ARKANSAS DEPARTMENT OF TRANSPORTATION



STATE JOB NO.	BR4707	
FEDERAL AID PROJECT NO.	STPB-0047(71)	
	PEMISCOTT BAYOU STR. & APPRS. (S)	
COUNTY ROAD NO.	CR 197	
IN	MISSISSIPPI	COUNTY

The information contained herein was obtained by the Department for design and estimating purposes only. It is being furnished with the express understanding that said information does not constitute a part of the Proposal or Contract and represents only the best knowledge of the Department as to the location, character and depth of the materials encountered. The information is only included and made available so that bidders may have access to subsurface information obtained by the Department and is not intended to be a substitute for personal investigation, interpretation and judgment of the bidder. The bidder should be cognizant of the possibility that conditions affecting the cost and/or quantities of work to be performed may differ from those indicated herein.



ARKANSAS DEPARTMENT OF TRANSPORTATION

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MATERIALS DIVISION

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July 22, 2019

TO:

Mr. Rick Ellis, Bridge Engineer

SUBJECT:

Job No. BR4707

Pemiscot Bayou Str. & Apprs. (S)

County Road 197 Mississippi County

Transmitted herewith are a brief summary of the geology and site conditions, D50 scour analysis, summary of percent material passing #200 sieve and Atterberg Limits test results (for liquefaction susceptibility analysis), and the logs of the borings conducted for the structure and approaches of the above referenced project. The samples obtained by the Standard Penetration Tests were brought to the laboratory and visually classified by experienced lab personnel to confirm the field identifications.

This project consists of replacing the bridge crossing Pemiscot Bayou, on Mississippi County Road 197, west of Blytheville. The existing bridge is out of service and the new bridge is to be constructed on the existing alignment. Due to steep slopes, conflicts with utilities, and high water levels in the channel only two of the four requested borings were obtained. The borings that were obtained are located at: 103+77 8' Rt. of Construction Centerline and 105+61 8' Rt. of Construction Centerline.

Based on plans provided by Bridge Division and the findings from this subsurface investigation, it is anticipated that all bents will be founded on concrete filled steel shell piling.

Embankment analyses included global stability with seismic design consideration utilizing a horizontal acceleration coefficient of 0.936, as provided by Bridge Design. It is assumed that the operational classification for this bridge is "other", as defined in Section 3.10.5 of the AASHTO LRFD Bridge Design Specification, Seventh Edition, 2014. Since this is not a "critical" or "essential" bridge the large expense and additional time associated with removing existing embankments and reconstructing reinforced embankments with significant ground improvement, to satisfy seismic consideration, is not recommended. Embankment displacement is expected to occur in a large seismic event. The proposed embankment configuration provides for a satisfactory Factor of Safety for static conditions.

Michael C. Benson Materials Engineer

MCB:rpt:mlg

cc: State Construction Engineer - Master File Copy

District 10 Engineer State Aid

G.C. File

GEOLOGY AND SITE CONDITIONS Job No. BR4707 Pemiscot Bayou Str. & Apprs. (S) County Road 197 Mississippi County

Site Conditions

The existing bridge is a seven span structure over Pemiscot Bayou. The bridge deck is constructed of concrete supported by 11 timber beams on timber pilings. The deck has curbs, but no guardrail. Many of the pilings show damage, and the bridge is currently closed to traffic. Overhead power lines parallel the west side of the roadway. There are residences on both sides of the roadway up-station from the bridge. There is also a residence on the west side of the road down-station from the bridge. There is an agricultural field located on the west side of the road, down-station from the bridge. A slope failure is located under the up-station end of the bridge (See Figure 1.).

Site Geology

The Pemiscot Bayou is a former distributary of the Mississippi River. It was active for approximately 2,000-3,000 years, flowing southwest toward the St. Francis River. When active the channel in this area would have been over 1,000 feet wide and, based on soil samples recovered, had a depth of at least 36.5 feet. The present channel is mostly infilled with silt and sandy silt. The current Pemiscot Bayou at the job site has been incorporated into the extensive system of ditches in northeast Arkansas.

The two borings drilled as part of this subsurface investigation encountered very different soils in the upper 36.5 feet. The up-station boring (**Boring 2**) encountered very loose to loose clayey sand in the upper 10 feet. These may represent backswamp deposits. Below this layer, sand with silt was encountered most likely representing point bar deposits, which grades into the generally coarser glacial outwash sands.

The down-station boring (**Boring 1**) encountered soils indicative of the infilled channel. These soils consist of loose to very loose silt to silt with sand and soft silty clay down to a depth of 36.5 feet (the uppermost sample in this zone was medium dense). As water stopped flowing in the channel, the channel started to fill up with fine-grained sediment. Below the infilled sediment layer, the soils in Boring 1 are part of the same sand unit observed in Boring 2 where sand with silt most likely representing point bar deposits grades into the generally coarser glacial outwash sands. The sands become siltier in both borings at depths greater than 80 feet.

