points near the beam ends. Disregard of this requirement may lead to collapse of the beam. The contractor's proposed lifting details shall be submitted on shop drawings to the Engineer for approval. The use of holes for lifting purposes will not be permitted.

The points of support and directions of the reactions with respect to the member shall be approximately the same during transportation and storage as when the

Beam lengths shown on the design plans are net lengths measured horizontally along the beam centerlines. The beam manufacturer shall make the necessary allowances for grade and shortening due to elastic shortening, creep, and shrinkage

Reinforcing steel shall be AASHTO M 31 or M 322, Type A (fy = 60,000 psi) with mill test reports,

After detensioning, saw cut, grind, or bend up strands as designated by the plans. Heat-cutting or bending methods shall not be used within 6" of the beam.

The Contractor may submit alternate strand patterns with design calculations for review and approval in accordance with Subsection 802.22.

Void filler shall be non-absorptive cellular polystyrene according to ASTM C578, designed to withstand the forces imposed during fabrication without substantial deformation or collapsing. Cardboard void filler will not be allowed. The outside dimensions of void filler shall be as shown in the plans. When two or more sections of void filler are used to make up a required length, the individual sections shall be effectively taped or spliced together.

Drawings show general features of design only. Shop drawings shall be made in accordance with specifications, submitted, and approved before fabrication is begun.

REINFORCING STEEL:

All reinforcing steel shall conform to AASHTO M 31 or M 322, Type A (fy = 60,000 psi) with mill test reports and shall be epoxy coated. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly but will be considered subsidiary to the item "EPOXY COATED REINFORCING STEEL (GRADE 60)".

CONCRETE:

All concrete in slab, parapet and diaphragms shall be Class S(AE) with a minimum 28 day compressive strength, f'c = 4,000 psi. Concrete shall be poured in the dry, and all exposed corners shall be chamfered ¾" unless otherwise noted. All end bent and intermediate bent diaphragms shall be cast in place and poured a minimum of 48 hours before the slab is poured. Removable forms shall be used when pouring diaphragms. The slab and diaphragms shall not be poured prior to 90 days following release of the prestressed beam strands.

The superstructure details shown are for use when removable deck forming is used and are the basis for measurement of the item "CLASS S(AF) CONCRETE - BRIDGE". See Standard Drawing No. 55005 for allowable modifiations and for tolerances when Permanent Steel Bridge Deck Forms are used.

Concrete in bridge superstructure shall be placed, consolidated and screeded off for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent.

The concrete deck (roadway surface) shall be given a tine finish in accordance with Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour. Sufficient concrete must be placed ahead of the strike-off to fully load the beam. When permitted, the use of a longitudinal strike-off will require that a vertical camber adjustment in the strike-off to account for future dead load deflection due to parapet railing. Any railing pours made before the entire slab has been placed and cured must be approved

STRUCTURAL STEEL:

All structural steel shall be ASTM A709, Gr. 36 or 50 unless noted otherwise.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on approved shop drawings. Shapes and materials shown in the plans will be the basis of payment, and no additional compensation will be made for any adjustments due to substitutions

Drawings show general features of design only. Shop drawings shall be prepared in accordance with the specifications, submitted and approved before fabrication is begun.

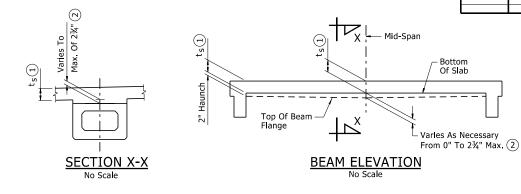
All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether temporary or permanent, a formal request with detailed drawings shall be submitted to the Engineer for approval; however, additional welds used for attaching falsework support devices or screed rail supports to the structural steel that do not exceed the limitations of Subsection 802.13 will not require approval prior to construction. All welding shall conform to Subsection 807.26.

SPECIAL CAMBER NOTES

The camber and dead load deflection values shown on the plans are estimated based on the required minimum concrete strength for the prestressed concrete beams. The contractor shall provide the Engineer with the following information

- A. Actual 28-Day concrete strength of prestressed concrete beams
- B. Estimated age of prestressed concrete beams at time of erection which shall not be less than 90 days from release.
- C. Profile of each beam under its own weight in final position.

Following receipt of the above data, the Engineer will evaluate the dead load and, if necessary, will provide an updated deflection diagram to the contractor.



ADJUSTMENT FOR SLAB THICKNESS TOLERANCE

 t_S = slab thickness as shown on superstructure details

See "TYPICAL ROADWAY SECTION" on Dwg. No. 63794.

JOB NO

07514

FED. AID PROJ. NO

101013

159'-0" UNIT

49

70

63802

1 Tolerance when removable deck forming is used is $+\frac{1}{2}$ ", $-\frac{1}{4}$ ". Haunch forming is required and shall be adjusted to maintain slab thickness tolerance. See Std. Dwg. No. 55005 for tolerances when permanent steel deck forms are used.

The "BEAM ELEVATION" sketch shows the range of acceptability of the top of beam relative to bottom of slab after the placement of the slab. When the top corner of the beam projects more than 3/8" into the slab, a raise in grade will be necessary. Beams shall be set in a sufficient number of spans over suitable increments so the revised grade line will produce a smooth riding surface. Variation of haunch height will be at the Contractor's expense.

(2) Haunch dimensions are measured at CL Beam.

SPAN PT.	INCHES				
SPAN PT.	W _A	X _A			
0.00	0.000	0.000			
0.10	0.292	0.112			
0.20	0.508	0.219			
0.30	0.652	0.302			
0.40	0.734	0.355			
0.50	0.761	0.372			

Table symmetric about mid-span

strengths may require adjustments.

See "SPECIAL CAMBER NOTES"

(3) Note: Camber and Deflection Values shown are based on a concrete beam strength, f'c = 8000 psi. Greater

				×ď	1			sition o re relea	f beam ased	after
			/		≱			sition o ead loa	of beam id	I
_			1	-		1-				-
0.00	0.10	0.20	0.30	0.40	0.50	0.60	0.70	0.80	0.90	1.00
ing	Д				1					in g
Bearing			-			— Mid	I-Span			Bearing
CL B					:					ال ا
9		"W	" is Ca	amber (of beam	ı (Pres	tress +			١

Dead Load of beam @ 90 Days After Release)

"X_A" Is Dead Load Deflection of Slab + Diaphragms + Composite Dead Load

3 CAMBER & DEFLECTIONS (INCHES) - 49'-0" BEAM

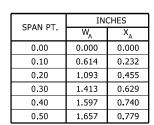


Table symmetric about mid-span

(4) Note: Camber and Deflection Values shown are based on a concrete beam strength, f'c = 8000 psi. Greater strengths may require adjustments See "SPECIAL CAMBER NOTES".

Initial position of beam strands are released Final position of beam under dead load	
0.00 0.10 0.20 0.30 0.40 0.50 0.60 0.70 0.80 0.90	1.00
A Mid-Span	L Bearing
기 "W _A " is Camber of beam (Prestress + Dead Load of beam @ 90 Days After Release)	리

"X_A" Is Dead Load Deflection of Slab + Diaphragms + Composite Dead Load

(4) CAMBER & DEFLECTIONS (INCHES) - 59'-0" BEAM No Scale



SHEET 9 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

CWT DATE: MAY 2020 FILENAME: b101013_s9.dgn DRAWN BY: ___ CHECKED BY: JHR DATE: JULY 2020 SCALE: As Shown DESIGNED BY: JME DATE: MAY 2020

BRIDGE NO. 07514 DRAWING NO. 63802

	BAR LIST								
	MARK	NO. REQ'D	LENGTH	P.D.					
	D501E	8	42'-2"	Str.					
	D502E	32	4'-5"	Str.					
END BENT DIAPHRAGM	D503E	12	5'-10"	2½"					
[유장]									
	D601E	100	7'-8"	4½"					
	D701E	8	42'-2"	Str.					
ш									
	D504E	76	9'-7"	2½"					
▎▁ऻ	D505E	64	4'-5"	Str.					
INT. BENT DIAPHRAGM	D506E	16	6'-2"	2½"					
l¤šŀ	D507E	12	3'-6"	2½"					
Ĭ₽₽₽	D508E	228	4'-6"	2½"					
				2					
F	D602E	12	42'-2"	Str.					
ш				0					
	P401E	576	5'-11"	2½"					
	P402E	64	5'-2"	2½"					
l _⊢ ⊦	P403E	576	3'-5"	3"					
PARAPET	P404E	72	5'-7"	Str.					
¼	P405E	64	9'-8"	Str.					
╽╏┞	P406E	64	21'-8"	Str.					
l ⊦	P407E	64	7'-8"	Str.					
l ⊦	P407E	04	/ - 0	5u.					
┌┌	S401E	640	0! ["	3"					
l ⊦		250	8'-5" 34'-0"						
l ⊦	S402E	250	34 -0	Str.					
l ⊦	CEO1E	620	42'-2"	C+r					
╽ᇕ├	S501E	628 236	42 -2	Str.					
SLAB	S502E S503E		4'-0"	Str.					
l ⊦	5503E	80	4 -0	Str.					
l ⊦	CC01E	00	201.011	Chu					
l ⊦	S601E	98	20'-0" 13'-1"	Str.					
l ⊦	S602E	98	13'-1"	4½"					
┌	D401E	60	FI 4411	23/11					
l ⊦	R401E	60	5'-11"	3¾"					
l ⊦	R402E	16	5'-10" 5'-2"	3¾" 2"					
l ⊦	R403E	4							
l ⊦	R404E	8	9'-4"	Str.					
l ⊦	R405E	24	9'-8"	Str.					
-	R406E	16	4'-0"	Str.					
l LLS	R407E	8	3'-11"	6"					
≰ -	R408E	32	5'-8"	Str.					
WINGWA	C4025	1	71.6"	2.11					
>	S403E	12	7'-6"	3"					
-	W401E	122	21.0"	Ct					
	W401E	120	2'-9"	Str.					
-	W402E	80	3'-11"	3¾"					
	WEG: 5		71.0"	224					
	W501E	32	7'-3"	3¾"					
	14406 : =		491.5"						
ш	W801E	40	12'-8"	Str.					

D506E D601E D508E 9¾" <u>D507E</u> <u>P401E</u> 2½" P.D. 7'-11" S401E 3¾" P.D. -10'-0" 1'-3" <u>S403E</u> R401E R407E R403E 4¾" W501E R402E <u>W402E</u>

BAR BENDING DIAGRAMS

NOTES: Dimensions of bars are out-to-out. Bar designations ending with "E" indicate epoxy coated bars.



SHEET 10 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

101013

159'-0" UNIT

07514

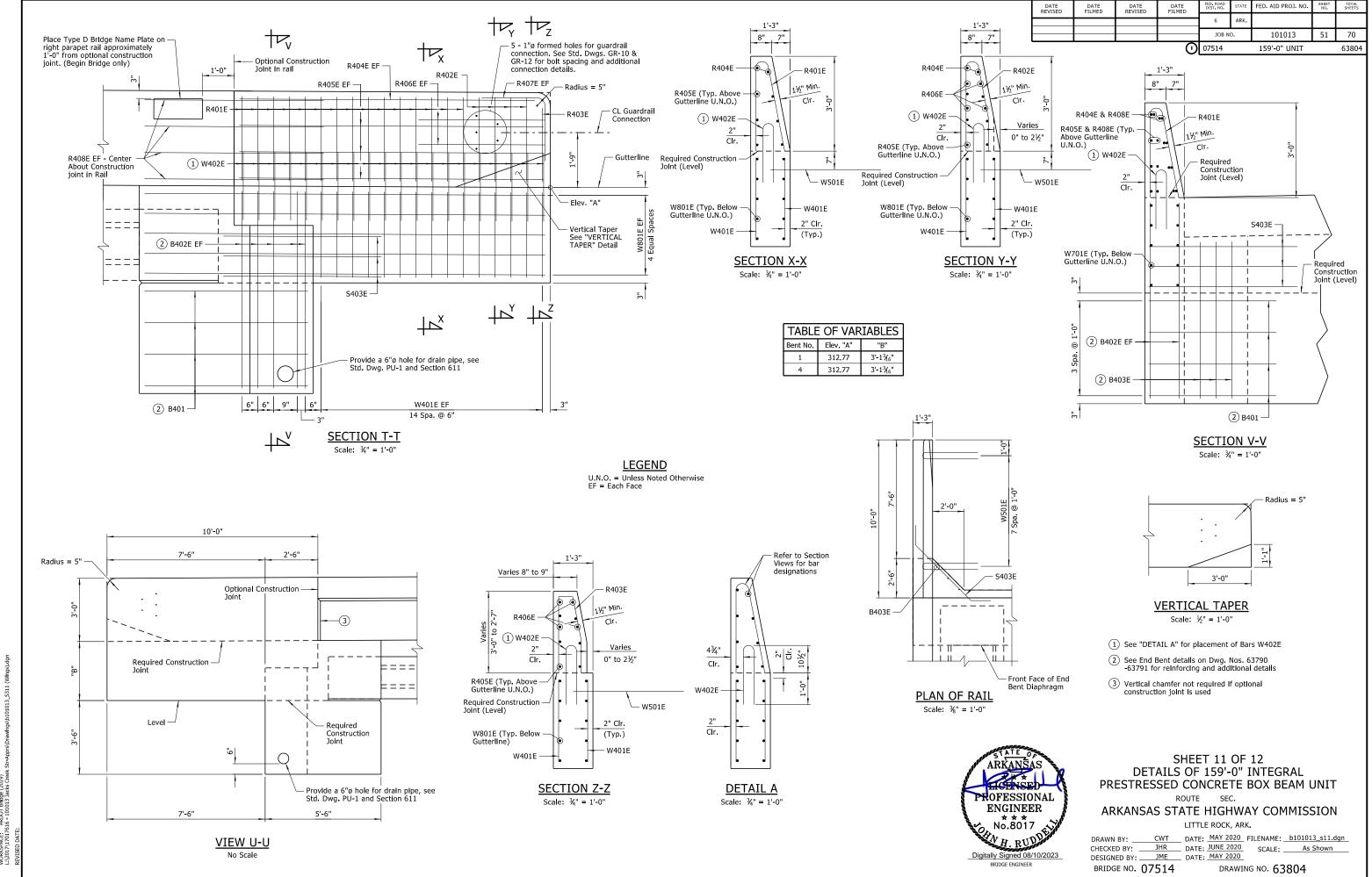
50

70

63803

LITTLE ROCK, ARK.

DRAWN BY:	CWT	DATE: MAY 2020 FILENAME: b101013_s10.dgn
CHECKED BY: _	JHR	DATE: JULY 2020 SCALE: As Shown
DESIGNED BY: _	JME	DATE: MAY 2020
BRIDGE NO.	07514	DRAWING NO. 63803



abhall 8/10/2023 1 WORKSPACE: ARDOT Bridge (2019) L:\2017\17017616 - 101013 Jacks Greek §

159'-0" UNIT 63805

(P) (F)"A" "A" Closed Parapet Open Parapet P401E & P403E P402E P401E & P403E P402E P401E & P403E, P401E & P403E "B" Spa. @ 6" Max. "D" Spa. "D" Spa. 3 Spa 11 Spa. @ 6" 3 Spa @ 6" @ 6" Max. @ 6" Max. @ 6" P4"T" EF T/A P4"T" EF ヤ₢ **1** ✓ A Required Construction -Joint at Top of Slab 2'-0" 6'-0' 2'-0" Drain Drain P404E EF - Lapped WIth P4"T" Bars As Shown

1'-2½" 1" Chamfer Or Radius — 1½" P401E 1½" Clr. P403E (Typ.) 4¾" Clr. Required Construction Joint (Match Roadway Slope)

SECTION A-A Scale: 1" = 1'-0"

1'-2½" 1" Chamfer Or Radius — 7½" 1½" P402E 1½" Clr. (Typ.) Smooth Surface With Trowel SECTION B-B Scale: 1" = 1'-0" 1" Chamfer (Typ.-Around Drain OpenIng)

1 Measured at Edge of Deck

8'-0"

DETAILS OF PARAPET RAIL

Scale: ½" = 1'-0"

TABLE OF PARAPET VARIABLES PANEL LENGTH CLOSED PARAPET OPEN PARAPET "D" 10'-0' 05E 19 06E 6'-0" 22'-0' 11 07E

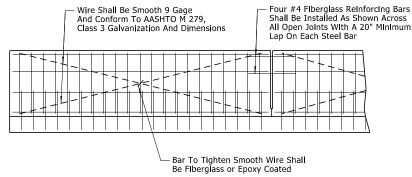
15

(Typ. At All Partial-Depth

LEGEND EF = Each Face

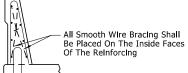
- F CL Full-Depth Parapet Joint (½"-1" max.) Stop 6" from Top of Slab.
- P CL Partial-Depth Parapet Joint (¼"-1" max.) Stop 16" from Top of Slab.

For locations of open and closed parapet panels and full-depth and partial-depth parapet joints, see "HALF REINFORCING PLAN & DECK POURING SEQUENCE" on Dwg. No. 63797.



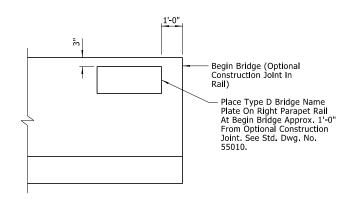
All panels shall be braced as required to prevent racking. All parapet joints shall be sawed as soon as practical to a minimum width of ¼". To control cracking before sawing, all joints must be grooved before the concrete is set. Sawing of the joints must be control led so it will follow the grooved joint.

For actual placement of reinforcing steel, see "DETAILS OF PARAPET RAIL".



The extruded parapet shall conform to the horizontal and vertical lines shown on the plans or as directed by the Engineer and shall present a smooth, uniform appearance and texture. Unless otherwise noted, exposed surfaces may be given a light brush finish or a Class 3, Textured Coating Finish, in place of the Class 2, Rubbed Finish.

DETAILS OF OPTIONAL SLIPFORMING OF CONCRETE PARAPET RAIL No Scale



VIEW SHOWING LOCATION OF NAME PLATE

(Showing Inside Face Of Parapet) No Scale



SHEET 12 OF 12 DETAILS OF 159'-0" INTEGRAL PRESTRESSED CONCRETE BOX BEAM UNIT

ROUTE SEC.

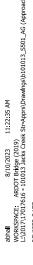
ARKANSAS STATE HIGHWAY COMMISSION

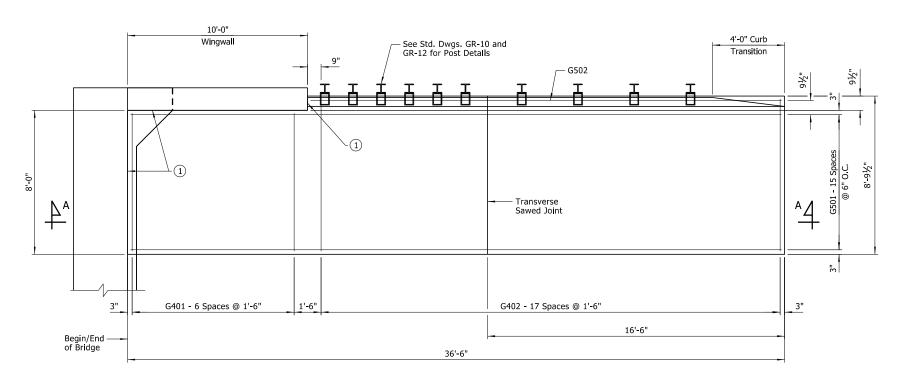
LITTLE ROCK, ARK.

JME DATE: MAY 2020 FILENAME: b101013_s12.dgn CHECKED BY: JHR DATE: JUNE 2020 SCALE: As Shown
DESIGNED BY: JME DATE: MAY 2020

DRAWING NO. 63805 BRIDGE NO. 07514

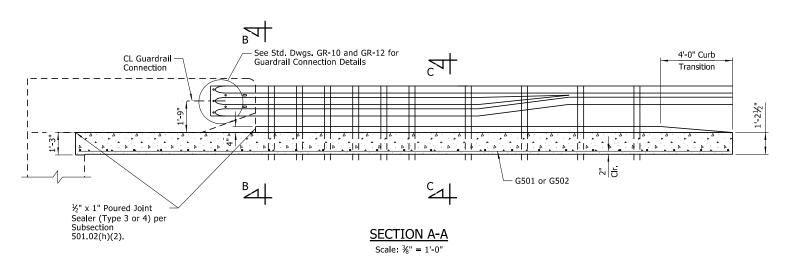
; AM < Str-Apprs\Drawings\b abhaii 8/10/2023 II;22;35 WORKSPACE: ARDOT Bridge (2019) L;\2017\17017616 - 101013 Jacks Creek





HALF PLAN OF TYPE SPECIAL APPROACH GUTTERS

Scale: 3/8" = 1'-0"



① Eliminate Type 1 Preformed Joint at the concrete diaphragm and at the face of wingwalls. ½" x 1" Poured Joint Sealer is required, however, backer rod shall be eliminated.

QUANTITIES FOR ONE TYPE SPECIAL APPROACH GUTTER

(FOR INFORMATION ONLY)

Reinforcing	Concrete
Steel (Lbs.)	(Cu. Yds.)
768	14.20

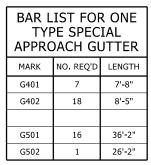
GENERAL NOTES

All concrete shall be Class S or Class S(AE) or mixture used for Portland Cement Concrete Pavement and shall be poured in the dry.

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322, Type A, with mill test reports.

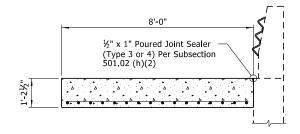
Approch Gutters will be measured and paid for in accordance with Section 504.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		101010		7.0
						101013	53	70
<u> </u>			07514		ADDD CUTTED		62906	



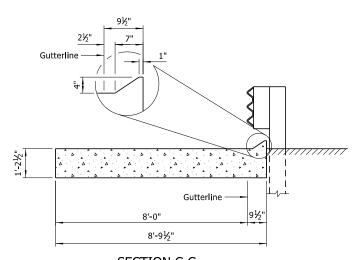
NOTE:

Quantities Shown are for One Type Special Approach Gutter. Four Type Special Approach Gutters are Required.



SECTION B-B

Scale: ½" = 1'-0"



Section c-c

Scale: $\frac{1}{2}$ " = 1'-0"



BRIDGE ENGINEER

DETAILS OF TYPE SPECIAL APPROACH GUTTERS

ROUTE SEC.

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

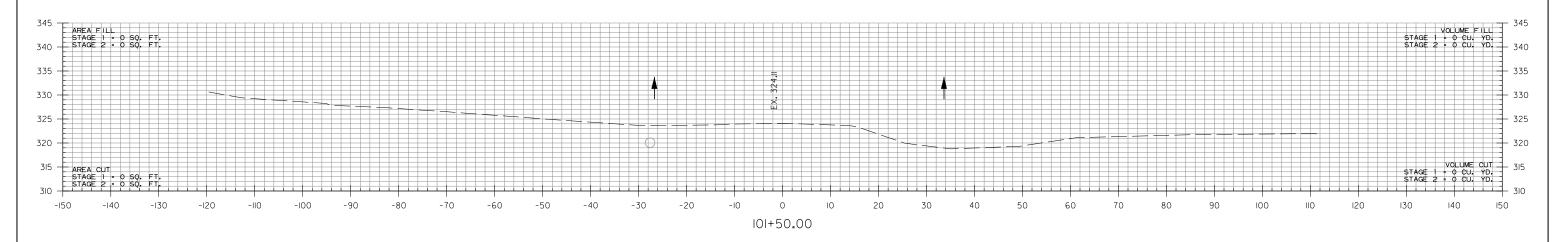
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 CWT
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 MAY 2020
 FILENAME:
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 CHECKED BY:
 JHR
 DATE:
 JUNE 2020
 SCALE:
 As Shown

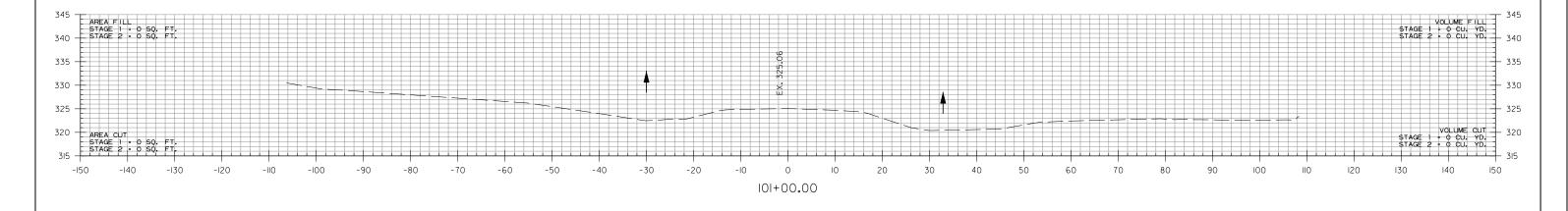
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 JME
 DATE:
 MAY 2020
 SCALE:
 As Shown

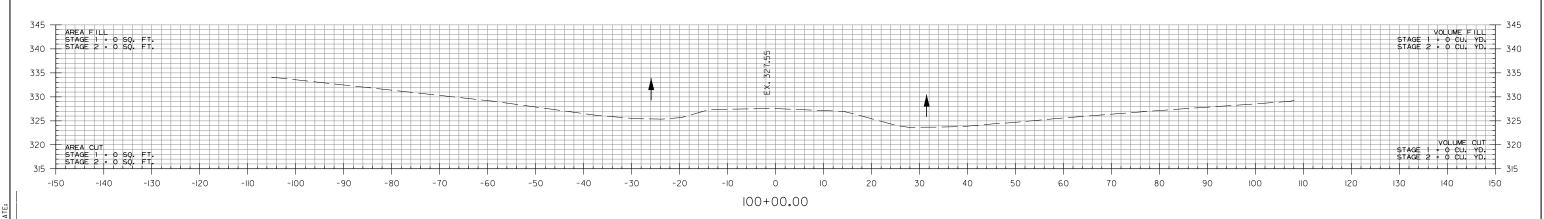
BRIDGE NO. 07514 DRAWING NO. 63806

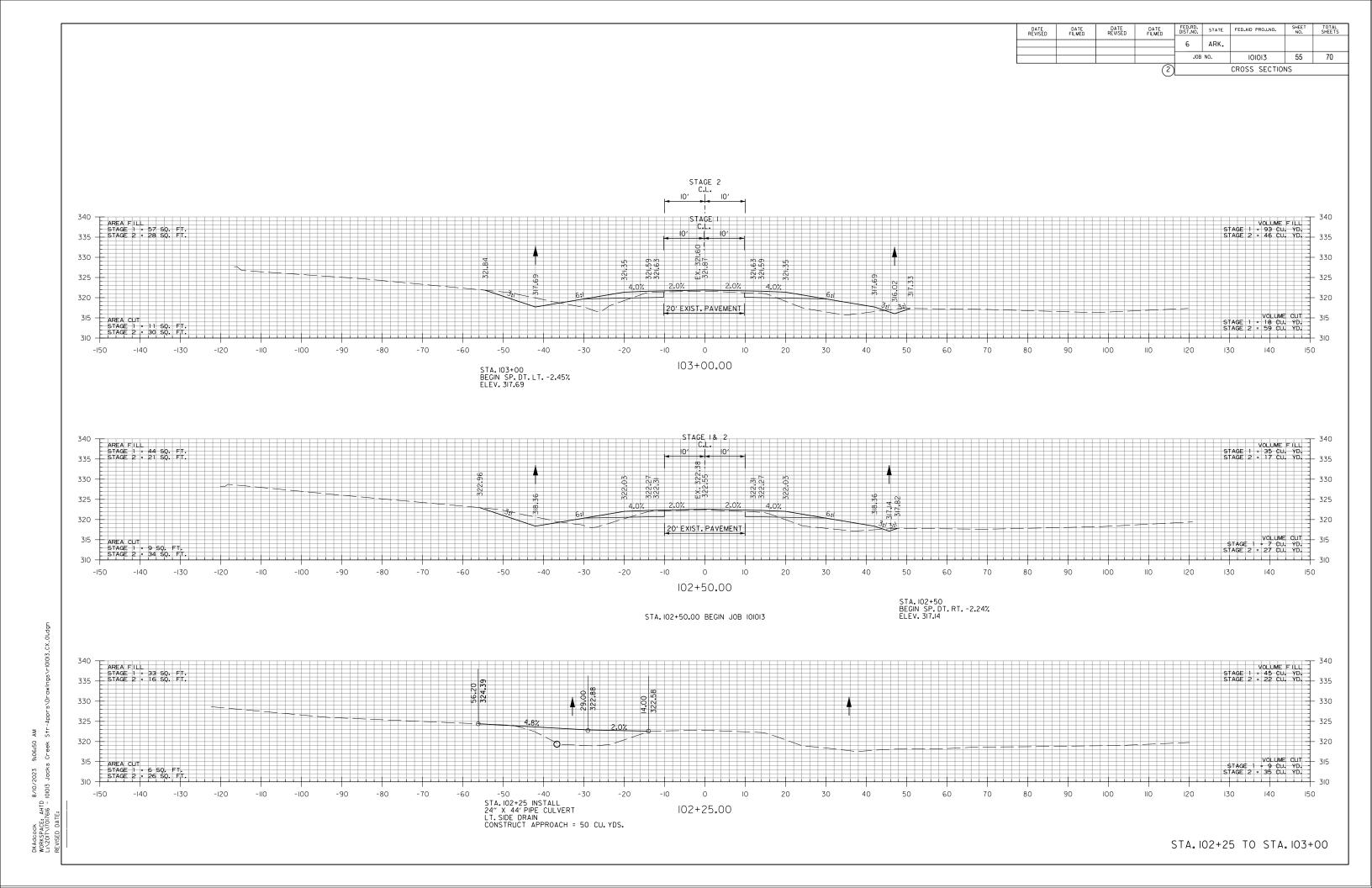
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED.RD. DIST.NO.	STATE	FED.AID PROJ.NO.	SHEET NO.	TOTAL SHEETS	
				6	ARK.				
				JOB NO.		101013	54	70	
2					CROSS SECTIONS				

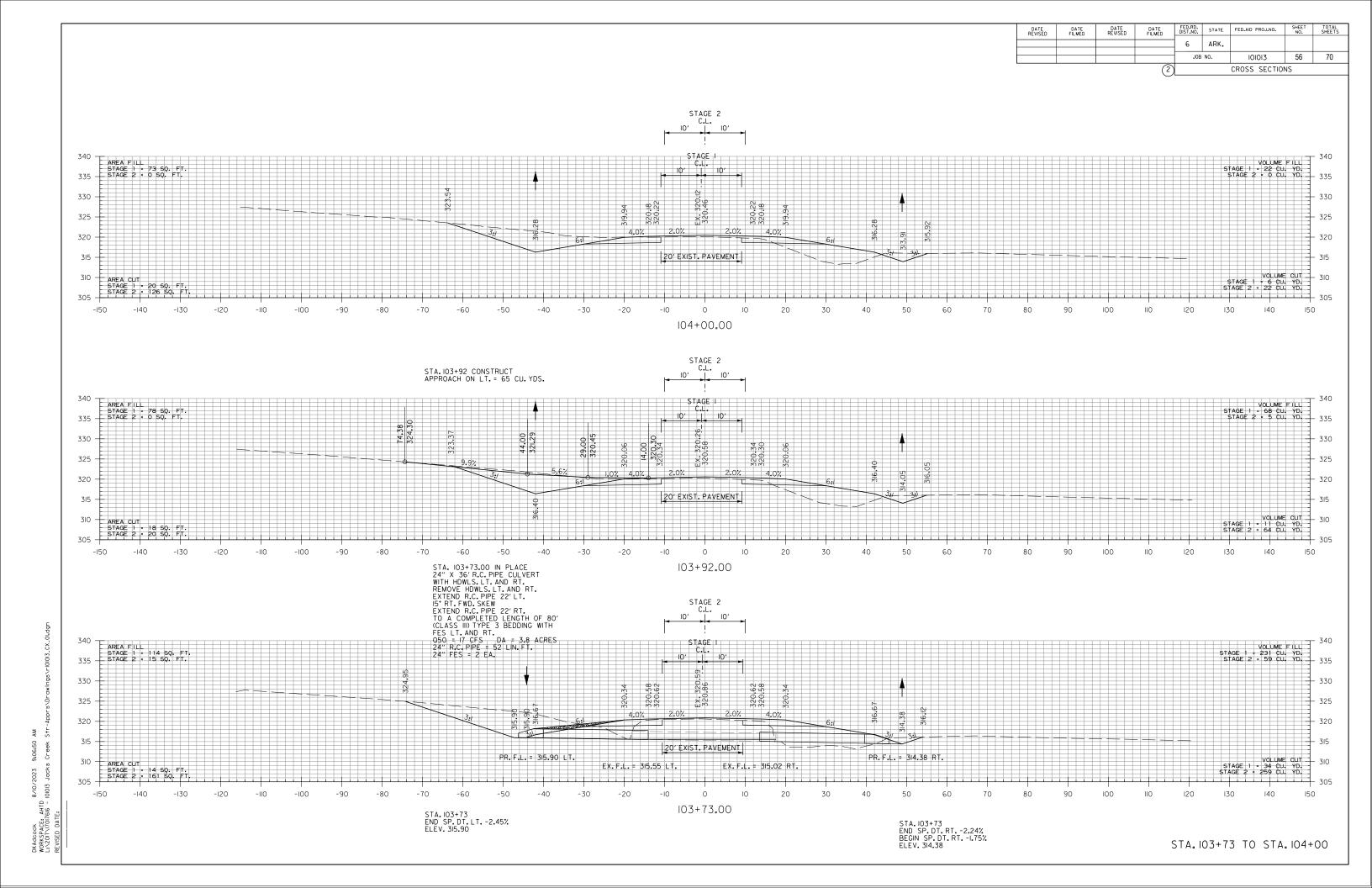


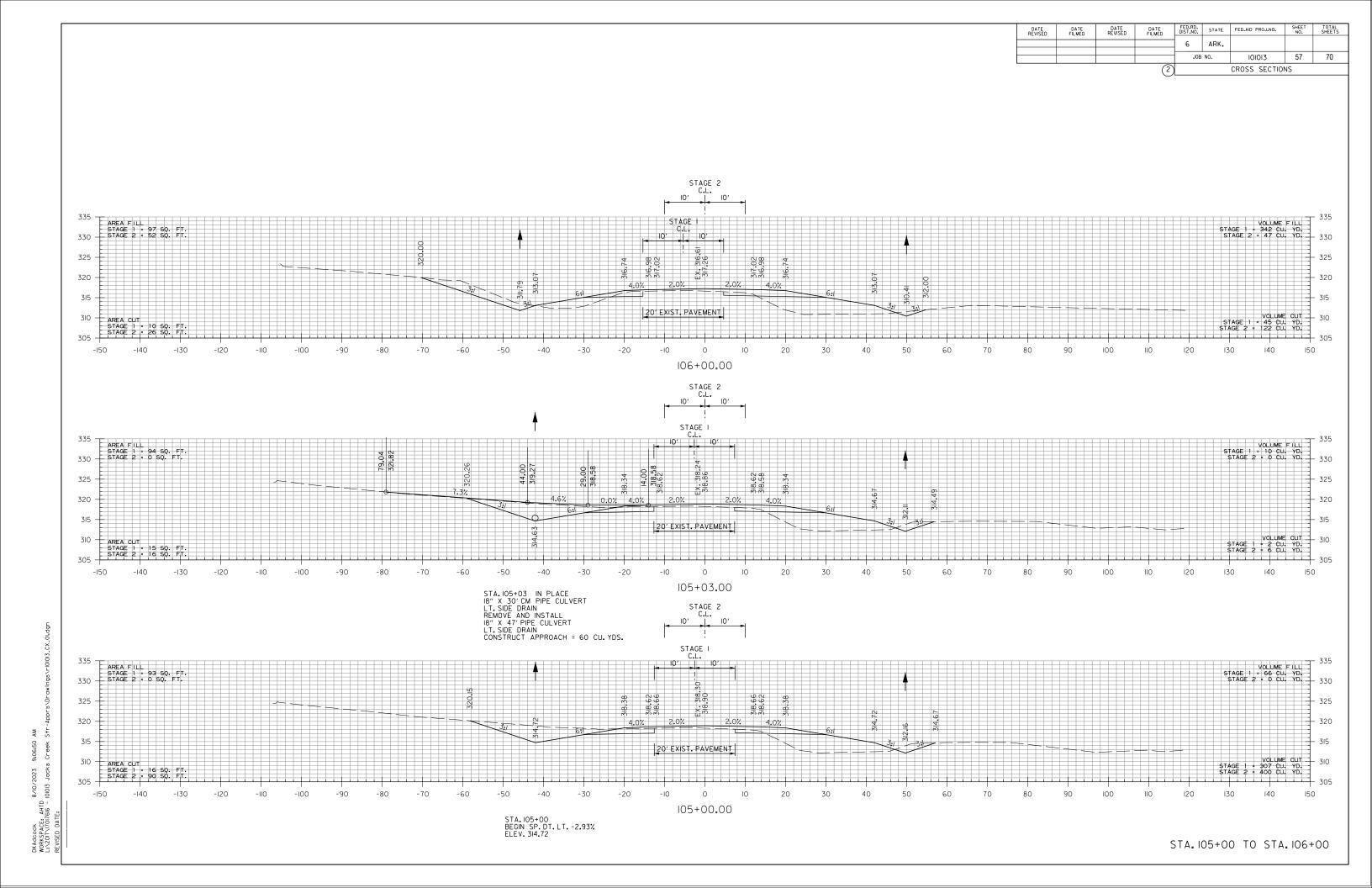
STA. IOI+50.00 BEGIN IOO' TRANSITION

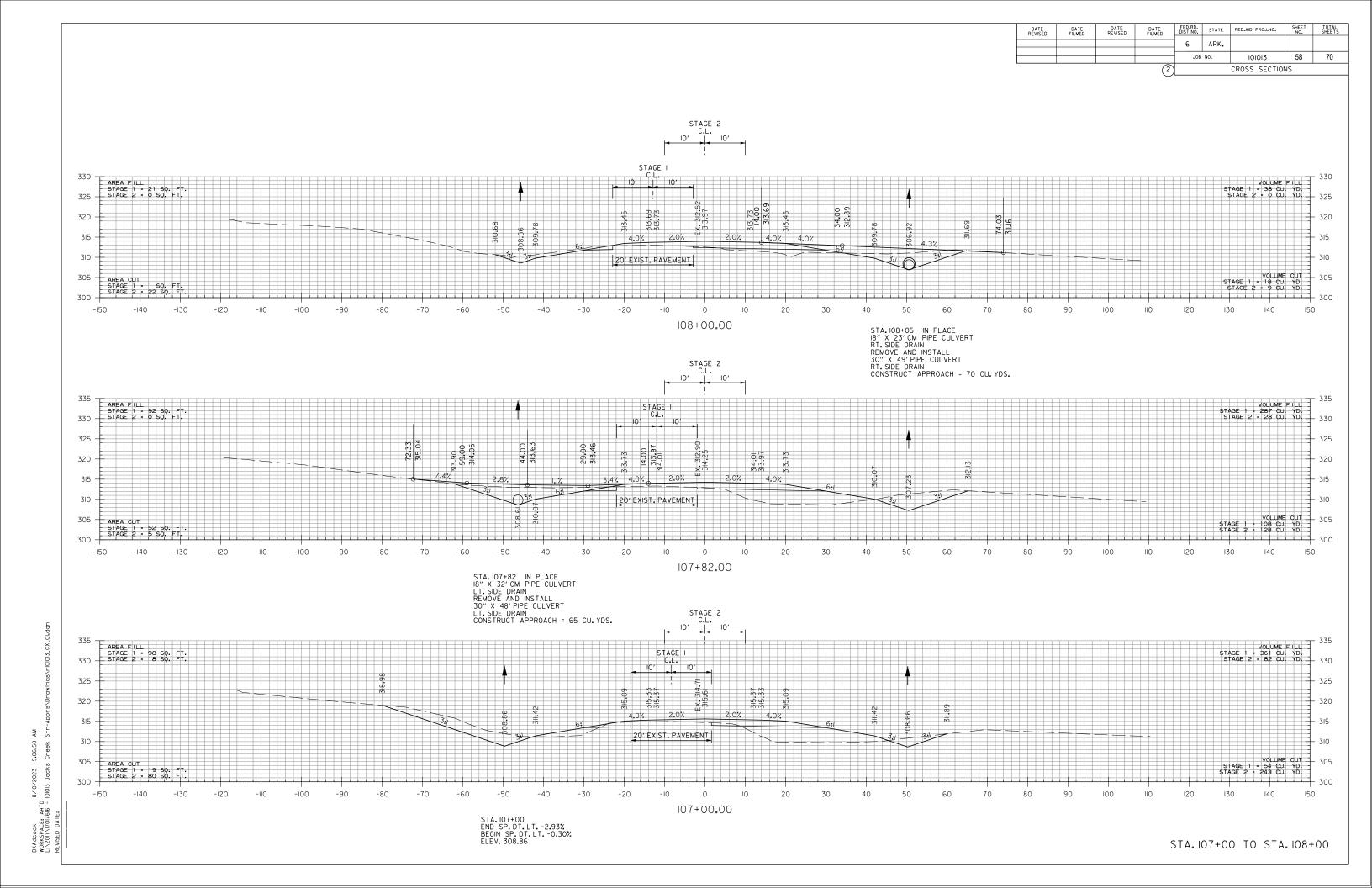


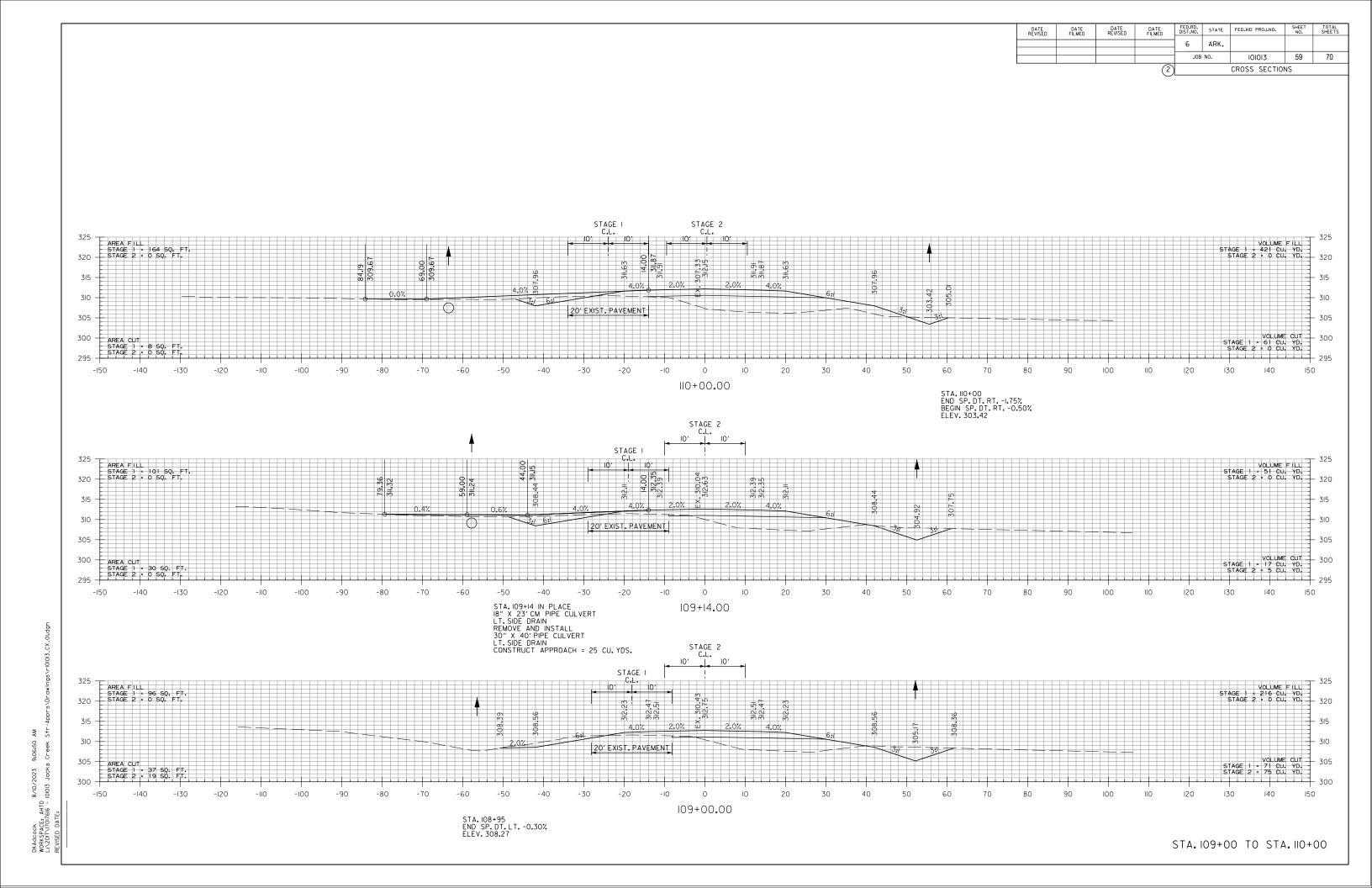


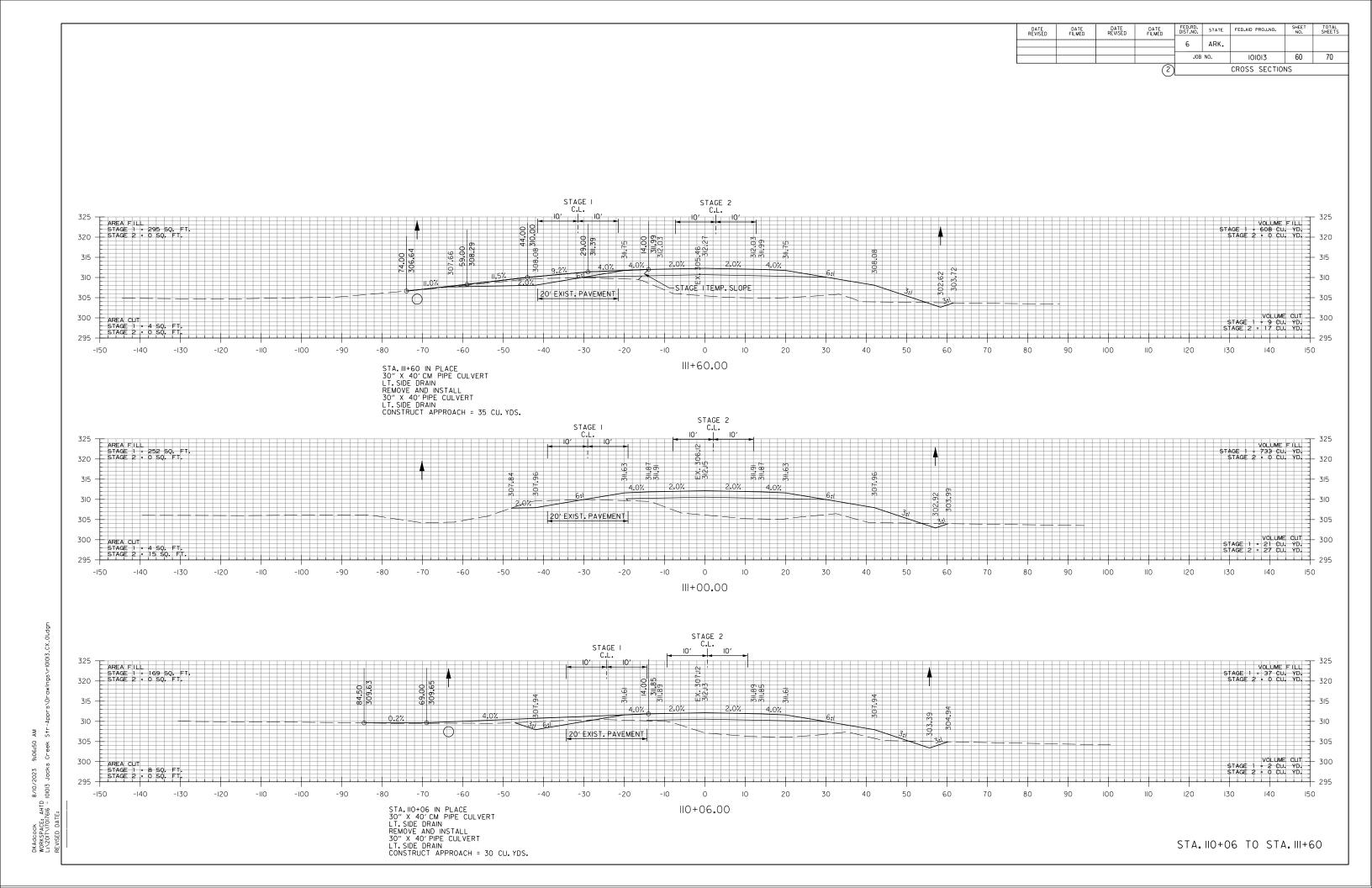












STA. II7+66.93 BEGIN TOE OF SLOPE

AT ELEVATION 304.00'

STACE 1 = 0 SQ. FT. STAGE 1 = 0 SQ. FT. STAGE 2 =

STA. 117+59 BEGIN SP. DT. LT. 0.63% ELEV. 296.60

315 310

305

300

295

AREA CUT STAGE 1 = 166 SQ, FT, STAGE 2 = 462 SQ, FT.