Fine soil was recovered from the up-station side of the channel while sampling for the D-50 scour test. This may indicate that the high silt content channel fill soils may continue to the up-station side of the stream.



Figure 1. Up-station stream bank under the bridge

Scour Potential

Scouring is occurring in association with ditches on both sides of the roadway. Riprap has been placed in the ditches as they enter the channel to minimize erosion. The stream is clear and flowing indicating that there is little active erosion in the channel (See Figures 2. and 3.).



Figure 2. Scouring in drainage on the down-station side of the channel and east side of the roadway.



Figure 3. Scouring in drainage on the up-station side of the channel and west side of the roadway.

Subsurface Conditions

Based on the results of the borings, the subsurface stratigraphy may be generalized as follows:

0 to 40 Feet:

Varies from moist to wet, very loose, to medium dense, brown to gray silt to sandy silt to clayey sand. One sample in this zone

contained wet, soft silty clay.

40 to 65 Feet:

Consists of wet, medium dense to dense, brown sand to sand with

silt with trace gravel.

65 to 85 Feet:

Consists of wet, dense to very dense, brown sand to sand with silt

with trace gravel.

85 to 101.5 Feet:

Consists of wet, very dense, brown sand with silt.

D₅₀ AGGREGATE ANALYSIS FOR SCOUR CALCULATIONS

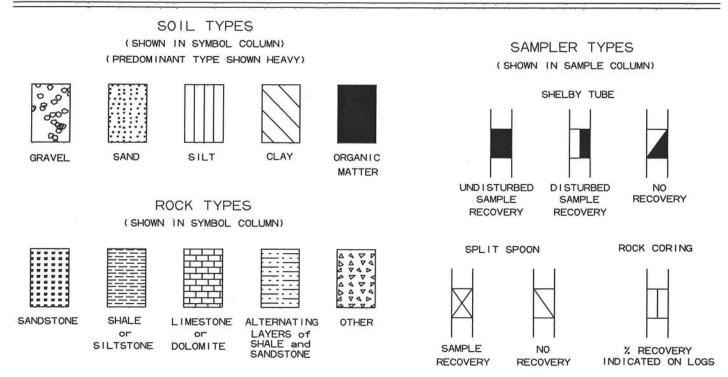
Job No. BR4707											
Creek Name	Station	Sample Type	Location	Depth (ft.)	Aggregate Size (D50) (in.)						
Pemiscot Bayou	105+19	Creek Bank	60' Rt. of Const. C.L.	NA	Less Than 0.0029						

Lab Test Summary Project Number: BR4707 Project Name: Pemiscot Bayo

Pemiscot Bayou Str. & Apprs. (S)

Ctation	1 4:	Depth	Plastic	Liquid	Plasticity	% Passing
Station	Location	(ft.)	Limit	Limit	Index	No. 200
103+77	8' RT.	4.6	NP			58
103+77	8' RT.	9.7	20	27	7	90
103+77	8' RT.	14.6	NP			92
103+77	8' RT.	20	NP			91
103+77	8' RT.	25	NP			92
103+77	8' RT.	30	NP			74
103+77	8' RT.	35	NP			52
103+77	8' RT.	40	NP			8
103+77	8' RT.	45	NP			9
103+77	8' RT.	50	NP			3
103+77	8' RT.	55	NP			4
103+77	8' RT.	60	NP	3 8	1 2 1019	4
103+77	8' RT.	65	NP			5
103+77	8' RT.	70	NP			4
103+77	8' RT.	75	NP			ND
103+77	8' RT.	80	NP			7
103+77	8' RT.	85	NP	ii.	100 10 10	8
103+77	8' RT.	90	NP			8
103+77	8' RT.	95	NP			20
103+77	8' RT.	100	NP			7
105+60.94	8' RT.	4.8	ND			31
105+60.94	8' RT.	9.8	14	35	11	19
105+60.94	8' RT.	15	NP			8
105+60.94	8' RT.	20	NP			5
105+60.94	8' RT.	25	NP			4
105+60.94	8' RT.	30	NP			5
105+60.94	8' RT.	35	NP			4
105+60.94	8' RT.	40	NP			4
105+60.94	8' RT.	45	NP			2
105+60.94	8' RT.	50	NP			4
105+60.94	8' RT.	55	NP			3
105+60.94	8' RT.	60	NP			3
105+60.94	8' RT.	65	NP			3
105+60.94	8' RT.	70	NP			4
105+60.94	8' RT.	75	NP			3
105+60.94	8' RT.	80	NP			2
105+60.94	8' RT.	85	NP			6
105+60.94	8' RT.	90	NP			5
105+60.94	8' RT.	95	NP			6
105+60.94	8' RT.	100	NP			5