STA. 117+32.75 BEGIN BRIDGE EXCAVATION AT ELEVATION 304.00'

117+66.93

STA. 117+20 BEGIN SP. DT. RT. 0.40% ELEV. 299.59

ADDITIONAL VOLUME FILL BRIDGE EXCAVATION AND SPECIAL DITCHES STAGE 1 = 15 CU. YD. STAGE 2 = 0 CU. YD. 310

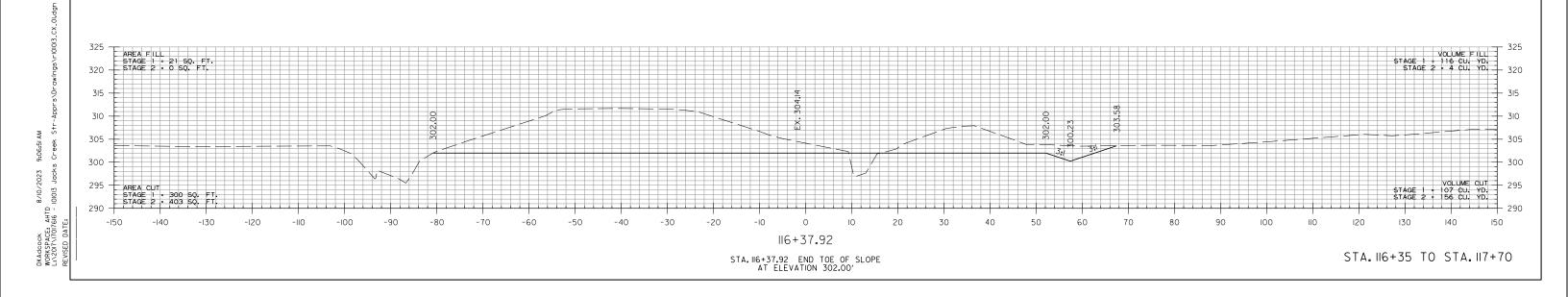
305

300

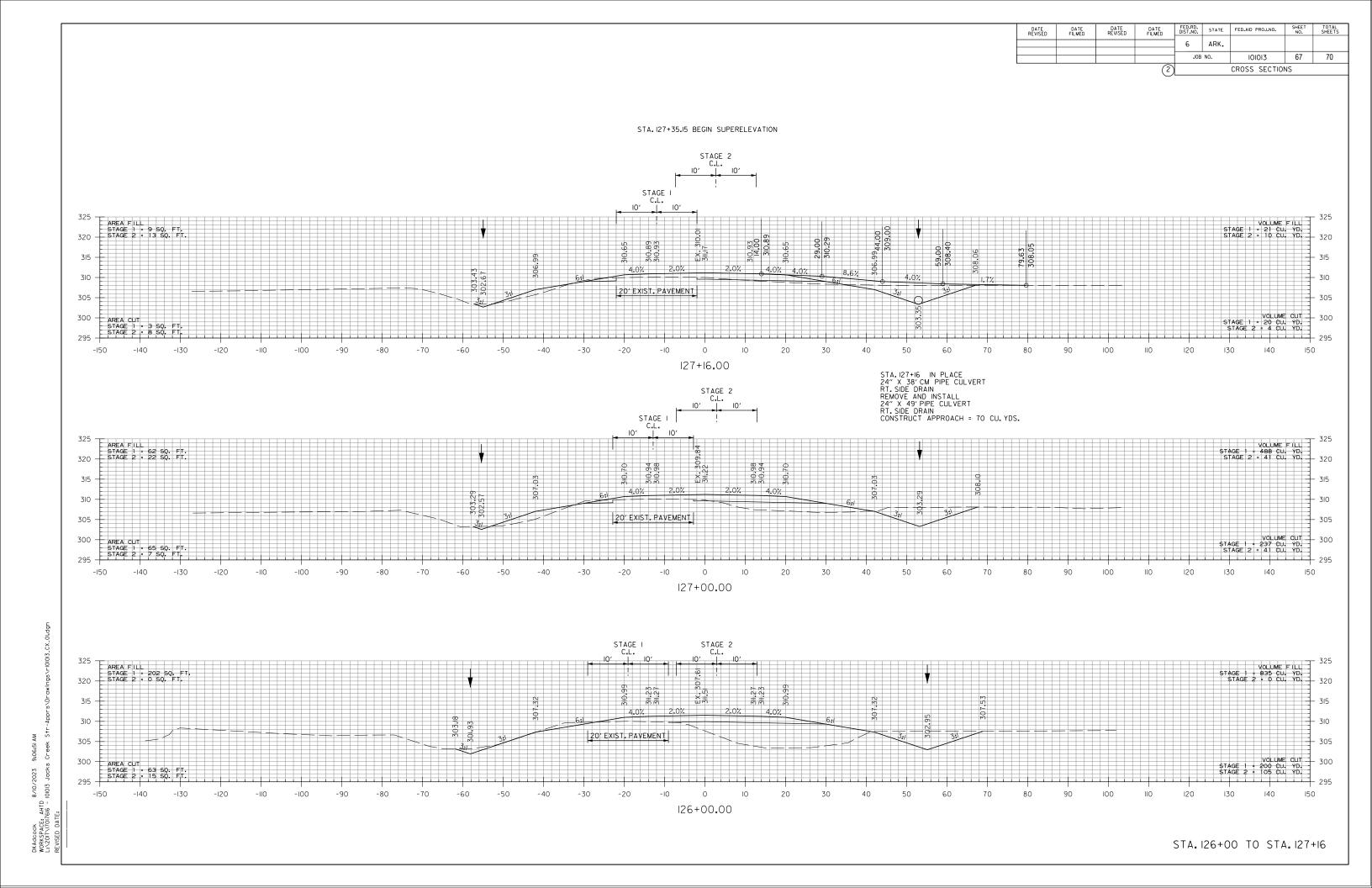
ADDITIONAL VOLUME CUT BRIDGE EXCAVATION AND SPECIAL DITCHES STAGE 1 = 256 CU, YD. STAGE 2 = 281 CU. YD.

STA. 116+83 END SP. DT. RT. -0.50% ELEV. 300.00

STA. II6+75.62 END BRIDGE EXCAVATION AT ELEVATION 302.00'

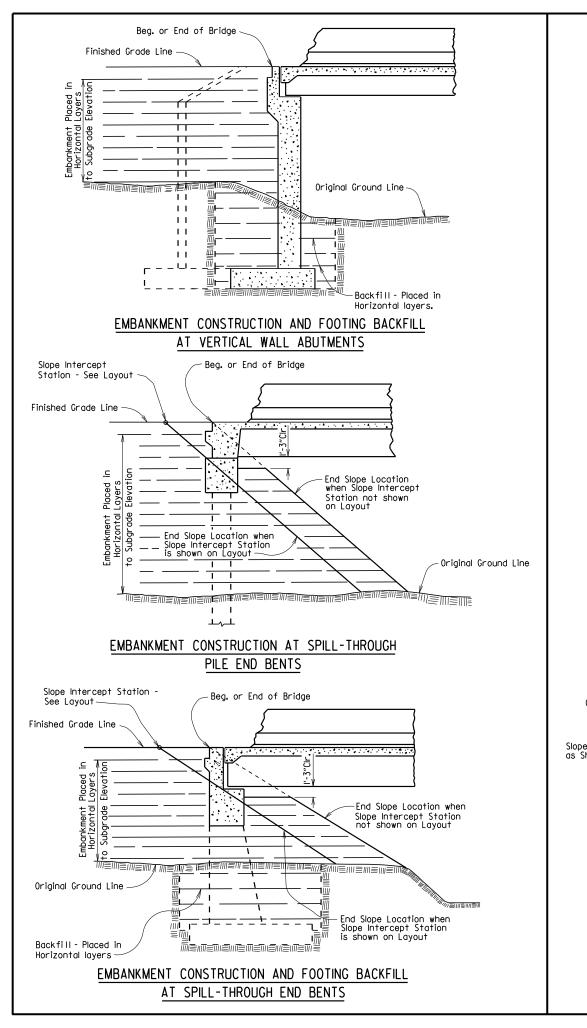


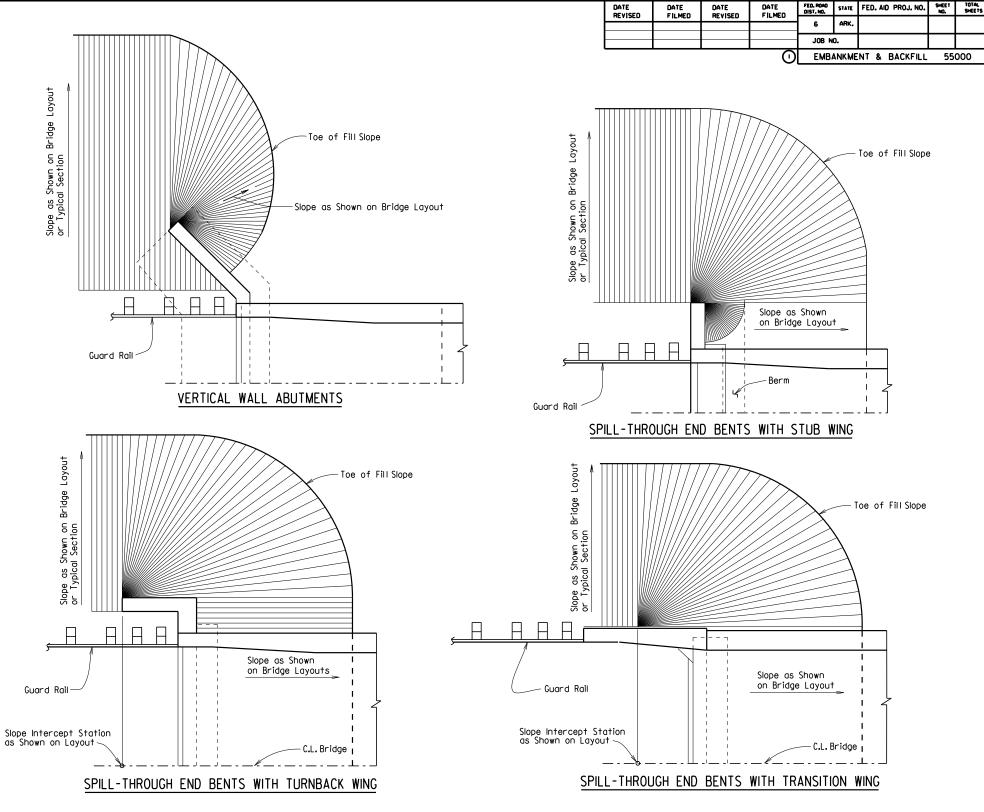
DATE REVISED STATE FED.AID PROJ.NO.



DATE REVISED STATE FED.AID PROJ.NO.

ARK.





METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 6 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to Subsections 210.09, 210.10 and 801.08 for construction requirements.

STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

ARKANSAS STATE HIGHWAY COMMISSION

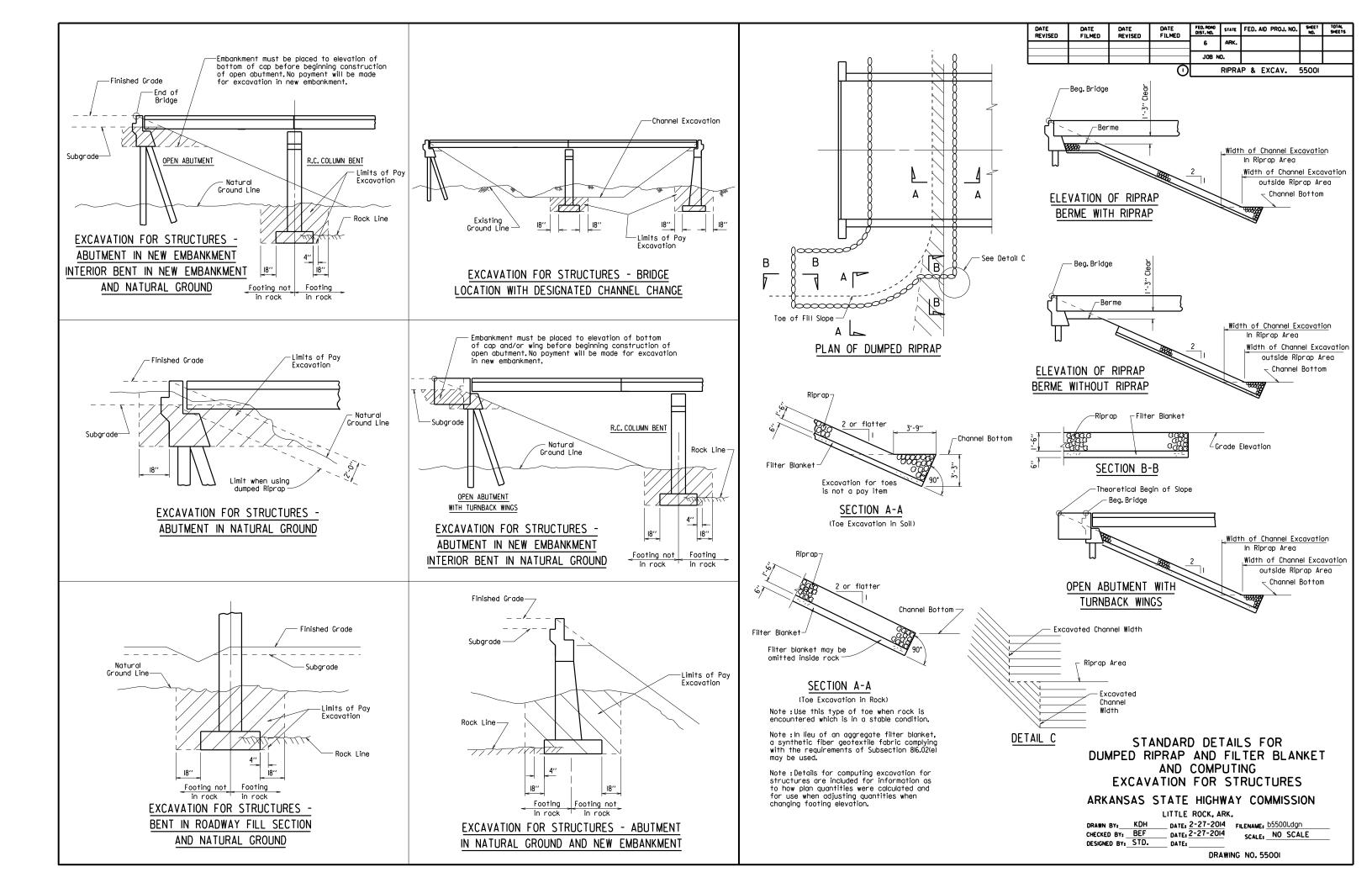
LITTLE ROCK, ARK.

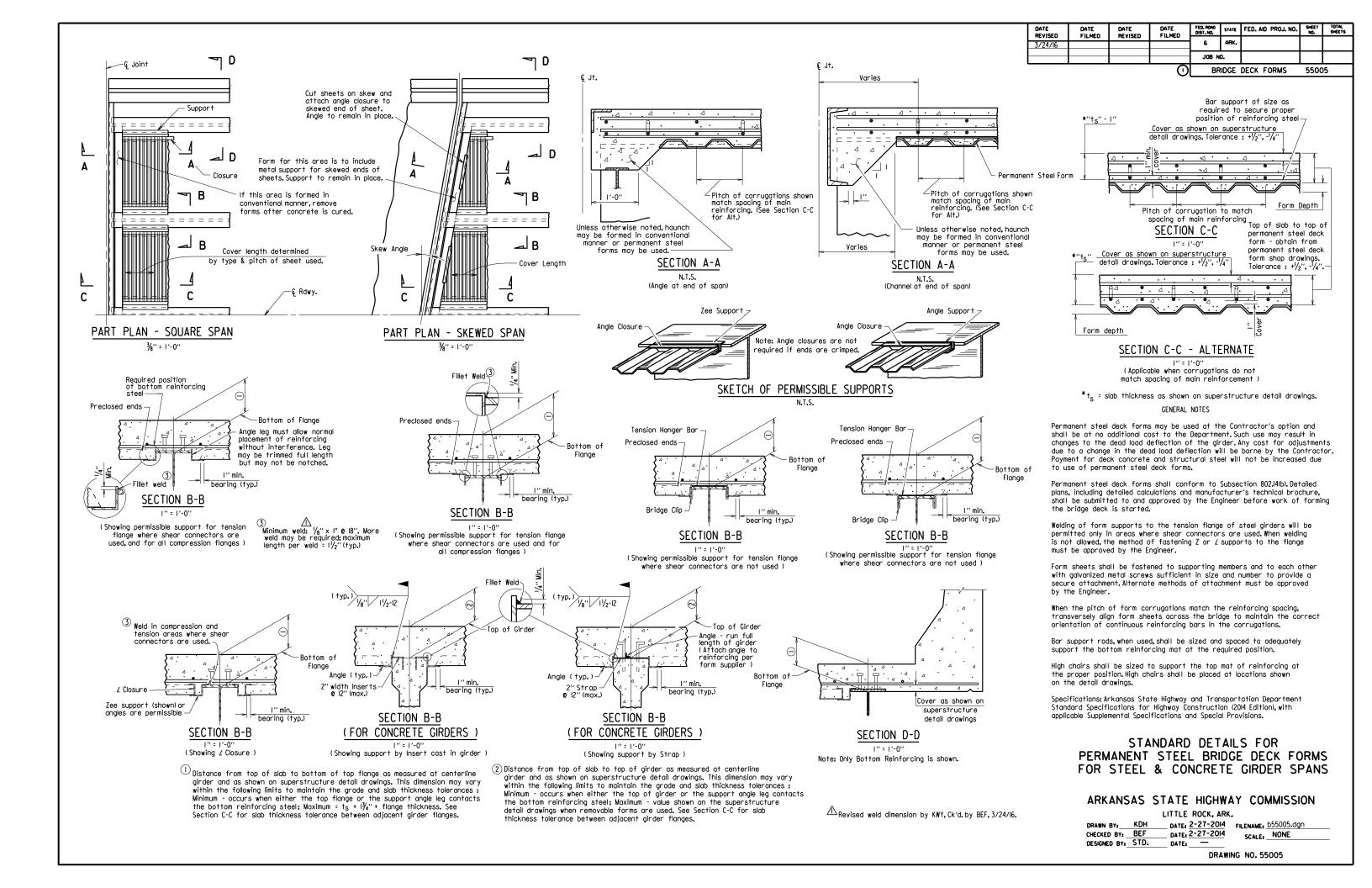
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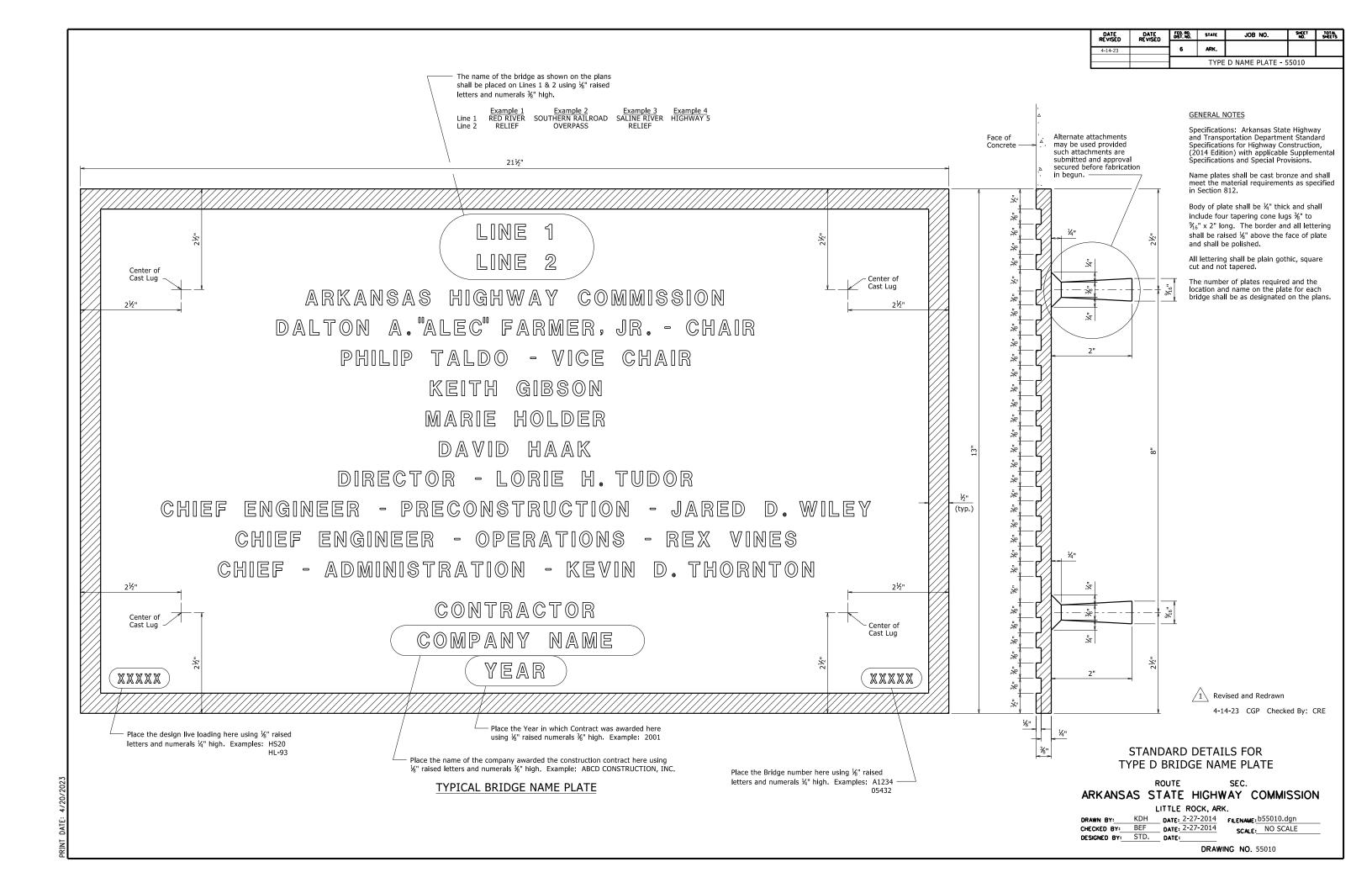
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 2-27-2014
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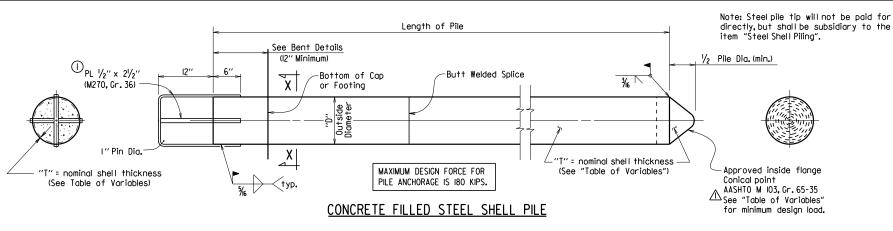
 CHECKED BY:
 BEF
 DATE:
 2-27-2014
 SCALE:
 NO SCALE

 DESIGNED BY:
 STD.
 DATE:
 NO SCALE



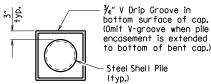






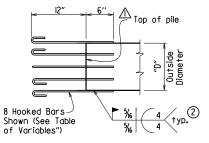
Pile anchorage shall be placed to minimize interference with anchor bolts and reinforcing in cap or footing.

Welding shall comply with ANSI/AWS DI.4 Structural Welding Code-Reinforcing Steel and applicable portions of ANSI/AWS DL5 Bridge Welding Code.



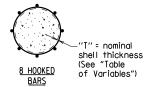
VIEW X-X

The Contractor may use No.7 hooked reinforcing bars equally spaced around piles. Reinforcing bars shall be ASTM A706, Grade 60. See "Table of Variables" for number required.



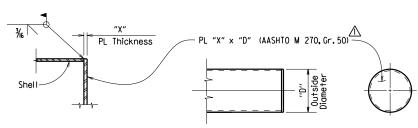






ALTERNATE PILE ANCHORAGE DETAIL

Note: Hooked bars shall be oriented to provide the required concrete clearances shown in the plans.



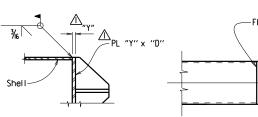
PART SECTION

ELEVATION

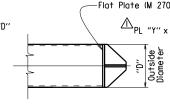
ALTERNATE FLAT TIP DETAIL

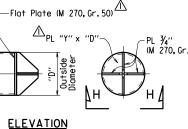
Note: The alternate flat tip detail shall not be used on steel shell piling to be driven through embankments constructed with internal geosynthetic reinforcement.

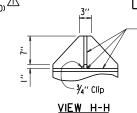
ALTERNATE VANED TIP DETAIL



PART SECTION









GENERAL NOTES FOR CONCRETE FILLED

Steel shells shall conform ASTM A252, Grade 3 (Fy = 45,000 psi.)

Concrete used for filling of steel shall be Class S with

a minimum 28-day compressive strength, f'c = 3,500 psi. and

Steel shell piling that extends above the ground and is not

protected by pile encasement shall be painted in accordance

See Bridge Layout for size and estimated length of steel shell

Concrete, structural steel, reinforcing steel (including welding), and painting shall not be paid for directly, but shall be

considered subsidiary to the item "Steel Shell Piling".

TYPICAL SPLICE DETAILS

Min. I" x .250" Split

Backing Ring

STEEL SHEEL PILES:

shall be poured in the dry.

piles and for driving information.

with Subsection 805.02.

B-U4a

OUTSIDE DIAMETER "D"	NOMINAL SHELL THICKNESS "T"	PLATE THICKNESS "X"	PLATE THICKNESS "Y"	NO.OF HOOKED BARS FOR ALTERNATE PILE ANCHORAGE	MINIMUM CONICAL TIP DESIGN LOAD (KIPS)
14"	0.50"	21/4"	11/2"	5	859
16"	0.50"	21/4"	11/2"	5	986
18"	0.50"	21/2"	11/2"	6	I , I 14
20"	0 . 50''	21/2"	13/4"	6	1,241
24"	0.50"	2¾"	13/4"	8	I , 495

1'-6" Hooked Bar

HOOKED BAR DETAIL

Revised and added various details by KWY, Ck'd. by BEF, 3/24/16.

DATE REVISED	OATE FILMED	DATE REVISED	DATE FILMED	FEO. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS				
UE A 19ED	FILMED	WEALDED	FILMED									
3/24/16				J 6	ARK,							
				-	_							
				JOB NO.		IOR NO.		I IOR NO.				

55021

STEEL SHELL PILES

GENERAL NOTES FOR PILE ENCASEMENTS:

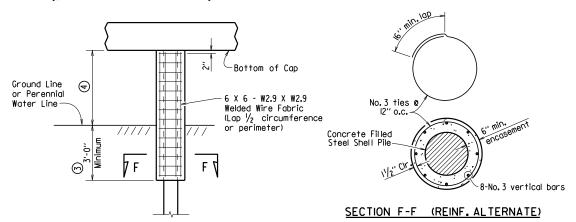
 11 See Bridge Layout for additional notes,any pile encasement restrictions and required location of pile encasements.

Concrete shall be Class S with a minimum 28-day compressive strength, f'c = 3,500 psi. If concrete cannot be placed in the dry, Seal Concrete may be used from top to bottom of encasement.

Reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322. Type A.

Welded wire fabric shall conform to AASHTO M 55 or M 221.

Concrete, welded wire fabric or reinforcing steel, and galvanized pipe shall not be paid for directly, but shall be considered subsidiary to the item "Pile Encasement".



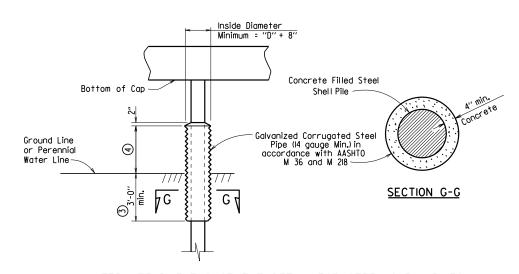
PILE ENCASEMENT DETAIL FOR STEEL SHELL PILES

(Shown with Encasement to Bottom of Cap)

Unless otherwise noted on Bridge Layout.

See Bridge Layout for height of pile encasement (3'-0" Minimum).

(5)
Pile encasement, when not extended to bottom of cap, shall have 2" concrete taper for water runoff as shown in the detail for partial height encasement.



ALTERNATE PILE ENCASEMENT DETAIL FOR STEEL SHELL PILES

(Shown with Partial Height Encasement)

This document was originally issued and sealed by Charles R. Ellis, PE No. 9235, on March 24, 2016. This copy is not a signed and sealed document.



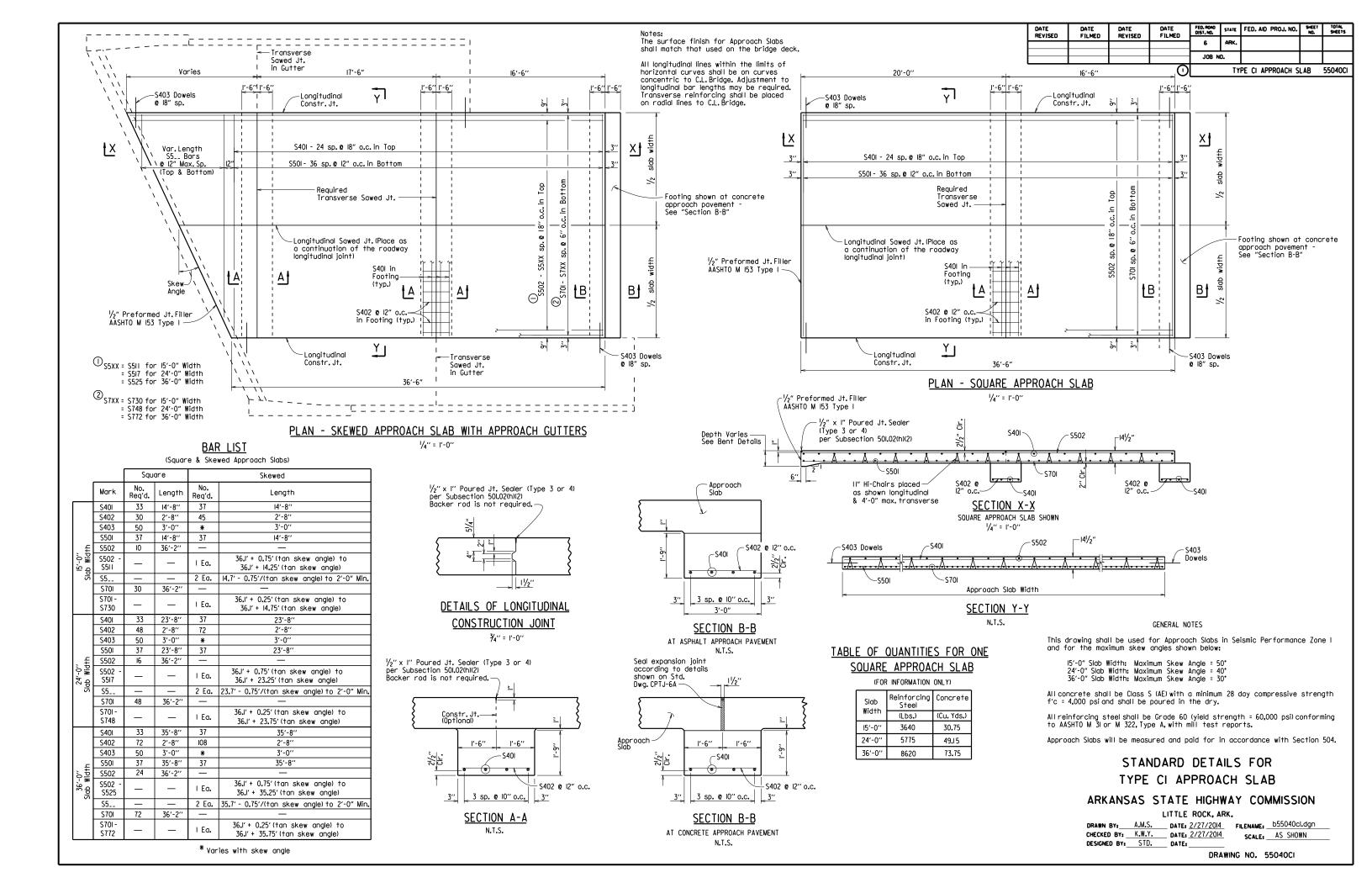
STANDARD DETAILS FOR CONCRETE FILLED STEEL SHELL PILES AND PILE ENCASEMENTS

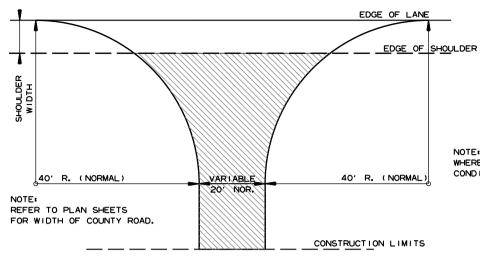
ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

DRAWN BY: A.M.S. DATE: 2/27/2014 FILENAME: 655021.dgn CHECKED BY: B.E.F. DATE: 2/27/2014 SCALE: NO SCALE DESIGNED BY: STD. DATE: -

DRAWING NO. 55021



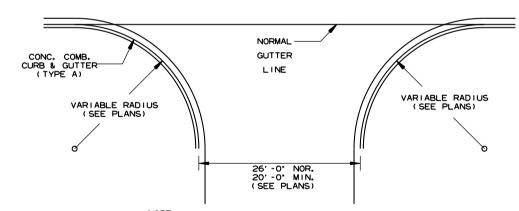


DETAIL FOR COUNTY ROAD TURNOUTS

OPEN SHOULDER SECTION

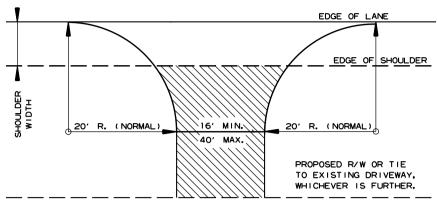
NOTE: TURNOUTS SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

> ACHM SURFACE COURSE (1/2") (220 LBS. PER SQ. YD.) AND AGGREGATE BASE COURSE (CLASS 7) 7" COMP. DEPTH, UNLESS OTHERWISE SPECIFIED IN PLANS.



PAVEMENT STRUCTURE FOR STATE HIGHWAYS, CITY STREETS, & COUNTY ROADS TO BE SAME AS MAIN LANES.

DETAIL OF TURNOUTS, ASPHALT STREETS, COUNTY ROADS & STATE HIGHWAYS CURB & GUTTER SECTION



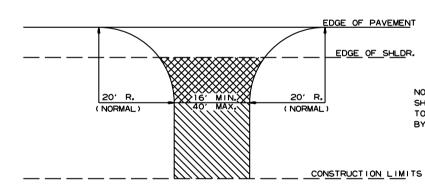
OPEN SHOULDER SECTION

(ARTERIALS)

DETAIL FOR DRIVEWAY TURNOUTS

NOTE: TURNOUTS AND PRIVATE DRIVES SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

ACHM SURFACE COURSE (1/2") (220 LBS, PER SQ, YD.) AND AGGREGATE BASE COURSE (CLASS 7) 7" COMP. DEPTH IF ASPHALT OR GRAVEL DRIVE EXISTING, OR 6 CONCRETE IF CONCRETE DRIVE EXISTING.



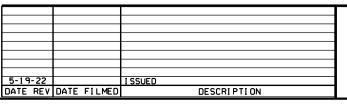
NOTE: TURNOUTS AND PRIVATE DRIVES SHALL BE MODIFIED WHERE NECESSARY TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

ASPHALT CONCRETE HOT MIX SURFACE COURSE (220 LBS, PER SQ, YD.) AGGREGATE BASE COURSE (CLASS 7) 7' COMP, DEPTH IF ASPHALT DRIVE EXIST OR 6' CONCRETE IF CONCRETE DRIVE EXIST.



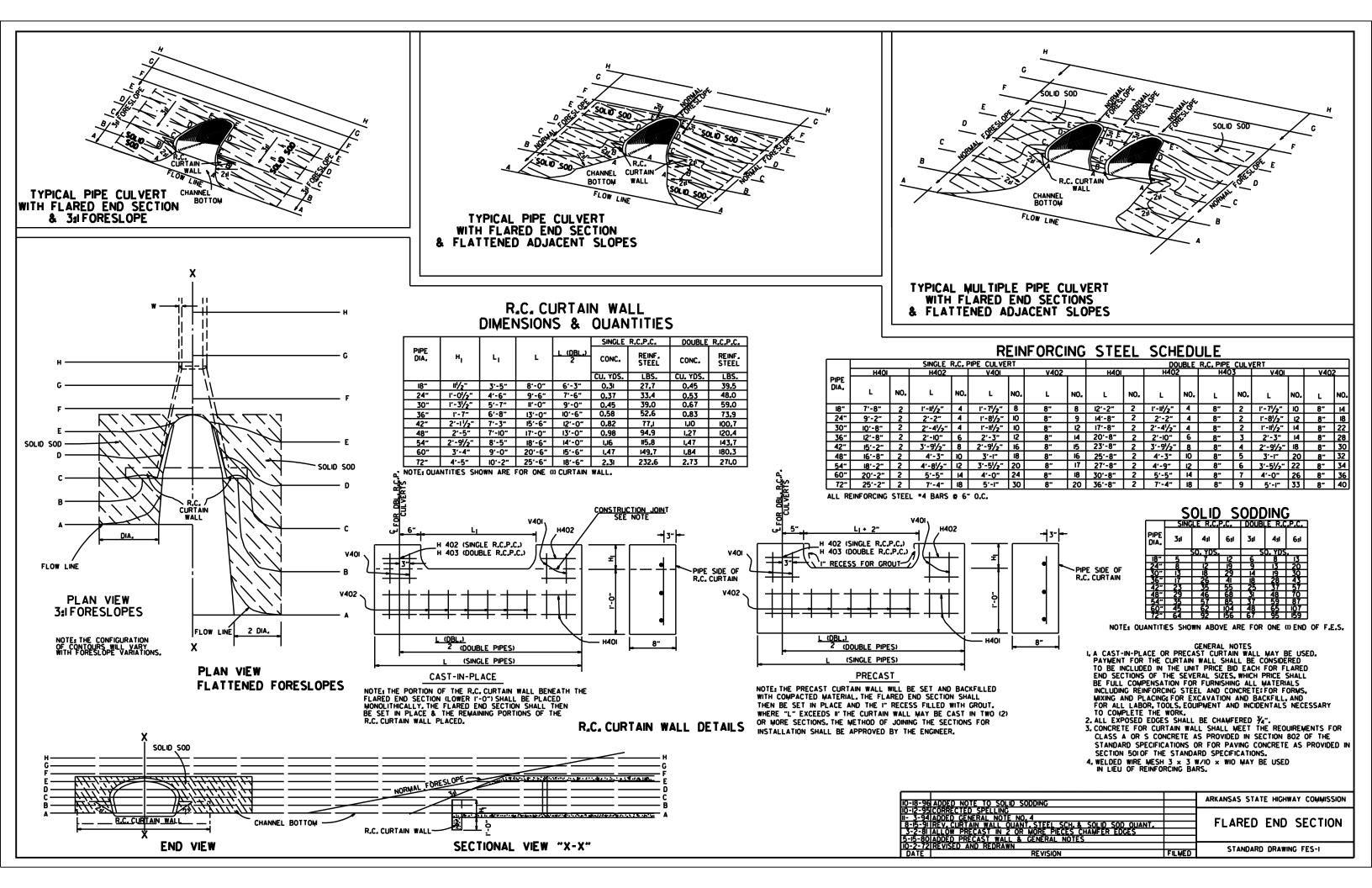
AGGREGATE BASE COURSE (CLASS 7)
9° COMP. DEPTH OR CONFORM
TO EXISTING DRIVEWAY

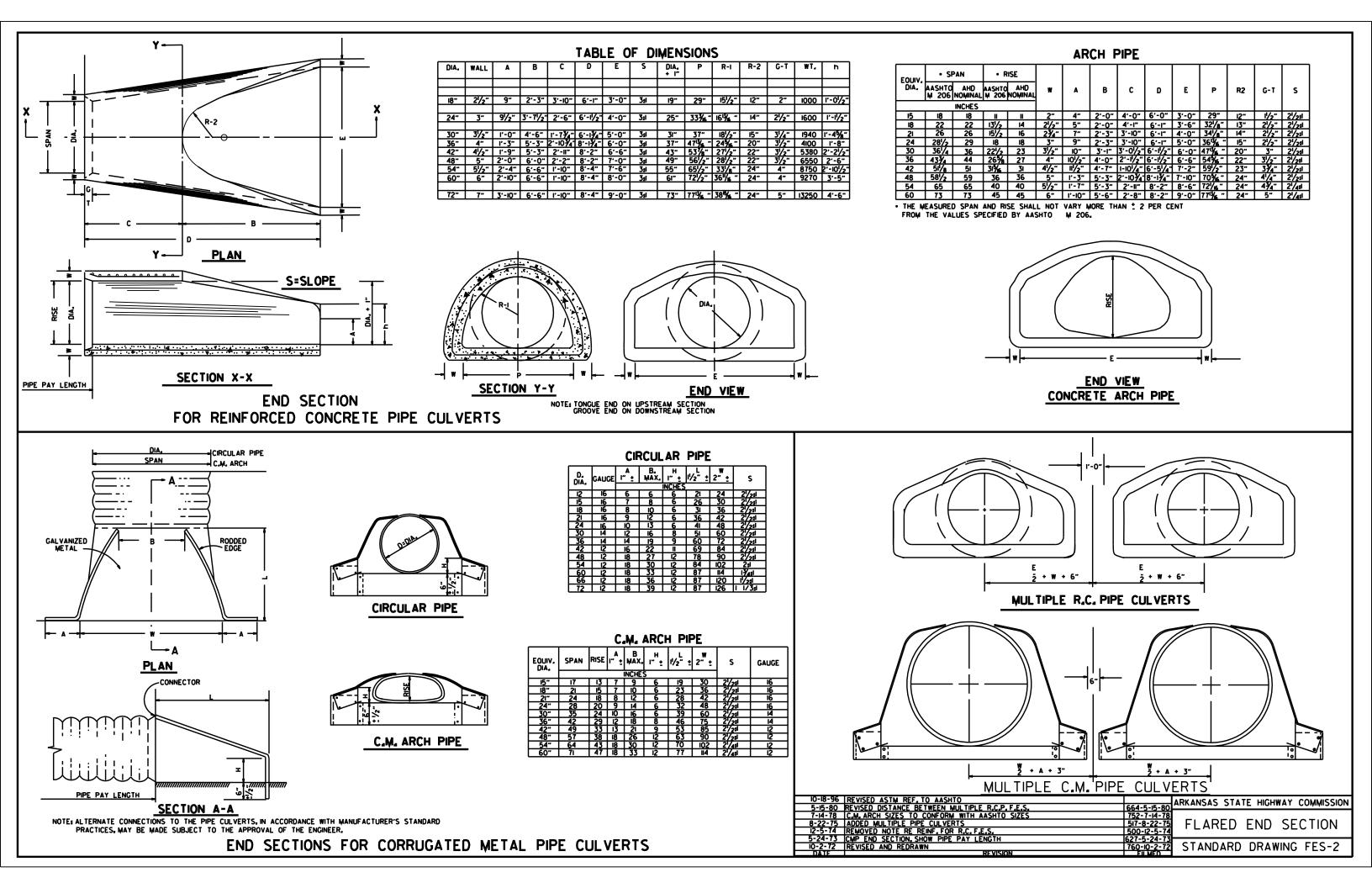
DETAIL FOR DRIVEWAY TURNOUTS (COLLECTORS)

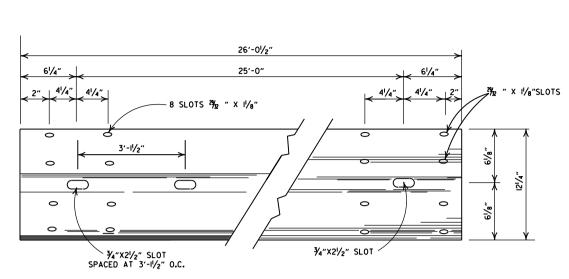


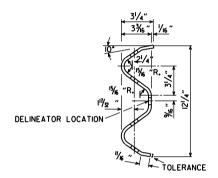
ARKANSAS STATE HIGHWAY COMMISSION DETAILS OF DRIVEWAYS & STREET TURNOUTS

STANDARD DRAWING DR-2



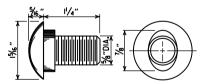




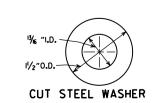


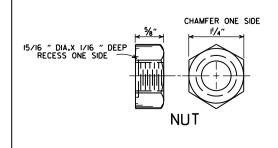
DETAILS OF W-BEAM GUARDRAIL

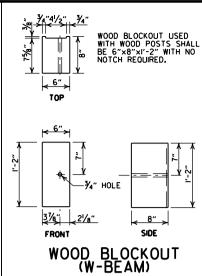
RAIL SECTION OF CLOSELY SIMILAR DIMENSIONS AND COMPARABLE STRENGTH MAY BE SUBSTITUTED IF APPROVED BY THE ENGINEER.