LEGEND



TERMS DESCRIBING CONSISTENCY OR CONDITION

GRANU	LAR SOIL		CLAY	CL	AY-SHALE	SHALE				
"N" Value	Density	"N" Value	Consistency	"N" Value	Consistency	"N" Value	Consistency			
0-4	Very Loose	0-1	Very Soft	0-1	Very Soft					
5-10	Loose	2-4	Soft	2-4	Soft	31-60	Soft			
11-30	Medium Dense	5-8	Medium Stiff	5-8	Medium Stiff	Over 60				
31-50	Dense	9-15	Stiff	9-15	Stiff	More than	2'			
Over 50	Very Dense	16-30	Very Stiff	16-30	Very Stiff	Penetratio	on			
		31-60	Hard	31-60	Hard	in 60 Blow	vs: Medium Hard			
		Over 60	Very Hard	0ver 60	Very Hard	Less than	2'			
						Penetratio	on			
						in 60 Blow	ısı Hard			

- 1. Ground water elevations indicated on boring logs represent ground water elevations at date or time shown on boring log. Absence of water surface implies that no ground water data is available but does not necessarily mean that ground water will not be encountered at locations or within the vertical reaches of these borings.
- 2. Borings represent subsurface conditions at their respective locations for their respective depths. Variations in conditions between or adjacent to boring locations may be encountered.
- Terms used for describing soils according to their texture or grain size distribution are in accordance with the Unified Soil Classification System.

Standard Penetration Test – Driving a 2.0" O.D., 1-3/8" I.D. sampler a distance of 1.0 foot into undisturbed soil with a 140-pound hammer free falling a distance of 30 inches. It is customary to drive the spoon 6.0 inches to seat into undisturbed soil, and then perform the test. The number of hammer blows for seating the spoon and performing the test are recorded for each 6 inches of penetration on the drill log. The field "N" Value (N_f) can be obtained by

adding the bottom two numbers for example: $\frac{6}{8-9} \Rightarrow 8+9=17 blows / ft$. The "N" Value corrected to 60% efficiency (N₆₀) can be obtained by multiplying N_f by the hammer correction factor published on the boring log.

777070107207077			DEPARTMENT OF TRANSPORTATION DIVISION - GEOTECHNICAL SEC.		BORING NO. 1 PAGE 1 OF 3								
JOB N			BR4707 Mississippi County		DATE:	·	-	-	ch 2	6, 20	19		-
JOB N			Pemiscot Bayou Str. & Apprs. (S)	- 1	TYPE OF DRILLING:								
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(COMMONSORIAL)			ustin Dillman		HAMMI	ER CC	RRECT	ION I	FACT	OR:		1.37	
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JOB N			BR4707 Mississippi County		DATE:		OI	_	ch 2	7, 20	19		-		
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	RKANSAS DEPARTMENT OF TRANSPORTATION ATERIALS DIVISION - GEOTECHNICAL SEC.						BORING NO. 2 PAGE 3 OF 3								
JOB N			BR4707 Mississippi County		DATE:		- 01		ch 2	7, 20	19		-		
JOB N			Pemiscot Bayou Str. & Apprs. (S)		TYPE O	F DRI	LLING		011 2	,, 20	.,				
			Co. Rd. 197				tem A		- R	otary	Was	sh			
STATI	ION:		105+61		EQUIPN					ME 7					
LOCA			8' Right of Construction Centerline												
			ustin Dillman		HAMMI	ER CC	RRECT	ION I	FACT	OR:		1.37			
COM	PLET	ION	DEPTH: 101.5										- 1		
D		s													
E	S	Α											5		
Р	M	M	DESCRIPTION OF MATERIAL	SOIL				ΤŦ	FT.	M.S		% T	% R		
T	В	P		GROUP	7.)	<u>۱</u>		IGI	7	3LC	ا بــ	C	Q		
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FT.	L	S	SURFACE ELEVATION: 245.6		PLASTIC LIMIT	MOIST.	LIQUID	DRY WEIGHT	LBS PER CU.FT	NO. OF BLOWS	PER 6-IN.				
	/111/1		CONTACE ELEVATION: 245.0		NP	0	77	Ω		12			-		
		X		SP	INE					16-					
<u> </u>			Wet, Dense, Brown, Poorly-Graded Sand with												
<u> </u>			Trace Gravel and Trace Organic Matter										ľ		
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75	<u> </u>														
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			with Silt and Trace Gravel	_											
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			with Silt and Trace Gravel	-											
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