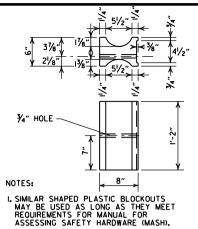


SPLICE BOLT POST BOLT - SAME EXCEPT LENGTH



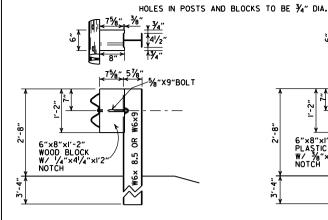






2.DIMENSIONS ARE SUBJECT TO MANUFACTURERS TOLERANCES.

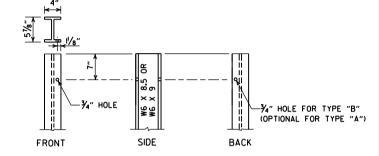
PLASTIC BLOCKOUT (W-BEAM)



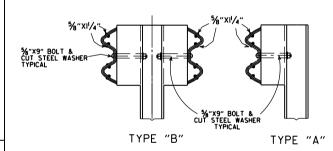
WOOD BLOCKOUT CONNECTIONS

7%" 5%" 5%" 5%" 5%" 5%" X9"BOLT 6"×8"×1'-2" PLASTIC BLOCK W/ 3/8"×4¹/2"×1'2" NOTCH PLASTIC BLOCKOUT CONNECTIONS

DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)



STEEL POST



DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)

-GENERAL NOTES-

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND
THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN

Y'' BEYOND IT.

WHERE W-BEAM GUARDRAIL CONTINUES, THE INTERMEDIATE SECTIONS
SHALL HAVE A POST SPACING OF 6'-3" UNLESS OTHERWISE NOTED.

W-BEAM GUARDRAIL REPRESENTING INTERMEDIATE SECTIONS
WILL BE MEASURED ALONG THE ROADWAY FACE FROM CENTERLINE OF
POST TO CENTERLINE OF POST.

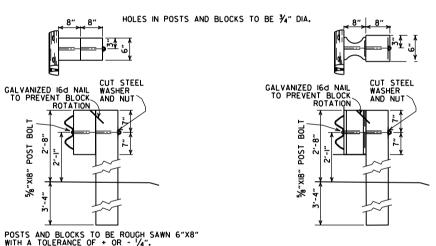
USE W-BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. FOR EXTENSIONS OR MODIFICATION OF EXISTING GUARDRAIL, W-BEAM GUARDRAIL COMPONENTS OF THE SAME TYPE AS THOSE EXISTING SHALL BE USED.

ANY BACKFILLING UNDER OR AROUND POST SHALL BE DAMP SAND THOROUGHLY TAMPED IN PLACE.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO. I 1350 f SOUTHERN PINE.

CONTRACTOR SHALL HAVE THE OPTION OF USING WOOD BLOCKOUTS FOR W-BEAM GUARDRAIL OR PLASTIC BLOCKOUTS, AS LONG AS BLOCKOUT USED MEETS REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) FOR W-BEAM GUARDRAIL.

DELINEATORS SHALL BE MOUNTED AT 37.5' SPACING ON THE FRONT FACE OF THE GUARDRAIL. SPACING MAY BE REDUCED IN CURVES, AS DIRECTED BY THE ENGINEER. COLOR SHALL BE IN ACCORDANCE WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES. PAYMENT FOR DELINEATORS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID PER LIN.FT.FOR GUARDRAIL.

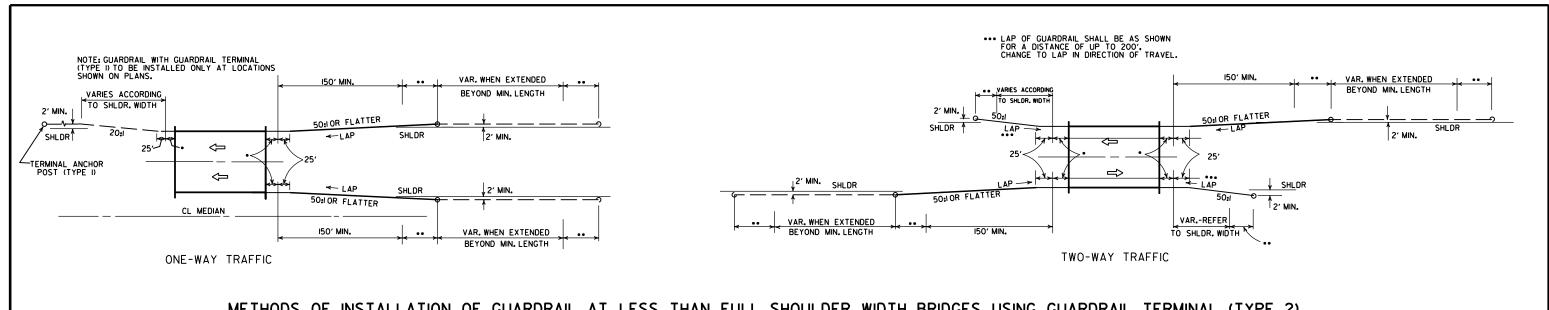


WOOD BLOCKOUT CONNECTIONS

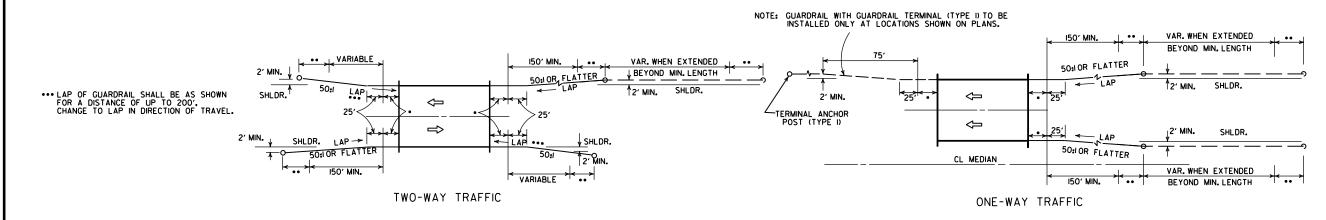
PLASTIC BLOCKOUT CONNECTIONS

DETAILS OF WOOD LINE POST CONNECTIONS (W-BEAM)

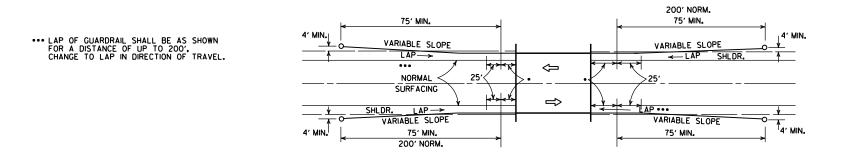
	REVISED GENERAL NOTES.		1
05-19-22	ADDED DELINEATOR LOCATION.		
11-07-19	RENUMBERED AND RENAMED		
	REVISED GENERAL NOTES AND RAISED		
11-16-17	GUARDRAIL HEIGHT 3"		
07-14-10	RAISED HEIGHT OF GUARDRAIL I"		
10-15-09	ADDED REFERENCE TO MASH		
04-10-03	REVISED GENERAL NOTES		
04-10-03	REVISED DIMENSION ON WOOD & PLASTIC		
08-22-02	BLOCKOUT CONNECTIONS & STEEL POST		
11-16-01	REVISED WOOD BLOCKOUT & DETAILS OF WOOD LINE POST CONNECTIONS		
03-30-00	REMOVED GUARDRAIL AT BRIDGE ENDS		
01-12-00	ADDED PLASTIC BLOCKOUT		
01-12-00	REV. BLOCKOUTS TO WOOD, DELETED CONC.		
1	POST & REV. GENERAL NOTE. DELETED DET.		
1	OF GUARDRAIL REPLACE, BEHIND CURB &		
08-12-98	DET. OF POST PLACE. IN SOLID ROCK.&		
08-12-98	ADDED DETAILS OF STEEL LINE POST		
	CONN, REMOVED BACK-UP PLATE, REVISED		
1	HOLES IN STEEL POLES		
	REMOVED "LAP IN DIRECTION OF TRAFFIC"		
04-03-97	NOTE & PLACED ARROWS ON WASHERS		
10 10 00			
10-18-96 06-02-94	REVISED WOOD POST NOTE ADDED ALT. STEEL POST SIZE		
08-02-94	REVISED STEEL POST SIZE	8-5-93	
10-01-92	REDRAWN & REVISED	10-1-92	IARKANSAS STATE HIGHWAY COMMISSION
08-15-91	REVISED WASHER NOTE	8-15-91	ANNANSAS STATE HIGHWAT COMMISSION
	REV. GEN. NOTE & DEPTH OF ANC. POST		
08-02-90	IN ROCK	8-2-90	
07-15-88	REVISED SECTION 3 & GENERAL NOTES		GUARDRAIL DETAILS
01-15-00	REV. ANCHOR POST .ELEV. NOTES & POST		GUANDNAIL DETAILS
03-04-88	IN ROCK	780-3-4-88	
10-30-87	REVISED WOOD LINE POST DETAIL	546-10-30-87	
10-30-87	REDRAWN & REVISED	802-10-9-87	CTANDADD DDAWING CD C
			STANDARD DRAWING GR-6
DATE	REVISION	FILMED	



METHODS OF INSTALLATION OF GUARDRAIL AT LESS THAN FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



METHOD OF INSTALLATION OF GUARDRAIL AT FULL SHOULDER WIDTH BRIDGES USING GUARDRAIL TERMINAL (TYPE 2)



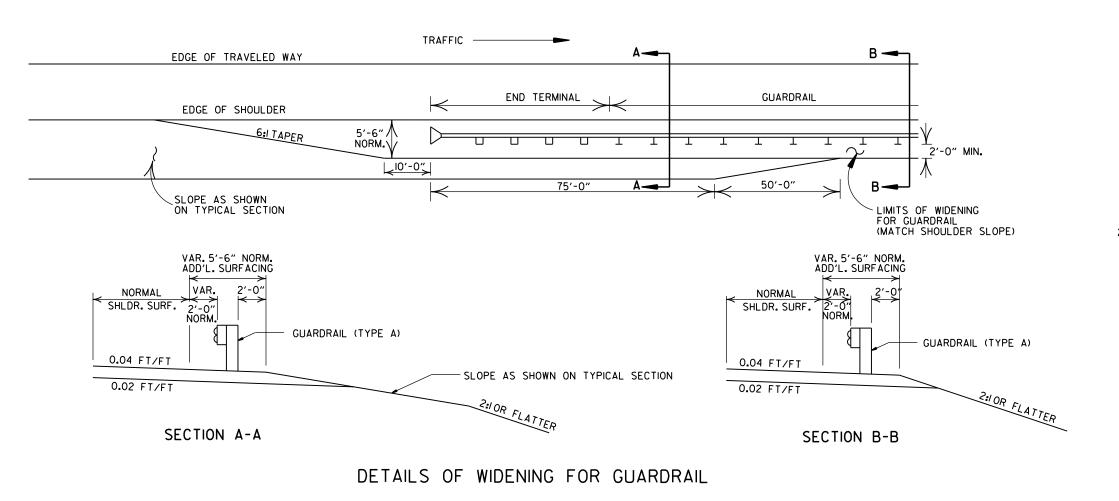
METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERMINAL (TYPE I) (FULL SHOULDER WIDTH OR LESS BRIDGES)

		_	
			ARKANSAS STATE HIGHWAY COMMISSION
11-07-19	RENUMBERED AND RENAMED		
4-17-08	REVISED LAYOUTS		
	REMOVED GUARDRAIL NOTES AND DETAILS		7
11-16-01	DELETED NOTE-METHOD OF INSTALLATION OF GUARDRAIL USING GUARDRAIL TERM. (TY. I)		GUARDRAIL DETAILS
1-12-00	ADDED CONSTRUCTION NOTE	1-12-00	
6-26-97	REVISED LAYOUT		
10-1-92	REDRAWN & REVISED	10-1-92	
	ADDED NOTE		
10-9-87	REDRAWN & REVISED		STANDARD DRAWING GR-8
DATE	REVISION	DATE FILM	

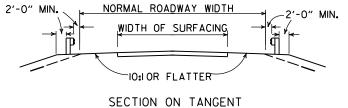
LEGEND

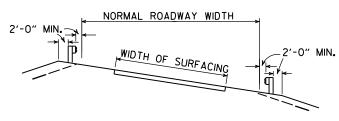
. THRIE BEAM GUARDRAIL TERMINAL

•• GUARDRAIL TERMINAL (TYPE 2)



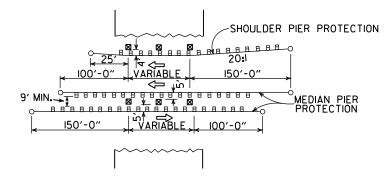
NOTE: NORMAL SECTION TO BE WIDENED APPROX.5'-6" EACH SIDE TO SUPPORT GUARDRAIL.





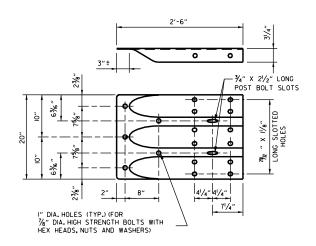
SECTION ON CURVE

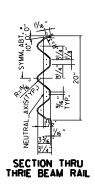
DETAILS SHOWING POSITION OF GUARDRAIL ON HIGHWAY

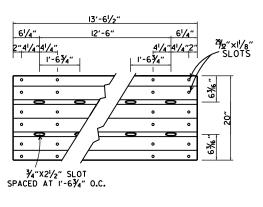


METHOD OF INSTALLATION OF GUARDRAIL AT FIXED OBSTACLE

			ARKANSAS STATE HIGHWAY COMMISSION
			GUARDRAIL DETAILS
			OUTHER DETTHE
11-07-19	RENUMBERED AND RENAMED		
4-17-08	MINOR REVISION		
11-10-05	DRAWN		STANDARD DRAWING GR-9
DATE	REVISION	DATE FILM	





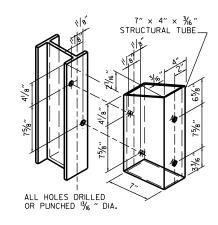


3/4"X21/2" SLOT -6¹/4" 71/4" 61/4" 61/4" 311/16 20" 63/6"

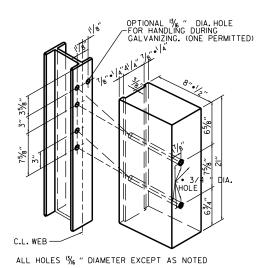
SPECIAL END SHOE

THRIE BEAM RAIL

TRANSITION SECTION



ATTACH BLOCKOUT TO POST USING 5%" DIA. HEX HEAD BOLTS WITH 11/2" O.D. CUT STEEL WASHERS AND NUT.

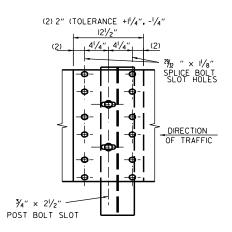


HOLE PUNCHING DETAIL

FOR STEEL POST & WOOD OR PLASTIC BLOCKOUTS

NOTE: BLOCKS SHALL BE THE SAME TYPE THROUGHOUT THE PROJECT LIMITS.

STRUCTURAL STEEL TUBING BLOCKOUT DETAIL



THRIE BEAM RAIL SPLICE AT POST

GENERAL NOTES:

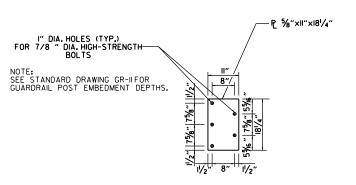
THE THRE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I. RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN 3*4" BEYOND IT.

ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-13.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

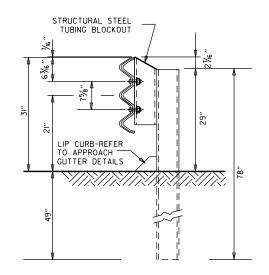
USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB. WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (4000 f) OR NO. I 1350 f SOUTHERN PINE.



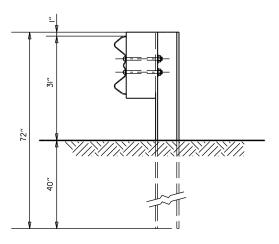
CONNECTOR PLATE

CONNECTOR PLATE SHALL BE AASHTO M270, GR. 36 AND SHALL BE GALVANIZED AFTER FABRICATION, GALVANIZING SHALL CONFORM TO SUBSECTION 807.19 OF THE STANDARD SPECIFICATIONS. CONNECTOR PLATE TO BE BOLTED TO SPECIAL END SHOE USING % DIA, HIGH STRENGTH BOLTS, WITH THE HEADS PLACED ON THE TRAFFIC FACE. WASHERS SHALL BE USED UNDER THE HEAD AND NUT. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND SHALL CONFORM TO SUBSECTION 807.06.

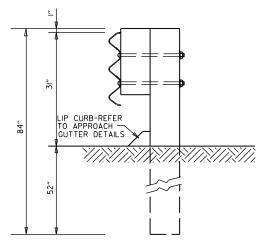
11-07-19	RENAMED AND REVISED REFERENCES		
11-16-17	REVISED TRANSITION SECTION, GUARD RAIL HEIGHT, AND GENERAL NOTES; MOYED THRIE BEAM GUARD RAIL CONNECTIONS AT BRIDGES ENDS TO STD. DRWG. GR-12		
07-14-10	RAISED HEIGHT OF W-BEAM I"		
11-29-07	ADDED PLASTIC BLOCKOUTS		ADVANCAC CTATE HICHWAY COMMICCION
11-10-05	ADDED NOTE FOR ATTACHING STEEL		ARKANSAS STATE HIGHWAY COMMISSION
	BLOCKOUT		
II-I8-04	REVISED GENERAL NOTES		
10-9-03	REVISED GENERAL NOTES		
04-10-03	REVISED GENERAL NOTES		GUARDRAIL DETAILS
08-22-02	REVISED NOTE (2)		
06-29-00	MOVED DIMENSION LINES		
05-18-00	ADDED NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-10
DATE	REVISION	FILMED	



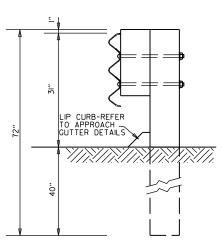
THRIE BEAM RAIL WITH STEEL TUBING BLOCKOUT AND STEEL POST POSTS 1-7



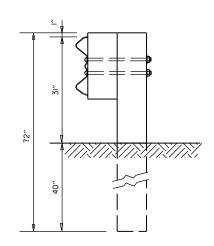
W-BEAM TO THRIE BEAM TRANSITION RAIL
WITH WOOD OR PLASTIC BLOCKOUT AND STEEL POST
POST 8



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUTS & WOOD POSTS
POSTS I-6



THRIE BEAM RAIL
WITH WOOD OR PLASTIC
BLOCKOUT & WOOD POST
POST 7



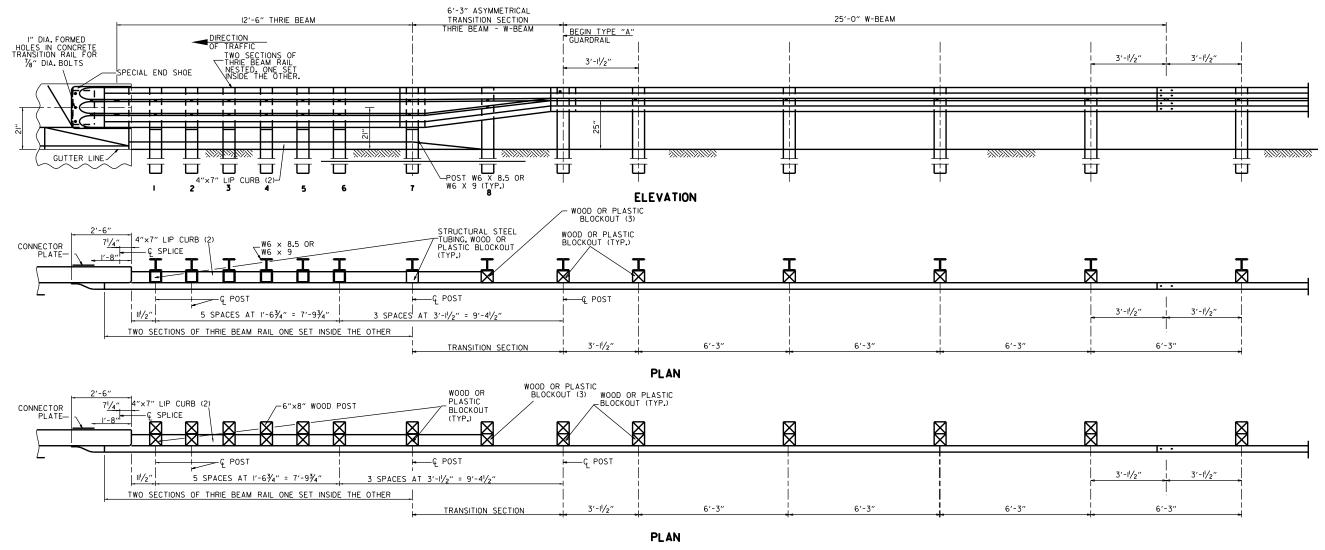
W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT & WOOD POST POST 8

GENERAL NOTES:

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (4400 f) OR NO.11350 f SOUTHERN PINE.

			ARKANSAS STATE HIGHWAY COMMISSION
11-07-19	RENAMED		
11-16-17	REVISED GUARDRAIL HEIGHT, CHANGED STD. DWG. NUMBER FROM GR-IOA TO GR-II		GUARDRAIL DETAILS
07-14-10	REVISED POST 8 DIMENSIONS]
II-29-07	ADDED PLASTIC BLOCKOUTS		
08-22-02	REVISED LIP CURB NOTE		
03-30-00	DRAWN & ISSUED		STANDARD DRAWING GR-II
DATE	REVISION	FILMED	STANDARD DRAWING GR II



- (1) VERIFY BOLT SPACING FROM RAIL TRANSITION PRODUCER.
 (2) REFER TO APPROACH GUTTER DETAILS.
- (3) LENGTH OF BLOCKOUT ON POST 8 TO BE MODIFIED TO FIT RAIL WIDTH.

THRIE BEAM GUARDRAIL CONNECTION AT BRIDGE ENDS

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE I.

RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN $3\prime4^{\prime\prime}$ BEYOND IT.

ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-8 & GR-13.

REFER TO STD. DRWG. GR-II FOR POST DETAILS.

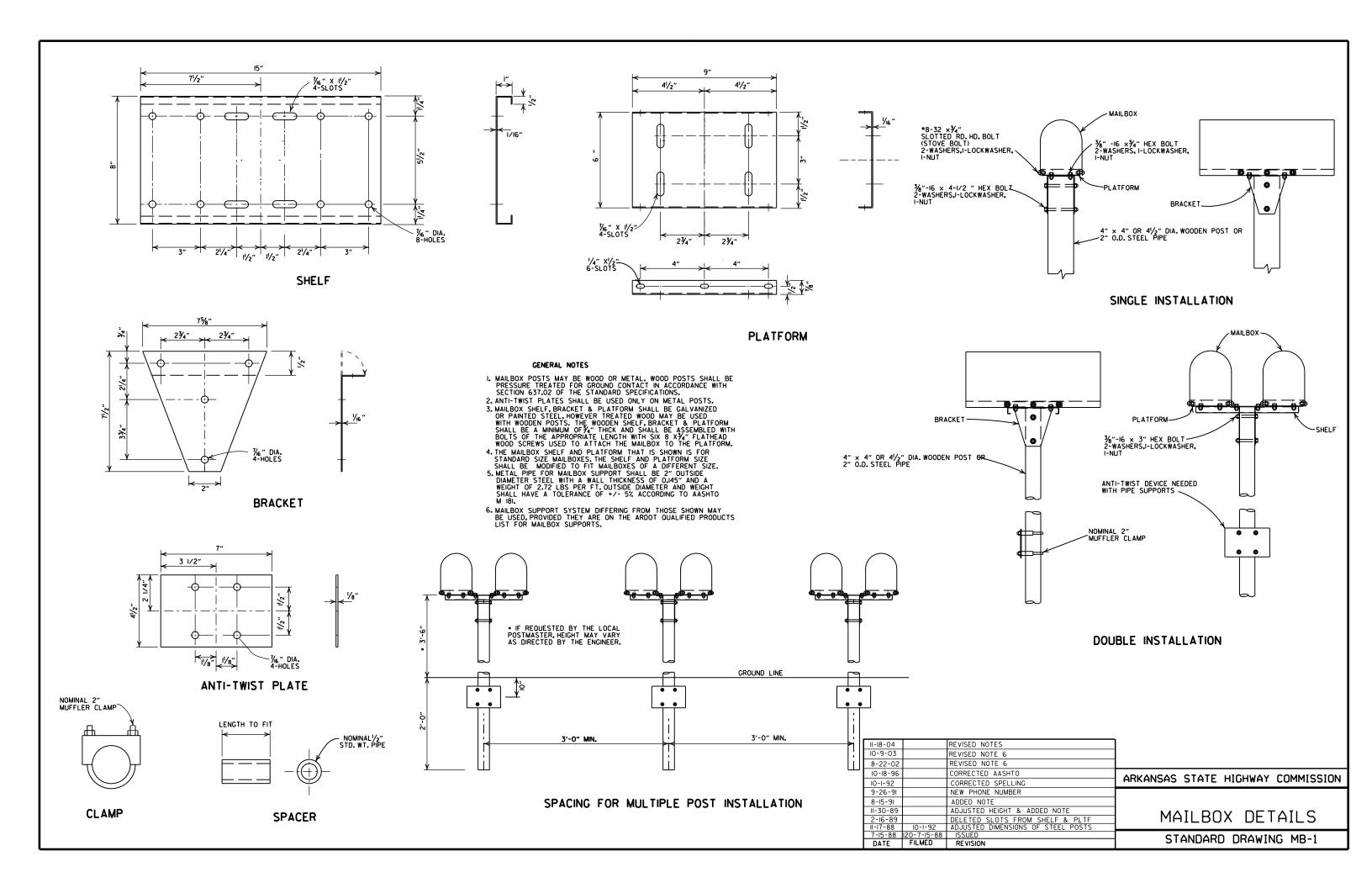
USE THRIE BEAM GUARDRAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB.

THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W-BEAM POSTS FOR ENTIRE JOB.

POSTS SHALL NOT BE PLACED AT SPLICE LOCATIONS ALONG W-BEAM RAILS.

WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. ISTRUCTURAL OR BETTER 9.7f (1400 f) OR NO.1 1350 f SOUTHERN PINE.

			ARKANSAS STATE HIGHWAY COMMISSION
			GUARDRAIL DETAILS
05-14-20 II-07-19	REVISED NOTES RENAMED & REVISED REFERENCES		GUANDINALE DETAILS
II-16-17 DATE	RE-DRAWN FROM STD. DWG. GR-10 & ISSUED REVISION	FILMED	STANDARD DRAWING GR-12



REINFORCED CONCRETE ARCH PIPE DIMENSIONS

EQUIV.	SP	AN	RISE			
DIA.	AASHTO M 206	ARDOT NOMINAL	AASHTO M 206	ARDOT NOMINAL		
INCHES		INC	HES			
15 18 21 24 30 36 42 48 54 60 72 84 90 96 108 120 132	AASIIIO		11 13½2 15½2 18 22½ 26% 31% 36 40 45 54 62 77½ 87½ 96% 106½	11 14 16 18 23 27 31 36 40 45 54 62 77 87 97		

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN + 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE DIMENSIONS

'	11 _	DILICIADIONS					
	EQUIV.	AASHTO M 207					
	DIA.	SPAN	RISE				
	INCHES	INC	HES				
	18	23	14				
	24	30	19				
	27	34	22				
	30	38	24				
	33	42	27				
	36	45	29				
	39	49	32				
	42	53	34				
	48	60	38				
	54	68	43				
	60	76	48				
	66	83	53				
	72	91	58				
	78 84	98	63				
		106	68				

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN
± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE. 5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(I).

NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE

- LEGEND -

- D1 = NORMAL INSIDE DIAMETER OF PIPE
- Do OUTSIDE DIAMETER OF PIPE
 H = FILL COVER HEIGHT OVER PIPE (FEET)
 MIN. = MINIMUM
- = UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

- *SM-3 WILL NOT BE ALLOWED.
- ** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.

MINIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

	CLASS OF PIPE				
	CLASS	III	CLASS IV	CLASS V	
INSTALLATION TYPE	TYPE 1 OR 2	TYPE 3	ALL	ALL	
PIPE ID (IN.)		FEE	Т		
12-15	2	2.5	2	1	
18-24	2.5	3	2	1	
27-33	3	4	2	1	
36-42	36-42 3.5		2	1	
48	48 4.5		2	1	
54-60	5	7	2	1	
66-78 6		8	2	1	
84-108	7.5	8	2	1	

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MINIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS OF PIPE			
INSTALLATION TYPE	CLASS III	CLASS IV		
	FEET			
TYPE 2 OR TYPE 3	2.5	1.5		

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

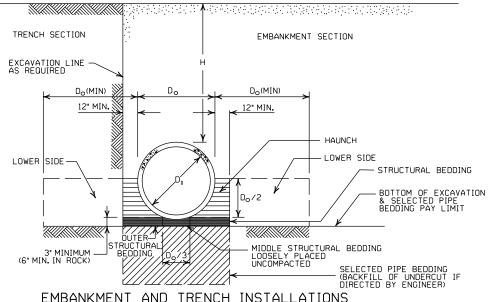
	CLASS OF PIPE				
INSTALLATION TYPE	CLASS III	CLASS IV	CLASS V		
1117	FEET				
TYPE 1	21	32	50		
TYPE 2	16	25	39		
TYPE 3	12	20	30		

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MAXIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

	CLASS	OF PIPE		
INSTALLATION TYPE	CLASS III	CLASS IV		
1175	FEET			
TYPE 2	13	21		
TYPE 3	10	16		

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.



EMBANKMENT AND TRENCH INSTALLATIONS

- I. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH. IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND ECCOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
- 3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

- I. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. ALL PIPE SHALL CONFORM TO SECTION 606. CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO MI70, R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES.
 THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD.DWG.FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING. HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SQUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
- 9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- IO. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

[
	2-27-14	REVISED GENERAL NOTE I.		
	12-I5-II	REVISED FOR LRFD DESIGN SPECIFICATIONS		
ĺ	5-18-00	REVISED TYPE 3 BEDDING & ADDED NOTE		
ı	3-30-00	REVISED INSTALLATIONS		
	II-06-97	ISSUED		
	DATE	REVISION	DATE	FILMED

ARKANSAS STATE HIGHWAY COMMISSION CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING

STANDARD DRAWING PCC-1



CORRUGATED STEEL PIPE (ROUND)

PIPE	① MINUMUM COVER TOP OF	MAX.FILL	HEIGHT "	H" ABOVE	TOP OF PI	PE (FEET)
DIAMETER	PIPE TO TOP OF GROUND		METAL	THICKNESS	(INCHES)	
(INCHES)	"H" (FEET)	0.064	0.079	0.109	0.138	0.168
	2¾ INCH BY ½ INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM					
12 15 18 24		84 67 56 42	91 73 61 46	59		
30 36 42 48	2 2 2 2 2	34	36 30 43 37	47 39 67 58	41 70 61	73 64
	② 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, BOLTED, OR HELICAL LOCK-SEAM					
36 42 48 54 60 66 72 78 84 90 96 102 108 114	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	48 48 41 36 32 29 26 24	60 51 45 40 36 33 30 28 26 24 22	88 88 72 64 59 53 47 44 41 38 35 33 31 30 28	III 90 71 64 58 53 49 45 43 40 38 35 34 32	III8 II02 85 79 71 64 59 54 51 45 44 42 37 35

CORRUGATED ALUMINUM PIPE (ROUND)

① MINUMUM	MAX.FILL	HEIGHT '	'H'' ABOVE	TOP OF F	PIPE (FEET
PIPE TO TOP		METAL TH	HICKNESS I	N INCHES	
"H" (FEET)	0.060	0.075	0.105	0.135	0.164
	2 ²/3	INCH B	Y 1/2 INCH	CORRUGA	TION
	R	IVETED OF		LOCK-SEA	м
I	45	45			
2	30	30	52		
2	22	22	39	41	
2		18	31	32	34
2.5		15	26	27	28
			43	43	44
			40	41	43
			35	37	38
2					34
					31
2					29
-					
C	OVER TOP OF PIPE TO TOP OF GROUND "H" (FEET) I 2 2	OVER TOP OF PIPE TO TOP OF GROUND O.060 O.	Note	METAL THICKNESS METAL THIC	METAL THICKNESS IN INCHES No.105 O.105 O.105

CONSTRUCTION SEQUENCE

- 1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
 2. INSTALL PIPE TO GRADE.
 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
 4. COMPLETE STRUCTURAL BACKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE, WHITCHEVER IS LESS
- NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4,5,6,0R 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

3 SM-3 WILL NOT BE ALLOWED.

EQUIVALENT METAL THICKNESSES AND GAUGES

METAL			
ST	EEL		GAUGE NUMBER
ZINC COATED	UNCOATED	ALUMINUM	
0.064	0.0598	0.060	16
0.079	0.0747	0.075	14
0.109	0.1046	0.105	12
0.138	0.1345	0.135	10
0.168	0.1644	0.164	8

ALUMINUM

2 3 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM

MAX.HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

13

TYPE 1

(1) MIN. HEIGHT OF

FILL, "H" (FT.)

INSTALLATION

TYPE 1

2.25 2.5

CORRUGATED METAL PIPE ARCHES

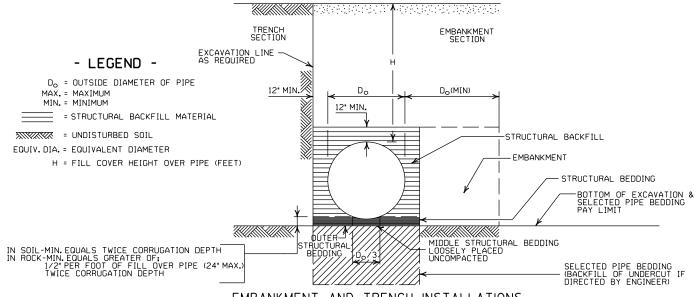
MINUMUM MIN. (1) MIN. HEIGHT OF

MAX. HEIGHT OF

MIN.

	LILE	INDIADIAL		I (I) MIN' HEI			IGHT OF	ITITIN.	l
EQUIV.	DIMENSION		THICKNESS	FILL, "	H'' (FT.)	FILL,"	H'' (FT.)	THICKNESS	
DIA.	SPAN X RISE	RADIUS	REQUIRED	INSTAL	LATION	INSTAL	LATION	REQUIRED	
(INCHES)	(INCHES)	(INCHES)	INCHES	TYPE	Ξ 1	TYPE	1	INCHES	
			2		BY ½ INCH (١
						AL LOCK-SEA			
15	17×13	3	0.064	2		15		0.060	
18	21×15	3 3 3	0.064	2		15		0.060	
21	24×18	3	0.064	2.2		15		0.060	I
24	28×20	3	0.064	2.		15		0.075	
30	35×24		0.079	3 3		12		0.075	
36	42×29 49×33	31/2	0.079 0.079] 3		12 12		0.105 0.105	
42 48	57×38	4	0.109	3				0.135	
54	64×43	5 6	0.109] 3		13		0.135	I
60	71×47	7	0.138	3		15		0.153	I
66	77×52	8	0.168	3		15		0.104	l
72	83×57	9	0.168	3		15			
			3 INCH	BY 1 INCH I	OR 5 INCH E	BY 1 INCH CO	RRUGATION	1	
			RIVE	TED, WELDE	D, OR HELIC	AL LOCK-SE	AM		
				INSTAL	LATION	INSTAL	LATION	1	ı
				TYPE 2	TYPE 1	TYPE 2	TYPE 1	2	
36	40×31	5	0.079	3	2	12	15	1	١
42	46×36	6	0.079	3	2	13	15		ί
48	53×4I	7	0.079	3	2	13	15		
54	60×46	8	0.079	3	2	13	15		
60	66×5I	9	0.079	3	2	13	15		
66	73×55	12	0.079	3	2	15	15		
72	81×59	14	0.079	3	2	15	15		
78	87×63	14	0.079	3	2	15	15		
84	95×67	16	0.109	3	2	I5	15		
90	103×71	16	0.109	3	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	15	15		
96	112×75	18	0.109	3		15	15		
102 108	117×79	18 18	0.109	3 3	2 2	15 15	15 15		
108	128×83	10	0.138			[ID	13	J	

- ① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.
- ② WHERE THE STANDARD 2 2/3'x ½ CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3"x 1" OR 5"x 1" CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS CAUGED FOR A FILL HEIGHT CONDITION EQUAL TO OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.



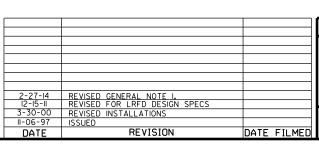
EMBANKMENT AND TRENCH INSTALLATIONS

- I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
- 2. INSTALLATION TYPE FOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
- 3. INSTALALTION TYPE ISHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 23" X 1/2"
- 4. INSTALLATION TYPE IOR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X I" OR 5" X I" CORRUGATION.

GENERAL NOTES

- I. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
- 2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
- 4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
- 5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
- 6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
- 7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL),
 BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE.

 IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."



ARKANSAS STATE HIGHWAY COMMISSION METAL PIPE CULVERT

FILL HEIGHTS & BEDDING

STANDARD DRAWING PCM-1



INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	*SELECTED MATERIALS (CLASS SM-I, SM-2 OR SM-4)

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF HOPE PIPE.

MULTIPLE INSTALLATION OF HIGH DENSITY POLYETHYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2′-6″
36"	3′-0″
42"	3′-6″
48"	4′-0″

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)			
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= IO'-0"		
18"	4'-6"	4′-6″		
24"	5′-0″	6′-0″		
30"	5′-6″	7′-6″		
36"	6′-0″	9'-0"		
42"	7′-0″	10'-6"		
48"	8'-0"	12'-0"		

(DNOTE:

18" MIN. (18" - 30" DIAMETERS)
24" MIN. (36" - 48" DIAMETERS)
MINIMUM COVER VALUES, "H"
SHALL INCLUDE A MINIMUM 12"
OF PAVEMENT AND/OR BASE.

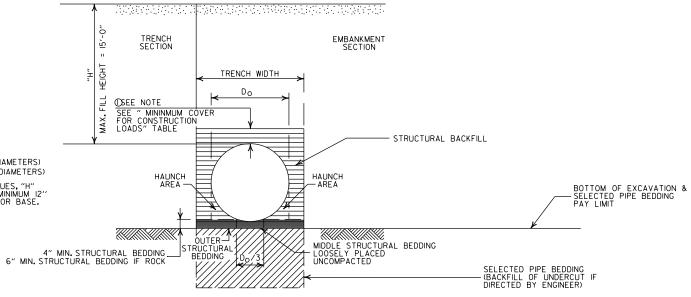
MINIMUM COVER FOR CONSTRUCTION LOADS

	Ø MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-II0.0 (KIPS)	110.0-175.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3′-0″	3'-0"
42" OR GREATER	3'-0"	3'-0"	3′-6″	4'-0"

MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M294, TYPE S.INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING, THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. HIGH DENSITY POLYETHYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR HDPE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I. 12-15-11 REVISED GENERAL NOTES & MINIMUM COVER NOTE 11-17-10 ISSUED DATE REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (HIGH DENSITY POLYETHYLENE)

STANDARD DRAWING PCP-1

INSTALLATION TYPE	•• MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING	
TYPE 2	•SELECTED MATERIALS (CLASS SM-I, SM-2, OR SM-4)	

• AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.

SM3 WILL NOT BE ALLOWED.

•• STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INNCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF PVC PIPE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0'	
18"	4'-6"	4'-6"	
24"	5′-0″	6′-0"	
30"	5′-6″	7′-6″	
36"	6′-0″	9′-0″	

MULTIPLE INSTALLATION OF PVC PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	l'-6"
24"	2'-0"
30"	2′-6″
36"	3′-0″

MAXIMUM FILL HEIGHT BASED ON STRUCTURAL BACKFILL

PIPE DIAMETER	"H"
18"	45'-0"
24"	45'-0"
30"	40'-0"
36"	40'-0"

① NOTE: 12" MIN. (18" - 36" DIAMETERS) MINIMUM COVER VALUE, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

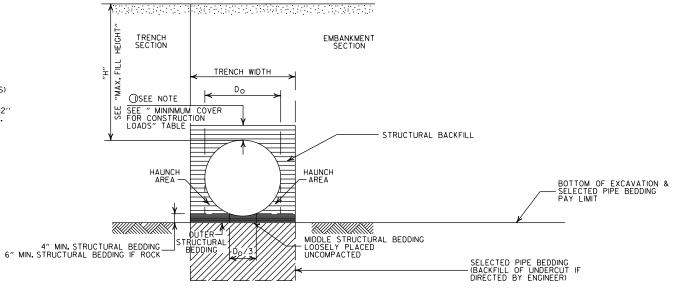
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	IIO.0-175.0 (KIPS)
18" THRU 36"	2'-0"	2'-6"	3'-0"	3'-0"

 ${}^{\textcircled{O}}$ MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I.PIPE SHALL CONFORM TO ASTM F949, CELL CLASS 12454. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE, IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. PVC PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR PVC PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.)

 D_{O} = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM

MIN. = MINIMUM

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

2-27-14 REVISED GENERAL NOTE I. 12-15-II REV GENERAL NOTES & MINIMUM COVER NOTE; DELETED SM3 MATERIAL II-17-10 ISSUED REVISION DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (PVC F949)

STANDARD DRAWING PCP-2



INS	STALLATION TYPE	**MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
	TYPE I	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
	TYPE 2	*SELECTED MATERIALS (CLASS SM-1, SM-2 OR SM-4) OR TYPE I INSTALLATION MATERIAL

*SM3 WILL NOT BE ALLOWED.

** STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF INCH, STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF POLYPROPYLENE PIPE.

MULTIPLE INSTALLATION OF POLYPROPYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2′-6″
36"	3′-0″
42"	3′-6″
48"	4′-0″
60"	5′-0″

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

	TRENCH WIDTH (FEET)		
PIPE DIAMETER	"H" < 10'-0"	"H" >OR= 10'-0"	
18"	4'-6"	4′-6″	
24"	5′-0″	6′-0″	
30"	5′-6″	7′-6″	
36"	6′-0"	9′-0″	
42"	7′-0″	10'-6"	
48"	8'-0"	12'-0"	
60"	10'-0"	15'-0"	

12" MIN. (18" - 42" DIAMETERS) 24" MIN. (60" DIAMETER) MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12'' OF PAVEMENT AND/OR BASE.

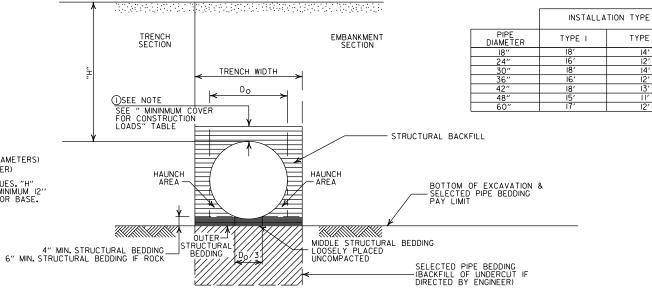
MINIMUM COVER FOR CONSTRUCTION LOADS

	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
PIPE DIAMETER	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-IIO.0 (KIPS)	110.0-150.0 (KIPS)
36" OR LESS	2'-0"	2′-6″	3'-0"	3'-0"
42" OR GREATER	3'-0"	3'-0"	3′-6″	4'-0"

②MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

GENERAL NOTES

- I. PIPE SHALL CONFORM TO AASHTO M330, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICIATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
- 2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SIXTH EDITION (2012) WITH 2013 INTERIMS.
- 3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
- 4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
- 5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUIANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
- 6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
- 7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
- 8. POLYPROPYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
- 9. JOINTS FOR POLYPROPYLENE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN SECTION 26.4.2.4 AND 30.4.2 OF THE AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS 3RD EDITION (2010) WITH 2012 INTERIMS. JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.



EMBANKMENT AND TRENCH INSTALLATIONS

I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

- I. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
- 2. INSTALL PIPE TO GRADE.
- 3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
- 4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
- 5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND

- LEGEND

H = FILL HEIGHT (FT.) Do = OUTSIDE DIAMETER OF PIPE MAX. = MAXIMUM MIN. = MINIMUM

MAXIMUM HEIGHT OF FILL "H"

TYPE 2

= STRUCTURAL BACKFILL MATERIAL

= UNDISTURBED SOIL

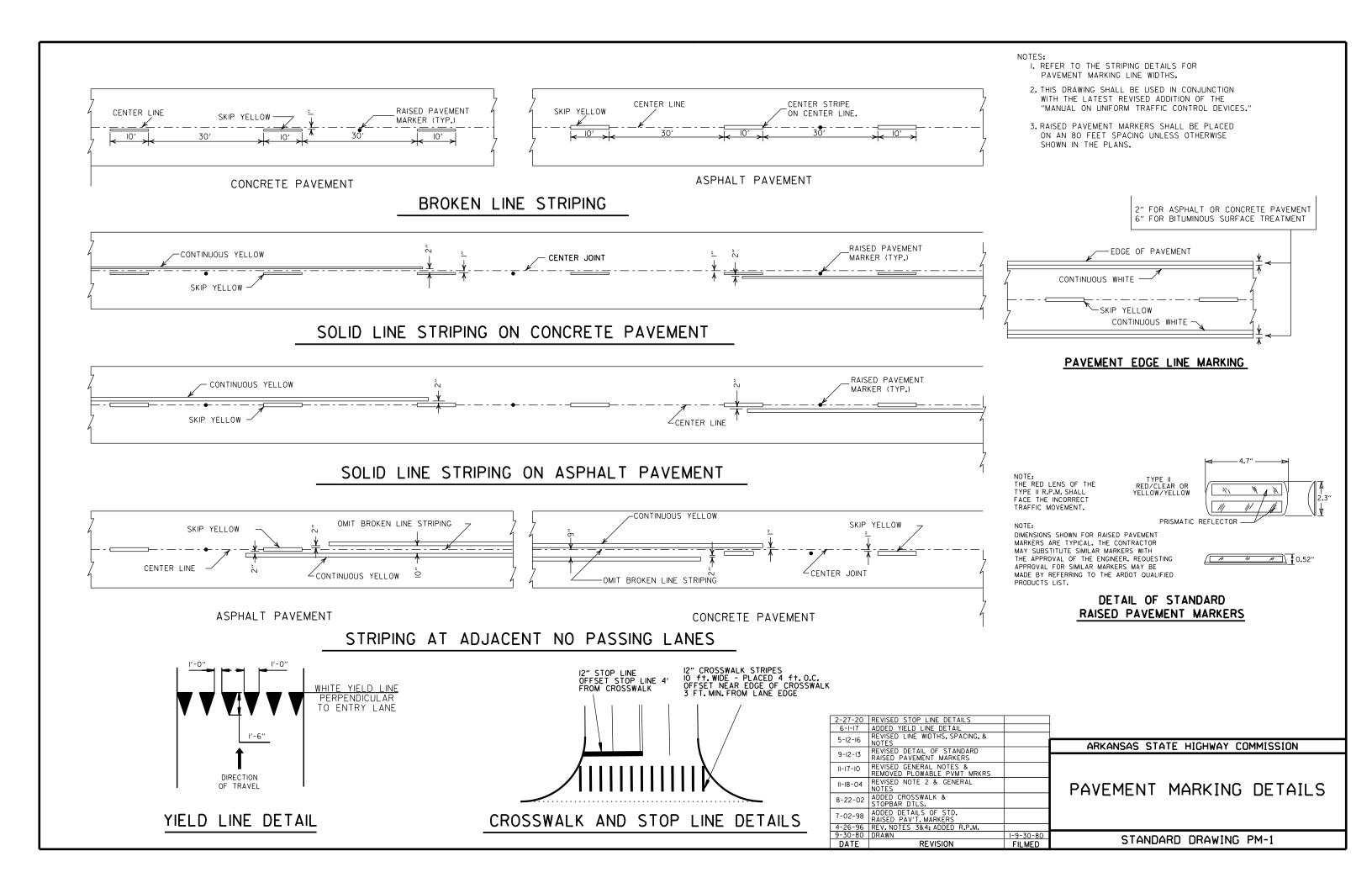
02-27-20	REVISED	
11-07-19	ISSUED	
DATE	REVISION	DATE FILMED

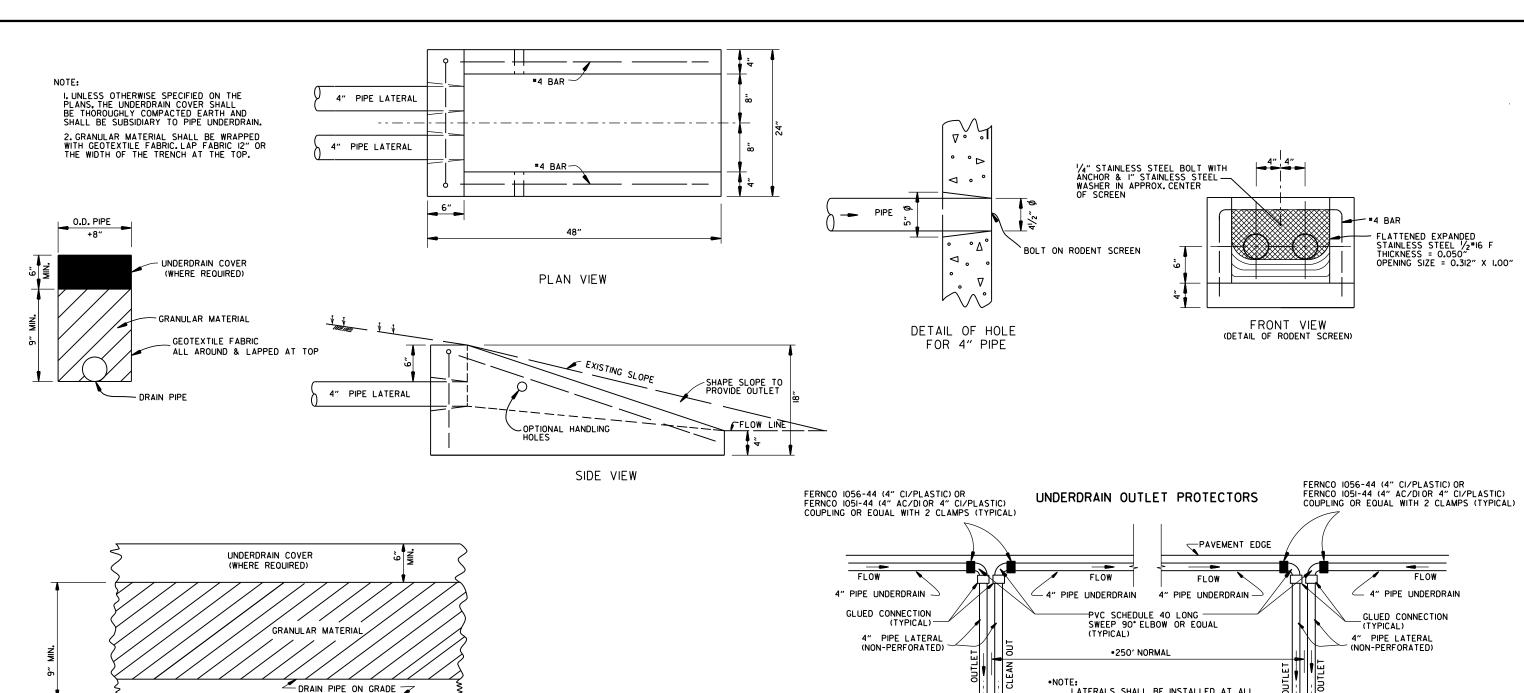
ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT (POLYPROPYLENE)

STANDARD DRAWING PCP-3







DETAILS OF PIPE UNDERDRAIN

NOTES FOR PIPE UNDERDRAINS

I. GEOTEXTILE FABRIC SHALL MEET THE REQUIREMENTS OF SECTION 625 FOR TYPE I. PAYMENT FOR GEOTEXTILE FABRIC AND GRANULAR FILTER MATERIAL SHALL BE INCLUDED IN THE PRICE BID PER LIN. FT. FOR "4" PIPE UNDERDRAINS" IN ACCORDANCE WITH SECTION 611 OF THE STANDARD SPECIFICATIONS.

2.4" NON-PERFORATED SCHEDULE 40 PVC PIPE LATERALS WITH OUTLET PROTECTORS SHALL BE INSTALLED AS SHOWN HEREON, LATERALS WILL BE MEASURED AND PAID FOR BY THE UNIT IN ACCORDANCE WITH SECTION GII OF THE STANDARD SPECIFICATIONS.

3. EXISTING 4" PIPE UNDERDRAINS MAY BE CONNECTED TO PROPOSED DROP INLETS OR EXTENDED WHERE DIRECTED BY THE ENGINEER. PAYMENT FOR CONNECTING TO DROP INLETS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR "4" PIPE UNDERDRAINS."

4. THE LOCATION OF ALL LATERALS SHALL BE MARKED WITH 4" X 12" PERMANENT PAVEMENT MARKING TAPE (TYPE III WHITE) AT THE OUTSIDE EDGE OF THE SHOULDER, PLACED TRANSVERSE TO TRAFFIC. PAYMENT FOR THIS WORK SHALL BE INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS.

5. PAYMENT FOR THE RODENT SCREEN SHALL BE INCLUDED IN THE PRICE BID PER EACH FOR "UNDERDRAIN OUTLET PROTECTORS."

6. ANY EXISTING UNDERDRAINS THAT INTERFERE WITH INSTALLATION OF THE NEW UNDERDRAIN SYSTEM SHALL BE REMOVED AND DISPOSED OF AS DIRECTED BY THE ENGINEER, PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS CONTRACT ITEMS. EXISTING UNDERDRAIN OUTLET PROTECTORS SHALL BE REMOVED UNDER THE ITEM "REMOVAL AND DISPOSAL OF UNDERDRAIN OUTLET PROTECTORS."

7. AT LOCATIONS WHERE A SINGLE LATERAL IS USED THE CONTRACTOR SHALL HAVE THE FOLLOWING OPTIONS: I. INSTALL OUTLET PROTECTOR AS SHOWN ON STANDARD DRAWING PU-LAND GROUT THE UNUSED HOLE OR 2. INSTALL AN OUTLET PROTECTOR WITH A SINGLE HOLE.

FERNCO 1051-44 (4" AC/DIOR 4" CI/PLAS COUPLING OR EQUAL WITH 2 CLAMPS (TY		COUPLING OR EQUAL WITH 2 CLAMPS (TYPIC
FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (TYPICAL) 4" PIPE LATERAL (NON-PERFORATED) ON GRADIENT	PAVEMENT EDGE FLOW FLOW FLOW FLOW FLOW 4" PIPE UNDERDRAIN PVC SCHEDULE 40 LONG SWEEP 90° ELBOW OR EQUAL (TYPICAL) •250' NORMAL •NOTE:	FLOW 4" PIPE UNDERDRAIN GLUED CONNECTION (ITYPICAL) 4" PIPE LATERAL (NON-PERFORATED) AT SAGS
	DETAIL OF DIDE LINDEDDDAIN LATEDALC	

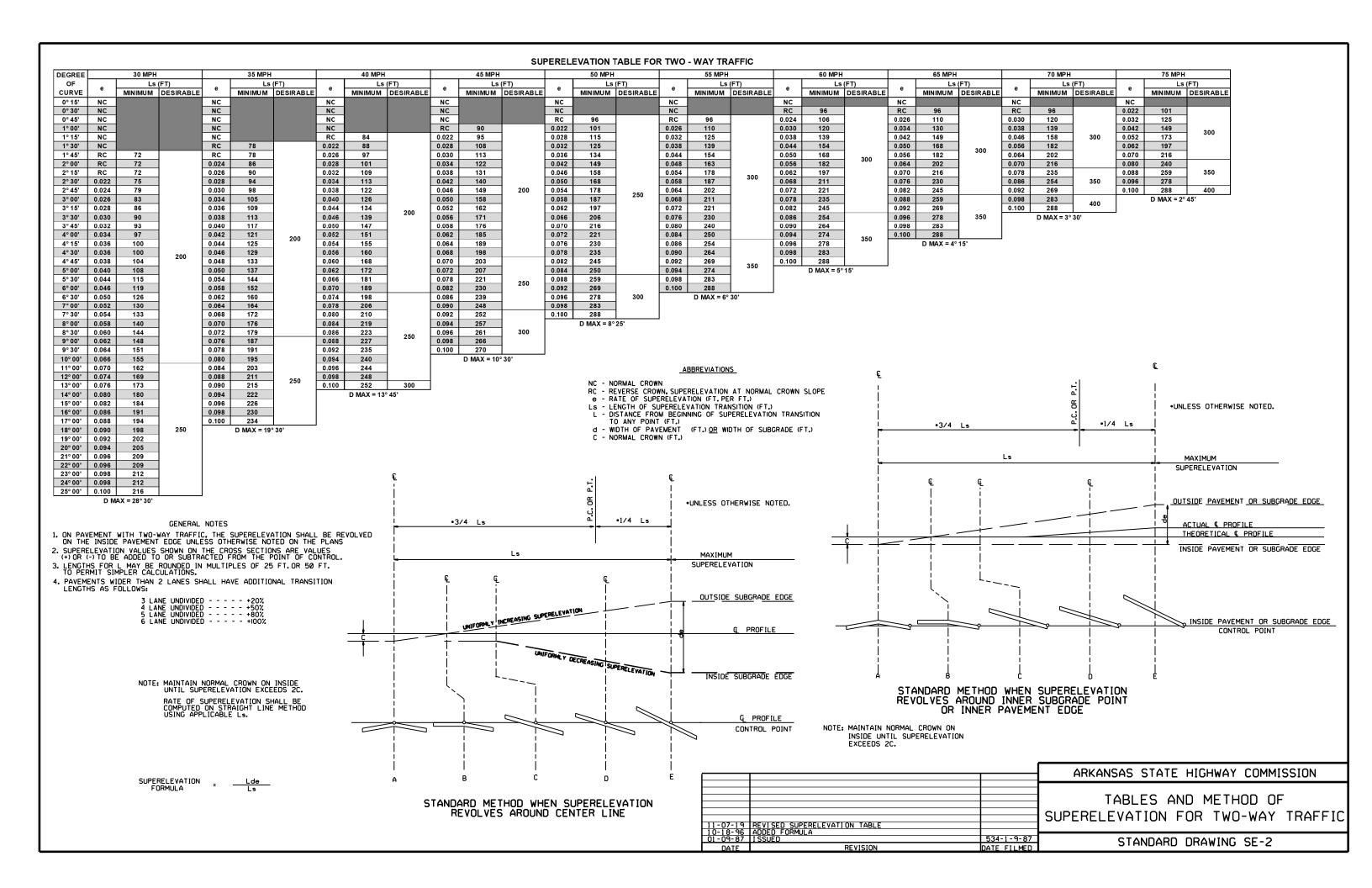
DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.

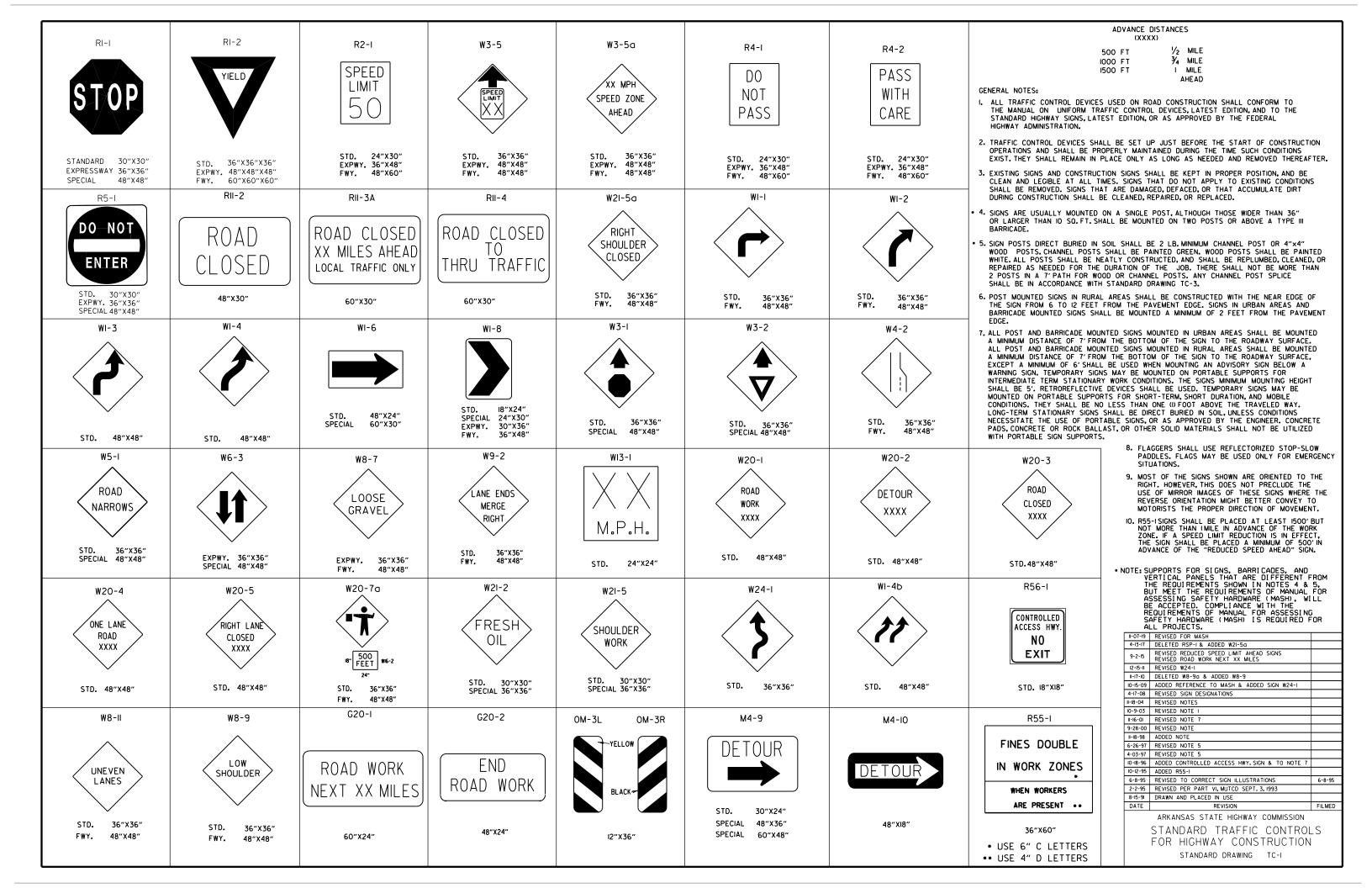
12-8-16	ADDED NOTES FOR PIPE UNDERDRAINS, REVISED RODENT SCREEN DETAIL AND NOTES, REMOVED NOTE IFOR GRANULAR MATERIAL, ADDED NOTE FOR GEOTEXTILE FABRIC		
4-10-03	REVISED NOTE 3		ı
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS		ı
11-18-98	REVISED NOTE		ı
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC		ı
4-26-96	ADDED LATERAL NOTE: 51/2" TO 5"		ı
II-22-95	REVISED LATERALS		
7-20-95	REVISED LATERALS & ADDED NOTE		Г
II- 3-94	REVISED FOR DUAL LATERALS	II- 3-94	ı
10- 1-92	SUBSTITUTED GEOTEXTILE	10- 1-92	Г
8-15-91	ADDED POLYEDTHYLENE PIPE	8-15-91	ı
II- 8-90	DELETED ALTERNATE NOTE	II- 8-90	ı
I-25-90	ADDED 4" SNAP ADAPTER	I-25-90	ı
II-30-89	DEL.(SUBGRADE); ADDED (WHERE REQUIRED)	II-30-89	⊢
7-15-88	ISSUED P.L.M.	647-7-15-88	ı
DATE	REVISION	DATE FILMED	

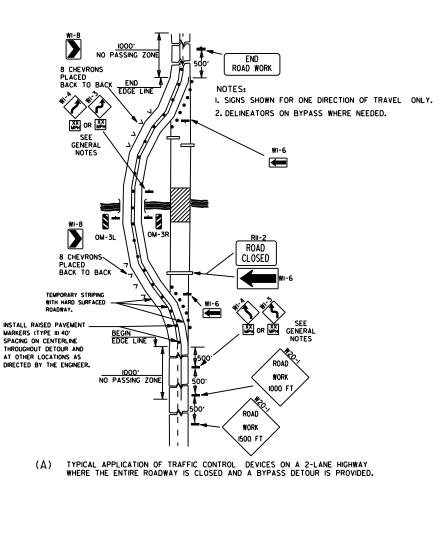
ARKANSAS STATE HIGHWAY COMMISSION

DETAILS OF PIPE UNDERDRAIN

STANDARD DRAWING PU-I







(DETOUR)

DETOUR

DETOUR

1

DETOUR

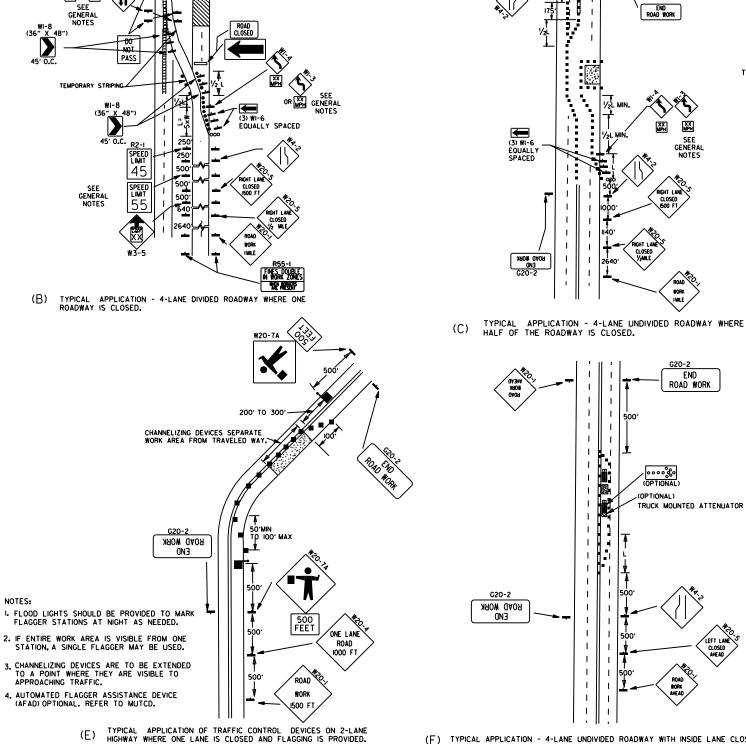
()) TYPICAL APPLICATION - ROADWAY CLOSED BEYOND DETOUR POINT.

WEST 4

I.REGULATORY TRAFFIC CONTROL DEVICES TO BE MODIFIED AS NEEDED FOR THE DURATION OF THE DETOUR.

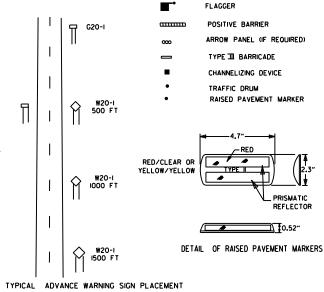
2.STREET NAMES MAY BE USED WHEN DESIRABLE FOR DIRECTING DETOURED TRAFFIC.

NOTES:



IL COMPLETE SIGNING SHOWN ONLY IN CROSSOVER DIRECTION. 2. TWO WAY TRAFFIC SEPARATED WITH POSITIVE BARRIER.

R4-7a RIGHT



KEY:

G20-2

END ROAD WORK

G20-2 ROAD WORK

(OPTIONAL)

(F) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WITH INSIDE LANE CLOSED.

TRUCK MOUNTED ATTENUATOR

TAPER FORMULAES

L=SXW FOR SPEEDS OF 45MPH OR MORE.

L= WS FOR SPEEDS OF 40MPH OR LESS.

WHERE:

L= MINIMUM LENGTH OF TAPER.

S= NUMERICAL VALUE OF POSTED SPEED LIMIT PRIOR TO WORK OR 85TH PERCENTILE SPEED.

W= WIDTH OF OFFSET.

GENERAL NOTES:

I. THE MAINTENANCE DIVISION SHALL CONDUCT A BALL BANK STUDY TO DETERMINE THE ADVISORY SPEED LIMIT PRIOR TO OPENING TO TRAFFIC. THE ADVISORY SPEED WILL BE POSTED ON WI-3 OR WI-4 CURVE WARNING SIGNS. USE WI-4 WHEN SPEED IS GREATER THAN 30MPH AND WI-3 WHEN 30MPH OR LESS

SOMPH OR LESS
2. WHEN THE EXISTING SPEED LIMIT IS 55MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 45MPH, THE R2-K55) SHALL BE
OMITTED AND THE W3-5 SHALL BE INSTALLED AT THAT
LOCATION, ADDITIONAL R2-145MPH SPEED LIMIT SIGNS SHALL
INSTALLED AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK AREA A R2-KXX)
SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

3. WHEN THE EXISTING SPEED LIMIT IS 65MPH AND THE PLANS
REQUIRE A SPEED LIMIT OF 55MPH, THE R2-I459 SHALL BE OMITTED.

ADDITIONAL R2-I55MPH SPEED LIMIT SIGNS SHALL BE INSTALLED
AT A MAXIMUM OF IMILE INTERVALS. AT THE END OF THE WORK
AREA A R2-I(XX) SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.

4. THE MAXIMUM SPACING BETWEEN CHANNELIZING DEVICES IN A TAPER
SHOULD BE APPROXIMATELY EQUAL IN FEET TO THE SPEED LIMIT.

BEYOND THE TAPER, MAXIMUM SPACING SHALL BE TWO TIMES
THE SPEED LIMIT, OR AS DIRECTED BY THE ENGINEER.

5. WARNING LIGHTS AND/OR FLAGS MAY BE MOUNTED
TO SIGNS OR CHANNELIZING DEVICES AT NIGHT AS NEEDED.

6. PAYEMENT MARKINGS NO LONGER APPLICABLE WHICH MIGHT CREATE CONFUSION IN THE MINDS OF VEHICLE OPERATORS SHALL BE REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.

7. TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE DELINEATED BY AFFIXING CONSPICUITY MATERIAL IN A CONTINUOUS LINE ON THE FACE OF THE TRAILER. WHEN PLACED ON OR ADJACENT TO THE SHOULDER AND NOT BEHIND A POSITIVE BARRIER, THESE DEVICES SHALL BE DELINEATED BY PLACING FIVE (5) TRAFFIC DRUMS, EQUALLY SPACED ALONG THE TRAFFIC SIDE OF THE DEVICE, PAYMENT FOR TRAFFIC DRUMS SHALL BE CONSIDERED INCLUDED IN THE PRICE BID FOR VARIOUS TRAILER MOUNTED DEVICES.

B. DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE ARDOT OUALIFIED PRODUCTS LIST.

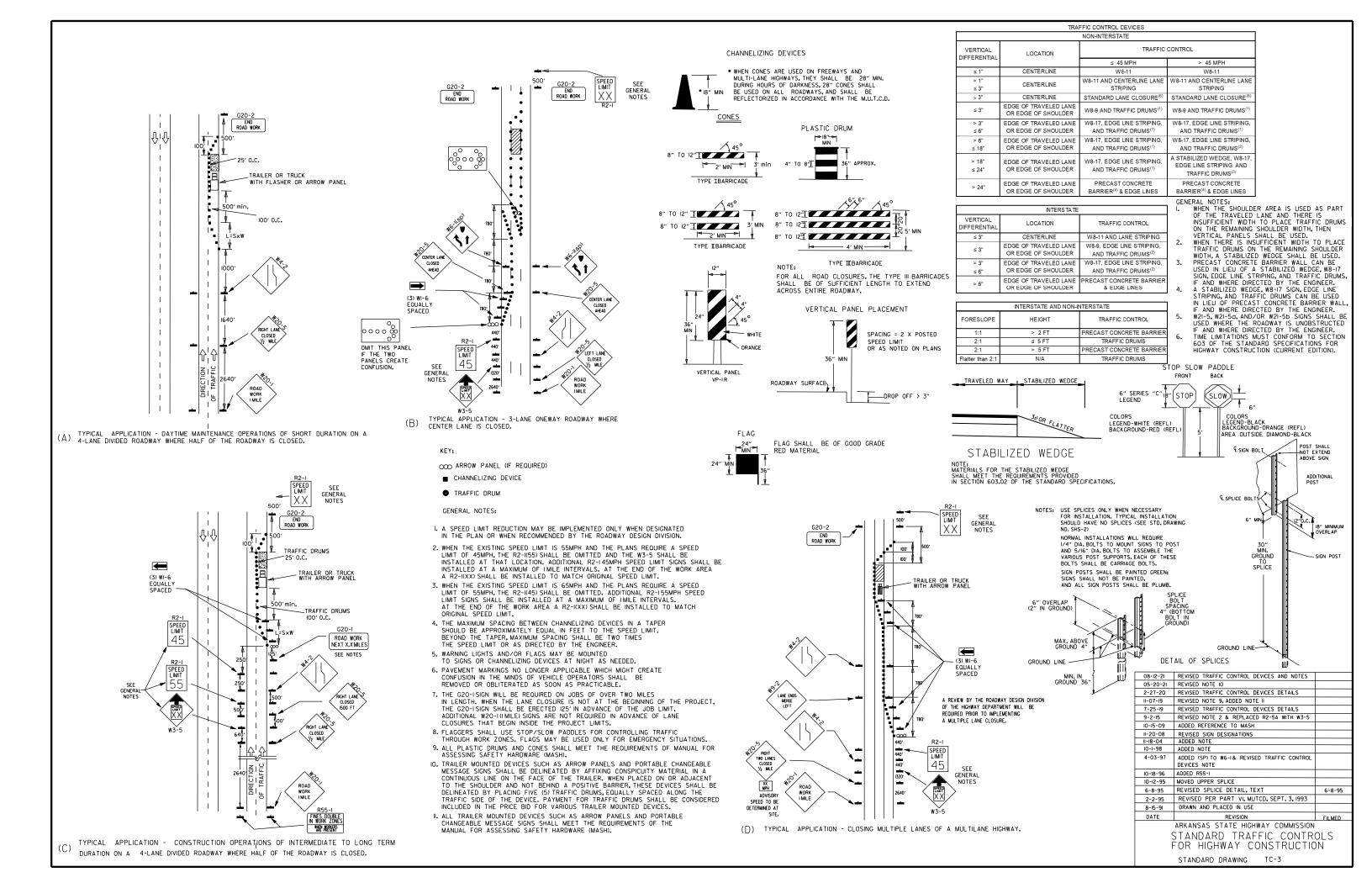
9. ALL TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL MEET THE REQUIREMENTS OF THE MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).

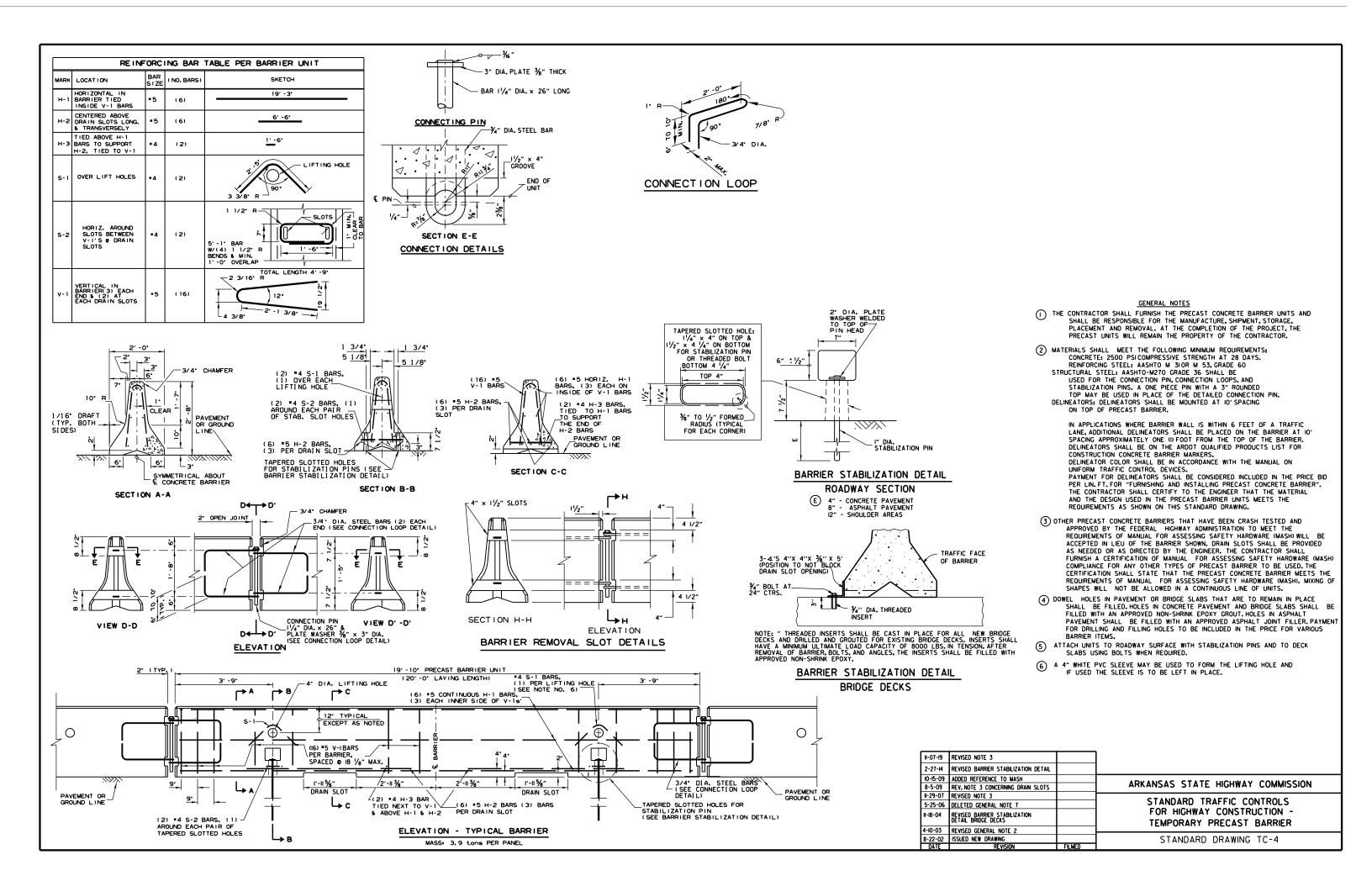
05-20-21	REVISED NOTE 7	
11-07-19	REVISED NOTE I, ADDED NOTE 9	
9-2-15	REVISED NOTE 2, ADDED NOTE 8, REVISED DRAWING (A) & REPLACED R2-5A WITH W3-5	
9-12-13	REVISED DETAIL OF RAISED PAVEMENT MARKERS	
3-11-10	ADDED (AFAD)	
II-20-08	REVISED SIGN DESIGNATIONS	
11-18-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-I	
4-26-96	CORRECTED (a) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON WI-4A	6-8-95
2-2-95	REVISED PER PART VI. MUTCO, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED

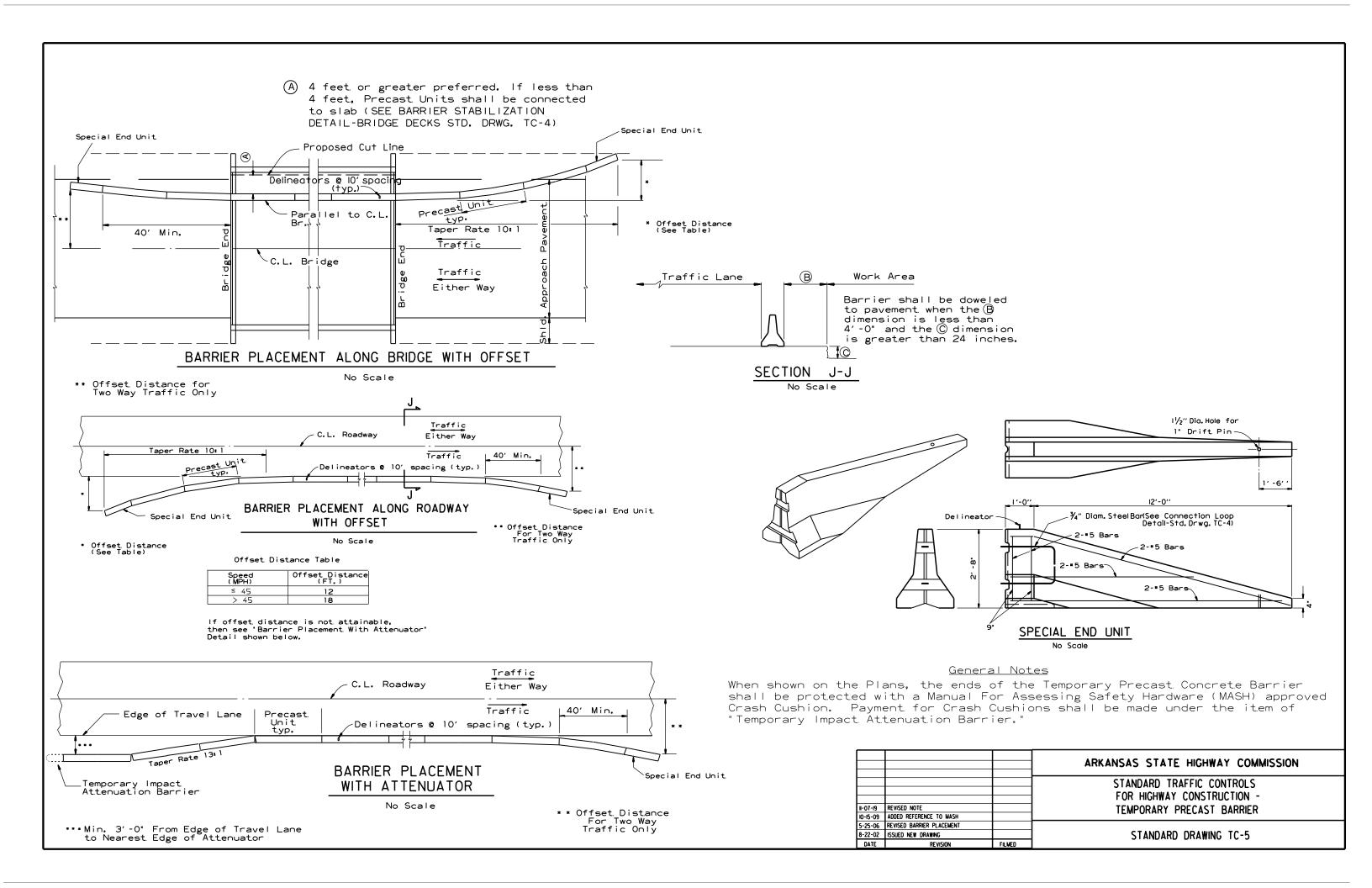
ARKANSAS STATE HIGHWAY COMMISSION

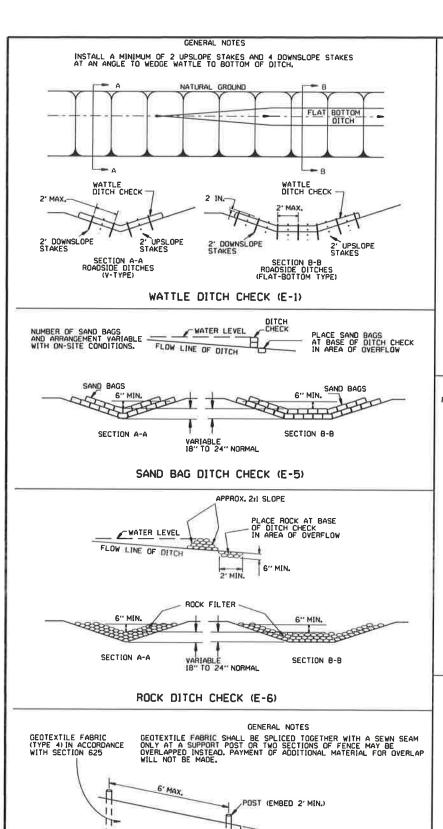
STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION

STANDARD DRAWING TC-2







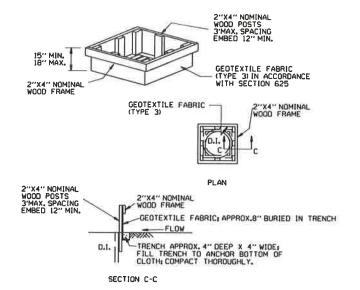


-6" MIN, BURIED

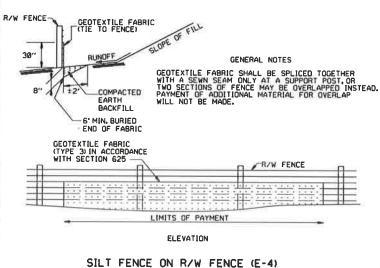
RUNOFF

COMPACTED EARTH

SILT FENCE (E-11)



DROP INLET SILT FENCE (E-7)

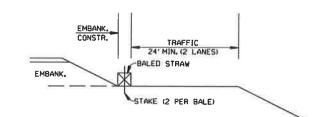


GENERAL NOTES

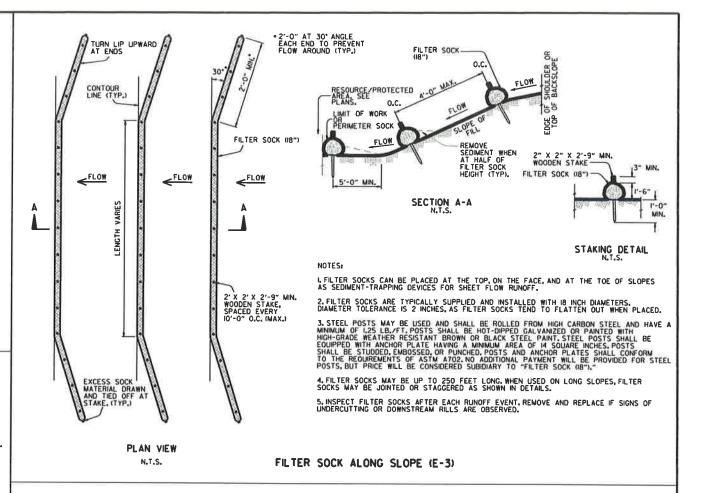
I. STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.

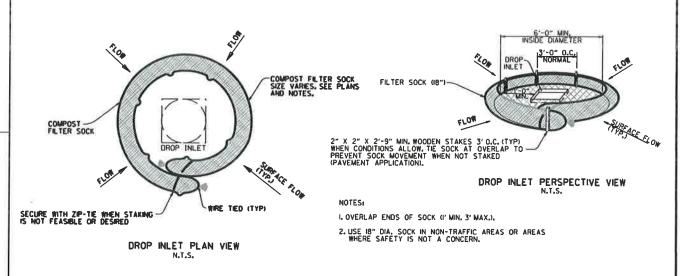
2. NO GAPS SHALL BE LEFT BETWEEN BALES.

3. BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.



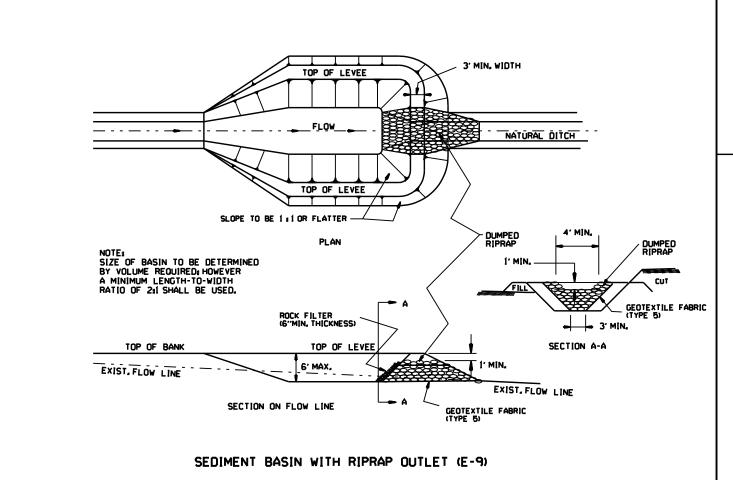
BALED STRAW FILTER BARRIER (E-2)

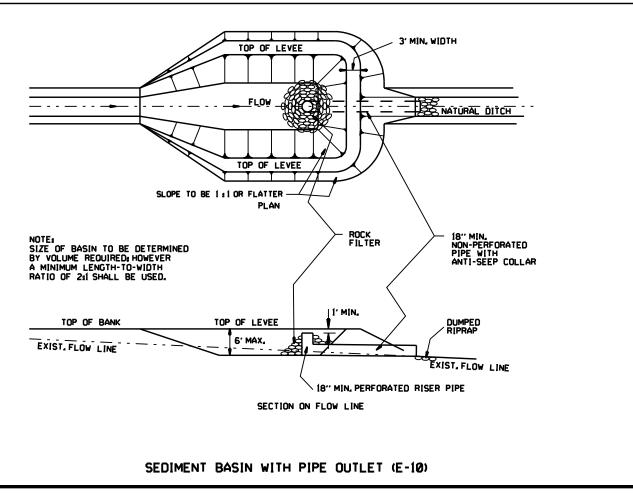


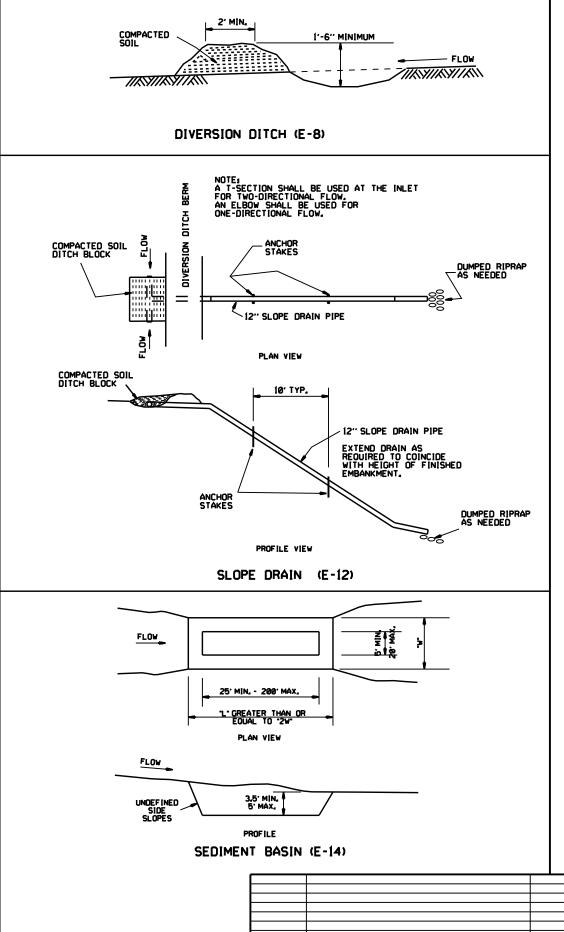


COMPOST FILTER SOCK DROP INLET PROTECTION (E-I3)

11-16-17	ADDED FILTER SOCK E-3 AND E-13			
12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ADVANCAS STATE UIGURAY COMBUSSION	
11-18-98	ADDED NOTES		ARKANSAS STATE HIGHWAY COMMISSION	
07-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)			
07-20-95	REVISED SILT FENCE E-4 AND E-II	7-20-95	TEMPORARY EROSION	
	REV. E-4 & E-II MIN. 13" BURIED END OF FABRIC			
06-02-94	REVISED E-1,4,7 & II; DELETED E-2 & 3	6-2-94	CONTROL DEVICES	
10-01-92	REDRAWN		CONTROL DEVICES	
08-02-76	ISSUED R.D.M.	298-7-28-76	CTANDADD DDAWING TEC I	
DATE	REVISION	FILMED	STANDARD DRAWING TEC-I	







6-2-94 Revised E-8 & E-12: Added E-14 & Deleted E-13
4-1-93 ISSUED REVISION

ARKANSAS STATE HIGHWAY COMMISSION

TEMPORARY EROSION CONTROL DEVICES

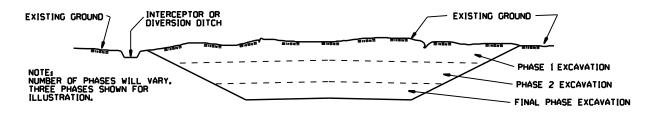
STANDARD DRAWING TEC-2

CLEARING AND GRUBBING

CONSTRUCTION SEQUENCE

- 1. PLACE PERIMETER CONTROLS (I.E. SILT FENCES , DIVERSION DITCHES, SEDIMENT BASINS, ETC.)
- 2. PERFORM CLEARING AND GRUBBING OPERATION.

EXCAVATION



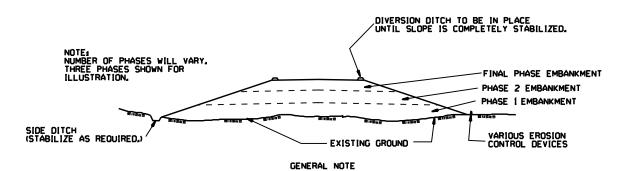
GENERAL NOTE

ALL CUT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

- 1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
- 2. PERFORM PHASE 1 EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING.
- 3. PERFORM PHASE 2 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
- 4. PERFORM FINAL PHASE OF EXCAVATION, PLACE PERMANENT OR TEMPORARY SEEDING. STABILIZE DITCHES. CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

EMBANKMENT



ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.

2. PLACE PHASE I EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING.
PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION
IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.

4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

			45.4
			ARK
44			
11-03-94	CORRECTED SPELLING		
6-2-94	Drawn & Issued	6-2-94	
DATE	REVISION	FILMED	

KANSAS STATE HIGHWAY COMMISSION

TEMPORARY EROSION CONTROL DEVICES

STANDARD DRAWING TEC-3

